

**DATA COLLECTION NOTEBOOK**

**GRANITE REEF WASH**

**DRAINAGE MASTER PLAN**



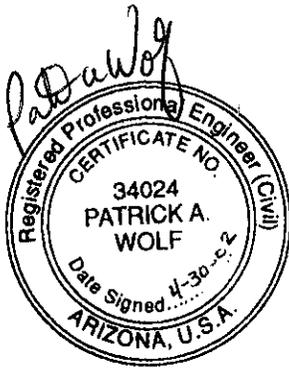
**DATA COLLECTION NOTEBOOK**

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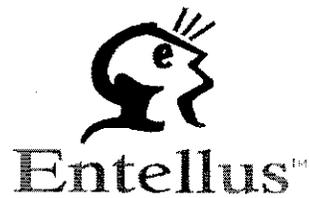
**Contract FCD 2000CO38**

**APRIL 30, 2002**



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GRANITE REEF WASH DRAINAGE MASTER PLAN



**GRANITE REEF DRAINAGE MASTER PLAN**

**FCD -2000CO38**

**TECHNICAL DATA NOTEBOOK**

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## SECTION 1: INTRODUCTION

This study was initiated by the Flood Control District of Maricopa County (District) in 2000 to identify flooding potential along the Granite Reef Wash from Thomas Road to its confluence with the Salt River by developing new hydrology and making recommendations for improvements. The study was conducted in coordination with the District, The City of Scottsdale (City) and the Salt River-Pima Maricopa Indian Community (SRPMIC).

The area of study for the Granite Reef Drainage Master Plan (DMP) falls within the City of Scottsdale and the SRPMIC as presented in **Figure 1**. The Granite Reef watershed is bounded in the north by the Arizona Canal, in the south by the Salt River, on the east by the Loop 101 Freeway and in the west by the Indian Bend Wash watershed. The west boundary meanders along the watershed following the divider between the two watersheds but it is generally located between Granite Reef Road and Hayden Road. The total study area is approximately six square miles.

The storm water conveyance system within the Granite Reef Wash drainage area has insufficient capacity to collect and convey major storm events, in particular, the 100-year flood event as defined in the *Granite Reef Wash Floodplain Delineation Study (Reference 1)*. Additionally, the wash does not have an adequate outfall to the Salt River through the SRPMIC land south of McKellips Road.

Several hydrologic studies had been performed in this watershed. The first study was the *Flood Insurance Study for the City of Scottsdale (Reference 2)*, which was completed in 1984. There were two studies done in 1995, one was the *Granite Reef Wash Drainage Study (Reference 3)*, and the other was the *Report on Granite Reef Wash Drainage Study (Reference 4)*. The last study for this area was the *Granite Reef Watershed Flood Mitigation Candidate Assessment Report (Reference 5)*, which was done in 1999. The results of these reports vary significantly and none of these studies contain procedures and assumptions that are acceptable to all interested parties. **Table 1** shows the flows from these reports at the three relevant locations.

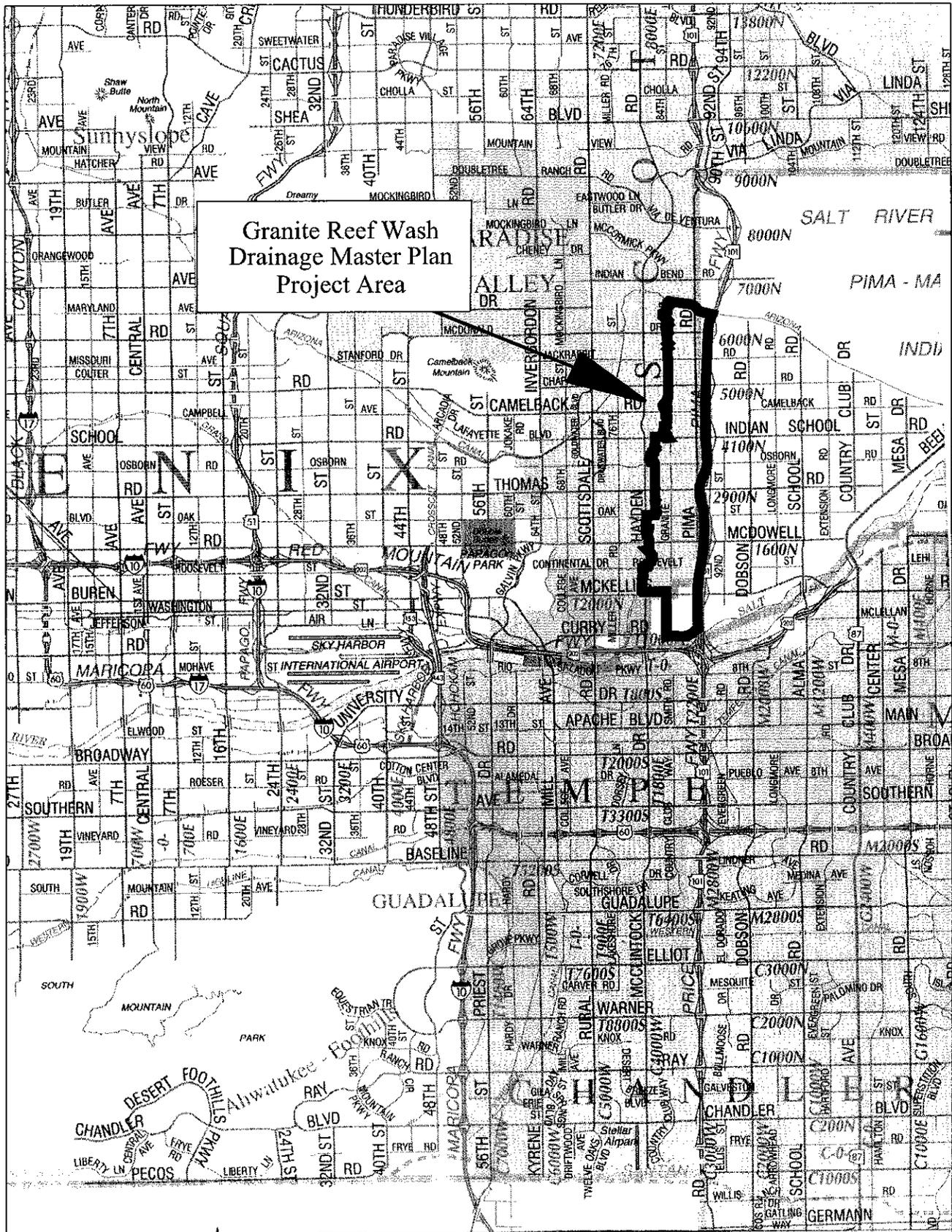


Figure 1.  
Vicinity Map

**TABLE 1**

Summary of 100-year Discharges from Current Hydrology Studies

Model *	100-yr Flows (cfs)		
	<i>Granite Reef Wash at:</i>		
	<i>Indian School Road</i>	<i>Thomas Road</i>	<i>McKellips Road</i>
FEMA	644	1,240	1,417
SLA	-	2,050	4,160
KVL	1,590	3,387	5,205

\*FEMA = *Flood Insurance Study for the City of Scottsdale (Reference 2)*

\*SLA = *Granite Reef Wash Drainage Study (Reference 3)*

\*KVL = *Report on Granite Reef Wash Drainage Study (Reference 4)*

**SECTION 2: ADWR/FEMA FORMS**

These forms are not required for this project.



## SECTION 3: MAPPING AND SURVEY INFORMATION

### 3.1 Field Survey Information

Field surveys were conducted, mainly, at roadway intersections to supplement aerial topographic mapping. Results of the field survey are documented in **Appendix C**.

### 3.2 Mapping

Topographic mapping and aerial photography used in this study were provided by the City. The SRPMIC provided more recent aerial photography for the study area with the Indian Community. The topographic mapping furnished by the City consisted of a 100-scale, 1-foot topographic map provided in digital format (**Reference 6**). The SRPMIC aerial photography consisted of a 2-foot pixel resolution digital image (**Reference 7**). Vertical and horizontal controls tied into the State Plane Coordinate System (1983 NAD) and the elevations are based on the North American Vertical Datum of 1988 (NAVD 88).

#### 3.2.1 Watershed Map

Watershed boundaries and hydrologic parameters were determined primarily from the City of Scottsdale topographic mapping (**Reference 6**). Additionally, aerial photography provided by the City and the SRPMIC (**References 7 and 8**) was overlaid with the watershed map to verify the basin boundaries.

#### 3.2.2 Floodplain Maps

The 100-scale, 1-foot contour interval mapping provided by the City was used to delineate the floodplain boundaries. The cross-section information was obtained from the *Granite Reef Wash Floodplain Delineation Study* (**Reference 1**).

### 3.2.3 Soils Map

Electronic soils maps were furnished by the District's GIS Department. This mapping is a digital version of the Natural Resources Conservation Service NRCS (formerly SCS) *Soil Survey of Aguila-Carefree Area, Parts of Maricopa and Pinal Counties, Arizona* (**Reference 9**) and *Soil Survey of Eastern Maricopa County and Northern Pinal Counties, Arizona* (**Reference 10**). The soils map is presented on **Plate 4-3**.

### 3.2.4 Land Use Map

The land use information within the City was developed based on Aerial photography provided by the City. This portion of the watershed is fully developed and recent photography provided the most accurate information on the type of development within the study area. However, an impervious area study conducted by the City (**Reference 11**) was used to adjust the percentage of impervious area for each type of land use identified from the aerial photography. Results of the impervious study indicated a much higher impervious percentage than the standard land use guidelines provided in *The Flood Control District of Maricopa County Drainage Manual – Volume I-Hydrology* (hereinafter referred as *FCDMC Drainage Manual*) (**Reference 12**). To verify the accuracy of the impervious area study, some areas were measured from the aerial photographs to check the impervious area. These checks came within five percent of the results provided by the impervious study.

Land use for the SRPMIC portion of the watershed was assumed to be commercial. Under current conditions, this portion of the watershed is either vacant or agricultural. However, the area is at a prime commercial location and it is expected to be developed in the near future. The Land Use Map is presented on **Plate 4-4**.

## SECTION 4: HYDROLOGY

### 4.1 Method Description

The peak flows in this study area were obtained through precipitation/runoff modeling. The hydrologic modeling was performed by means of the 1991 version of the HEC-1 computer program as developed by the U.S. Army Corps of Engineers (Reference 13). The models were developed using Green and Ampt methodology to estimate the rainfall losses. Excess rainfall was then routed to the concentration points using the Clark Unit Hydrograph. The estimation procedures for model parameters and components were based on the *FCDMC Drainage Manual* (Reference 12).

Schematic flow routing diagrams for runoff modeling are presented on **Plate 4-2**. A six-hour duration storm was used as the base model for this study because it produced higher peak flows than the 24-hour storm. However, the 24-hour storm was used to analyze storage requirements. Flow through regional detention basins was modeled by diverting the flow out of the model up to the capacity of the basin. The Normal-Depth routing method was predominantly utilized for routing hydrographs from one concentration point to the next. However, the Muskingum-Cunge methodology was used at locations with normal depth methodology became numerically unstable. Kinematic Wave methodology was used to route the storm drain routing reaches.

### 4.2 Parameter Estimation

#### 4.2.1 Drainage Area Boundary

Based on the topographic mapping provided and the aerial photography provided by the City and the SRP-MIC, Entellus developed a preliminary set of basin boundaries. These boundaries were generated considering the existing infrastructure, which included roadway layout, storm drain and other

features affecting the flow patterns through the watershed. Basin Boundaries are shown in **Plate 4-1**.

#### 4.2.2 Watershed Work Map

The watershed maps were based on a 100 scale, 1 foot contour interval topographic mapping provide by the City. Mapping outside the City boundaries was not available. In these areas, The USGS 7.5 minutes Quadrangle maps in combination with the aerial photography provided by the SRP-MIC were used.

#### 4.2.3 Gage Data

Most of the conveyance in the watershed is through the roadways. This type of flow is not suited for gauging. No gauging stations are located within the study area.

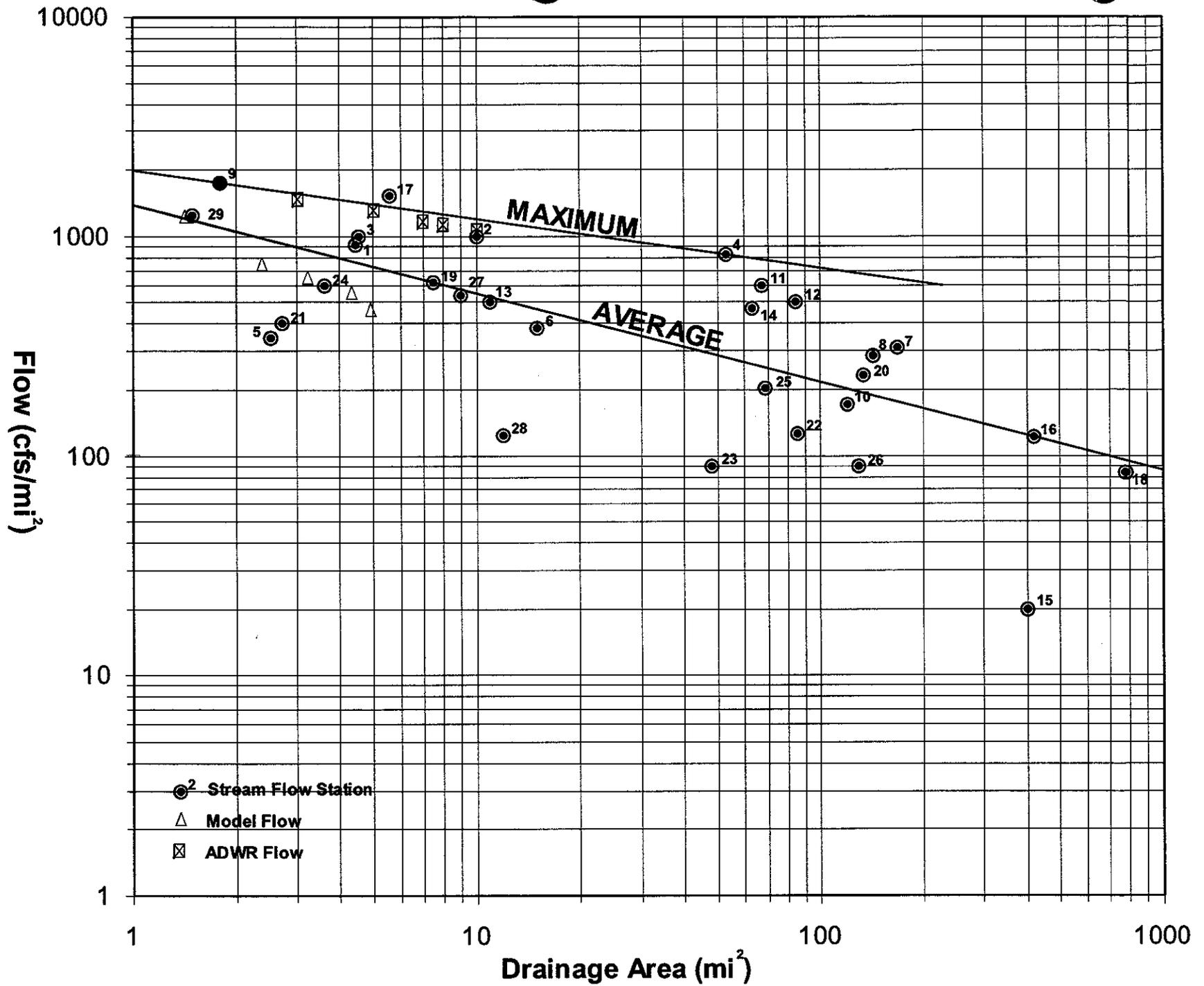
#### 4.2.4 Statistical Parameters

The Arizona Department of Water Resources (ADWR) State Standard Attachment SSA 2-96 (**Reference 14**), documents regional regression equations developed for the State of Arizona. The project area is located within Region 12, which includes most of the central portion of the state. This equation is shown in **Figure 3**. The ADWR regression equations were developed based on gauge data across the entire region and better represent a much larger, better-defined watercourse.

#### 4.2.5 Precipitation

Precipitation data was obtained from the isopluvial maps contained in the NOAA Atlas of Arizona (**Reference 15**). Selected isopluvial maps used for Maricopa County are also encountered in *FCDMC Drainage Manual (Figures 2.1 through 2.13)* (**Reference 12**). Copies of these figures, as well as the precipitation frequency (PreFre) output, are included in **Appendix D**.

FIGURE 3  
VERIFICATION OF RESULTS



## 4.2.6 Physical Parameters

### 4.2.6.1 Watershed parameters (HEC-1 Model)

Soils and land use percentages were estimated using the District's Geographic Information System (GIS) database. The land use type was determined from recent aerial photography. The percentage of impervious area for each land use type was estimated using the *City of Scottsdale Impervious Study (Reference 11)*. This information, as well as other physical parameters estimated from the work maps, was entered into the *Drainage Design Menu System (DDMS) (Reference 16)* to obtain the Green and Ampt rainfall loss parameters.

The Clark Unit Hydrograph was used in the computation of peak discharges in this study because the average subbasin area is less than five square miles. Clark Unit Hydrograph parameters were estimated using the MCUHP1 module of the DDMS software (**Reference 16**). Supporting documentation and DDMS reports are included in **Appendix D**.

### 4.2.6.2 Storage routing parameters

Storage for regional basins identified from contours and aerial photographs were included in the development of the HEC-1 model.

Storage volumes were obtained from the contour maps. The storage was modeled as a diversion where all the flow entering the basin, up to the capacity of the basin, is diverted out of the model. Once the basin is full, no more flow is diverted. Detailed calculations are also included in **Appendix E**.

The basins located in the SRPMIC were modeled assuming they were completely developed with a commercial land use and retained the 100-year, 2-hour storm. They<sup>re</sup> were three basins in the project area that had schools with a large playground area. These playground areas are bermed so that they are self-detained. The playground areas were subtracted from the basin calculations to account for the storage.

### 4.3 Problems Encountered During the Study

#### 4.3.1 Special Problems and Solutions

##### 4.3.1.1 Storm Drain

There are several existing storm drains within the study area (Figure 2). They range in size from 18 inches to 72 inches. The total capacity of the storm drains was calculated for each reach and the results and detailed calculation are included in Appendix E. The amount of flow entering the storm drain at each concentration point was estimated by prorating the capacity of the storm drain between the different concentration points. The length of storm drain (number of catch basins within a reach) as well as the contributing area was used to determine the percentage of the storm drain capacity that could be taken at each concentration point (see Appendix D). These flows were then diverted out of the model and routed to Indian Bend Wash.

A sensitivity analysis was performed to determine if a more detailed analysis of the storm drains is needed to accurately estimate the flows at Granite Reef Wash. This sensitivity analysis shows that the storm drains have an insignificant effect on the flows at Granite Reef Wash. Therefore, a more detailed analysis

of the storm drain system was not performed. **Table 2** shows the results of this analysis at some key locations.

**TABLE 2**

Storm Drain Sensitivity Analysis Results

Location	Flow (cfs)	
	100-year, 6-hour Model	Sensitivity Model *
Thomas Road & Granite Reef Wash	1766	1958
McDowell Road & Granite Reef Wash	2203	2388
McKellips Road & Granite Reef Wash	2347	2550

\* No storm drains included in the model.

4.3.1.2 Flow Splits

The study area is very flat and each roadway intersection has a potential flow split. Particular attention was given to flow splits where the flow exits the watershed. Several of these splits were identified along Granite Reef Road where split flows to the west exit the watershed and flow into the Indian Bend Wash watershed. Split flows within the watershed were also modeled to insure flow estimates at relevant concentration points are as accurate as possible. The detailed split flow calculations are included in **Appendix E**.

4.3.1.3 Area Reduction

The upstream contributing area, for both surface diversions and storm drain diversions, was divided between the main path and the diverted path proportionately to the amount of flow estimated for each path. These values were hard-coded into the models to accurately reflect the upstream contributing area.

#### 4.3.2 Modeling Warning and Error Messages

There were no ERROR messages in the HEC-1 models. Warning messages generated by the HEC-1 were examined to ensure that the models were not adversely affected.

The warning message encountered for the 10-year, 6-hour HEC-1 model is as follows:

- WARNING --- MODIFIED PULS ROUTING MAY BE NUMERICALLY UNSTABLE FOR FLOWS BETWEEN (*Value*) TO (*Value*).

This warning specified a range of flows at which routing might be numerically unstable. This warning occurred in three routing reaches. The hydrographs of all three routing reaches were within the range of the specified flows, and these routing reaches were examined. These hydrographs showed no signs of instability and the warning was considered inconsequential.

The warning message encountered for the 100-year, 6-hour HEC-1 model is as follows:

- WARNING --- MODIFIED PULS ROUTING MAY BE NUMERICALLY UNSTABLE FOR FLOWS BETWEEN (*Value*) TO (*Value*).

This warning specified a range of flows at which routing might be numerically unstable. This warning occurred in three routing reaches. One of them became unstable at a range specified by the program that was lower than the flow being routed and the warning was ignored. The hydrographs of the other two that were within the range of specified flows were examined. These hydrographs

#### 4.3.2 Modeling Warning and Error Messages

There were no ERROR messages in the HEC-1 models. Warning messages generated by the HEC-1 were examined to ensure that the models were not adversely affected.

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- WARNING --- MODIFIED PULS ROUTING MAY BE NUMERICALLY UNSTABLE FOR FLOWS BETWEEN (*Value*) TO (*Value*).

This warning specified a range of flows at which routing might be numerically unstable. This warning occurred in three routing reaches. The hydrographs of all three routing reaches were within the range of the specified flows, and these routing reaches were examined. These hydrographs showed no signs of instability and the warning was considered inconsequential.

The warning message encountered for the 100-year, 6-hour HEC-1 model is as follows:

- WARNING --- MODIFIED PULS ROUTING MAY BE NUMERICALLY UNSTABLE FOR FLOWS BETWEEN (*Value*) TO (*Value*).

This warning specified a range of flows at which routing might be numerically unstable. This warning occurred in three routing reaches. One of them became unstable at a range specified by the program that was lower than the flow being routed and the warning was ignored. The hydrographs of the other two that were within the range of specified flows were examined. These hydrographs

for the routes showed no signs of instability, and the warning was considered inconsequential.

The warning message encountered for the 100-year, 24-hour HEC-1 model is as follows:

- WARNING --- MODIFIED PULS ROUTING MAY BE NUMERICALLY UNSTABLE FOR FLOWS BETWEEN (*Value*) TO (*Value*).
- WARNING --- TIME STEP FAILED TO CONVERGE. STABILITY PROBLEMS MAY RESULT

The first warning specified a range of flows at which routing might be numerically unstable. This warning occurs in two routing reaches. The hydrographs were within the range of specified flows, and were examined. These hydrographs for the routes showed no signs of instability, and the warning was considered inconsequential.

The last warning message occurred in routing reach RF02W and RE02W. These routing reaches are storm sewers that are under capacity for the 100-year, 24-hour storm. This warning was considered inconsequential because the hydrograph showed no signs of irregularity and the results appear to be reasonable.

#### 4.4 Calibration

Flows estimated for this project were based on a precipitation/runoff model using HEC-1 software and the methodology and procedures outlined by *The FCDMC Drainage Manual*. The result obtained during this study were plotted and compared with gage data collected by the Flood Control District of Maricopa County (Reference 17). These results were also compared with regional regression

equations. **Figure 3** shows that the flows estimated by the model are generally below the average values estimated from other means, but within the data envelope. This is to be expected because the shape of the watershed. The Granite Reef watershed is long (approximately 6 miles in length) and narrow (1/2 to 1 mile wide). This type of watershed tends to produce hydrographs that have a longer timeframe with lower peaks than the more conventional watersheds.

## 4.5 Final Results

### 4.5.1 Hydrologic Analysis

Results of the hydrologic analysis including the HEC-1 input files are presented in **Appendices F, G, and H**. **Table 3** shows the flow from the hydrology models at some key locations.

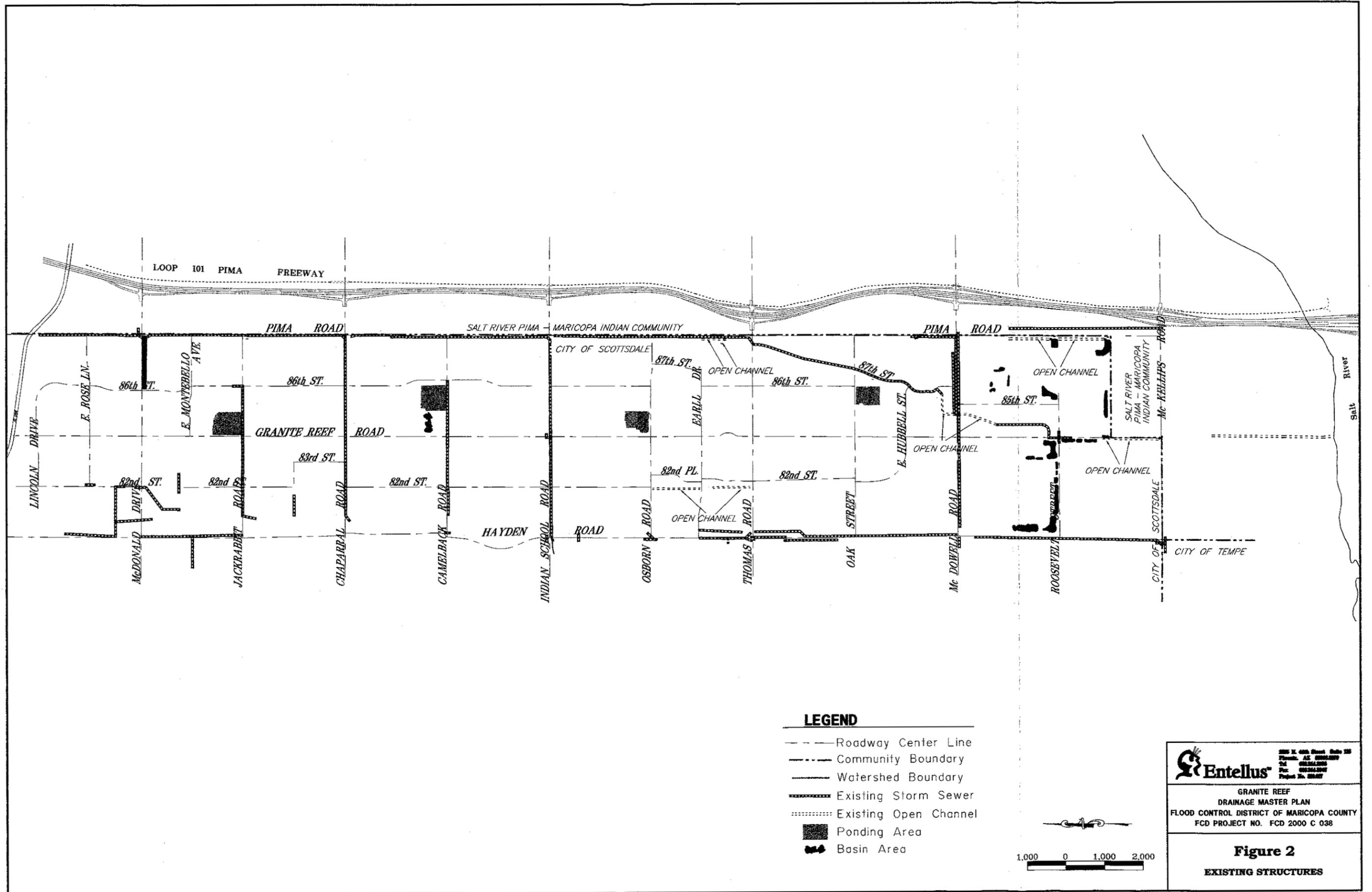
**TABLE 3**  
Summary of Discharges at Key Locations

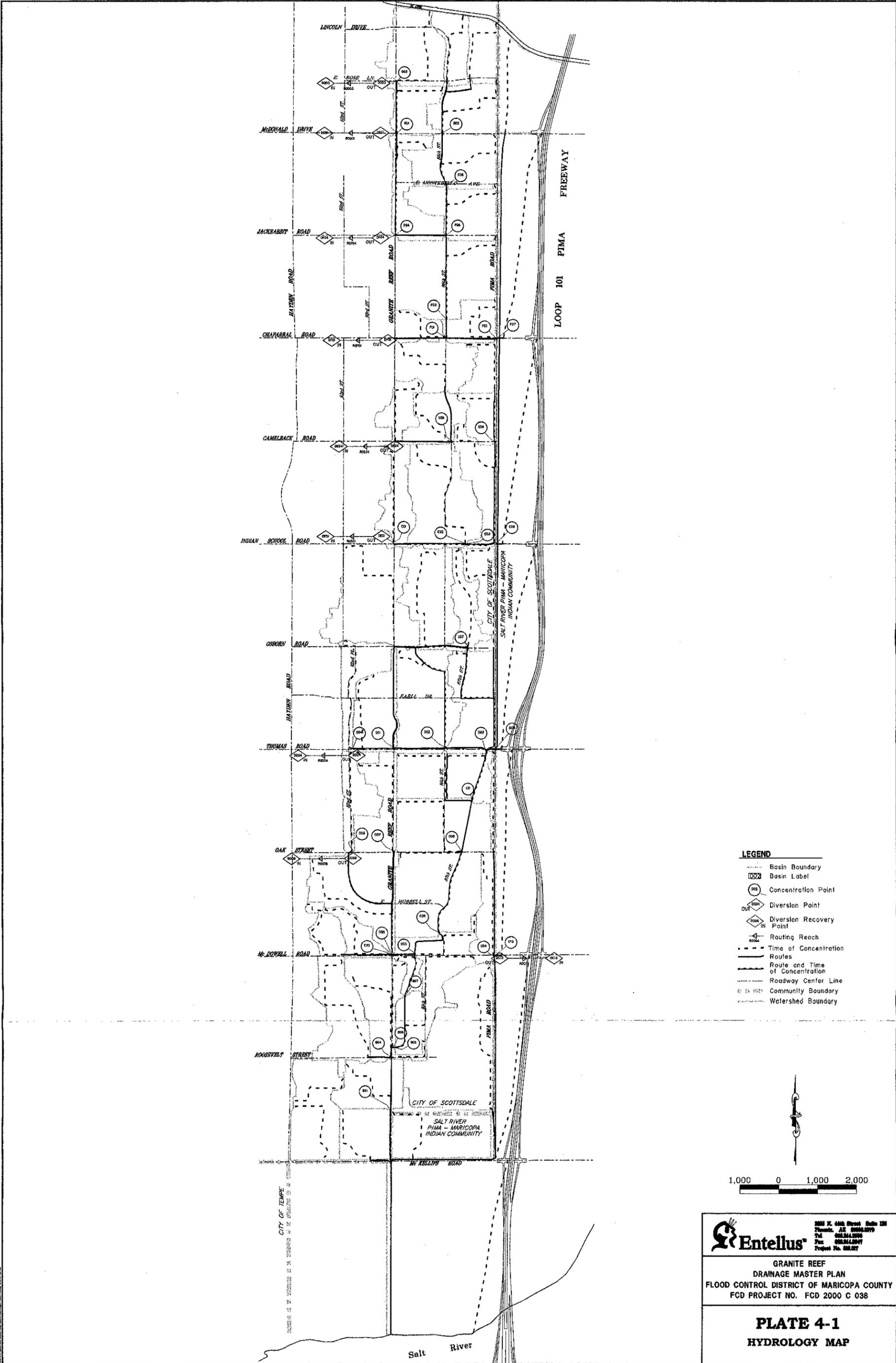
Location	Flow (cfs)		
	100-year, 6-hour	10-year, 6-hour	100-year, 24-hour
Camelback Road & 86 <sup>th</sup> Street	803	289	554
Thomas Road & Granite Reef Road	265	119	179
Thomas Road & Granite Reef Wash	1766	538	1484
McDowell Road & Granite Reef Wash	2203	739	1899
McKellips Road & Granite Reef Wash	2347	743	2122

### 4.5.2 Verification of Results

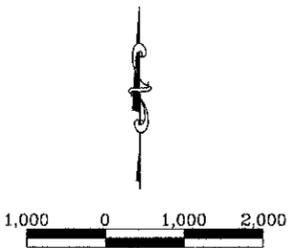
The results of the Hydrology analysis were plotted and compared with the Flood Control District gauge data (**Figure 3**) shows the data is within the lower portion of the data envelope as expected considering the physical

characteristics of the watershed. The flows are also consistent with flows estimated by other studies within the same general area.





- LEGEND**
- Basin Boundary
  - 000 Basin Label
  - 000 Concentration Point
  - OUT 000 Diversion Point
  - IN 000 Diversion Recovery Point
  - Routing Reach
  - Time of Concentration Routes
  - Route and Time of Concentration
  - Roadway Center Line
  - Community Boundary
  - Watershed Boundary

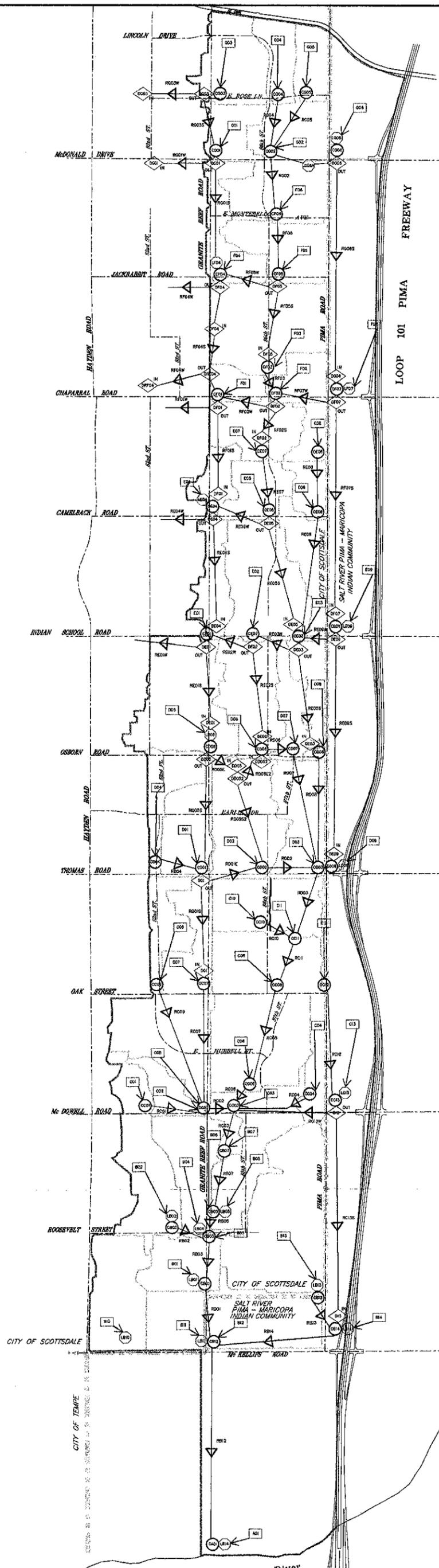


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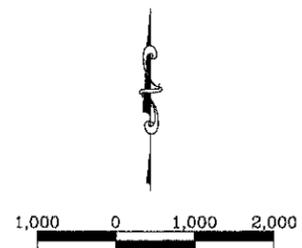
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**PLATE 4-1  
 HYDROLOGY MAP**



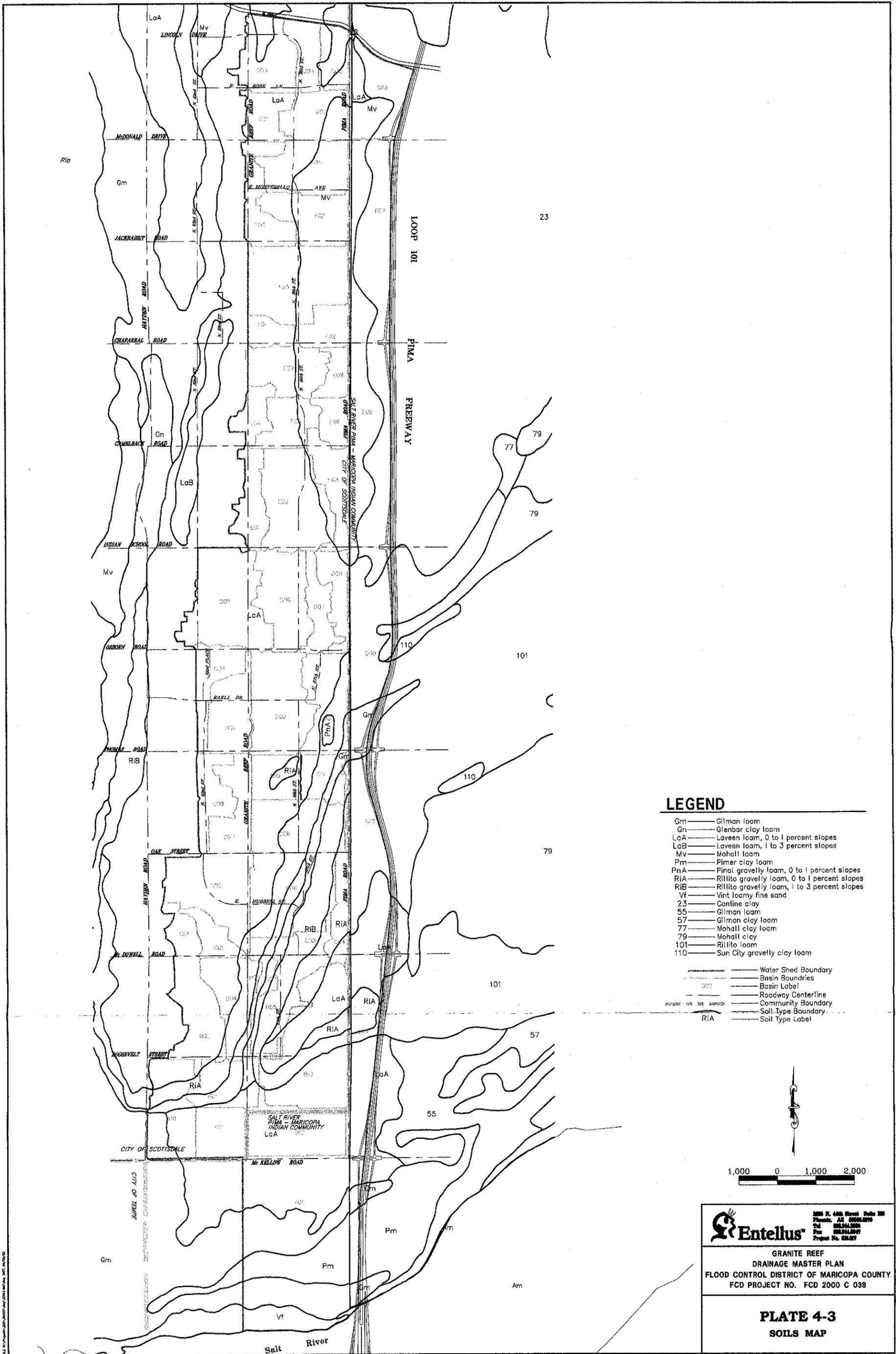
- LEGEND**
- Basin Boundary
  - Basin Label
  - Concentration Point
  - Diversion Point
  - Diversion Recovery Point
  - Routing Reach
  - Storage Routing
  - Roadway Center Line
  - Community Boundary
  - Watershed Boundary



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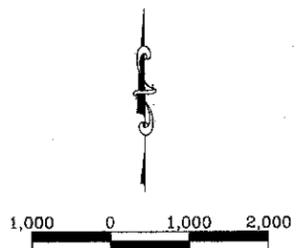
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**PLATE 4-2**  
**HEC-1 SCHEMATIC DRAWING**



**LEGEND**

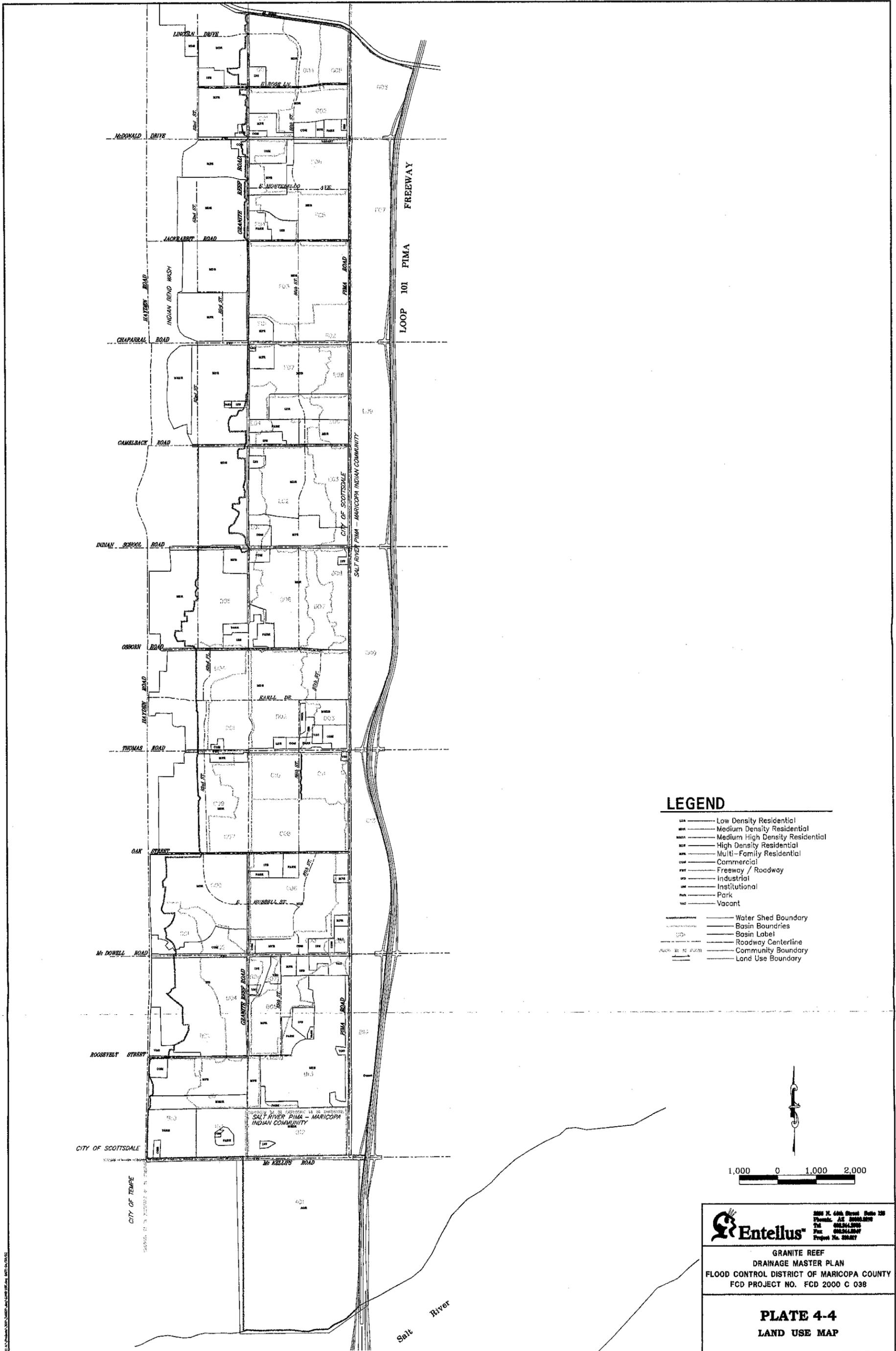
- Gm — Gilman loam
  - Gn — Glenbar clay loam
  - LaA — Laveen loam, 0 to 1 percent slopes
  - LaB — Laveen loam, 1 to 3 percent slopes
  - Mv — Mohall loam
  - Pm — Pimer clay loam
  - PnA — Pinal gravelly loam, 0 to 1 percent slopes
  - RIA — Rillito gravelly loam, 0 to 1 percent slopes
  - RIB — Rillito gravelly loam, 1 to 3 percent slopes
  - Vf — Vint loamy fine sand
  - 23 — Contine clay
  - 55 — Gilman loam
  - 57 — Gilman clay loam
  - 77 — Mohall clay loam
  - 79 — Mohall clay
  - 101 — Rillito loam
  - 110 — Sun City gravelly clay loam
- 
- Water Shed Boundary
  - Basin Boundaries
  - 002 Basin Label
  - Roadway Centerline
  - Community Boundary
  - Soil Type Boundary
  - RIA Soil Type Label




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 Project No. 02027

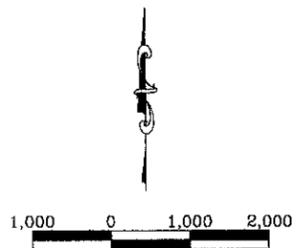
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**PLATE 4-3  
 SOILS MAP**



**LEGEND**

- LDR — Low Density Residential
- MDR — Medium Density Residential
- MHR — Medium High Density Residential
- HDR — High Density Residential
- MFR — Multi-Family Residential
- C — Commercial
- FWR — Freeway / Roadway
- I — Industrial
- IN — Institutional
- P — Park
- V — Vacant
- Water Shed Boundary
- Basin Boundaries
- Basin Label
- Roadway Centerline
- Community Boundary
- Land Use Boundary




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 FCD PROJECT NO. FCD 2000 C 038**

**PLATE 4-4  
 LAND USE MAP**

## APPENDIX A REFERENCES

### A.1 Data Collection Summary

The Data Collection summary is included in the *Granite Reef Wash Drainage Master Plan Part One (Reference 18)*.

### A.2 Reference Documents

The following is a list of the references used during the course of this study:

1. Flood Control District of Maricopa County, *Granite Reef Wash Floodplain Delineation Study*, May 1997.
2. *Flood Insurance Study for the City of Scottsdale*, Federal Emergency Management Association, June 1984.
3. *Granite Reef Wash Drainage Study*, Simons, Li and Associates, Jan. 1995.
4. *Report on Granite Reef Wash Drainage Study*, KVL Consultants Inc., Feb 1995.
5. *Granite Reef Watershed Flood Mitigation Candidate Assessment Report*, Primatech Engineers and Consultants, Oct 1999.
6. City of Scottsdale 100 scale, 1 foot contour mapping
7. SRPMIC Aerial Photography
8. City of Scottsdale, Aerial Photography
9. U.S. Department of Agriculture, Soil Conservation Service, *Soil Survey of Aguila-Carefree Area*, April 1986.
10. U.S. Department of Agriculture, Soil Conservation Service, *Soil Survey of Eastern Maricopa County and Northern Pinal Counties*, November 1974.
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*(Version 1.5)*, March 2000.
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23. Chow, V.T., *Open Channel Hydraulics*, McGraw-Hill, 1959
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*Delineation of Riverine Floodplains in Arizona*

**APPENDIX B GENERAL DOCUMENTATION AND CORRESPONDANCE**

**This will be included at the end of the project.**



APPENDIX C SURVEY FIELD NOTES



**PRELIMINARY ONLY**

82ND STREET

82ND



N.T.S.

HIGHLAND

End. Nail

ROAD 000°00'00"E  
P=53.30 1399.913'

REFEED RD. 000°01'55"E  
P=52.93  
P=52.82  
P=52.64  
P=52.50  
P=52.36  
P=52.18  
P=52.03

GRANITE

86TH STREET

86TH

End. Brass Cap (#3)  
EL=1252.9871

P=52.76 P=52.87 P=52.94  
S89°52'58"W  
P=53.05 (1311.902')

side slope west = 30.02  
3:10

CAMELBACK RD.

End. Brass Cap (#1)

RD.

Not Found

(#24) TC=53.26  
G=52.69  
(#46) NG=53.15  
(#24) TC=52.83  
G=52.27

side slope south = 31.4  
3:10

TC=52.97  
G=52.06  
NG=53.38 (#45)  
TC=52.45 (#20)  
G=51.89

side slope south = 26.8  
3:10

TC=53.24  
G=52.77  
(#40)  
NG=53.53  
(#33) TC=53.06  
G=52.59

side slope west = 30.16  
3:10

TC=52.99  
G=52.50  
NG=53.25  
TC=52.48  
G=51.97

P=53.15 P=53.21 P=53.43 P=53.30  
w=48'

P=52.57

(#24) TC=53.26  
G=52.69

P=52.64

P=52.50

(1041.098')

P=52.36

P=52.18

P=52.03

MONTECITO

AVE.

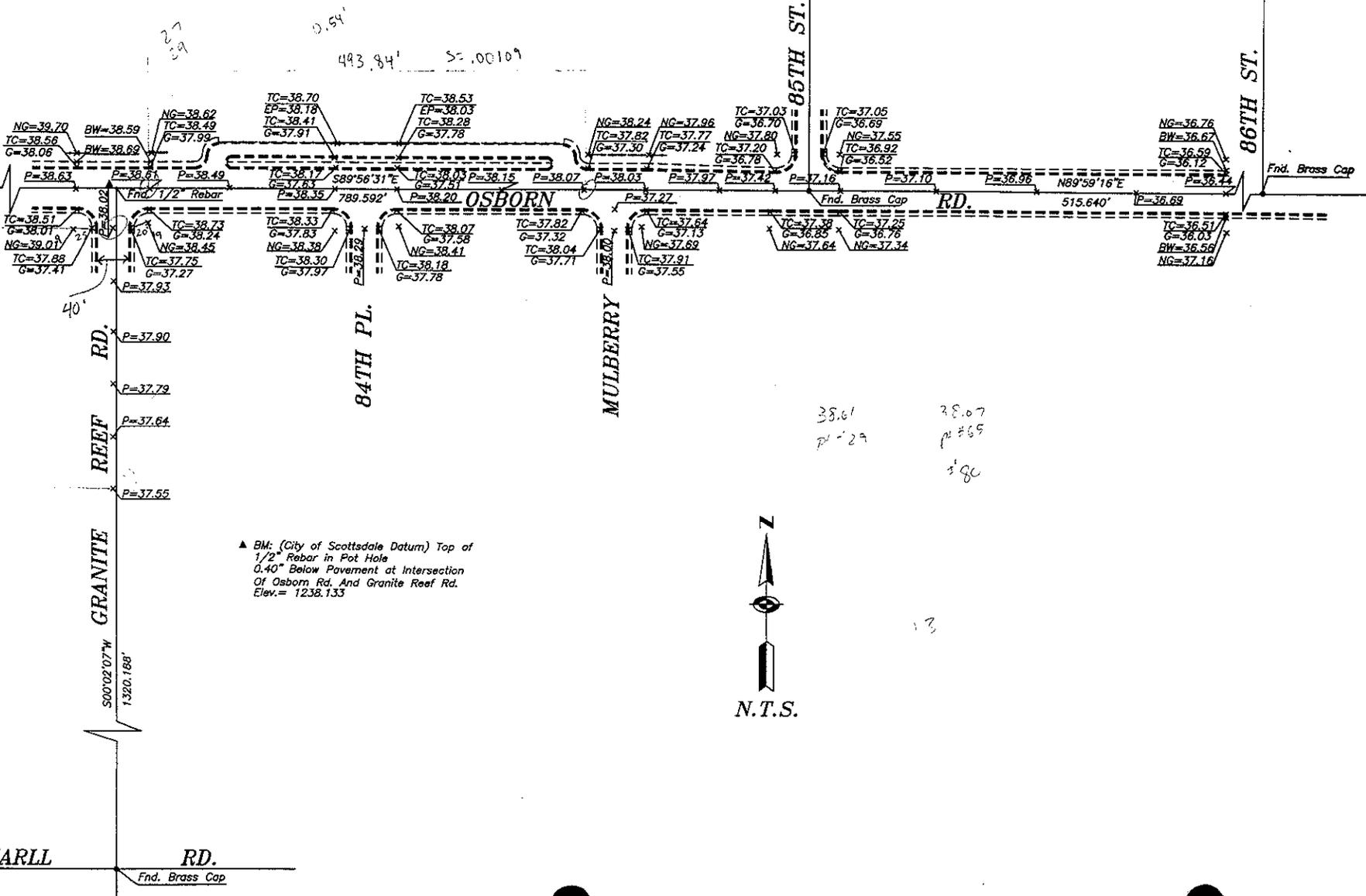
End. Brass Cap (#4)

▲ BM: (City of Scottsdale Datum)  
Top Of Brass Cap Flush  
At Intersection of Granite Reef Rd. &  
Camelback Rd. Elev=1252.932

B2ND STREET

**PRELIMINARY ONLY**

Fnd. Brass Cap 1310.724'



▲ BM: (City of Scottsdale Datum) Top of 1/2" Rebar in Pot Hole 0.40" Below Pavement at Intersection of Osborn Rd. And Granite Reef Rd. Elev.= 1238.133



38.61  
P=29

38.07  
P=65  
i.g.c.

13

**PRELIMINARY ONLY**

HAYDEN RD.

HAYDEN



N.T.S.

ST.

82ND

ST.

82ND

ST.

REEF RD.

GRANITE

P=28.43 P=28.44 N90.00.01 E P=28.27 P=28.30 P=28.48 P=28.60 P=28.58 P=28.60 P=28.66

Fnd. Brass Cap HH

OAK

25'-0" / 25'-0" / 25'-0"

P=28.40 Not Found

Not Found P=28.58

ST.

Fnd. Brass Cap

TC=27.57  
G=28.03  
NG=28.29  
TC=27.87  
G=27.56  
P=28.03

TC=28.10  
G=27.58  
NG=28.13  
TC=27.93  
G=27.57

No Split  
All flow is South.

▲ BM: (City of Scottsdale Datum) Top of Fnd. Brass Cap In Hand Hole At Intersection of Hayden Rd. & Oak Street Elev=1227.101'

CYPRESS

Fnd. 2" Iron Pipe

**PRELIMINARY ONLY**

PL.

82ND

GRANADA

RD.

Fnd. Brass Cap

*No Split, all flow east in the main*

ST.

84TH

Not Found

Not Found

McDOWELL

Fnd. Brass Cap HH

RD.

TC=18.59  
G=18.11  
BW=18.73

TC=17.55  
BW=18.14

NG=17.79\*

TC=17.56  
G=17.07  
BW=17.67

GRANITE REEF RD.

P=16.75 P=16.55 P=16.91 P=17.15 P=17.42 P=17.69 P=18.36 P=18.37

TC=18.34  
G=17.85  
NG=14.46

TC=18.15  
G=17.67

TC=17.42  
G=16.97  
NG=17.84  
BW=17.54

TC=18.21  
G=17.70  
NG=16.60

TC=18.25  
G=17.75

BW=17.97  
TC=17.87  
G=17.42

TC=18.53  
EP=18.01

TC=19.24  
EP=18.60

TC=19.33  
EP=18.82

TC=18.53  
G=18.04

TC=18.21  
G=17.70  
NG=15.98

N00°00'00"E  
799.255  
P=18.43  
P=18.62

▲ BM: (City of Scottsdale Datum)  
Top Of Fnd. Brass Cap In Hand Hole  
At Intersection Of McDowell Rd.  
And Granite Reef Rd.  
Elev= 1218.037'



N.T.S.

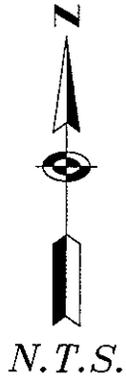
BELLEVUE ST.

Not Found

HAYDEN RD.

RD.

**PRELIMINARY ONLY**



THOMAS

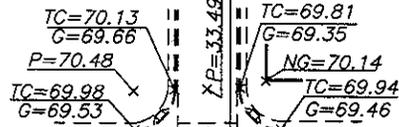
STREET

82ND

EDGEMONT

AVE.

REEF RD.  
GRANITE



No Split, all flow east.

P=34.33 P=34.18

P=33.99 P=33.78 P=33.70 P=33.67 P=33.68 P=33.57

TC=33.85 Not Found  
EP=33.34

TC=69.74  
EP=69.26

RD.

P=33.51 P=33.4

Fnd. Brass Cap HH

P=33.15

P=33.19

TC=33.19  
G=32.85

TC=33.30  
G=32.93

NG=33.56  
TC=32.98  
G=32.65

NG=33.67  
TC=33.00  
G=32.67

▲ BM: (City of Scottsdale Datum) Top of Fnd. Brass Cap In Hand Hole At Intersection of Granite Reef Rd. & Thomas Rd. Elev=1232.666'

Fnd. Brass Cap

82ND STREET

86TH STREET

PRELIMINARY ONLY



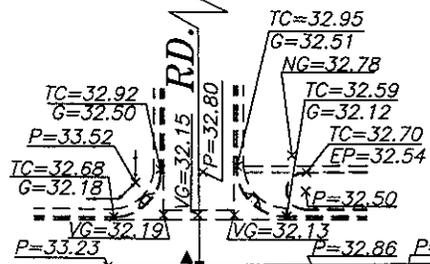
N.T.S.

THOMAS RD.

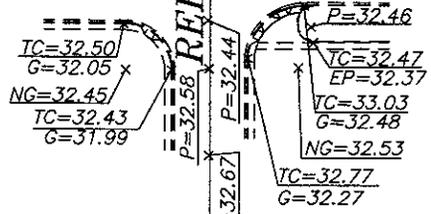
GRANITE REEF RD.

OAK ST.

N00°31'39"W  
330.145'  
Fnd. Brass Cap



No Split, all flow east.



▲ BM: (City of Scottsdale Datum) Top of Fnd. Brass Cap In Hand Hole At Intersection of Granite Reef Rd. & Thomas Rd. Elev=1232.666'

N00°00'00"E  
2647.707'  
P=32.36  
P=32.20

Fnd. Brass Cap

**PRELIMINARY ONLY**

CATALINA DR.

Fnd. Brass Cap

No split, all flow east.

STREET

Fnd. Brass Cap HH

PIMA

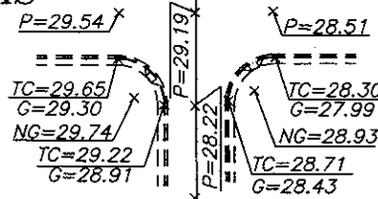
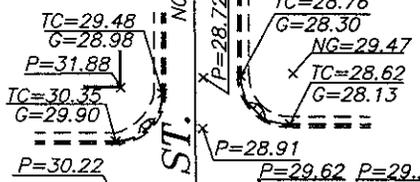
N89°26'12"E  
1301.494'

THOMAS RD.

Fnd. PK Nail

RD.

N89°25'52"E  
1301.497'



EDGEMONT AVE.

Fnd. Brass Cap

▲ BM: (City of Scottsdale Datum) Top of Fnd. Brass Cap In Hand Hole At Intersection of Granite Reef Rd. & Thomas Rd. Elev=1232.666'

P=28.74

86TH

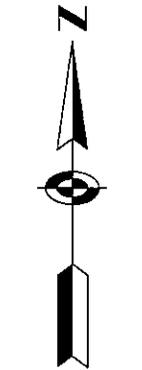
P=28.55

P=28.30

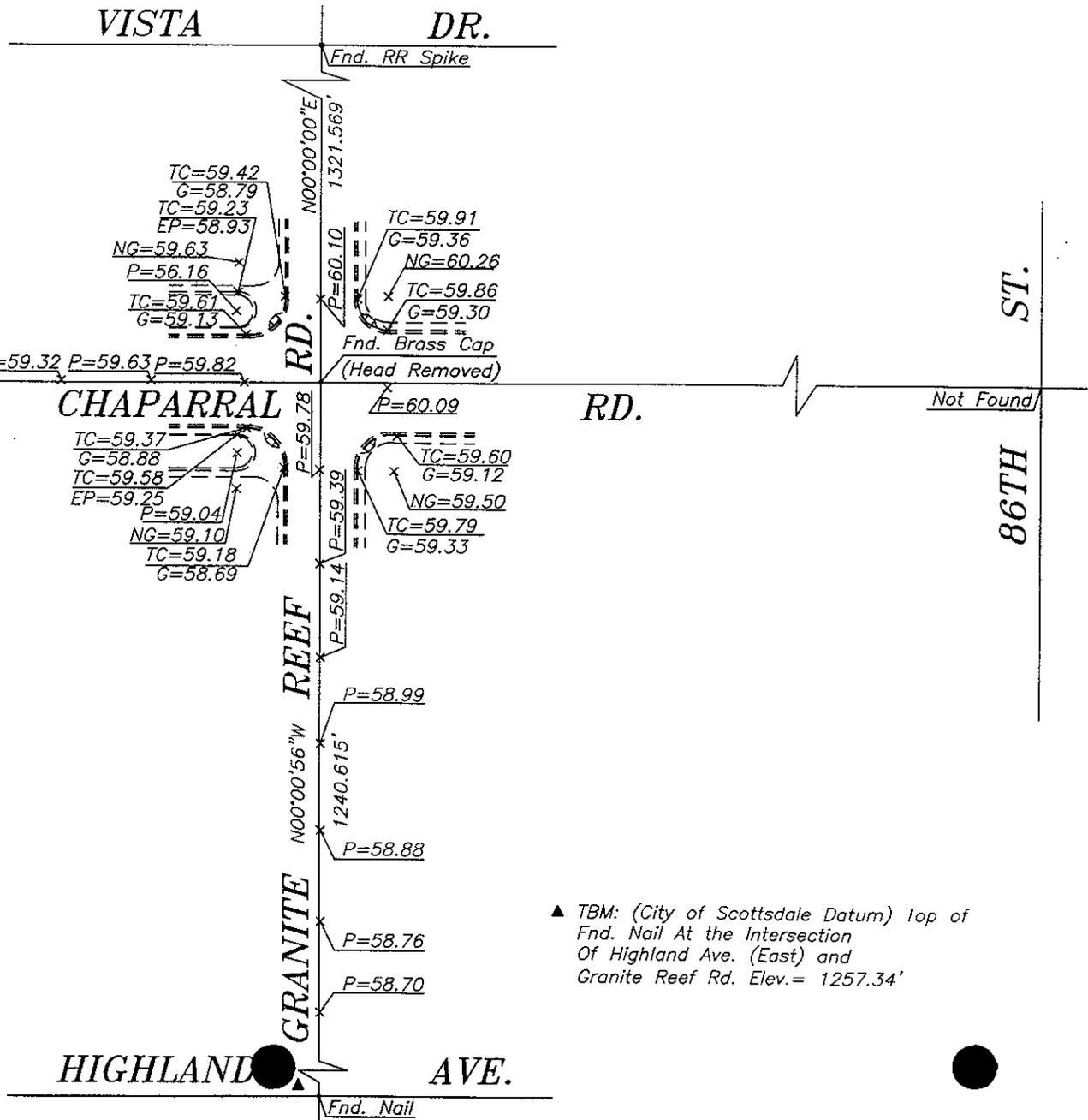


N.T.S.

**PRELIMINARY ONLY**



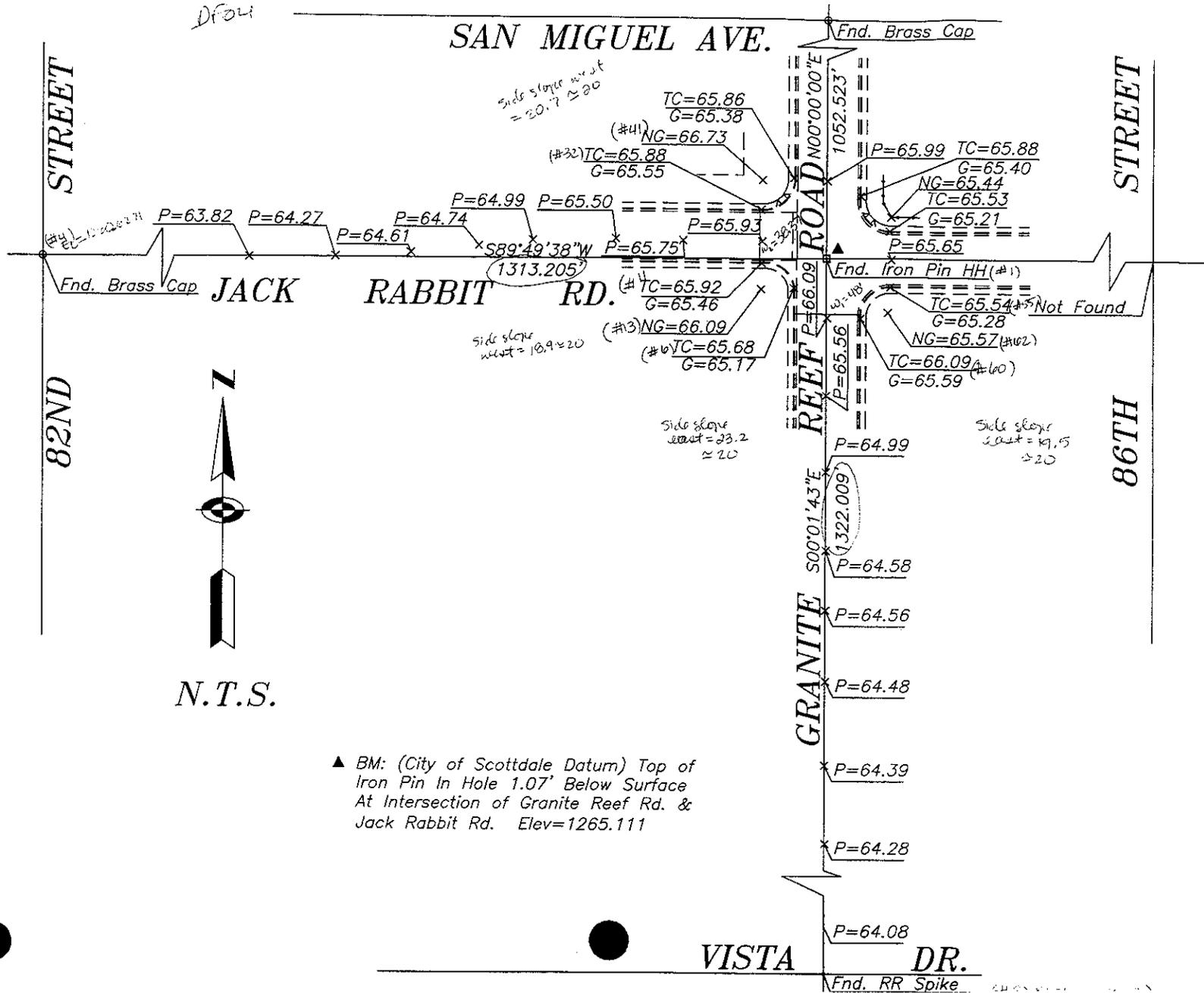
N.T.S.



▲ TBM: (City of Scottsdale Datum) Top of Fnd. Nail At the Intersection Of Highland Ave. (East) and Granite Reef Rd. Elev.= 1257.34'

**PRELIMINARY ONLY**

DFOU



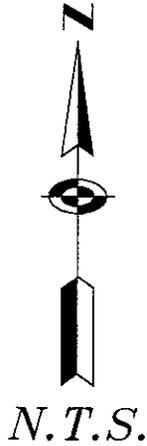
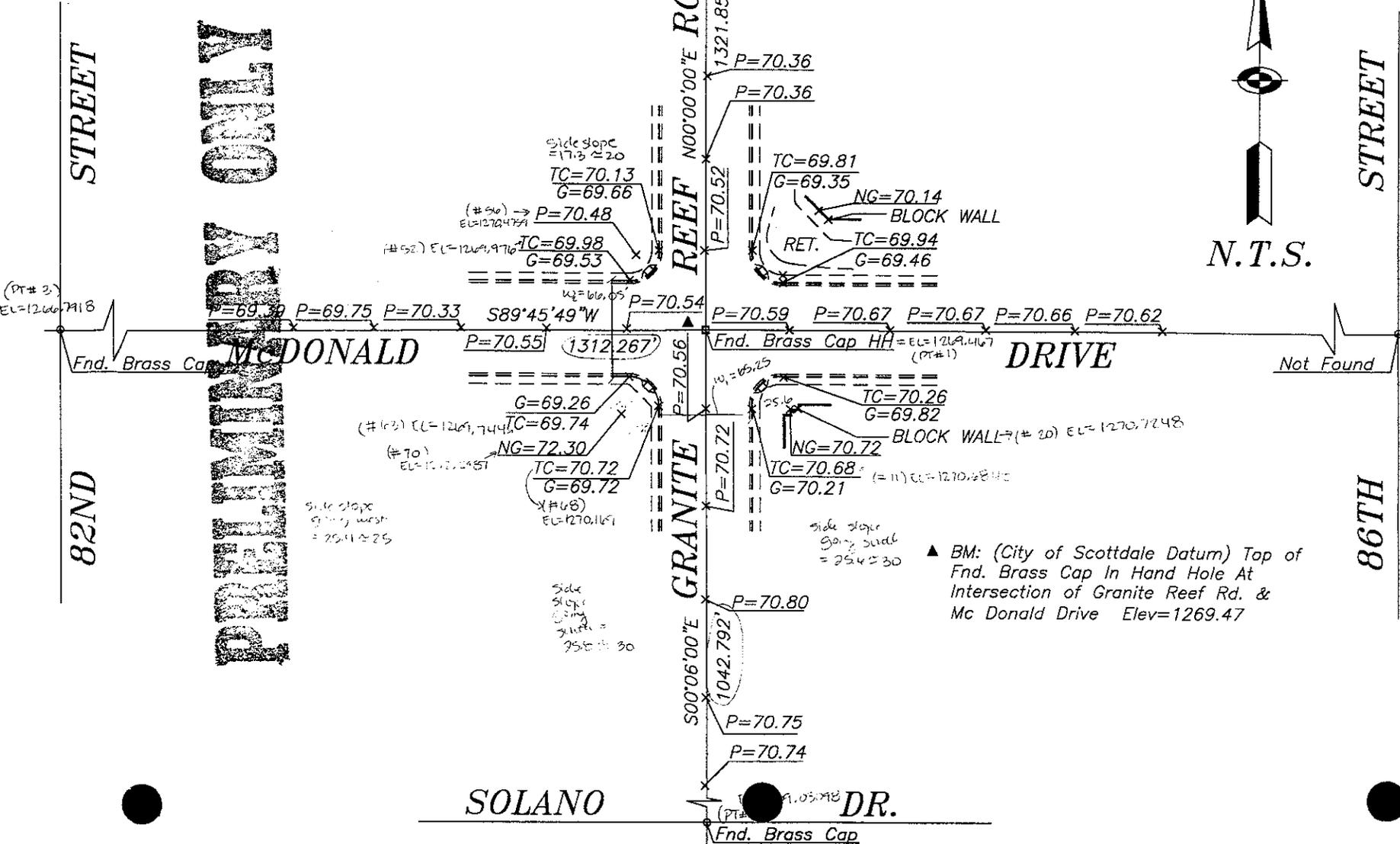
▲ BM: (City of Scottsdale Datum) Top of Iron Pin In Hole 1.07' Below Surface At Intersection of Granite Reef Rd. & Jack Rabbit Rd. Elev=1265.111

VISTA DR.

Fnd. RR Spike

Call Jit  
248-4203

Doc:



▲ BM: (City of Scottsdale Datum) Top of Fnd. Brass Cap In Hand Hole At Intersection of Granite Reef Rd. & Mc Donald Drive Elev=1269.47

## **APPENDIX D    HYDROLOGIC ANALYSIS SUPPORTING DOCUMENTATION**

**D.1    Precipitation Data**

**D.2    Physical Parameters Calculations**

**D.3    Hydrograph Routing Data**

**D.4    Storage Routing Data**

**D.5    Flow Splits and Diversions Data**



**D.1 Precipitation Data**

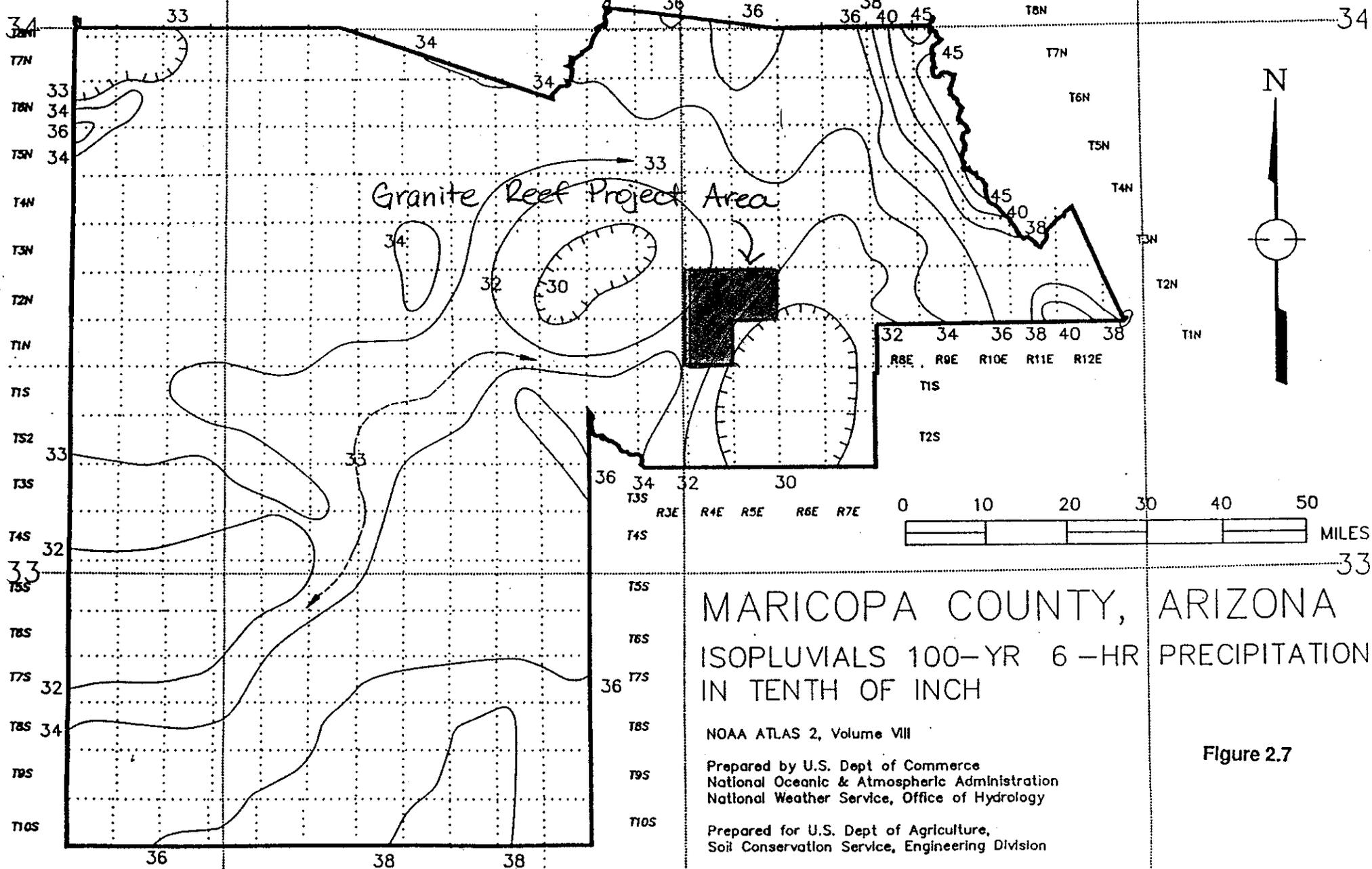


R10W R9W R8W R7W R6W R5W R4W R3W R2W R1W R1E R2E R3E R4E R5E R6E R7E R8E R9E R10E R11E R12E

113

112

111



# MARICOPA COUNTY, ARIZONA

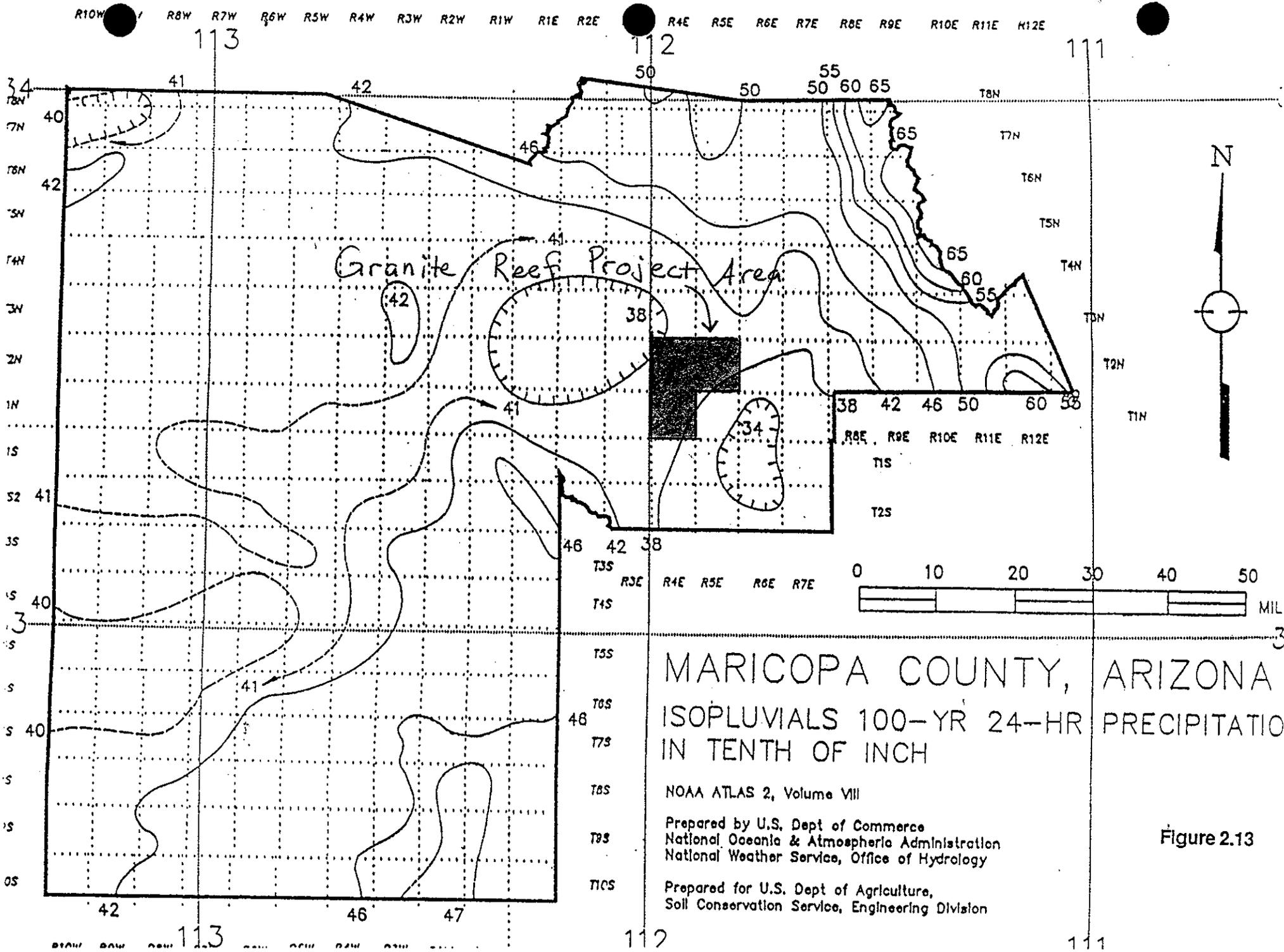
## ISOPLUVIALS 100-YR 6-HR PRECIPITATION IN TENTH OF INCH

NOAA ATLAS 2, Volume VIII

Prepared by U.S. Dept of Commerce  
National Oceanic & Atmospheric Administration  
National Weather Service, Office of Hydrology

Prepared for U.S. Dept of Agriculture,  
Soil Conservation Service, Engineering Division

Figure 2.7



MARICOPA COUNTY, ARIZONA  
 ISOPLUVIALS 100-YR 24-HR PRECIPITATIO  
 IN TENTH OF INCH

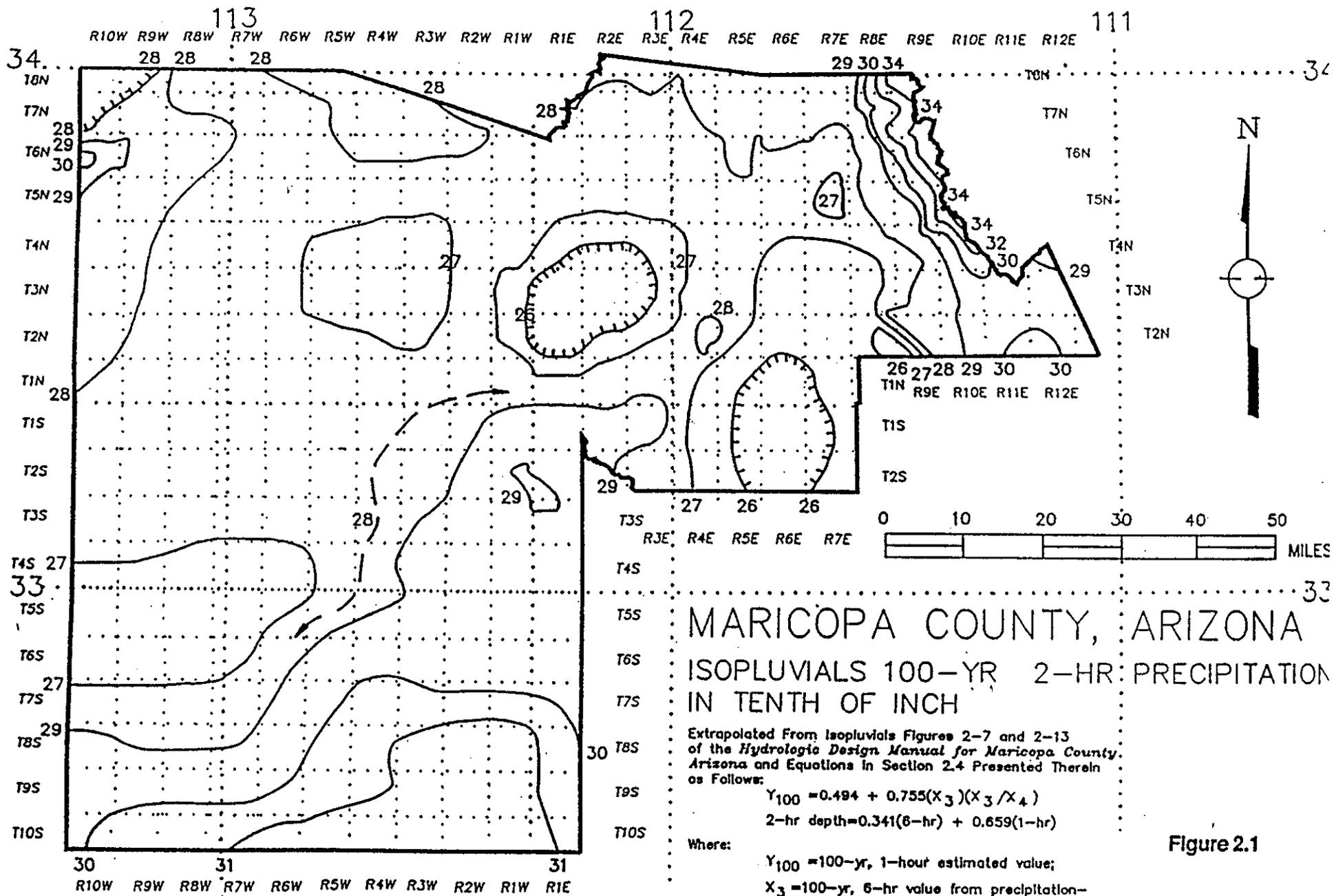
NOAA ATLAS 2, Volume VIII  
 Prepared by U.S. Dept of Commerce  
 National Oceanic & Atmospheric Administration  
 National Weather Service, Office of Hydrology  
 Prepared for U.S. Dept of Agriculture,  
 Soil Conservation Service, Engineering Division

Figure 2.13

Flood Control District of Maricopa County  
 SCOTTSDALE - GRANITE REEF ADMP  
**Rainfall Data**

Primary Zone Number: 7 Latitude: 0.0 Elevation: 0  
 Short Duration Zone Number: 8 Longitude: 0.0

Duration	Point Values (in)						
	2-Yr	5-Yr	10-Yr	25-Yr	50-Yr	100-Yr	
5 MIN	0.33	0.42	0.49	0.58	0.66	0.73	
10 MIN	0.49	0.64	0.74	0.89	1.00	1.12	
15 MIN	0.59	0.80	0.94	1.13	1.28	1.43	
30 MIN	0.79	1.07	1.26	1.53	1.74	1.94	
1 HOUR	0.96	1.32	1.57	1.90	2.17	2.43	
2 HOUR	1.04	1.45	1.73	2.10	2.40	2.69	
3 HOUR	1.10	1.54	1.83	2.24	2.55	2.87	
6 HOUR	1.20	1.70	2.03	2.49	2.85	3.20	
12 HOUR	1.30	1.88	2.26	2.79	3.20	3.60	
24 HOUR	1.40	2.06	2.49	3.09	3.54	4.00	



MARICOPA COUNTY, ARIZONA  
 ISOPLUVIALS 100-YR 2-HR PRECIPITATION  
 IN TENTH OF INCH

Extrapolated From isopluvials Figures 2-7 and 2-13  
 of the *Hydrologic Design Manual for Maricopa County,  
 Arizona* and Equations in Section 2.4 Presented Therein  
 as Follows:

$$Y_{100} = 0.494 + 0.755(X_3)(X_3/X_4)$$

$$2\text{-hr depth} = 0.341(6\text{-hr}) + 0.659(1\text{-hr})$$

Where:

- $Y_{100}$  = 100-yr, 1-hour estimated value;
- $X_3$  = 100-yr, 6-hr value from precipitation-frequency maps;
- $X_4$  = 100-yr, 24-hr value from precipitation-frequency maps;
- 6-hr-isopluvial values from figure 2.7;

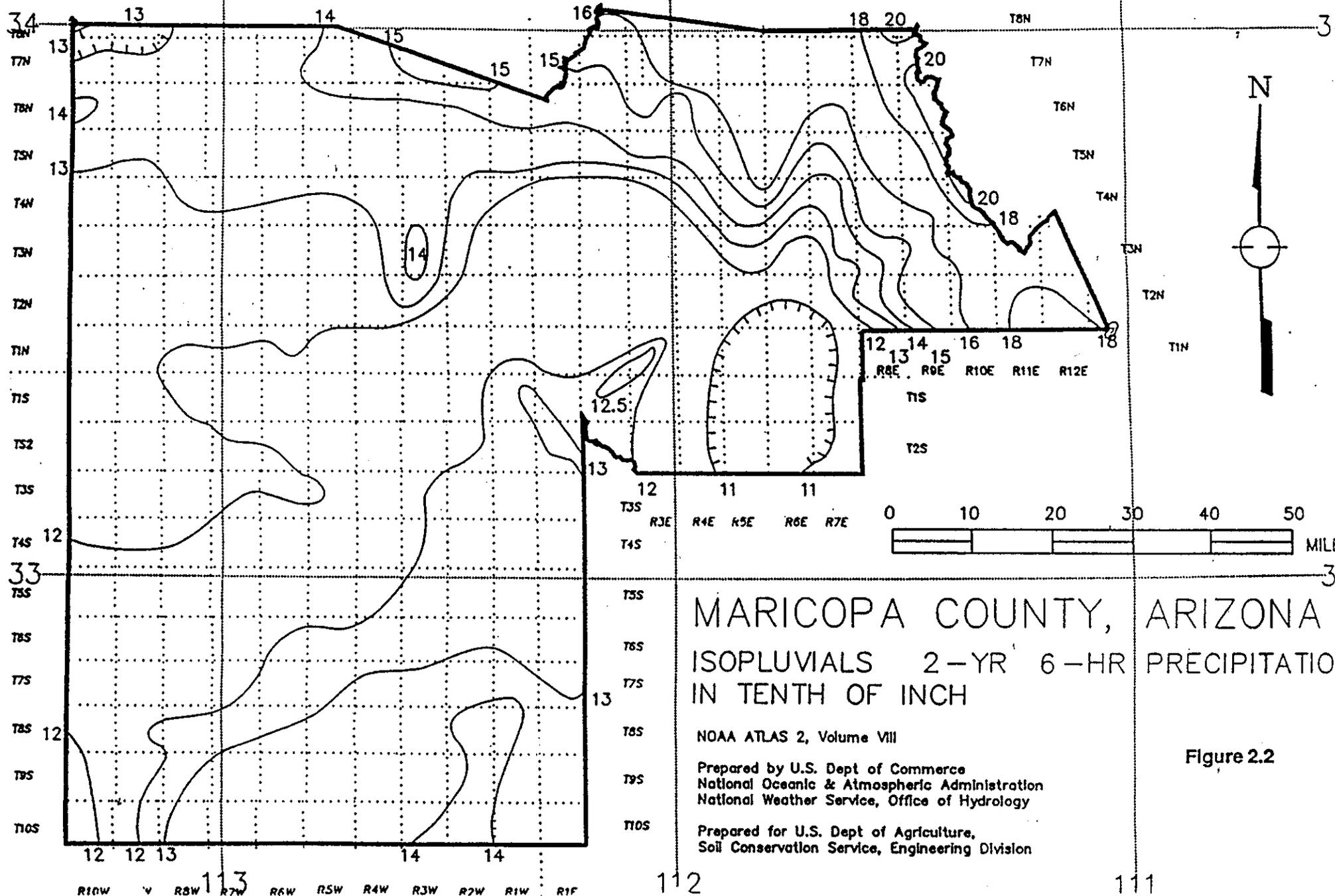
Figure 2.1

R10W R9W R8W R7W R6W R5W R4W R3W R2W R1W R1E R2E R3E R4E R5E R6E R7E R8E R9E R10E R11E R12E

113

112

111



MARICOPA COUNTY, ARIZONA  
 ISOPLUVIALS 2-YR 6-HR PRECIPITATION  
 IN TENTH OF INCH

NOAA ATLAS 2, Volume VIII

Prepared by U.S. Dept of Commerce  
 National Oceanic & Atmospheric Administration  
 National Weather Service, Office of Hydrology

Prepared for U.S. Dept of Agriculture,  
 Soil Conservation Service, Engineering Division

Figure 2.2

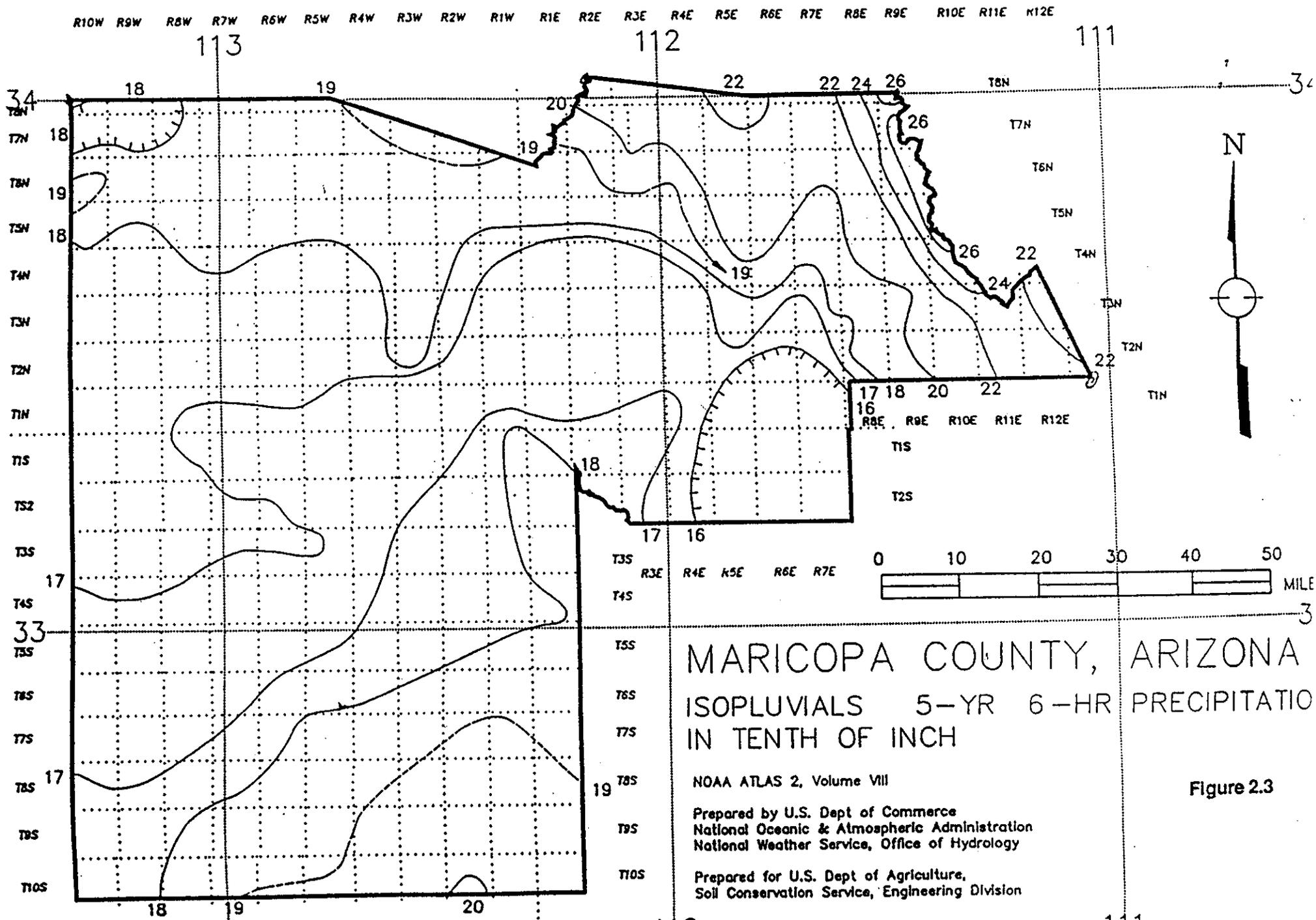
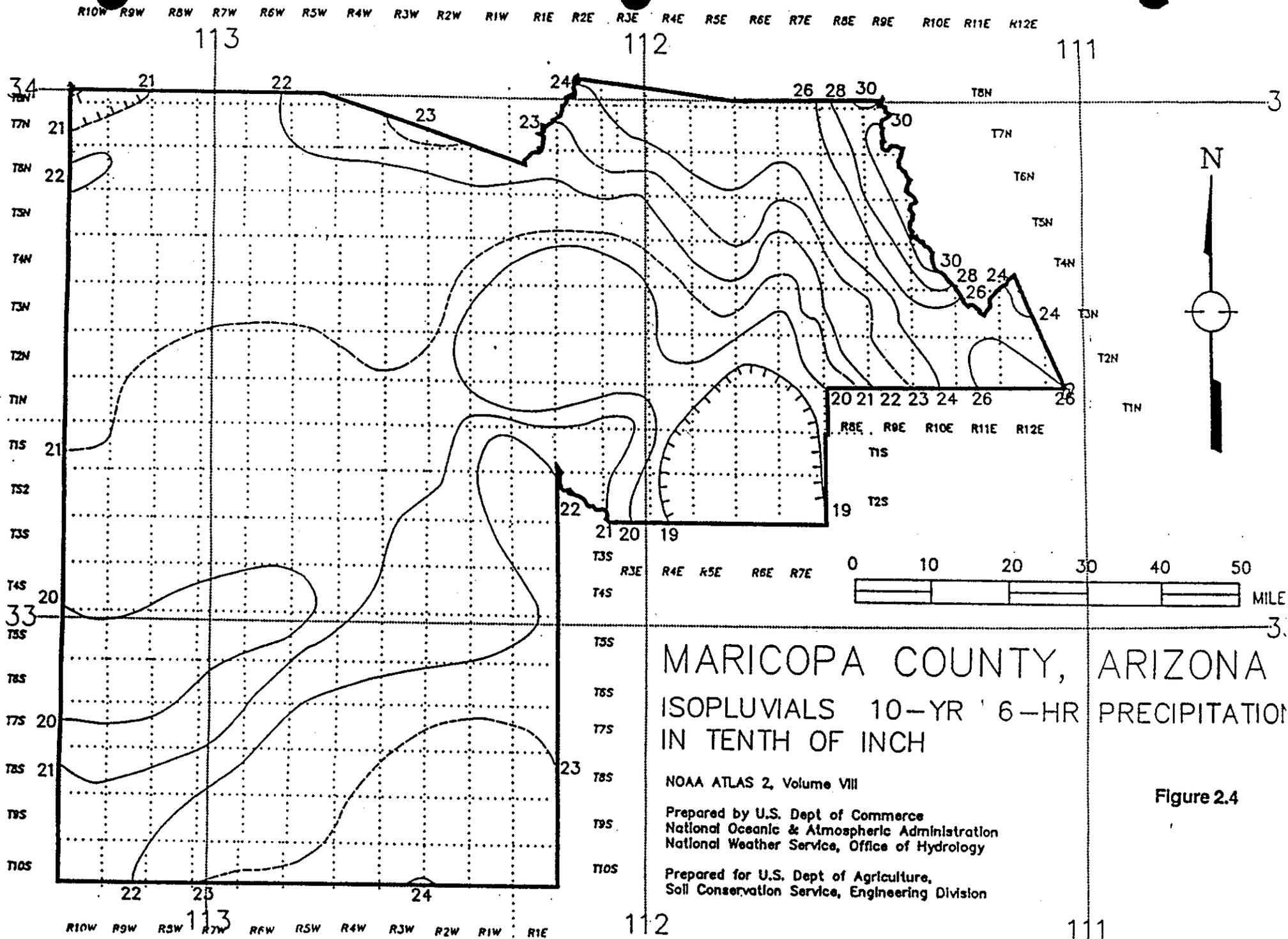


Figure 2.3



R10W R9W R8W R7W R6W R5W R4W R3W R2W R1W R1E R2E R3E R4E R5E R6E R7E R8E R9E R10E R11E R12E

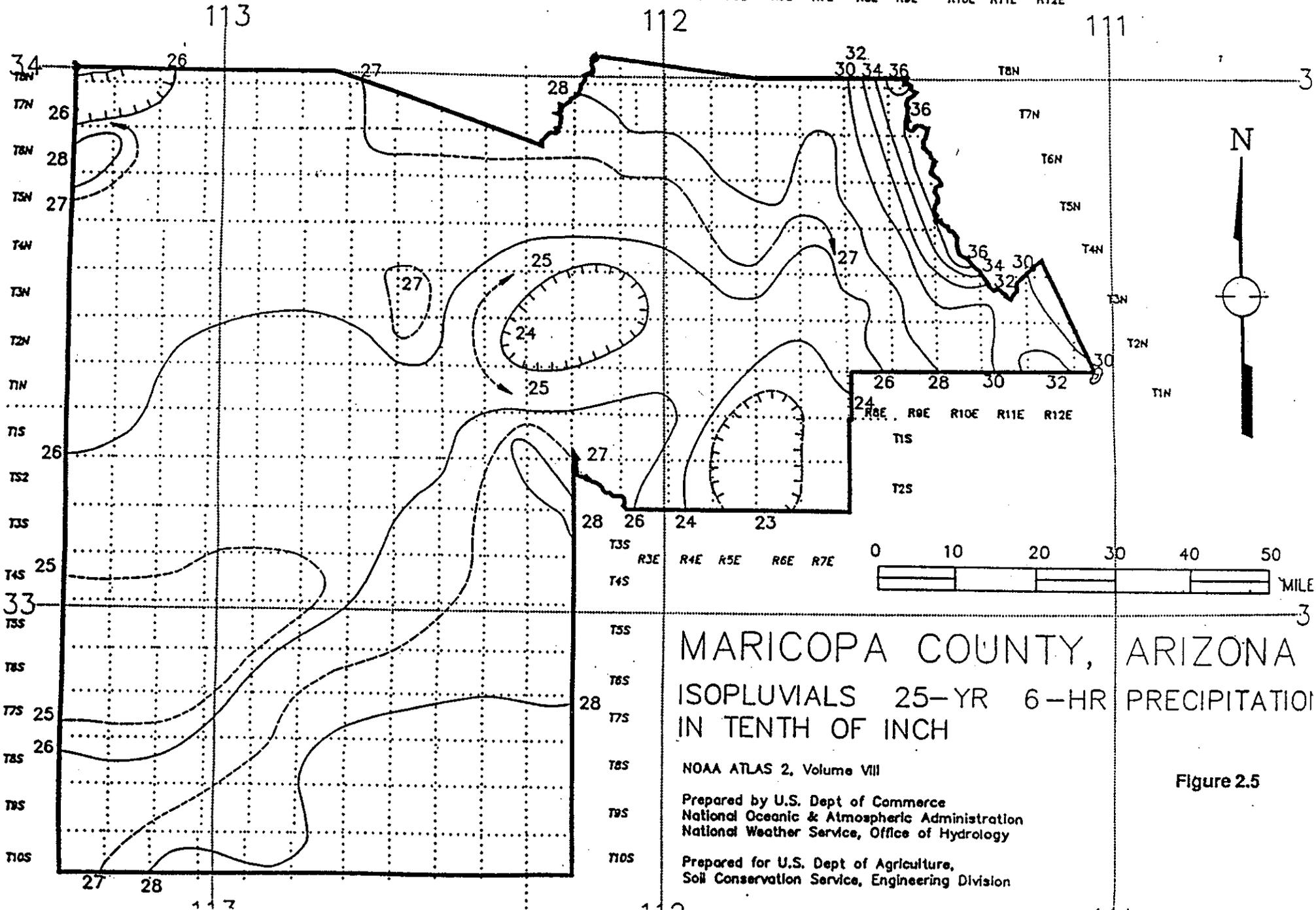


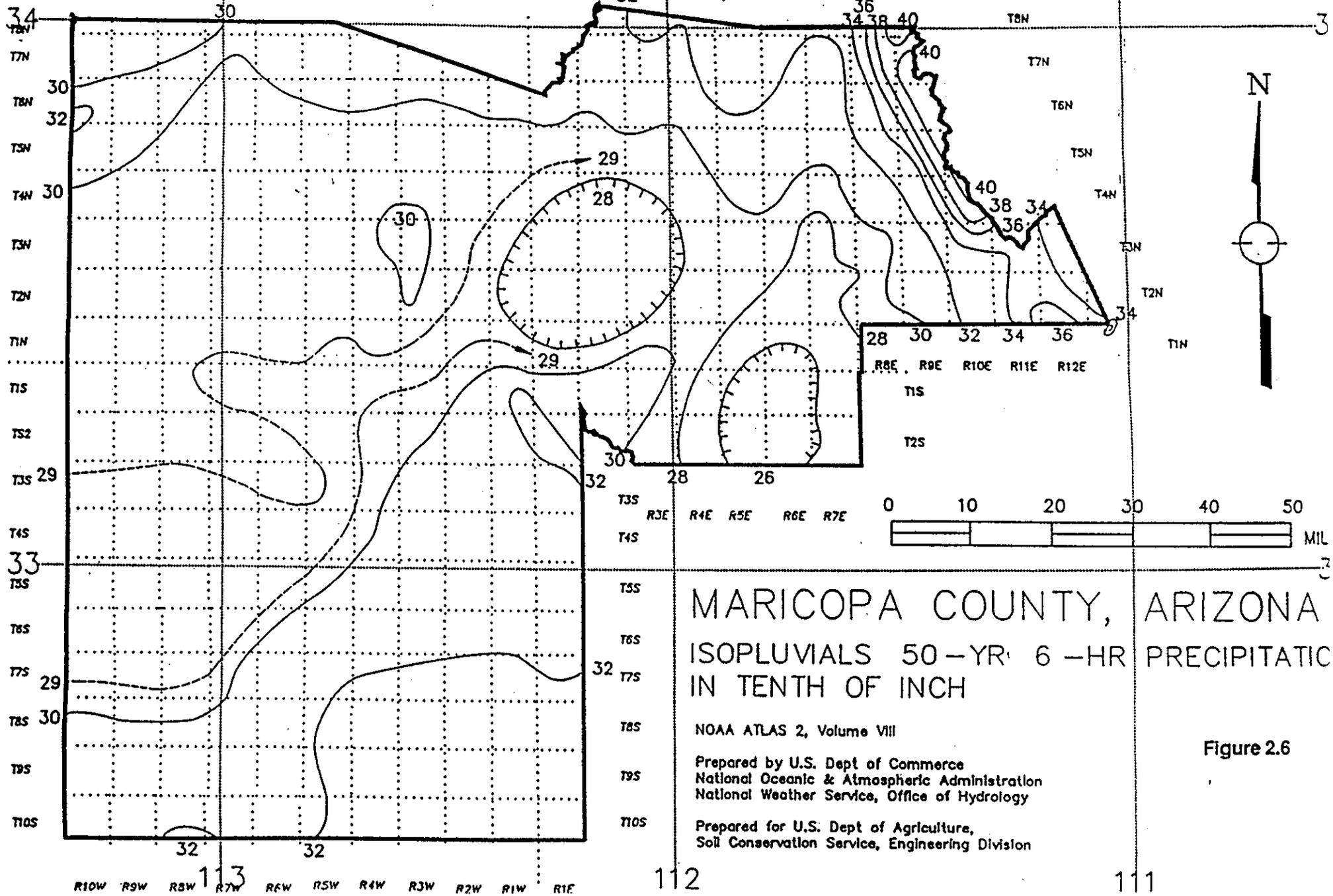
Figure 2.5

R10W R9W R8W R7W R6W R5W R4W R3W R2W R1W R1E R2E R3E R4E R5E R6E R7E R8E R9E R10E R11E R12E

113

112

111



MARICOPA COUNTY, ARIZONA  
 ISOPLUVIALS 50-YR 6-HR PRECIPITATION  
 IN TENTH OF INCH

NOAA ATLAS 2, Volume VIII  
 Prepared by U.S. Dept of Commerce  
 National Oceanic & Atmospheric Administration  
 National Weather Service, Office of Hydrology  
 Prepared for U.S. Dept of Agriculture,  
 Soil Conservation Service, Engineering Division

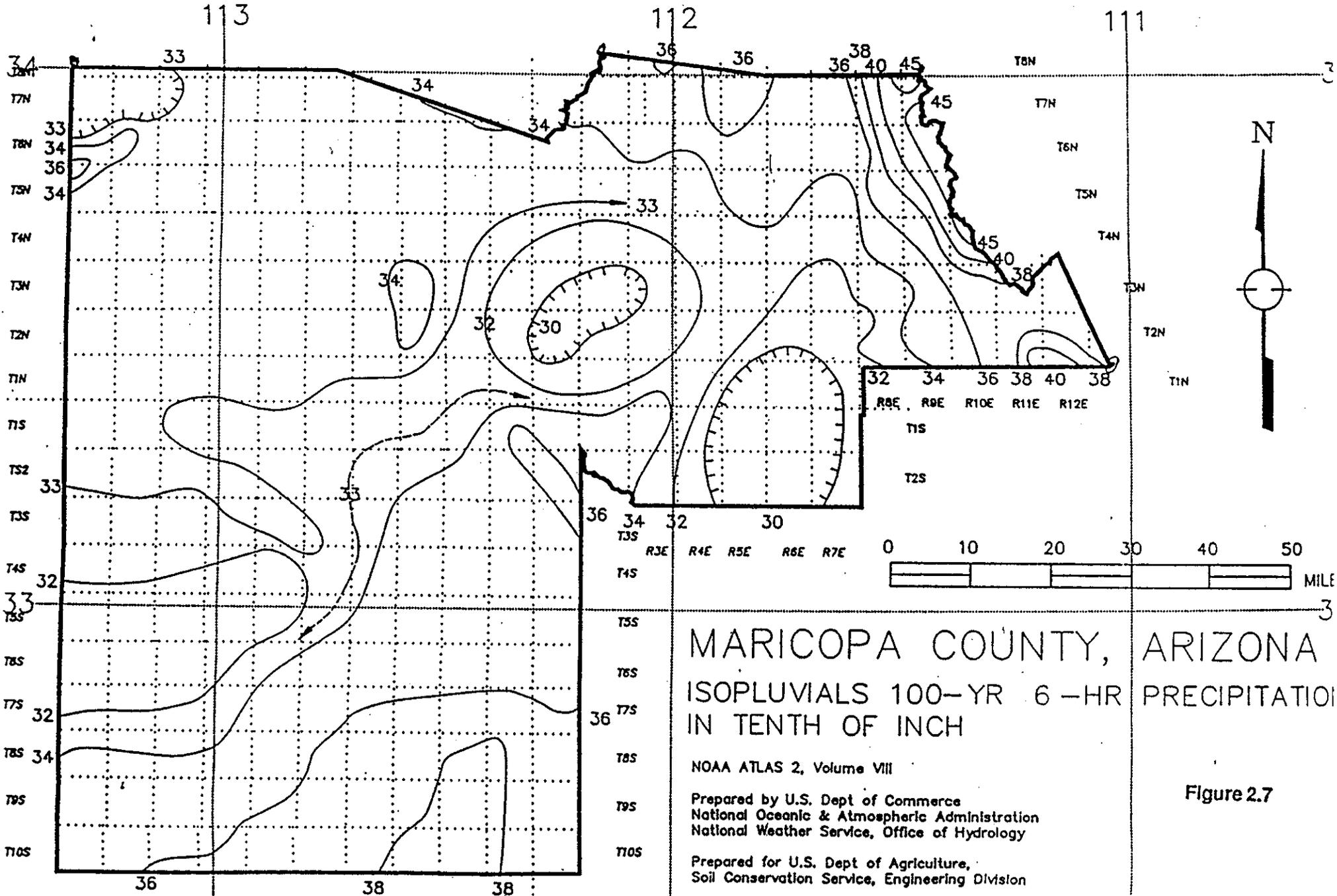
Figure 2.6

R10W R9W R8W R7W R6W R5W R4W R3W R2W R1W R1E

112

111

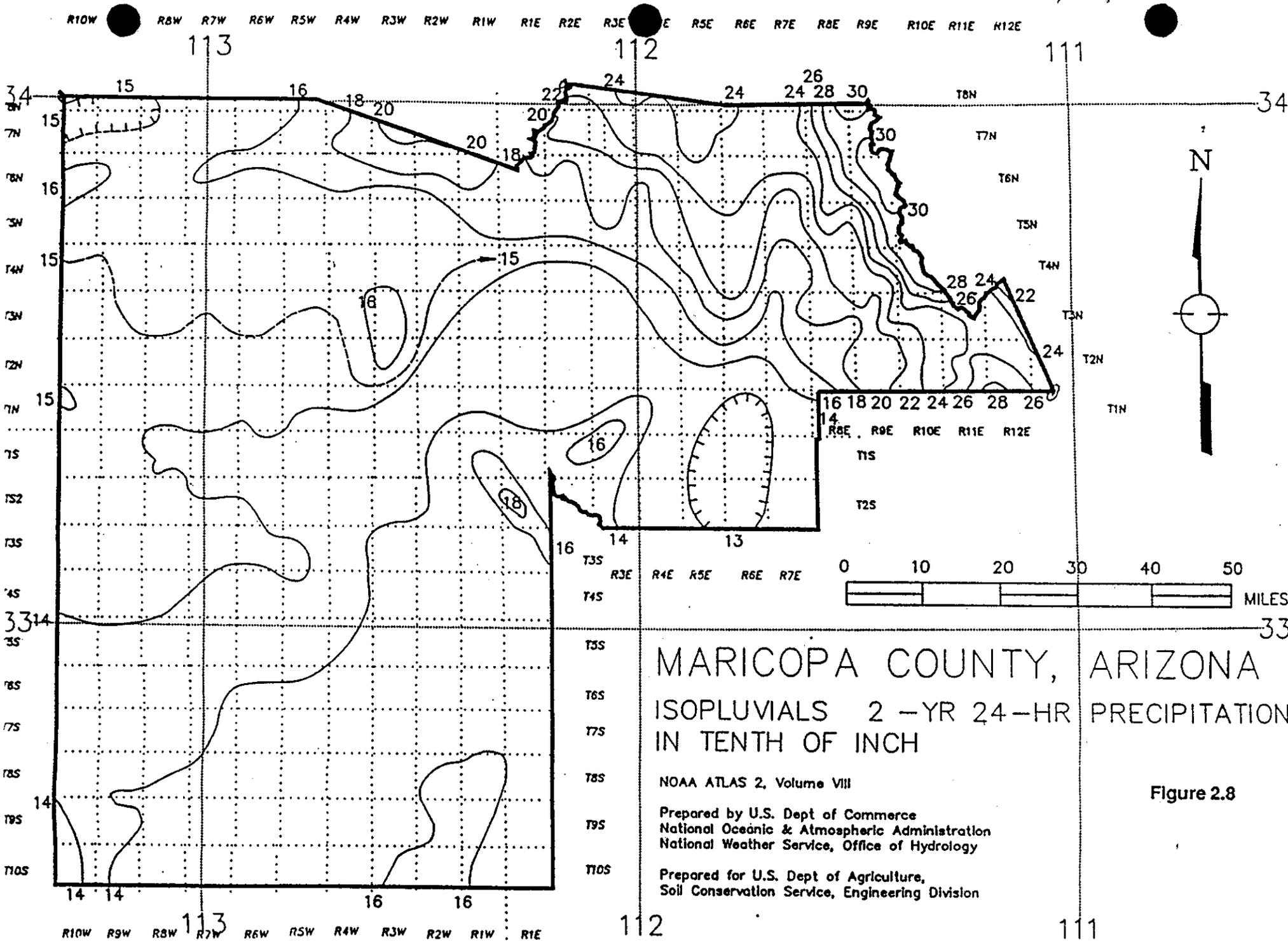
R10W R9W R8W R7W R6W R5W R4W R3W R2W R1W R1E R2E R3E R4E R5E R6E R7E R8E R9E R10E R11E R12E



MARICOPA COUNTY, ARIZONA  
ISOPLUVIALS 100-YR 6-HR PRECIPITATION  
IN TENTH OF INCH

NOAA ATLAS 2, Volume VIII  
Prepared by U.S. Dept of Commerce  
National Oceanic & Atmospheric Administration  
National Weather Service, Office of Hydrology  
Prepared for U.S. Dept of Agriculture,  
Soil Conservation Service, Engineering Division

Figure 2.7



MARICOPA COUNTY, ARIZONA  
 ISOPLUVIALS 2-YR 24-HR PRECIPITATION  
 IN TENTH OF INCH

NOAA ATLAS 2, Volume VIII  
 Prepared by U.S. Dept of Commerce  
 National Oceanic & Atmospheric Administration  
 National Weather Service, Office of Hydrology  
 Prepared for U.S. Dept of Agriculture,  
 Soil Conservation Service, Engineering Division

Figure 2.8

R10W R9W R8W R7W R6W R5W R4W R3W R2W R1W R1E R2E R3E R4E R5E R6E R7E R8E R9E R10E R11E R12E

113

112

111

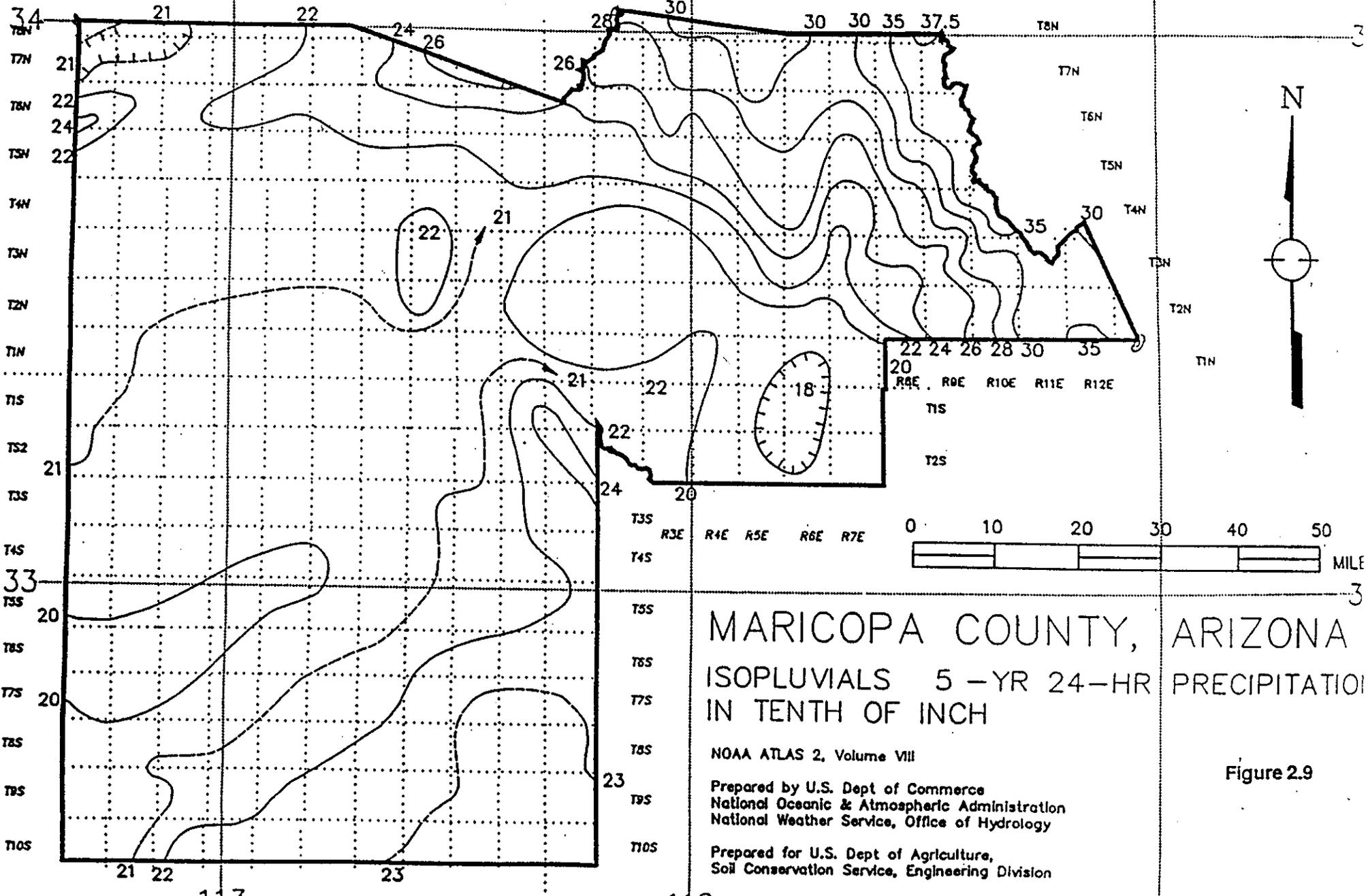
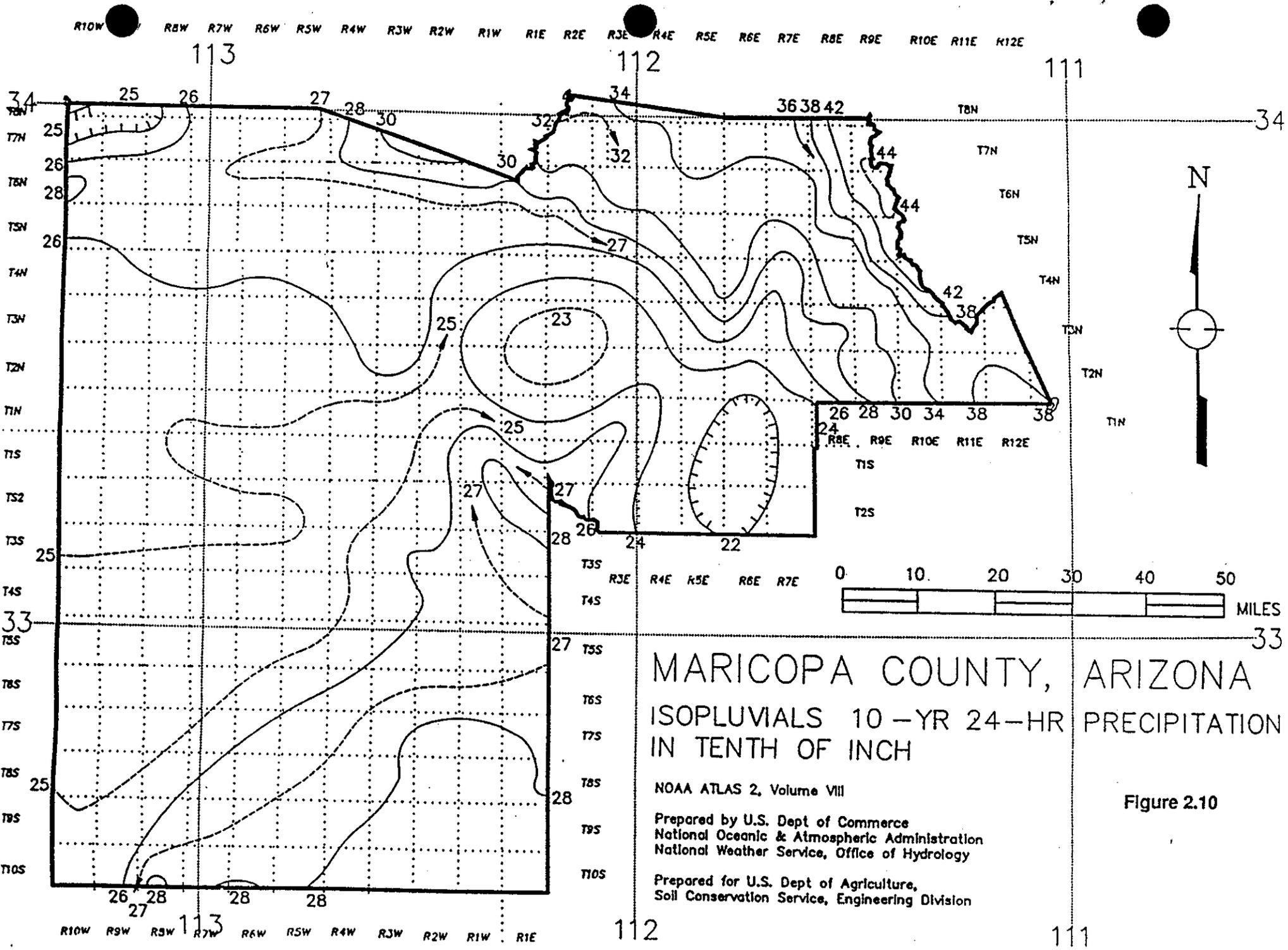


Figure 2.9



MARICOPA COUNTY, ARIZONA  
 ISOPLUVIALS 10-YR 24-HR PRECIPITATION  
 IN TENTH OF INCH

NOAA ATLAS 2, Volume VIII  
 Prepared by U.S. Dept of Commerce  
 National Oceanic & Atmospheric Administration  
 National Weather Service, Office of Hydrology  
 Prepared for U.S. Dept of Agriculture,  
 Soil Conservation Service, Engineering Division

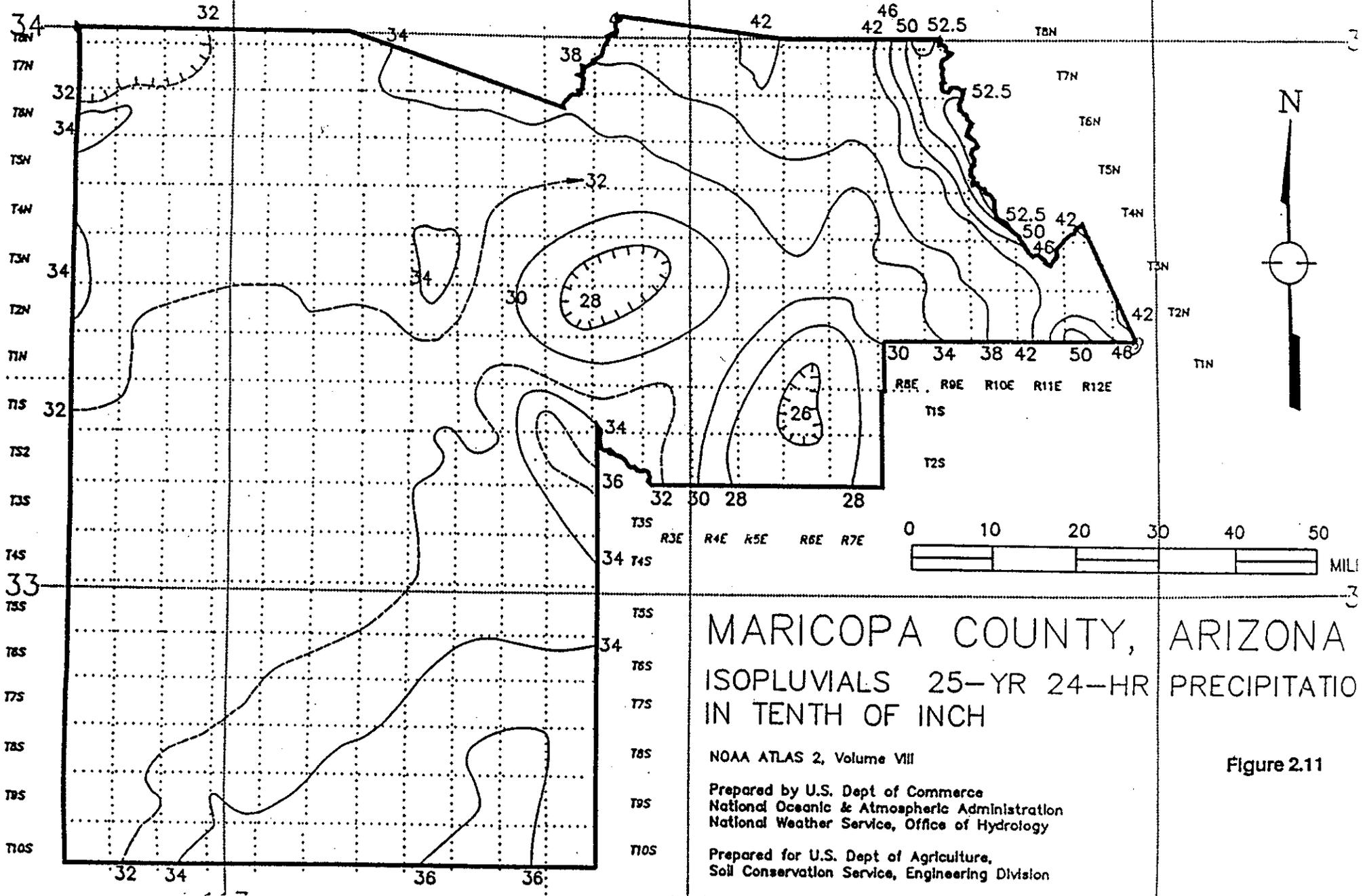
Figure 2.10

R10W R9W R8W R7W R6W R5W R4W R3W R2W R1W R1E R2E R3E R4E R5E R6E R7E R8E R9E R10E R11E R12E

113

112

111



MARICOPA COUNTY, ARIZONA  
ISOPLUVIALS 25-YR 24-HR PRECIPITATIO  
IN TENTH OF INCH

NOAA ATLAS 2, Volume VIII

Prepared by U.S. Dept of Commerce  
National Oceanic & Atmospheric Administration  
National Weather Service, Office of Hydrology

Prepared for U.S. Dept of Agriculture,  
Soil Conservation Service, Engineering Division

Figure 2.11

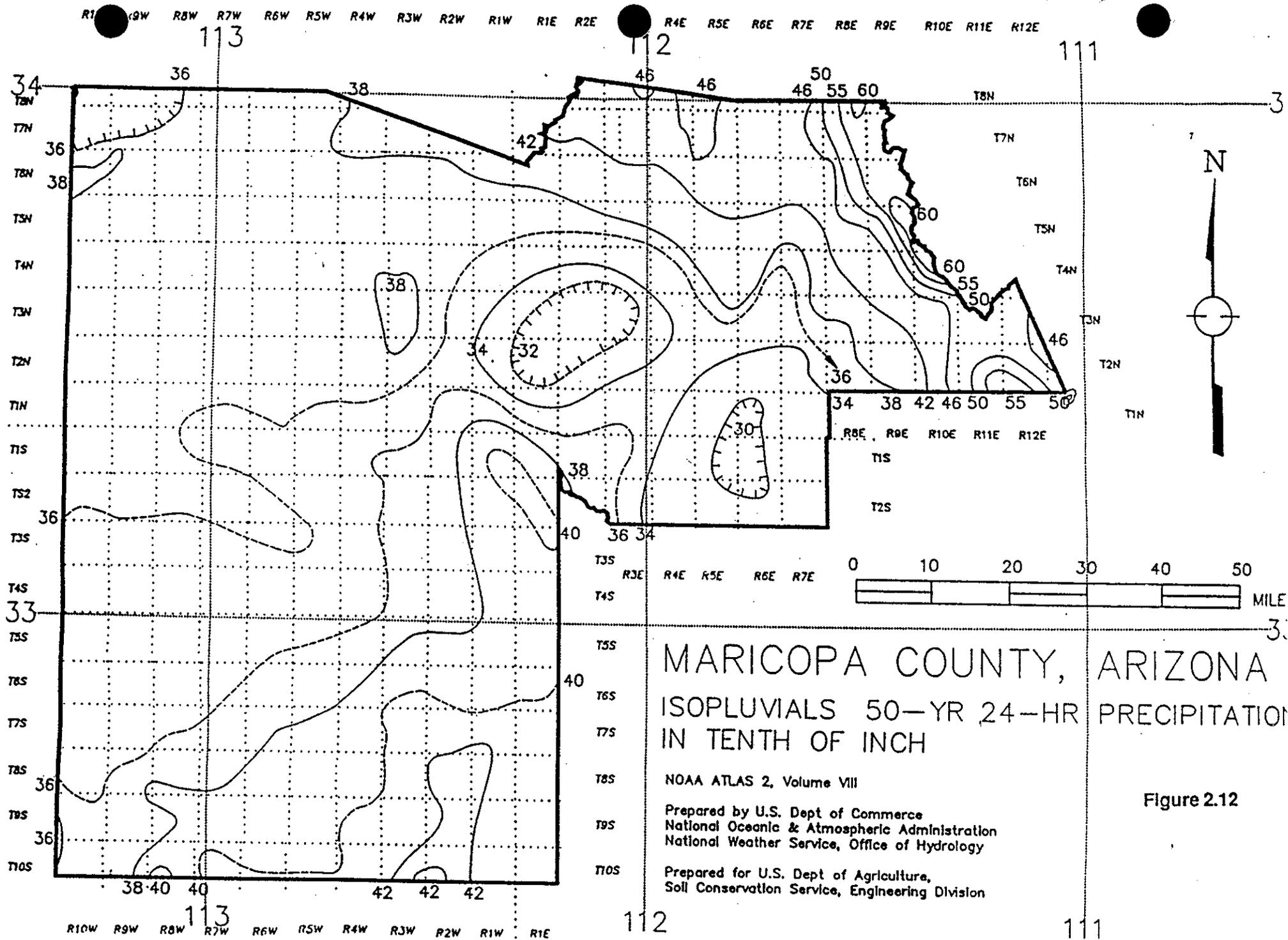


Figure 2.12

## D.2 Physical Parameters Calculations



Flood Control District of Maricopa County  
SCOTTSDALE - GRANITE REEF ADMP

Land Use Data

3/30/2002

Page 1

Area Id	Land Use Code	Area	Area Pct (%)	DTHETA Condition	Vegetation Cover (%)	RTIMP (%)	IA (in)	Kn	Kb	Type	Kb
A01	FWY	0.011	2.2	Normal	80.0	95	0.05	0.020	Low	0.068	
	COM-1	0.492	97.0	Normal	70.0	80	0.10	0.020	Min	0.024	
	MHDR	0.004	0.8	Normal	55.0	40	0.25	0.050	Low	0.074	
B01	MFR	0.055	77.5	Normal	60.0	45	0.25	0.050	Low	0.059	
	MHDR	0.016	22.5	Normal	55.0	40	0.25	0.050	Low	0.066	
B02	IND	0.090	89.1	Normal	60.0	70	0.15	0.030	Min	0.029	
	VACANT	0.003	3.0	Dry	15.0	5	0.10	0.030	Min	0.038	
	FWY	0.003	3.0	Normal	80.0	95	0.05	0.020	Low	0.076	
	MFR	0.005	5.0	Normal	60.0	45	0.25	0.050	Low	0.073	
B03	MFR	0.005	64.3	Normal	60.0	45	0.25	0.050	Low	0.074	
	MDR	0.003	35.7	Normal	50.0	50	0.25	0.050	Low	0.077	
B04	IND	0.072	93.5	Normal	60.0	70	0.15	0.030	Min	0.030	
	MFR	0.001	1.3	Normal	60.0	45	0.25	0.050	Low	0.083	
	FWY	0.004	5.2	Normal	80.0	95	0.05	0.020	Low	0.074	
B05	FWY	0.002	2.3	Normal	80.0	95	0.05	0.020	Low	0.079	
	IND	0.007	8.0	Normal	60.0	70	0.15	0.030	Min	0.036	
	MFR	0.069	78.4	Normal	60.0	45	0.25	0.050	Low	0.057	
	PARK	0.002	2.3	Normal	90.0	5	0.20	0.100	Hi	0.147	
	VACANT	0.008	9.1	Dry	15.0	5	0.10	0.030	Min	0.036	
B06	FWY	0.002	13.3	Normal	80.0	95	0.05	0.020	Low	0.079	
	MFR	0.004	26.7	Normal	60.0	45	0.25	0.050	Low	0.074	
	VACANT	0.002	13.3	Dry	15.0	5	0.10	0.030	Min	0.039	
	INS-1	0.007	46.7	Normal	85.0	75	0.10	0.020	Min	0.036	
B07	FWY	0.001	6.3	Normal	80.0	95	0.05	0.020	Low	0.083	
	INS-1	0.007	43.8	Normal	85.0	75	0.10	0.020	Min	0.036	
	MFR	0.003	18.8	Normal	60.0	45	0.25	0.050	Low	0.076	
	VACANT	0.005	31.3	Dry	15.0	5	0.10	0.030	Min	0.037	
B10	COM-1	0.011	10.0	Normal	70.0	80	0.10	0.020	Min	0.035	
	PARK	0.055	50.0	Normal	90.0	5	0.20	0.100	Hi	0.111	
	MHDR	0.013	11.8	Normal	55.0	40	0.25	0.050	Low	0.067	
	MFR	0.031	28.2	Normal	60.0	45	0.25	0.050	Low	0.062	
B11	COM-1	0.001	1.7	Normal	70.0	80	0.10	0.020	Min	0.041	

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Area Id	Land Use Code	Area	Area Pct (%)	DTHETA Condition	Vegetation Cover (%)	RTIMP (%)	IA (in)	Kn	Kb	Type	Kb
	PARK	0.006	10.0	Normal	90.0	5	0.20	0.100		Hi	0.135
	MHDR	0.053	88.3	Normal	55.0	40	0.25	0.050		Low	0.059
B12	INS-1	0.002	1.7	Normal	85.0	75	0.10	0.020		Min	0.039
	MHDR	0.111	93.3	Normal	55.0	40	0.25	0.050		Low	0.055
	PARK	0.005	4.2	Normal	90.0	5	0.20	0.100		Hi	0.137
	MFR	0.001	0.8	Normal	60.0	45	0.25	0.050		Low	0.083
B13	PARK	0.015	6.8	Normal	90.0	5	0.20	0.100		Hi	0.125
	INS-1	0.013	5.9	Normal	85.0	75	0.10	0.020		Min	0.034
	MFR	0.008	3.6	Normal	60.0	45	0.25	0.050		Low	0.070
	IND	0.003	1.4	Normal	60.0	70	0.15	0.030		Min	0.038
	VACANT	0.020	9.0	Dry	15.0	5	0.10	0.030		Min	0.033
	MHDR	0.001	0.5	Normal	55.0	40	0.25	0.050		Low	0.083
	MDR	0.161	72.9	Normal	50.0	50	0.25	0.050		Low	0.052
B14	COM-1	0.131	88.5	Normal	70.0	80	0.10	0.020		Min	0.028
	COM-1	0.016	10.8	Normal	70.0	80	0.10	0.020		Min	0.034
	MHDR	0.001	0.7	Normal	55.0	40	0.25	0.050		Low	0.083
C01	MDR	0.025	78.1	Normal	50.0	50	0.25	0.050		Low	0.063
	COM-1	0.006	18.8	Normal	70.0	80	0.10	0.020		Min	0.036
	FWY	0.001	3.1	Normal	80.0	95	0.05	0.020		Low	0.083
C02	MDR	0.014	38.9	Normal	50.0	50	0.25	0.050		Low	0.067
	COM-1	0.018	50.0	Normal	70.0	80	0.10	0.020		Min	0.033
	FWY	0.004	11.1	Normal	80.0	95	0.05	0.020		Low	0.074
C03	COM-1	0.008	20.0	Normal	70.0	80	0.10	0.020		Min	0.036
	MFR	0.012	30.0	Normal	60.0	45	0.25	0.050		Low	0.068
	INS-1	0.006	15.0	Normal	85.0	75	0.10	0.020		Min	0.036
	FWY	0.003	7.5	Normal	80.0	95	0.05	0.020		Low	0.076
	MDR	0.011	27.5	Normal	50.0	50	0.25	0.050		Low	0.068
C04	COM-1	0.002	4.5	Normal	70.0	80	0.10	0.020		Min	0.039
	FWY	0.005	11.4	Normal	80.0	95	0.05	0.020		Low	0.073
	INS-1	0.001	2.3	Normal	85.0	75	0.10	0.020		Min	0.041
	MDR	0.018	40.9	Normal	50.0	50	0.25	0.050		Low	0.065
	MFR	0.006	13.6	Normal	60.0	45	0.25	0.050		Low	0.072

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Area Id	Land Use Code	Area	Area Pct (%)	DTHETA Condition	Vegetation Cover (%)	RTIMP (%)	IA (in)	Kn	Kb	Type	Kb
C04	VACANT	0.012	27.3	Dry	15.0	5	0.10	0.030	Min		0.034
C05	COM-1	0.008	5.8	Normal	70.0	80	0.10	0.020	Min		0.036
	INS-1	0.002	1.5	Normal	85.0	75	0.10	0.020	Min		0.039
	FWY	0.006	4.4	Normal	80.0	95	0.05	0.020	Low		0.072
	MDR	0.121	88.3	Normal	50.0	50	0.25	0.050	Low		0.054
C06	PARK	0.003	1.6	Normal	90.0	5	0.20	0.100	Hi		0.145
	INS-1	0.015	9.8	Normal	85.0	75	0.10	0.020	Min		0.034
	FWY	0.002	1.3	Normal	80.0	95	0.05	0.020	Low		0.079
	MDR	0.133	87.2	Normal	50.0	50	0.25	0.050	Low		0.053
C07	MFR	0.003	4.2	Normal	60.0	45	0.25	0.050	Low		0.076
	FWY	0.001	1.4	Normal	80.0	95	0.05	0.020	Low		0.083
	MDR	0.068	94.4	Normal	50.0	50	0.25	0.050	Low		0.057
C08	FWY	0.002	1.7	Normal	80.0	95	0.05	0.020	Low		0.079
	MDR	0.116	98.3	Normal	50.0	50	0.25	0.050	Low		0.054
C09	MFR	0.005	8.8	Normal	60.0	45	0.25	0.050	Low		0.073
	FWY	0.001	1.8	Normal	80.0	95	0.05	0.020	Low		0.083
	MDR	0.051	89.5	Normal	50.0	50	0.25	0.050	Low		0.059
C10	MDR	0.054	100.0	Normal	50.0	50	0.25	0.050	Low		0.059
C11	VACANT	0.001	2.0	Dry	15.0	5	0.10	0.030	Min		0.041
	MDR	0.049	98.0	Normal	50.0	50	0.25	0.050	Low		0.059
C12	FWY	0.003	30.0	Normal	80.0	95	0.05	0.020	Low		0.076
	MDR	0.007	70.0	Normal	50.0	50	0.25	0.050	Low		0.071
C13	COM-1	0.176	100.0	Normal	70.0	80	0.10	0.020	Min		0.027
D01	FWY	0.004	4.9	Normal	80.0	95	0.05	0.020	Low		0.074
	COM-1	0.002	2.4	Normal	70.0	80	0.10	0.020	Min		0.039
	MDR	0.076	92.7	Normal	50.0	50	0.25	0.050	Low		0.057
D02	MDR	0.004	2.9	Normal	50.0	50	0.25	0.050	Low		0.074
	COM-1	0.004	2.9	Normal	70.0	80	0.10	0.020	Min		0.037
	PARK	0.004	2.9	Normal	90.0	5	0.20	0.100	Hi		0.140
	FWY	0.005	3.6	Normal	80.0	95	0.05	0.020	Low		0.073
	PARK	0.001	0.7	Normal	90.0	5	0.20	0.100	Hi		0.155

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Area Id	Land Use Code	Area	Area Pct (%)	DTHETA Condition	Vegetation Cover (%)	RTIMP (%)	IA (in)	Kn	Kb	Type	Kb
D03	INS-1	0.004	2.9	Normal	85.0	75	0.10	0.020		Min	0.037
	MHDR	0.004	2.9	Normal	55.0	40	0.25	0.050		Low	0.074
	MDR	0.114	81.4	Normal	50.0	50	0.25	0.050		Low	0.054
	FWY	0.009	8.7	Normal	80.0	95	0.05	0.020		Low	0.070
	PARK	0.004	3.8	Normal	90.0	5	0.20	0.100		Hi	0.140
	INS-1	0.001	1.0	Normal	85.0	75	0.10	0.020		Min	0.041
	VACANT	0.005	4.8	Dry	15.0	5	0.10	0.030		Min	0.037
	COM-1	0.013	12.5	Normal	70.0	80	0.10	0.020		Min	0.034
	MHDR	0.020	19.2	Normal	55.0	40	0.25	0.050		Low	0.065
	MDR	0.052	50.0	Normal	50.0	50	0.25	0.050		Low	0.059
D04	FWY	0.004	6.6	Normal	80.0	95	0.05	0.020		Low	0.074
	INS-1	0.001	1.6	Normal	85.0	75	0.10	0.020		Min	0.041
	MDR	0.056	91.8	Normal	50.0	50	0.25	0.050		Low	0.059
D05	INS-1	0.015	9.4	Normal	85.0	75	0.10	0.020		Min	0.034
	PARK	0.010	6.4	Normal	90.0	5	0.20	0.100		Hi	0.130
	FWY	0.006	3.8	Normal	80.0	95	0.05	0.020		Low	0.072
	MFR	0.014	8.8	Normal	60.0	45	0.25	0.050		Low	0.067
	MDR	0.114	71.6	Normal	50.0	50	0.25	0.050		Low	0.054
D06	FWY	0.001	0.9	Normal	80.0	95	0.05	0.020		Low	0.083
	COM-1	0.004	3.7	Normal	70.0	80	0.10	0.020		Min	0.037
	MDR	0.104	95.4	Normal	50.0	50	0.25	0.050		Low	0.055
D07	FWY	0.001	2.0	Normal	80.0	95	0.05	0.020		Low	0.083
	MDR	0.048	98.0	Normal	50.0	50	0.25	0.050		Low	0.060
D08	INS-1	0.002	3.8	Normal	85.0	75	0.10	0.020		Min	0.039
	FWY	0.002	3.8	Normal	80.0	95	0.05	0.020		Low	0.079
	MDR	0.049	92.5	Normal	50.0	50	0.25	0.050		Low	0.059
D09	COM-1	0.216	100.0	Normal	70.0	80	0.10	0.020		Min	0.027
E01	COM-1	0.008	13.8	Normal	70.0	80	0.10	0.020		Min	0.036
	FWY	0.006	10.3	Normal	80.0	95	0.05	0.020		Low	0.072
	MFR	0.005	8.6	Normal	60.0	45	0.25	0.050		Low	0.073
	INS-1	0.004	6.9	Normal	85.0	75	0.10	0.020		Min	0.037
	MDR	0.035	60.3	Normal	50.0	50	0.25	0.050		Low	0.061

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Area id	Land Use Code	Area	Area Pct (%)	DTHETA Condition	Vegetation Cover (%)	RTIMP (%)	IA (in)	Kn	Kb	Type	Kb
E02	FWY	0.004	2.7	Normal	80.0	95	0.05	0.020	Low		0.074
	COM-1	0.003	2.0	Normal	70.0	80	0.10	0.020	Min		0.038
	MFR	0.040	27.0	Normal	60.0	45	0.25	0.050	Low		0.061
	MDR	0.101	68.2	Normal	50.0	50	0.25	0.050	Low		0.055
E03	FWY	0.006	8.6	Normal	80.0	95	0.05	0.020	Low		0.072
	MFR	0.007	10.0	Normal	60.0	45	0.25	0.050	Low		0.071
	MDR	0.057	81.4	Normal	50.0	50	0.25	0.050	Low		0.059
E04	FWY	0.003	7.9	Normal	80.0	95	0.05	0.020	Low		0.076
	INS-1	0.002	5.3	Normal	85.0	75	0.10	0.020	Min		0.039
	PARK	0.007	18.4	Normal	90.0	5	0.20	0.100	Hi		0.134
	MDR	0.026	68.4	Normal	50.0	50	0.25	0.050	Low		0.063
E05	FWY	0.003	6.4	Normal	80.0	95	0.05	0.020	Low		0.076
	INS-1	0.008	17.0	Normal	85.0	75	0.10	0.020	Min		0.036
	MDR	0.025	53.2	Normal	50.0	50	0.25	0.050	Low		0.063
	MDR	0.011	23.4	Normal	50.0	50	0.25	0.050	Low		0.068
E06	FWY	0.002	5.6	Normal	80.0	95	0.05	0.020	Low		0.079
	MDR	0.017	47.2	Normal	50.0	50	0.25	0.050	Low		0.066
	MDR	0.017	47.2	Normal	50.0	50	0.25	0.050	Low		0.066
E07	MDR	0.003	3.6	Normal	50.0	50	0.25	0.050	Low		0.076
	MFR	0.012	14.3	Normal	60.0	45	0.25	0.050	Low		0.068
	FWY	0.001	1.2	Normal	80.0	95	0.05	0.020	Low		0.083
	VACANT	0.001	1.2	Dry	15.0	5	0.10	0.030	Min		0.041
	MDR	0.067	79.8	Normal	50.0	50	0.25	0.050	Low		0.058
E08	FWY	0.001	3.2	Normal	80.0	95	0.05	0.020	Low		0.083
	MDR	0.030	96.8	Normal	50.0	50	0.25	0.050	Low		0.062
E09	COM-1	0.223	100.0	Normal	70.0	80	0.10	0.020	Min		0.027
F01	FWY	0.004	13.3	Normal	80.0	95	0.05	0.020	Low		0.074
	MFR	0.015	50.0	Normal	60.0	45	0.25	0.050	Low		0.066
	MDR	0.011	36.7	Normal	50.0	50	0.25	0.050	Low		0.068
F02	FWY	0.005	10.9	Normal	80.0	95	0.05	0.020	Low		0.073
	MDR	0.041	89.1	Normal	50.0	50	0.25	0.050	Low		0.060

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Area Id	Land Use Code	Area	Area Pct (%)	DTHETA Condition	Vegetation Cover (%)	RTIMP (%)	IA (in)	Kn	Kb	Type	Kb
F03	FWY	0.003	17.6	Normal	80.0	95	0.05	0.020	Low		0.076
	MDR	0.014	82.4	Normal	50.0	50	0.25	0.050	Low		0.067
F04	INS-1	0.014	23.3	Normal	85.0	75	0.10	0.020	Min		0.034
	FWY	0.007	11.7	Normal	80.0	95	0.05	0.020	Low		0.071
	MFR	0.012	20.0	Normal	60.0	45	0.25	0.050	Low		0.068
	COM-1	0.016	26.7	Normal	70.0	80	0.10	0.020	Min		0.034
	MDR	0.011	18.3	Normal	50.0	50	0.25	0.050	Low		0.068
F05	FWY	0.003	3.4	Normal	80.0	95	0.05	0.020	Low		0.076
	INS-1	0.001	1.1	Normal	85.0	75	0.10	0.020	Min		0.041
	MFR	0.002	2.3	Normal	60.0	45	0.25	0.050	Low		0.079
	MDR	0.082	93.2	Normal	50.0	50	0.25	0.050	Low		0.056
F06	MFR	0.019	18.4	Normal	60.0	45	0.25	0.050	Low		0.065
	COM-1	0.006	5.8	Normal	70.0	80	0.10	0.020	Min		0.036
	VACANT	0.005	4.9	Dry	15.0	5	0.10	0.030	Min		0.037
	FWY	0.002	1.9	Normal	80.0	95	0.05	0.020	Low		0.079
	MDR	0.071	68.9	Normal	50.0	50	0.25	0.050	Low		0.057
F07	COM-1	0.250	100.0	Normal	70.0	80	0.10	0.020	Min		0.026
G01	MFR	0.015	30.6	Normal	60.0	45	0.25	0.050	Low		0.066
	COM-1	0.006	12.2	Normal	70.0	80	0.10	0.020	Min		0.036
	FWY	0.005	10.2	Normal	80.0	95	0.05	0.020	Low		0.073
	MDR	0.023	46.9	Normal	50.0	50	0.25	0.050	Low		0.064
G02	INS-1	0.002	2.7	Normal	85.0	75	0.10	0.020	Min		0.039
	FWY	0.005	6.7	Normal	80.0	95	0.05	0.020	Low		0.073
	PARK	0.008	10.7	Normal	90.0	5	0.20	0.100	Hi		0.132
	MFR	0.004	5.3	Normal	60.0	45	0.25	0.050	Low		0.074
	COM-1	0.008	10.7	Normal	70.0	80	0.10	0.020	Min		0.036
	MDR	0.048	64.0	Normal	50.0	50	0.25	0.050	Low		0.060
G03	INS-1	0.008	8.6	Normal	85.0	75	0.10	0.020	Min		0.036
	FWY	0.005	5.4	Normal	80.0	95	0.05	0.020	Low		0.073
	MDR	0.080	86.0	Normal	50.0	50	0.25	0.050	Low		0.056
G04	FWY	0.001	2.2	Normal	80.0	95	0.05	0.020	Low		0.083
	MDR	0.044	97.8	Normal	50.0	50	0.25	0.050	Low		0.060

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Area Id	Land Use Code	Area	Area Pct (%)	DTHETA Condition	Vegetation Cover (%)	RTIMP (%)	IA (in)	Kn	Kb	Type	Kb
G05	FWY	0.003	6.7	Normal	80.0	95	0.05	0.020	Low		0.076
	MDR	0.042	93.3	Normal	50.0	50	0.25	0.050	Low		0.060
G06	COM-1	0.127	100.0	Normal	70.0	80	0.10	0.020	Min		0.028

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Soil Data

Area Id	Soil Survey	Map Unit	Area	Area Pct (%)	XKSAT	Rock Outcrop (%)	Effective (%)
A01	Eastern County	LAA	0.156	30.8	0.25		
	Eastern County	GM	0.183	36.1	0.25*		
	Eastern County	PM	0.120	23.7	0.04		
	Eastern County	AM	0.002	0.4	1.20*		
	Eastern County	VF	0.045	8.9	1.20*		
	Aguila/Carefree	55	0.001	0.2	0.27		
B01	Eastern County	LAA	0.016	22.5	0.25		
	Eastern County	RIB	0.028	39.4	0.40		
	Eastern County	GM	0.007	9.9	0.25*		
	Eastern County	RIA	0.020	28.2	0.40		
B02	Eastern County	LAA	0.097	96.0	0.25		
	Eastern County	RIA	0.004	4.0	0.40		
B03	Eastern County	GM	0.001	14.3	0.25*		
	Eastern County	RIB	0.001	14.3	0.40		
	Eastern County	LAA	0.005	71.4	0.25		
B04	Eastern County	LAA	0.040	51.9	0.25		
	Eastern County	RIA	0.030	39.0	0.40		
	Eastern County	RIB	0.006	7.8	0.40		
	Eastern County	GM	0.001	1.3	0.25*		
B05	Eastern County	LAA	0.006	6.8	0.25		
	Eastern County	RIA	0.028	31.8	0.40		
	Eastern County	RIB	0.045	51.1	0.40		
	Eastern County	GM	0.009	10.2	0.25*		
B06	Eastern County	RIA	0.010	62.5	0.40		
	Eastern County	RIB	0.005	31.3	0.40		
	Eastern County	GM	0.001	6.3	0.25*		
B07	Eastern County	RIA	0.001	6.3	0.40		
	Eastern County	RIB	0.008	50.0	0.40		
	Eastern County	GM	0.007	43.8	0.25*		
B10	Eastern County	LAA	0.058	52.3	0.25		
	Eastern County	GM	0.025	22.5	0.25*		
	Eastern County	RIA	0.018	16.2	0.40		
	Eastern County	RIB	0.010	9.0	0.40		
B11	Eastern County	LAA	0.060	100.0	0.25		
B12	Eastern County	LAA	0.119	100.0	0.25		
B13	Eastern County	LAA	0.151	68.3	0.25		
	Eastern County	RIB	0.036	16.3	0.40		
	Eastern County	RIA	0.034	15.4	0.40		
B14	Eastern County	LAA	0.107	71.3	0.25		
	Aguila/Carefree	101	0.015	10.0	0.28		
	Eastern County	RIA	0.027	18.0	0.40		
	Eastern County	RIB	0.001	0.7	0.40		
C01	Eastern County	LAA	0.032	100.0	0.25		
C02	Eastern County	LAA	0.035	97.2	0.25		

Flood Control District of Maricopa County  
SCOTTSDALE - GRANITE REEF ADMP

Soil Data

Area Id	Soil Survey	Map Unit	Area	Area Pct (%)	XKSAT	Rock Outcrop (%)	Effective (%)
C02	Eastern County	RIA	0.001	2.8	0.40		
C03	Eastern County	RIA	0.015	37.5	0.40		
	Eastern County	RIB	0.020	50.0	0.40		
	Eastern County	GM	0.005	12.5	0.25*		
C04	Eastern County	LAA	0.009	20.5	0.25		
	Eastern County	RIA	0.035	79.5	0.40		
C05	Eastern County	LAA	0.133	97.1	0.25		
	Eastern County	RIA	0.004	2.9	*		
C06	Eastern County	LAA	0.040	26.2	0.25		
	Eastern County	RIA	0.032	20.7	0.40		
	Eastern County	RIB	0.064	42.0	0.40		
	Eastern County	GM	0.017	11.1	0.25*		
C07	Eastern County	LAA	0.072	100.0	0.25		
C08	Eastern County	LAA	0.047	39.8	0.25		
	Eastern County	RIA	0.038	32.2	0.40		
	Eastern County	GM	0.010	8.5	0.25*		
	Eastern County	RIB	0.023	19.5	0.40		
C09	Eastern County	LAA	0.057	100.0	0.25		
C10	Eastern County	LAA	0.044	81.5	0.25		
	Eastern County	RIA	0.010	18.5	0.40		
C11	Eastern County	LAA	0.013	26.0	0.25		
	Eastern County	RIA	0.018	36.0	0.40		
	Eastern County	GM	0.010	20.0	0.25*		
	Eastern County	RIB	0.009	18.0	0.40		
C12	Eastern County	GM	0.001	10.0	0.25*		
	Eastern County	RIB	0.004	40.0	0.40		
	Eastern County	RIA	0.005	50.0	0.40		
C13	Eastern County	GM	0.004	2.0	*		
	Eastern County	LAA	0.057	32.3	*		
	Aguila/Carefree	101	0.116	65.7	*		
D01	Eastern County	LAA	0.082	100.0	0.25		
D02	Eastern County	LAA	0.139	100.0	0.25		
D03	Eastern County	LAA	0.055	53.9	0.25		
	Eastern County	PNA	0.007	6.9	0.40		
	Eastern County	GM	0.006	5.9	0.25*		
	Eastern County	RIA	0.034	33.3	0.40		
D04	Eastern County	LAA	0.060	100.0	0.25		
D05	Eastern County	LAA	0.158	100.0	0.25		
D06	Eastern County	LAA	0.109	100.0	0.25		
D07	Eastern County	LAA	0.049	100.0	0.25		
D08	Eastern County	LAA	0.051	94.4	0.25		

Flood Control District of Maricopa County  
SCOTTSDALE - GRANITE REEF ADMP

Soil Data

Area Id	Soil Survey	Map Unit	Area	Area Pct (%)	XKSAT	Rock Outcrop (%)	Effective (%)
D08	Eastern County	MV	0.003	5.6	0.25*		
D09	Aguila/Carefree	110	0.003	1.6	*		
	Aguila/Carefree	79	0.013	6.1	*		
	Eastern County	GM	0.032	14.8	*		
	Aguila/Carefree	23	0.017	8.1	*		
	Eastern County	MV	0.004	1.7	*		
	Aguila/Carefree	101	0.146	67.8	*		
E01	Eastern County	LAA	0.058	100.0	0.25		
E02	Eastern County	LAA	0.135	90.6	0.25		
	Eastern County	MV	0.014	9.4	0.25*		
E03	Eastern County	LAA	0.003	4.3	0.25		
	Eastern County	MV	0.067	95.7	0.25*		
E04	Eastern County	LAA	0.038	100.0	0.25		
E05	Eastern County	LAA	0.030	63.8	0.25		
	Eastern County	MV	0.017	36.2	0.25*		
E06	Eastern County	MV	0.036	100.0	0.25*		
E07	Eastern County	LAA	0.050	59.5	0.25		
	Eastern County	MV	0.034	40.5	0.25*		
E08	Eastern County	MV	0.031	100.0	0.25*		
E09	Aguila/Carefree	23	0.121	54.2	*		
	Eastern County	MV	0.102	45.8	*		
F01	Eastern County	LAA	0.030	100.0	0.25		
F02	Eastern County	MV	0.046	100.0	0.25*		
F03	Eastern County	LAA	0.089	52.7	0.25		
	Eastern County	MV	0.080	47.3	0.25*		
F04	Eastern County	LAA	0.060	100.0	0.25		
F05	Eastern County	LAA	0.020	23.0	0.25		
	Eastern County	MV	0.067	77.0	0.25*		
F06	Eastern County	LAA	0.046	44.7	0.25		
	Eastern County	MV	0.057	55.3	0.25*		
F07	Aguila/Carefree	23	0.177	70.7	*		
	Eastern County	MV	0.073	29.3	*		
G01	Eastern County	LAA	0.049	100.0	0.25		
G02	Eastern County	LAA	0.052	68.4	0.25		
	Eastern County	MV	0.024	31.6	0.25*		
G03	Eastern County	LAA	0.078	83.0	0.25		
	Eastern County	CO	0.016	17.0	0.04*	*	
G04	Eastern County	LAA	0.045	100.0	0.25		
G05	Eastern County	CO	0.023	52.3	0.04*	*	

Flood Control District of Maricopa County  
 SCOTTSDALE - GRANITE REEF ADMP

**Soil Data**

Area Id	Soil Survey	Map Unit	Area	Area Pct (%)	XKSAT	Rock Outcrop (%)	Effective (%)
G05	Eastern County	LAA	0.021	47.7	0.25		
G06	Eastern County	MV	0.032	25.2	*		
	Eastern County	LAA	0.020	15.4	*		
	Aguila/Carefree	23	0.075	59.3	*		

Flood Control District of Maricopa County  
 SCOTTSDALE - GRANITE REEF ADMP  
**Sub Basin Data**

Basin: 01		Storms: Multiple		Duration: 6 Hour		Loss Method: Green-Ampt		Unit Hydrograph: Clark										
Area ID	Sub Basin Parameters						Rainfall Losses					Return Period (Years)						
	Area (sq mi)	Length (mi)	Slope (ft/mi)	Adj Slope	Time-Area	Kb	IA (in)	DTHETA	PSIF (in)	XKSAT (in/hr)	RTIMP (%)	2	5	10	25	50	100	
A01	0.51	0.83	6.0	6.0	Urban	0.025	0.10	0.25	5.40	0.32	80	<b>Tc (hrs)</b>	0.95	0.78	0.70	0.63	0.58	0.54
												<b>R (hrs)</b>	0.44	0.36	0.32	0.28	0.26	0.24
B01	0.07	0.41	55.4	55.4	Urban	0.061	0.25	0.25	4.30	0.54	44	<b>Tc (hrs)</b>	0.57	0.42	0.36	0.31	0.28	0.27
												<b>R (hrs)</b>	0.43	0.31	0.26	0.22	0.20	0.19
B02	0.10	0.70	19.9	19.9	Urban	0.033	0.15	0.25	4.70	0.41	68	<b>Tc (hrs)</b>	0.66	0.53	0.48	0.42	0.39	0.36
												<b>R (hrs)</b>	0.65	0.50	0.45	0.40	0.36	0.34
B03	0.01	0.24	31.7	31.7	Urban	0.075	0.25	0.25	4.65	0.41	47	<b>Tc (hrs)</b>	0.54	0.40	0.36	0.31	0.28	0.27
												<b>R (hrs)</b>	1.02	0.74	0.65	0.55	0.50	0.47
B04	0.08	0.55	28.2	28.2	Urban	0.033	0.15	0.25	4.45	0.50	71	<b>Tc (hrs)</b>	0.48	0.39	0.35	0.31	0.29	0.27
												<b>R (hrs)</b>	0.44	0.34	0.31	0.27	0.25	0.23
B05	0.09	0.81	23.5	23.5	Urban	0.056	0.22	0.26	4.15	0.58	44	<b>Tc (hrs)</b>	1.26	0.92	0.81	0.70	0.63	0.58
												<b>R (hrs)</b>	1.61	1.14	0.98	0.84	0.75	0.69
B06	0.02	0.47	36.3	36.3	Urban	0.052	0.13	0.26	4.00	0.64	60	<b>Tc (hrs)</b>	0.59	0.45	0.40	0.35	0.33	0.30
												<b>R (hrs)</b>	1.22	0.92	0.81	0.70	0.64	0.59
B07	0.02	0.29	54.8	54.8	Urban	0.047	0.13	0.28	4.35	0.51	49	<b>Tc (hrs)</b>	0.35	0.26	0.24	0.22	0.20	0.19
												<b>R (hrs)</b>	0.46	0.33	0.30	0.27	0.25	0.23
B10	0.11	0.78	27.4	27.4	Urban	0.084	0.21	0.25	4.60	0.49	28	<b>Tc (hrs)</b>	1.50	1.26	1.08	0.91	0.82	0.74
												<b>R (hrs)</b>	1.67	1.37	1.15	0.96	0.85	0.77
B11	0.06	0.34	17.9	17.9	Urban	0.066	0.24	0.25	4.80	0.39	37	<b>Tc (hrs)</b>	0.87	0.63	0.55	0.47	0.43	0.40
												<b>R (hrs)</b>	0.65	0.46	0.40	0.33	0.30	0.27
B12	0.12	0.38	13.1	13.1	Urban	0.058	0.25	0.25	4.80	0.38	39	<b>Tc (hrs)</b>	0.96	0.71	0.62	0.53	0.48	0.44
												<b>R (hrs)</b>	0.55	0.39	0.34	0.29	0.26	0.23
B13	0.22	0.74	51.1	51.1	Urban	0.055	0.22	0.26	4.55	0.42	44	<b>Tc (hrs)</b>	0.79	0.59	0.52	1.58	0.40	0.38
												<b>R (hrs)</b>	0.53	0.38	0.33	1.14	0.25	0.23
B14	0.15	1.00	8.0	8.0	Urban	0.029	0.10	0.25	4.60	0.48	80	<b>Tc (hrs)</b>	1.07	0.87	0.79	0.70	0.65	0.61
												<b>R (hrs)</b>	1.18	0.94	0.84	0.74	0.68	0.63

Flood Control District of Maricopa County  
 SCOTTSDALE - GRANITE REEF ADMP  
**Sub Basin Data**

Basin: 01		Storms: Multiple		Duration: 6 Hour		Loss Method: Green-Ampt		Unit Hydrograph: Clark										
Area ID	Sub Basin Parameters						Rainfall Losses					Return Period (Years)						
	Area (sq mi)	Length (mi)	Slope (ft/mi)	Adj Slope	Time-Area	Kb	IA (in)	DTHETA	PSIF (in)	XKSAT (in/hr)	RTIMP (%)	2	5	10	25	50	100	
C01	0.03	0.45	10.0	10.0	Urban	0.059	0.22	0.25	4.80	0.37	57	Tc (hrs)	1.06	0.83	0.74	0.65	0.59	0.55
												R (hrs)	1.48	1.14	1.00	0.86	0.78	0.71
C02	0.04	0.54	9.8	9.8	Urban	0.051	0.15	0.25	4.80	0.40	70	Tc (hrs)	1.00	0.81	0.73	0.65	0.60	0.55
												R (hrs)	1.51	1.20	1.07	0.93	0.85	0.78
C03	0.04	0.49	36.7	36.7	Urban	0.057	0.18	0.25	4.10	0.61	62	Tc (hrs)	0.63	0.49	0.44	0.39	0.35	0.33
												R (hrs)	0.79	0.60	0.53	0.46	0.42	0.39
C04	0.04	0.54	9.6	9.6	Urban	0.057	0.18	0.28	4.20	0.51	44	Tc (hrs)	1.40	1.03	0.90	0.78	0.71	0.65
												R (hrs)	1.94	1.38	1.19	1.02	0.91	0.83
C05	0.14	0.76	9.9	9.9	Urban	0.054	0.23	0.25	4.70	0.37	54	Tc (hrs)	1.50	1.53	1.02	0.89	0.81	0.75
												R (hrs)	1.45	1.48	0.95	0.81	0.73	0.68
C06	0.15	0.73	18.8	18.8	Urban	0.053	0.23	0.25	4.30	0.49	52	Tc (hrs)	1.12	0.85	0.76	0.66	0.60	0.55
												R (hrs)	0.95	0.70	0.62	0.53	0.48	0.44
C07	0.07	0.60	10.2	10.2	Urban	0.058	0.25	0.25	4.80	0.35	50	Tc (hrs)	1.36	1.03	0.91	0.79	0.72	0.67
												R (hrs)	1.54	1.13	0.99	0.85	0.76	0.70
C08	0.12	0.56	11.0	11.0	Urban	0.054	0.25	0.25	4.40	0.45	51	Tc (hrs)	1.20	0.91	0.80	0.70	0.64	0.59
												R (hrs)	0.96	0.70	0.62	0.53	0.48	0.43
C09	0.06	0.54	12.1	12.1	Urban	0.061	0.25	0.25	4.80	0.36	50	Tc (hrs)	1.21	0.92	0.82	0.71	0.65	0.60
												R (hrs)	1.42	1.05	0.92	0.78	0.71	0.65
C10	0.05	0.45	17.9	17.9	Urban	0.059	0.25	0.25	4.65	0.38	50	Tc (hrs)	0.87	0.67	0.59	0.51	0.47	0.43
												R (hrs)	0.89	0.66	0.58	0.49	0.45	0.41
C11	0.05	0.41	24.7	24.7	Urban	0.059	0.25	0.25	4.40	0.45	49	Tc (hrs)	0.73	0.55	0.48	0.42	0.38	0.36
												R (hrs)	0.71	0.52	0.45	0.39	0.35	0.32
C12	0.01	0.49	0.4	0.4	Urban	0.073	0.19	0.25	4.10	0.58	64	Tc (hrs)	1.50	1.50	1.50	1.50	1.50	1.50
												R (hrs)	4.51	4.51	4.51	4.51	4.51	4.51
C13	0.18	1.00	10.0	10.0	Urban	0.027	0.10	0.29	2.65	1.70	80	Tc (hrs)	1.55	0.79	0.71	0.63	0.58	0.54
												R (hrs)	1.62	0.77	0.68	0.59	0.55	0.50

Flood Control District of Maricopa County  
 SCOTTSDALE - GRANITE REEF ADMP  
**Sub Basin Data**

Basin: 01      Storms: Multiple      Duration: 6 Hour      Loss Method: Green-Ampt      Unit Hydrograph: Clark																		
Area ID	Sub Basin Parameters						Rainfall Losses					Return Period (Years)						
	Area (sq mi)	Length (mi)	Slope (ft/mi)	Adj Slope	Time-Area	Kb	IA (in)	DTHETA	PSIF (in)	XKSAT (in/hr)	RTIMP (%)	2	5	10	25	50	100	
D01	0.08	0.57	9.0	9.0	Urban	0.057	0.24	0.25	4.80	0.36	53	Tc (hrs)	1.34	1.02	0.90	0.79	0.72	0.67
												R (hrs)	1.35	0.99	0.87	0.75	0.68	0.62
D02	0.14	0.66	15.5	15.5	Urban	0.058	0.23	0.25	4.80	0.36	51	Tc (hrs)	1.19	0.91	0.81	0.70	0.64	0.59
												R (hrs)	0.99	0.74	0.65	0.55	0.50	0.46
D03	0.10	0.73	18.2	18.2	Urban	0.060	0.20	0.25	4.50	0.45	52	Tc (hrs)	1.23	0.94	0.84	0.73	0.67	0.61
												R (hrs)	1.32	0.98	0.86	0.74	0.67	0.61
D04	0.06	0.62	12.3	12.3	Urban	0.060	0.23	0.25	4.80	0.36	53	Tc (hrs)	1.28	0.98	0.87	0.76	0.69	0.64
												R (hrs)	1.65	1.22	1.08	0.92	0.83	0.76
D05	0.16	0.72	13.0	13.0	Urban	0.059	0.23	0.25	4.80	0.38	51	Tc (hrs)	1.41	1.07	0.94	0.83	0.75	0.69
												R (hrs)	1.19	0.87	0.76	0.66	0.59	0.54
D06	0.11	0.66	10.6	10.6	Urban	0.055	0.24	0.25	4.80	0.35	52	Tc (hrs)	1.35	1.03	0.91	0.80	0.73	0.67
												R (hrs)	1.31	0.97	0.85	0.73	0.66	0.60
D07	0.05	0.52	15.6	15.6	Urban	0.060	0.25	0.25	4.80	0.35	51	Tc (hrs)	1.02	0.79	0.70	0.60	0.55	0.51
												R (hrs)	1.25	0.94	0.82	0.70	0.63	0.58
D08	0.05	0.73	14.5	14.5	Urban	0.059	0.24	0.25	4.80	0.36	53	Tc (hrs)	1.33	1.01	0.90	0.78	0.71	0.66
												R (hrs)	2.08	1.53	1.35	1.16	1.04	0.96
D09	0.22	1.01	15.0	15.0	Urban	0.027	0.10	0.29	2.65	1.70	80	Tc (hrs)	0.80	0.67	0.60	0.53	0.49	0.46
												R (hrs)	0.70	0.57	0.51	0.44	0.41	0.38
E01	0.06	0.51	15.2	15.2	Urban	0.058	0.20	0.25	4.80	0.38	60	Tc (hrs)	0.93	0.74	0.66	0.58	0.53	0.49
												R (hrs)	1.01	0.78	0.68	0.59	0.53	0.49
E02	0.15	0.70	12.1	12.1	Urban	0.057	0.24	0.25	4.80	0.37	50	Tc (hrs)	1.40	1.05	0.93	0.81	0.74	0.68
												R (hrs)	1.19	0.87	0.76	0.65	0.59	0.54
E03	0.07	0.87	9.6	9.6	Urban	0.061	0.23	0.25	4.80	0.36	53	Tc (hrs)	1.50	1.44	1.27	1.10	0.99	0.92
												R (hrs)	2.37	2.26	1.97	1.67	1.50	1.37
E04	0.04	0.39	9.6	9.6	Urban	0.076	0.22	0.25	4.80	0.39	47	Tc (hrs)	1.29	0.97	0.86	0.75	0.68	0.62
												R (hrs)	1.47	1.07	0.94	0.80	0.72	0.66

Flood Control District of Maricopa County  
 SCOTTSDALE - GRANITE REEF ADMP  
**Sub Basin Data**

Basin: 01		Storms: Multiple		Duration: 6 Hour		Loss Method: Green-Ampt		Unit Hydrograph: Clark										
Area ID	Sub Basin Parameters						Rainfall Losses					Return Period (Years)						
	Area (sq mi)	Length (mi)	Slope (ft/mi)	Adj Slope	Time-Area	Kb	IA (in)	DTHETA	PSIF (in)	XKSAT (in/hr)	RTIMP (%)	2	5	10	25	50	100	
E05	0.05	0.33	15.5	15.5	Urban	0.060	0.21	0.25	4.80	0.37	57	<b>Tc (hrs)</b>	0.72	0.56	0.50	0.43	0.40	0.38
												<b>R (hrs)</b>	0.60	0.46	0.40	0.34	0.31	0.29
E06	0.04	0.40	12.8	12.8	Urban	0.067	0.24	0.25	4.80	0.36	53	<b>Tc (hrs)</b>	0.99	0.77	0.68	0.59	0.54	0.50
												<b>R (hrs)</b>	1.17	0.88	0.77	0.66	0.59	0.55
E07	0.08	0.42	15.8	15.8	Urban	0.060	0.25	0.25	4.80	0.36	49	<b>Tc (hrs)</b>	0.89	0.68	0.60	0.52	0.47	0.44
												<b>R (hrs)</b>	0.66	0.50	0.43	0.37	0.33	0.31
E08	0.03	0.36	10.3	10.3	Urban	0.063	0.24	0.25	4.80	0.35	51	<b>Tc (hrs)</b>	0.97	0.75	0.67	0.58	0.53	0.49
												<b>R (hrs)</b>	1.14	0.86	0.76	0.64	0.58	0.53
E09	0.22	1.00	10.0	10.0	Urban	0.027	0.10	0.29	2.65	1.70	80	<b>Tc (hrs)</b>	1.55	0.79	0.71	0.63	0.58	0.55
												<b>R (hrs)</b>	1.42	0.67	0.60	0.52	0.48	0.45
F01	0.03	0.34	11.9	11.9	Urban	0.068	0.22	0.25	4.80	0.39	53	<b>Tc (hrs)</b>	0.93	0.72	0.64	0.55	0.50	0.47
												<b>R (hrs)</b>	1.05	0.79	0.69	0.59	0.53	0.49
F02	0.05	0.44	11.4	11.4	Urban	0.061	0.23	0.25	4.80	0.36	55	<b>Tc (hrs)</b>	1.02	0.80	0.71	0.62	0.56	0.52
												<b>R (hrs)</b>	1.14	0.87	0.76	0.65	0.59	0.54
F03	0.17	0.60	12.4	12.4	Urban	0.069	0.21	0.25	4.80	0.37	58	<b>Tc (hrs)</b>	1.34	1.03	0.92	0.81	0.74	0.68
												<b>R (hrs)</b>	0.94	0.70	0.62	0.54	0.49	0.45
F04	0.06	0.61	11.5	11.5	Urban	0.051	0.15	0.25	4.80	0.42	68	<b>Tc (hrs)</b>	1.04	0.83	0.75	0.66	0.61	0.57
												<b>R (hrs)</b>	1.29	1.01	0.90	0.78	0.71	0.66
F05	0.09	0.44	9.0	9.0	Urban	0.057	0.24	0.25	4.80	0.36	52	<b>Tc (hrs)</b>	1.11	0.85	0.76	0.66	0.60	0.55
												<b>R (hrs)</b>	0.87	0.65	0.57	0.49	0.44	0.40
F06	0.10	0.48	8.6	8.6	Urban	0.057	0.23	0.25	4.80	0.36	49	<b>Tc (hrs)</b>	1.23	0.94	0.83	0.72	0.65	0.60
												<b>R (hrs)</b>	0.94	0.70	0.61	0.52	0.47	0.43
F07	0.25	1.02	10.0	10.0	Urban	0.026	0.10	0.29	2.65	1.70	80	<b>Tc (hrs)</b>	0.93	0.78	0.70	0.62	0.58	0.54
												<b>R (hrs)</b>	0.76	0.63	0.56	0.49	0.45	0.42
G01	0.05	0.44	9.0	9.0	Urban	0.062	0.21	0.25	4.80	0.38	57	<b>Tc (hrs)</b>	1.14	0.89	0.79	0.69	0.63	0.59
												<b>R (hrs)</b>	1.25	0.94	0.83	0.72	0.65	0.60

Flood Control District of Maricopa County  
 SCOTTSDALE - GRANITE REEF ADMP  
**Sub Basin Data**

Basin: 01		Storms: Multiple		Duration: 6 Hour		Loss Method: Green-Ampt		Unit Hydrograph: Clark										
Area ID	Sub Basin Parameters						Rainfall Losses					Return Period (Years)						
	Area (sq mi)	Length (mi)	Slope (ft/mi)	Adj Slope	Time-Area	Kb	IA (in)	DTHETA	PSIF (in)	XKSAT (in/hr)	RTIMP (%)	2	5	10	25	50	100	
G02	0.08	0.50	12.3	12.3	Urban	0.066	0.21	0.25	4.80	0.38	52	<b>Tc (hrs)</b>	1.18	0.91	1.56	0.70	0.64	0.59
												<b>R (hrs)</b>	1.11	0.83	1.51	0.63	0.56	0.52
G03	0.09	0.51	27.6	27.6	Urban	0.055	0.23	0.25	5.60	0.26	55	<b>Tc (hrs)</b>	0.70	0.55	0.49	0.43	0.39	0.37
												<b>R (hrs)</b>	0.57	0.43	0.38	0.32	0.30	0.28
G04	0.05	0.38	35.4	35.4	Urban	0.061	0.25	0.25	4.80	0.35	51	<b>Tc (hrs)</b>	0.58	0.45	0.40	0.35	0.32	0.30
												<b>R (hrs)</b>	0.55	0.41	0.36	0.31	0.28	0.26
G05	0.05	0.39	29.8	29.8	Urban	0.061	0.24	0.15	7.00	0.14	53	<b>Tc (hrs)</b>	0.58	1.58	0.40	0.36	0.34	0.32
												<b>R (hrs)</b>	0.55	1.68	0.37	0.33	0.30	0.28
G06	0.13	0.46	20.0	20.0	Urban	0.028	0.10	0.29	2.65	1.70	80	<b>Tc (hrs)</b>	0.43	0.36	0.33	0.29	0.27	0.25
												<b>R (hrs)</b>	0.25	0.21	0.19	0.16	0.15	0.14

### D.3 Hydrograph Routing Data



## Granite Reef ADMP Route Information

Route ID	Length (ft)	Upstream Elevation	Downstream Elevation	Slope (ft/ft)	Cross Section Type
RB01	1320	1191	1188.5	0.0019	Ditch
RB02	548	1207.8	1197.6	0.0186	Minor Arterial
RB03	1348	1202	1198	0.0030	Minor Arterial
RB05	271	1199	1197.6	0.0052	Minor Arterial
RB07	1525	1207.9	1204	0.0026	Minor Arterial
RB12	4395	*	*	0.0019 *(1)	Shallow Flow
RB13	1269	*	*	0.004 *(1)	Shallow Flow
RB14	2746	*	*	0.004 *(1)	Major Arterial
RC01	1217	1220.2	1218.6	0.0013	Major Arterial
RC03	1097	1204	1203	0.0009	Channel
RC04	1563	1221.5	1204.9	0.0106	Major Arterial
RC02	632	1218	1214	0.0063	Major Arterial
RC06	1271	1212	1204.9	0.0056	Channel
RC07	2638	1227	1219	0.0030	Minor Arterial
RC08	2205	1217.7	1212	0.0026	Minor Arterial
RC09	3368	1228	1218	0.0030	Minor Arterial
RC10	688	1225	1218.8	0.0090	Minor Arterial
RC11	1334	1218.8	1216.8	0.0015	Minor Arterial
RC12	2693	1223	1221	0.0007	Minor Arterial with a wall
RC13S	5197	*	*	0.0058 *(1)	Shallow Flow
RC13W	2052	*	*	0.0044*(2)	Storm Sewer
RD01E	1315	1232	1228.8	0.0024	Major Arterial
RD01S	2611	1233	1227	0.0023	Minor Arterial
RD02	1063	1228.8	1224	0.0045	Major Arterial
RD03	1307	1224	1218.8	0.0040	Minor Arterial
RD04	1084	1234	1232	0.0018	Major Arterial
RD05E	560	1239	1238	0.0018	Subdivision Road
RD05E2	767	1238	1237	0.0013	Minor Arterial
RD05S	2669	1239	1232	0.0026	Minor Arterial
RD05S2	3035	1238	1228.8	0.0030	Minor Arterial
RD06	609	1237	1235	0.0033	Minor Arterial
RD07	3657	1235	1224	0.0030	Minor Arterial
RD08	2751	1234.3	1224	0.0037	Minor Arterial with a wall
RE01S	2567	1245	1236.7	0.0032	Subdivision Road
RE02W	355	*	*	0.0009*(2)	Storm Sewer
RE02S	2617	1244	1236.7	0.0028	Subdivision Road
RE03S	3444	1244	1233	0.0032	Subdivision Road
RE03W	1504	*	*	0.0009	Storm Sewer
RE04S	2636	1252	1245.1	0.0026	Minor Arterial
RE05S	3053	1250	1243.7	0.0021	Minor Arterial
RE05W	1490	*	*	0.0021*(2)	Storm Sewer
RE06	3333	1249	1244.5	0.0014	Minor Arterial with a wall
RE07	1335	1253.3	1250	0.0025	Minor Arterial
RE08	1326	1252.8	1249	0.0029	Minor Arterial with a wall
RE09S	5501	1244.5	1224	0.0037	Minor Arterial along Pima Rd.
RE09W	779	*	*	0.0009*(2)	Storm Sewer
RF01S	2619	1260	1252	0.0031	Minor Arterial
RF02S	1338	1256.3	1253.3	0.0022	Minor Arterial
RF02W	1310	*	*	0.001*(2)	Storm Sewer
RF03	482	1257.7	1256	0.0035	Minor Arterial
RF04S	2652	1266	1259.8	0.0023	Minor Arterial
RF05S	2167	1264.4	1257.4	0.0032	Minor Arterial
RF05W	1302	*	*	0.004*(2)	Storm Sewer
RF06	1299	1267.4	1264.8	0.0020	Minor Arterial
RF07W	1315	*	*	0.001 *(2)	Storm Sewer
RF07S	5280	1257	1245	0.0023	Minor Arterial along Pima Rd.
RG01S	2625	1269.8	1266	0.0014	Minor Arterial
RG02	1358	1269.9	1267.4	0.0018	Minor Arterial
RG03S	1322	1273	1269.8	0.0024	Minor Arterial
RG04	1336	1274.3	1269.9	0.0033	Subdivision Road
RG05	1994	1275	1269.9	0.0026	Subdivision Road
RG06S	5286	1272	1257	0.0028	Minor Arterial along Pima Rd.

Notes: 1) The slope was estimated because there was no topo information.  
 2) The storm sewer's slope was taken from the as-built information.



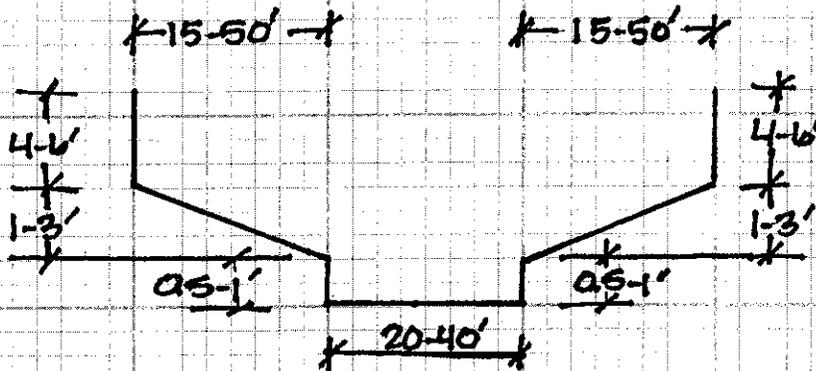
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BY AMG DATE 4/15/02  
CHECK PAW DATE 4/17/02

CLIENT FCDMIL

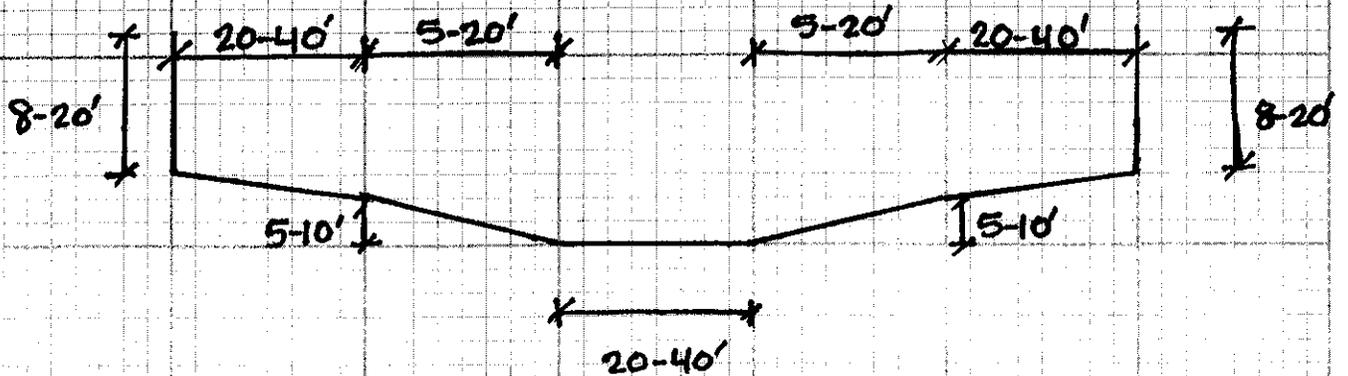
JOB NAME GRANITE REEF DMP

JOB NO. 310.021

# SUBDIVISION ROAD



# CHANNEL



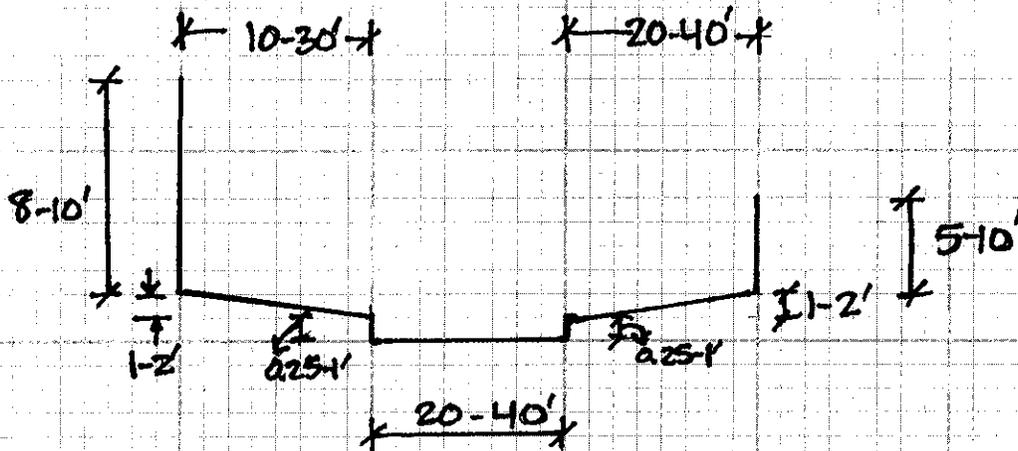


SHEET 2 OF 4  
BY AMG DATE 4/15/02  
CHECK PAW DATE 4/17/02

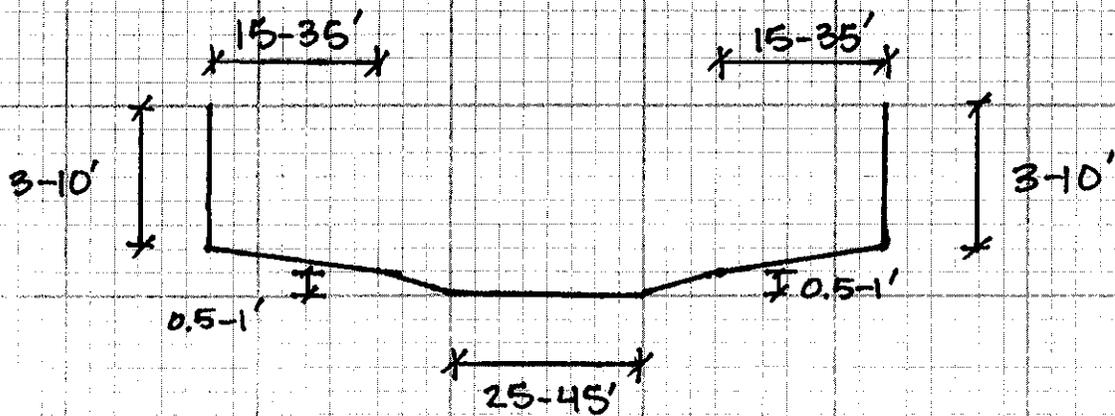
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JOB NAME GRANITE REEF DMP

JOB NO. 310.021

### MINOR ARTERIAL W/ WALL



### MINOR ARTERIAL ALONG PIMA RD.

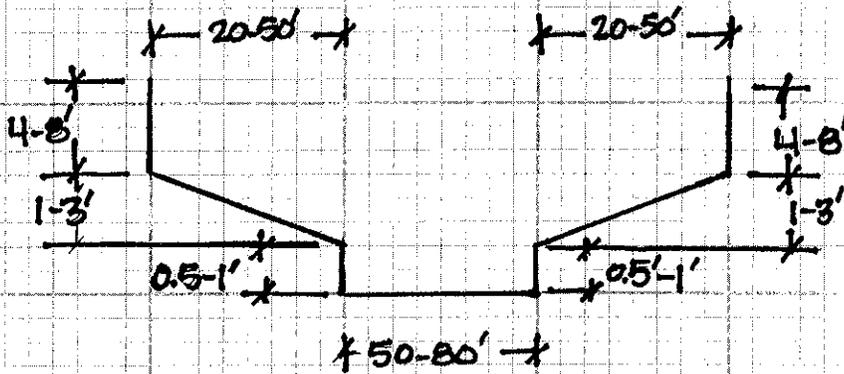




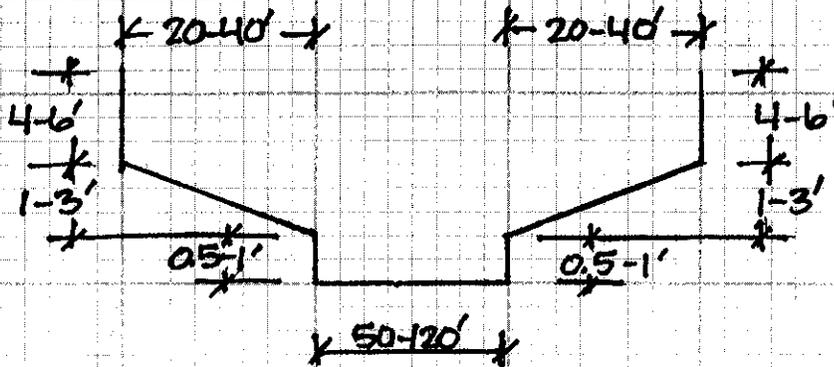
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JOB NAME GRANITE REEF DMP

JOB NO. 310.021

## MINOR ARTERIAL



## MAJOR ARTERIAL



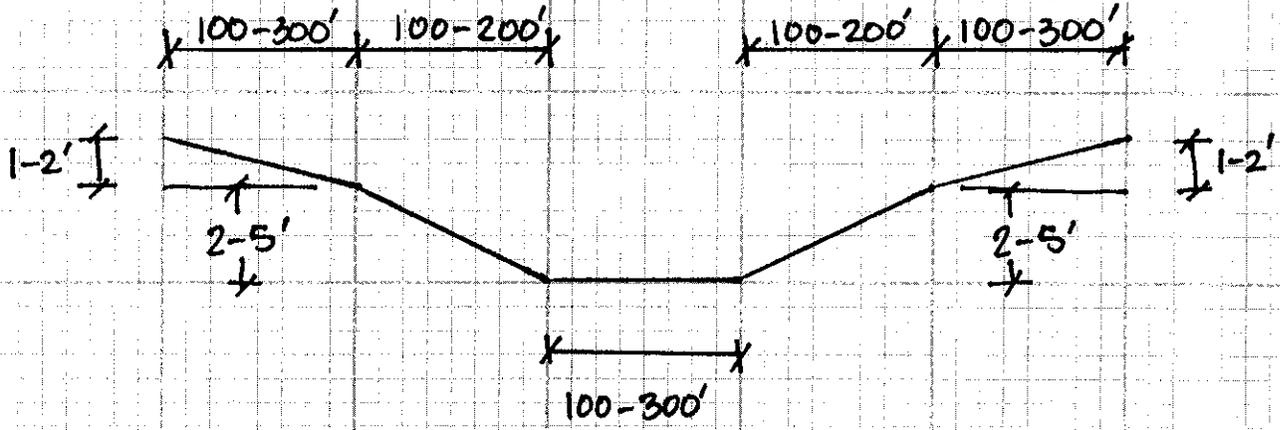


SHEET 4 OF 4  
BY AMG DATE 4/15/02  
CHECK PAW DATE 4/17/02

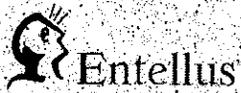
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JOB NAME GRANITE REEF DMP

JOB NO. 310.021

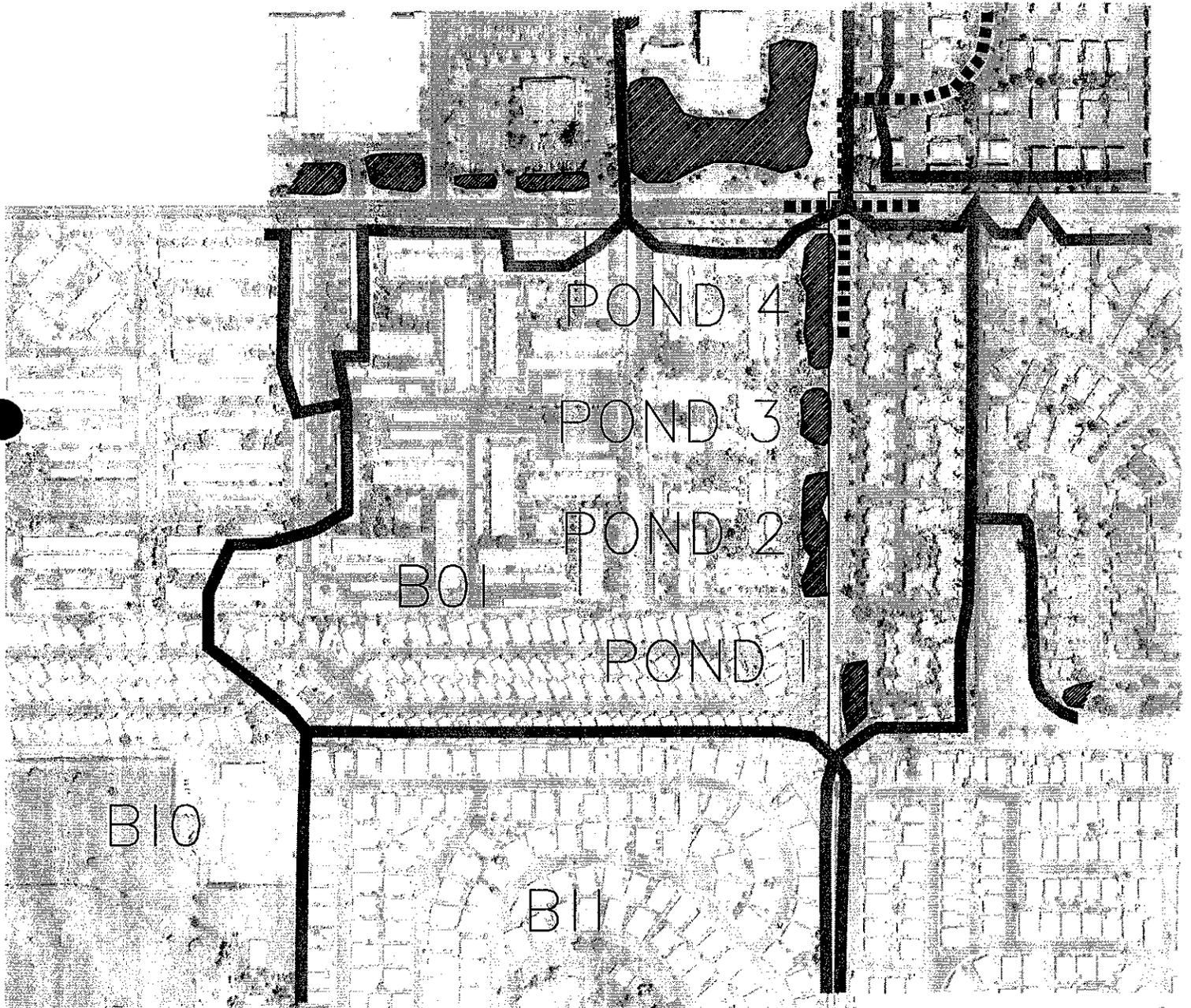
SHALLOW FLOW



**D.4 Storage Routing Data**



# BASIN B01



## Detention Basin Calculations

Note: See attached drawings of detention ponds for dimensions.

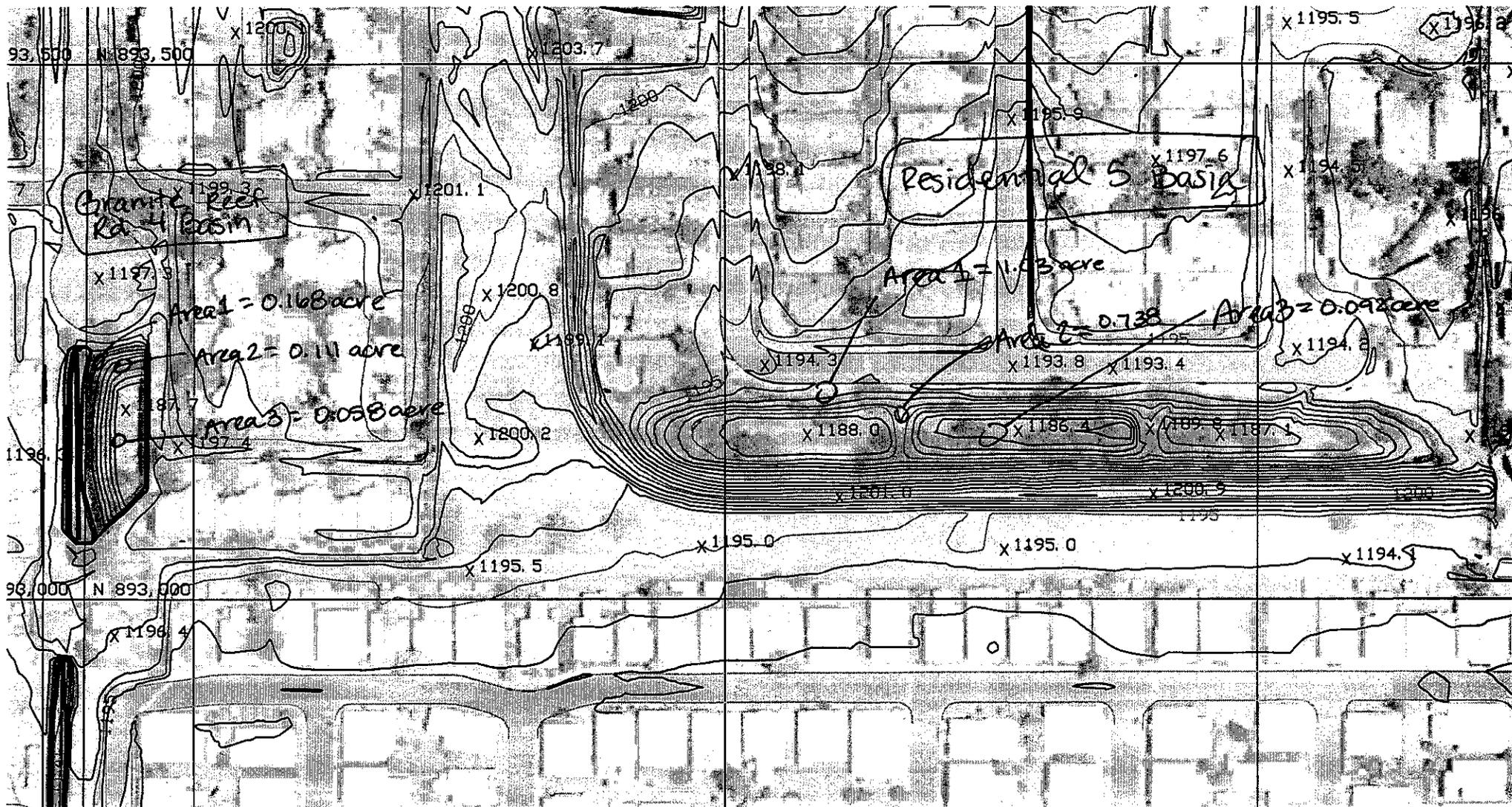
### Basin B01

Pond 1			Pond 2		Pond 3	Pond 4	Ponds 2,3,4		
Depth (ft)	Area	Storage	Depth (ft)	Area	Area	Area	Area	Storage	
0	0.058		0	0.042	0.133	0.093	0.268		
3	0.111	0.25	1	0.289	0.194	0.275	0.758	0.49	
6	0.168	0.42						<b>Total (ac ft)</b>	0.49
<b>Total (ac ft)</b>		0.66							

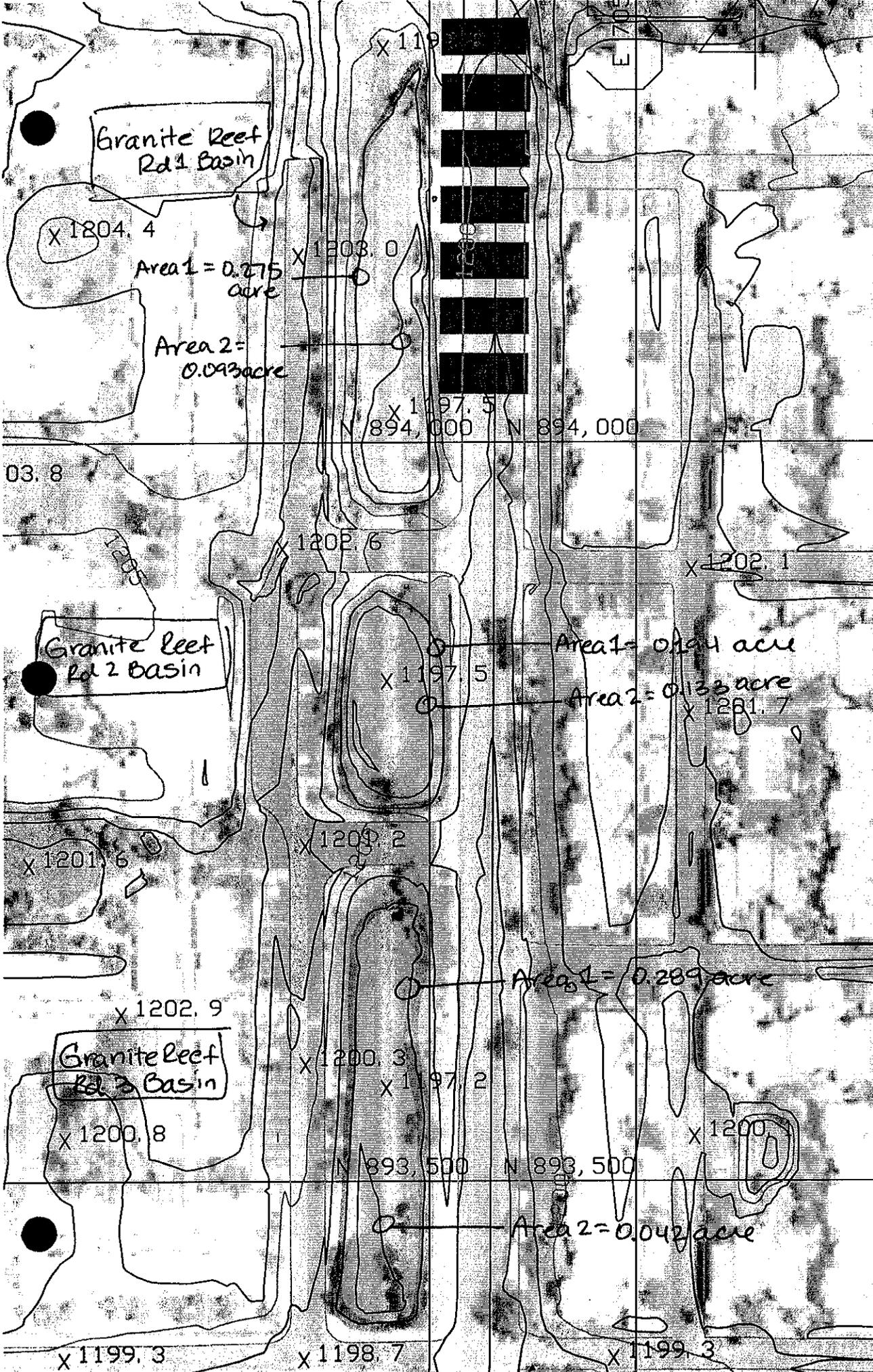
All Ponds = 0.66 + 0.49 = 1.15 ac ft.

BASIN  
B01 ↓

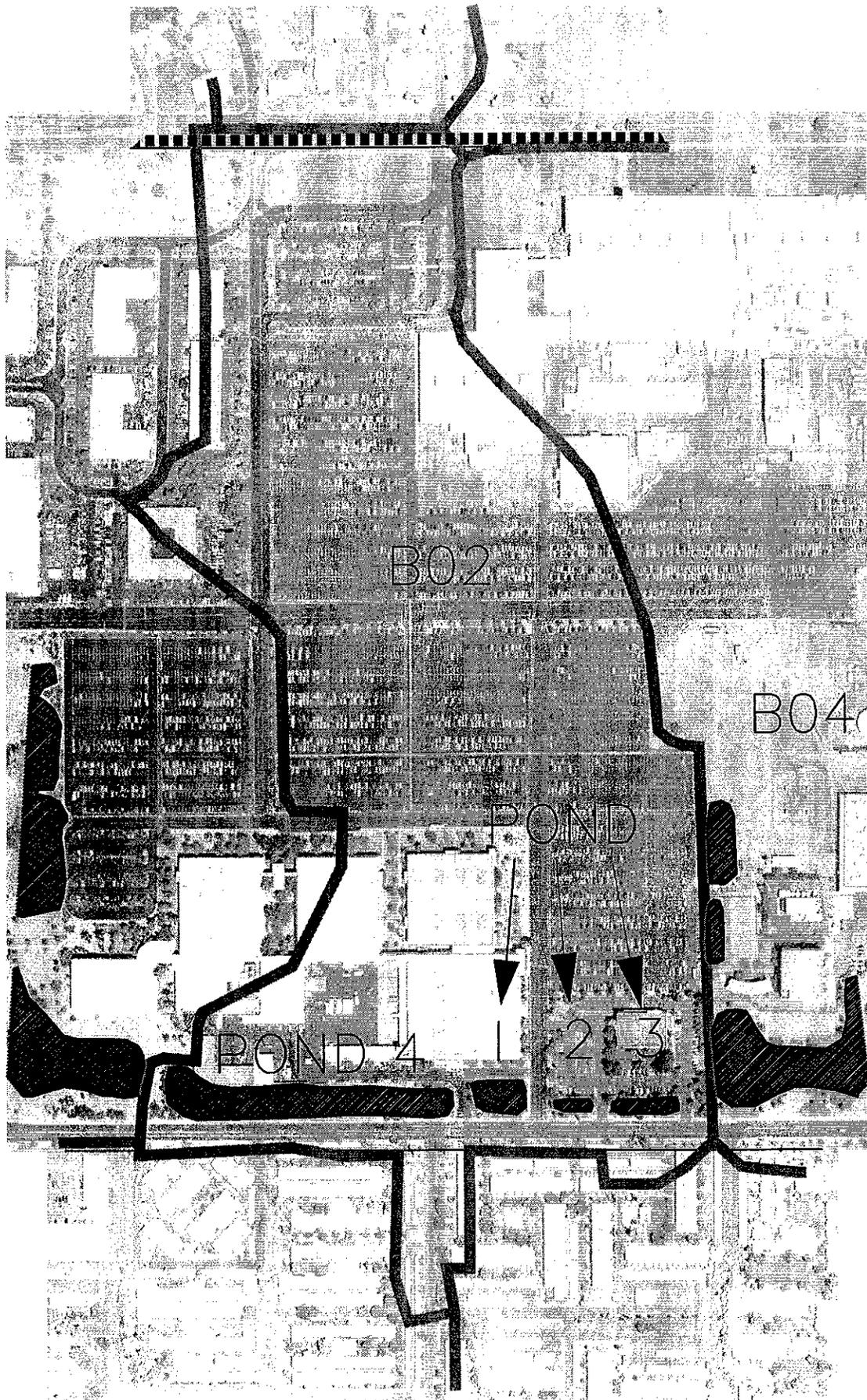
Basin  
↓



# B01 BASIN

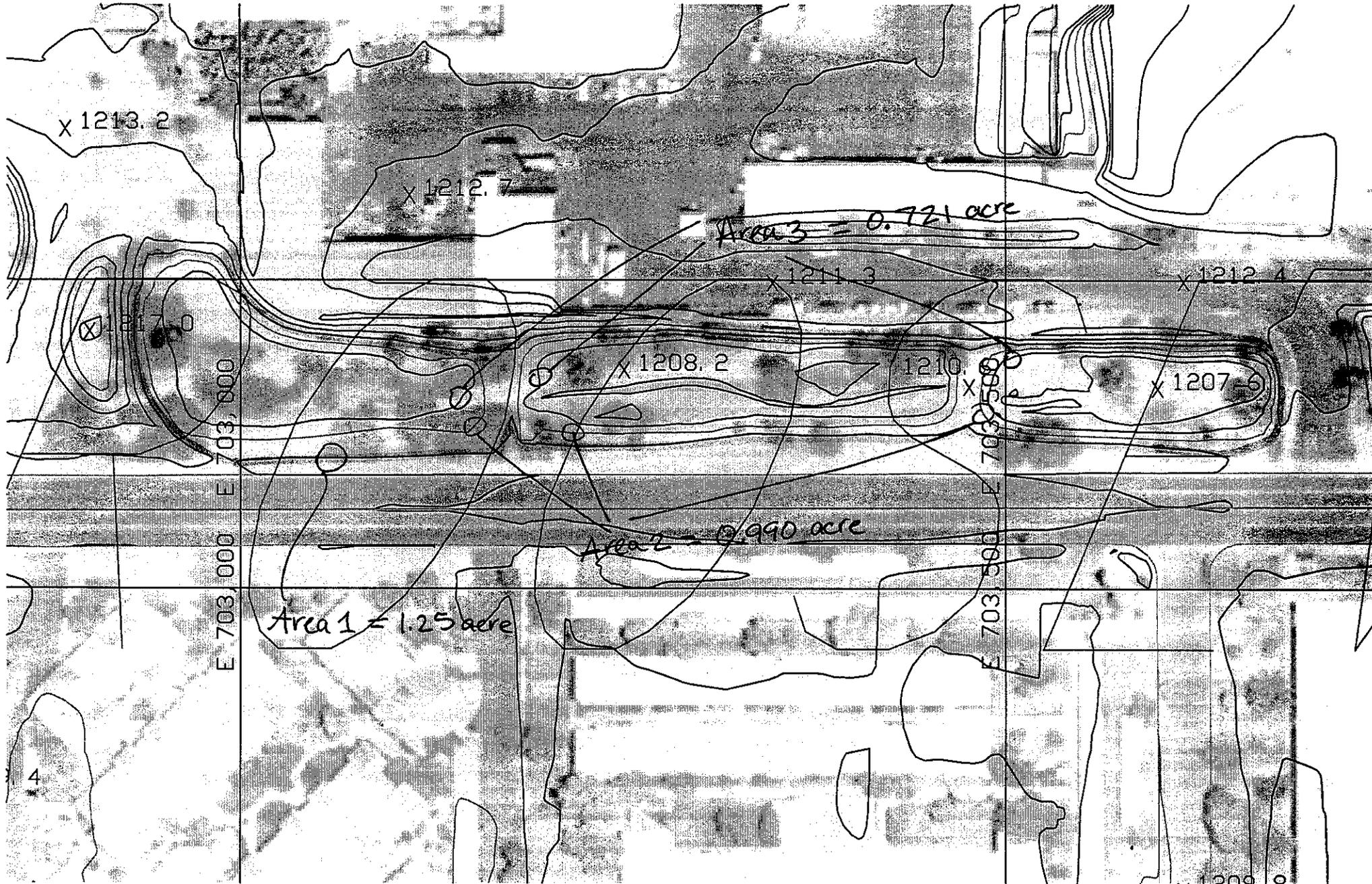


# BASIN B02

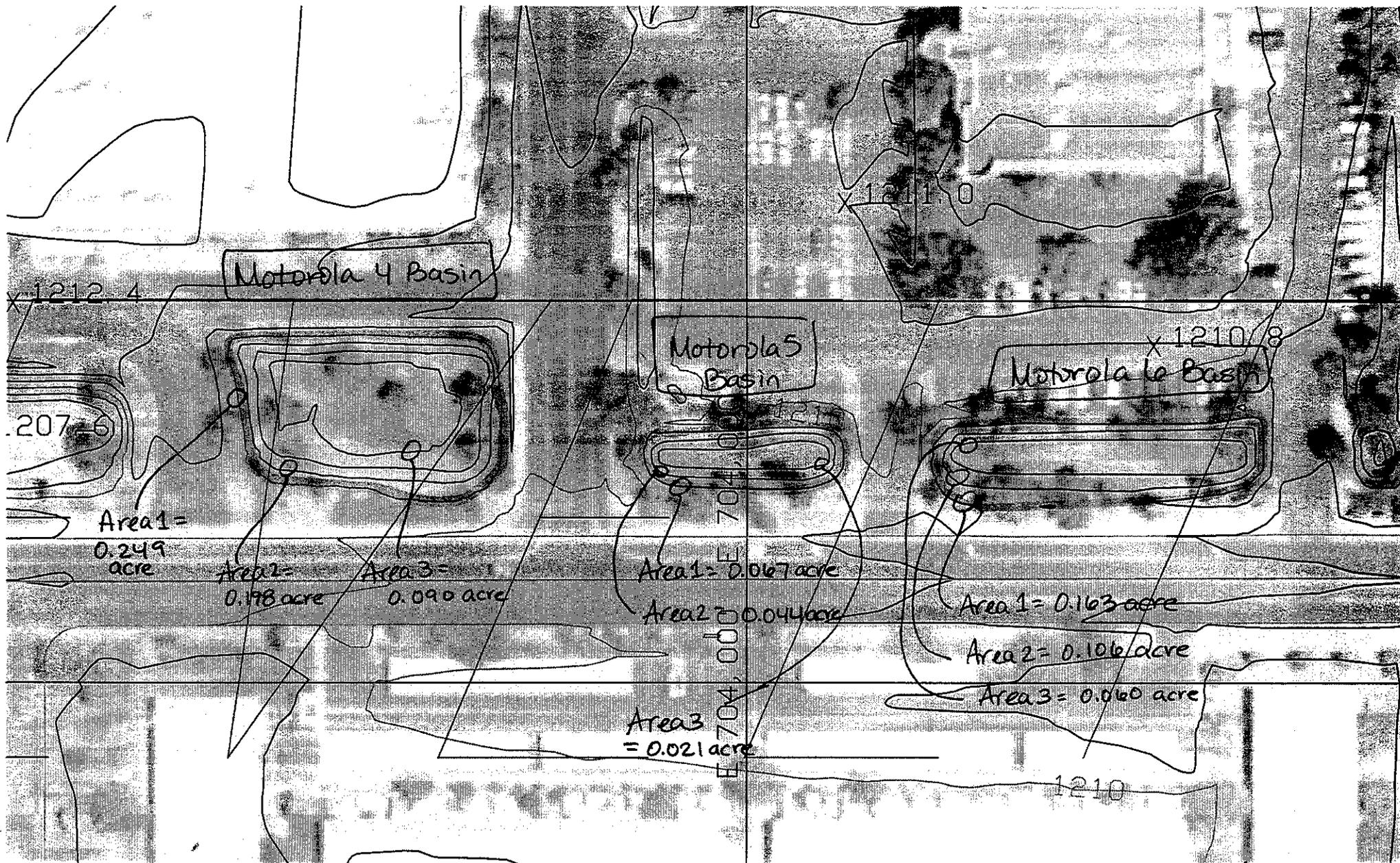


BASIN BOZ

Motorola 3 Basin



BASIN B02



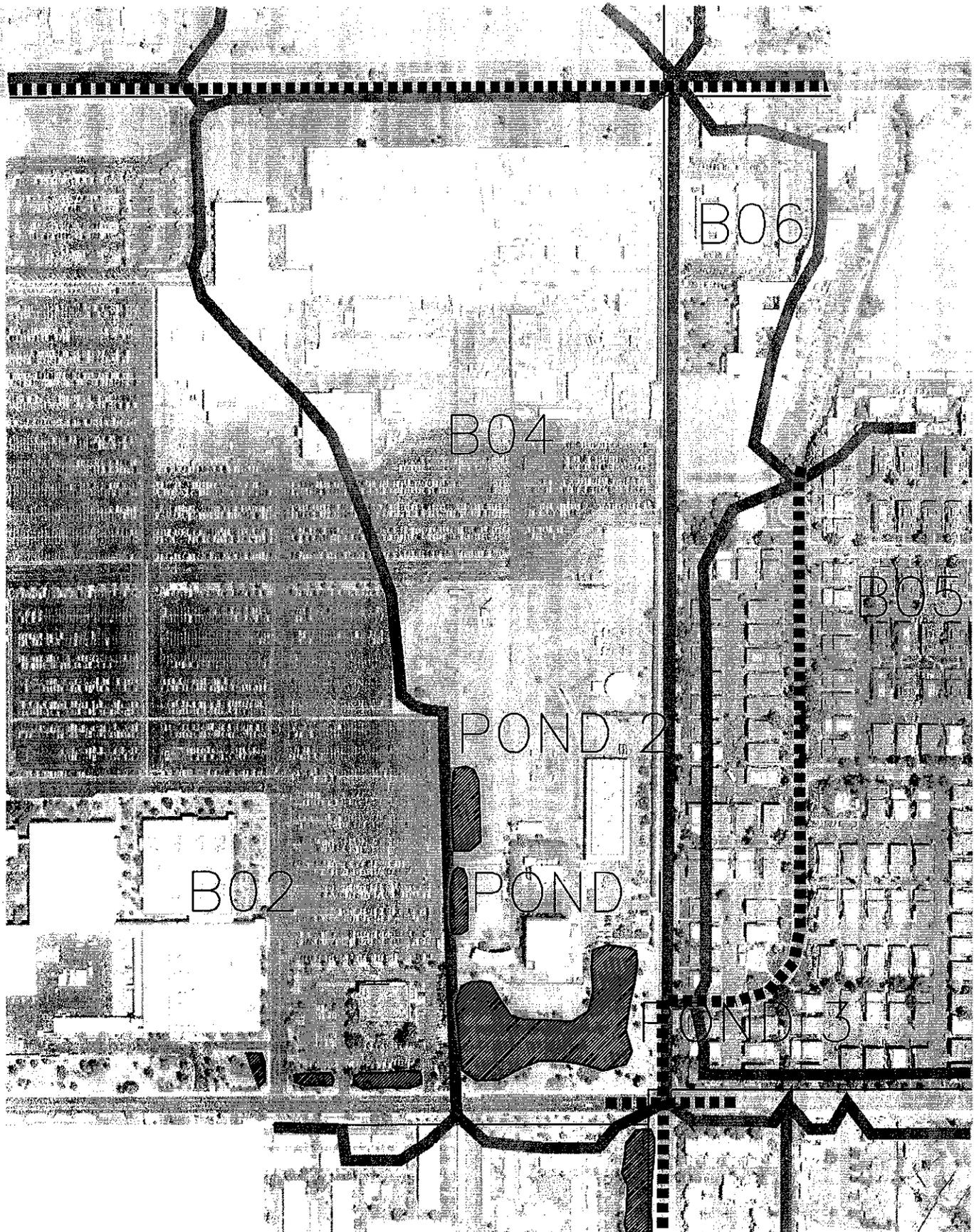
## Detention Basin Calculations

Note: See attached drawings of detention ponds for dimensions.

### Basin B02

Depth (ft)	Pond 1 Area	Pond 2 Area	Pond 3 Area	Pond 4 Area	All Ponds Area	Storage
0	0.09	0.021	0.06	0.721	0.892	
1	0.198	0.044	0.106	0.99	1.338	1.11
2	0.249	0.067	0.163	1.25	1.729	1.53
					<b>Total (ac ft)</b>	2.64

# BASIN B04



## Detention Basin Calculations

Note: See attached drawings of detention ponds for dimensions.

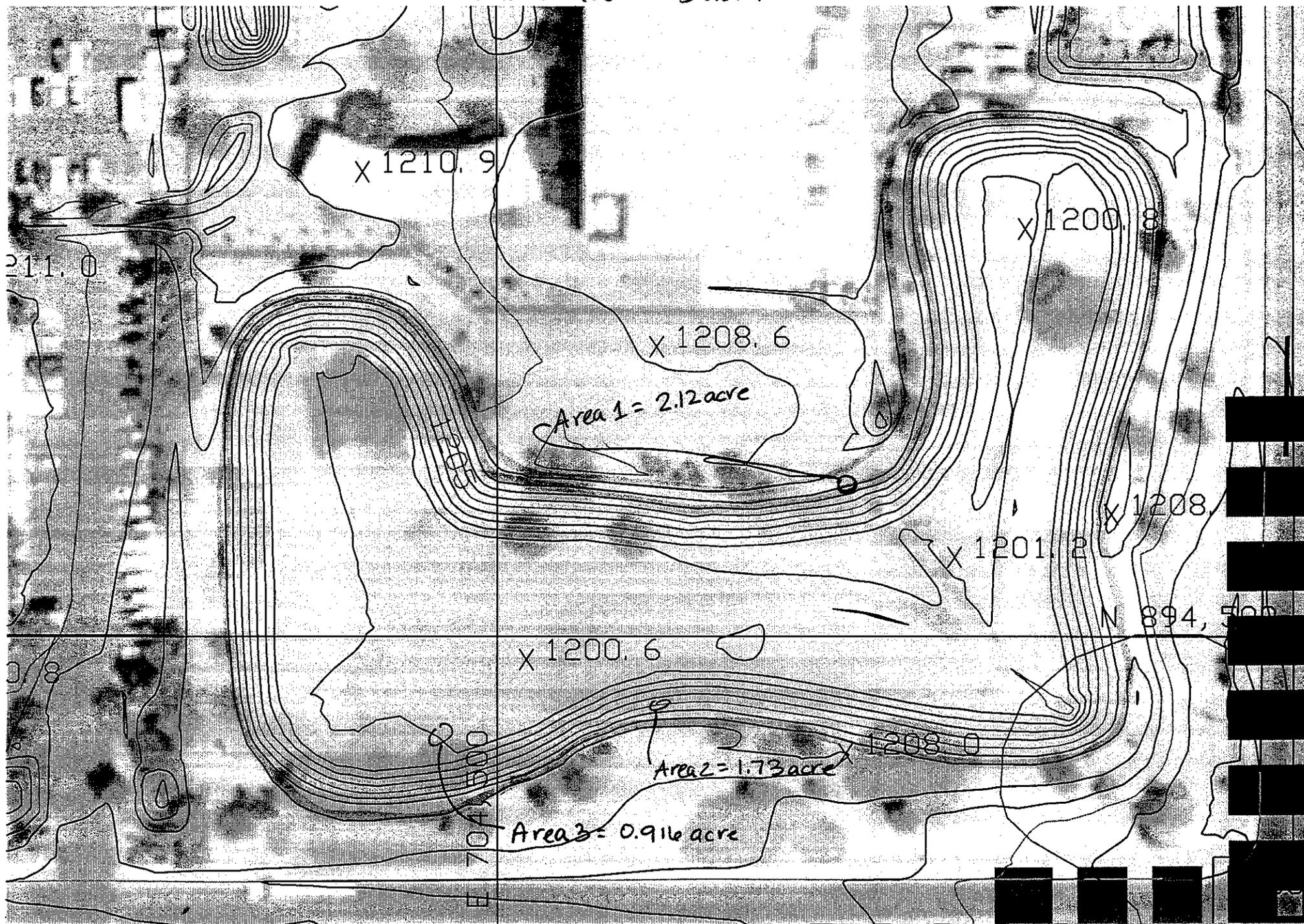
### Basin B04

Pond 1			Pond 2			Pond 3			
Depth (ft)	Area	Storage	Depth (ft)	Area	Storage	Depth (ft)	Area	Storage	
0	0.019		0	0.056		0	0.916		
4	0.095	0.21	2	0.206	0.25	4	1.73	5.21	
8	0.161	0.40	6	0.364	1.13	7	2.12	5.77	
<b>Total (ac ft)</b>		0.61				1.37			10.97

All Ponds = 0.61 + 1.37 + 10.97 = 12.95 ac ft.

Motorola 7 Basin

BASIN 304



X 1212.2

Motorola 9 Basin

Area 1 = 0.364 acre

Area 2 = 0.206 acre

Area 3 = 0.056 acre

X 1209

X 1217

X 1217.6

X 1210.9

X 1209.9

X 1210.5

Motorola 8 Basin

Area 1 = 0.161 acre

Area 2 = 0.95 acre

Area 3 = 0.019 acre

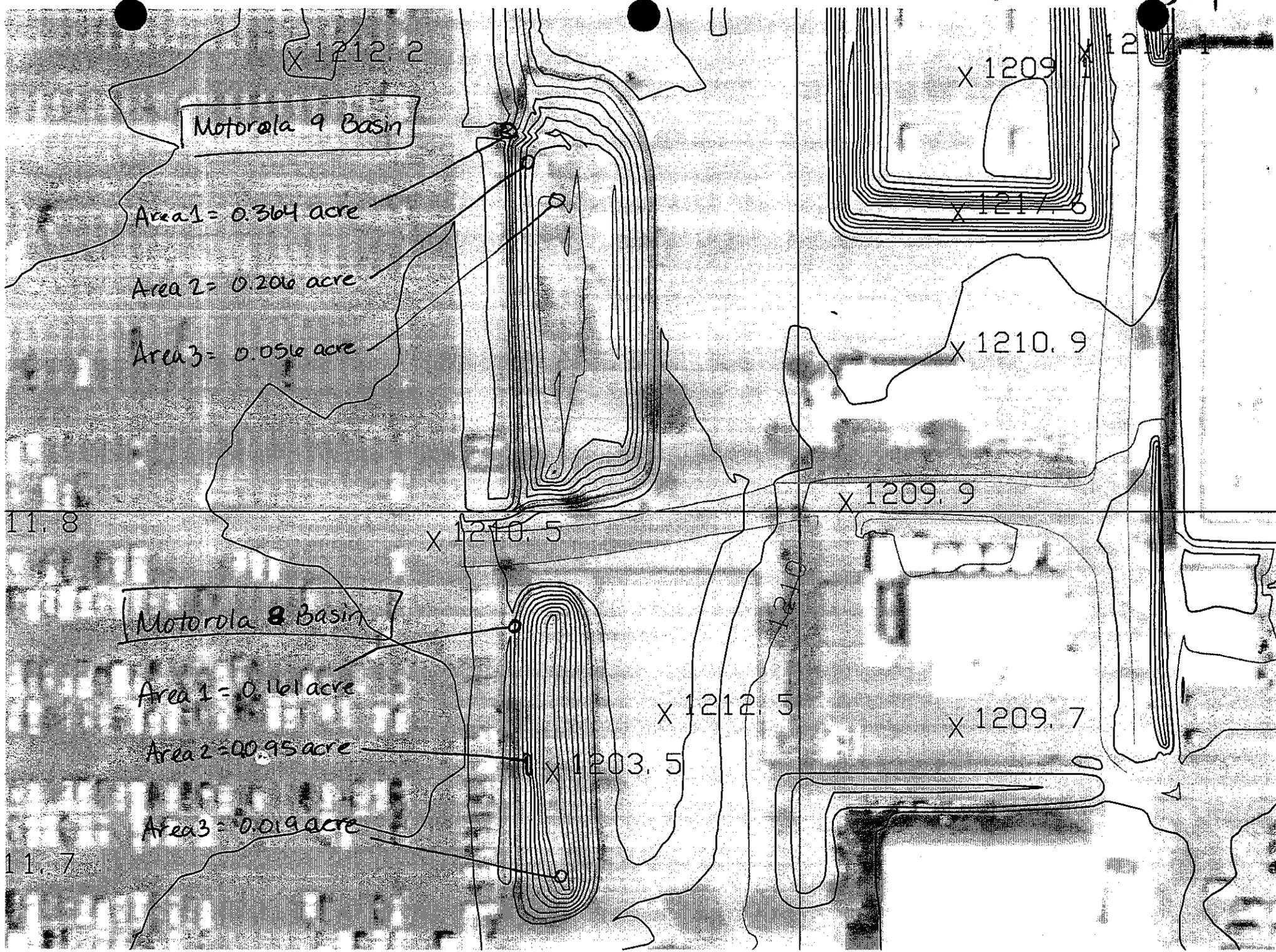
X 1212.5

X 1203.5

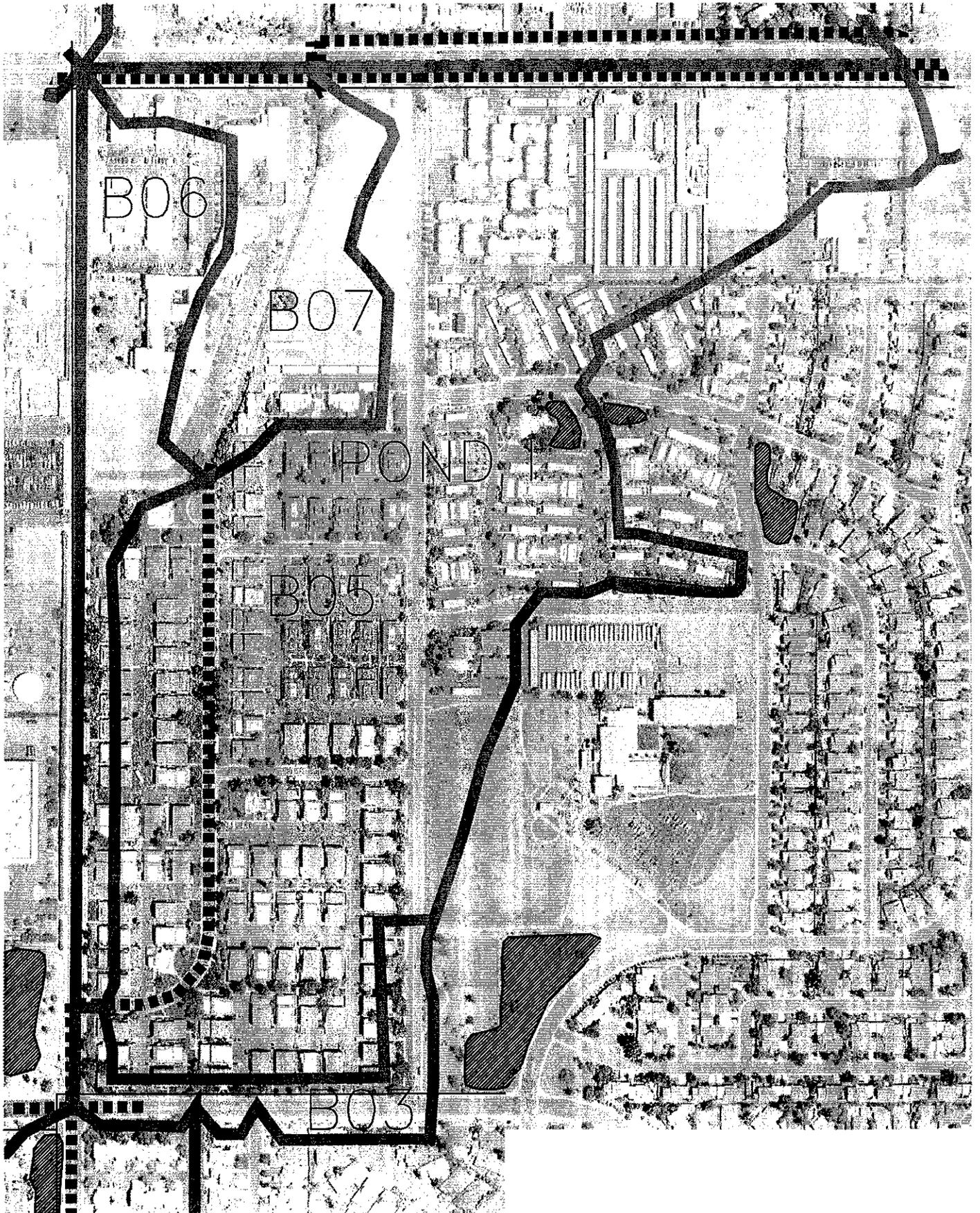
X 1209.7

11.8

11.7



# BASIN B05



## Detention Basin Calculations

Note: See attached drawings of detention ponds for dimensions.

### Basin B05

Pond 1		
Depth (ft)	Area	Storage
0	0.037	
1	0.082	0.058
2	0.126	0.103
Total (ac ft)		0.16



BASIN B05

Residential Basin

Residential Basin

BASIN B13

Area 1 = 0.126 acre  
Area 2 = 0.082 acre  
Area 3 = 0.037 acre

Area 1 = 0.380  
Area 2 = 0.187  
Area 3 = 0.116

1212.1

211.6

2

20.5

x 1216.0

x 1218

x 1217

x 1215.2

x 1215.7

x 1215.0

x 1214.6

x 1214.2

x 1215.8

x 1214.8

x 1216.1

x 1215.4

x 1215.3

x 1215.2

x 1214.5

x 1214.6

x 1212.8

x 1213.1

1214.0

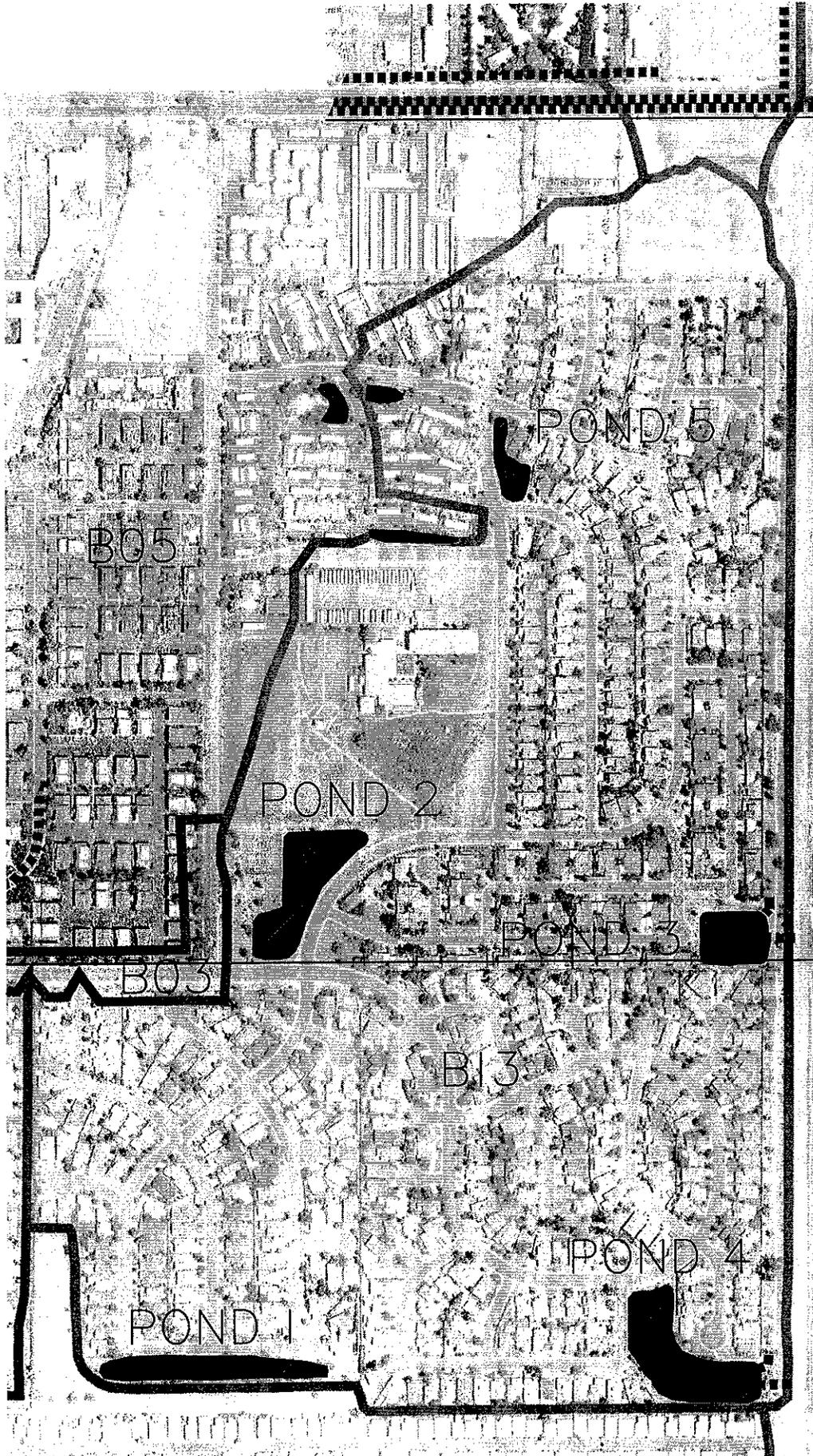
x 1213.9

x 1213.1

x 1212.7

x 1212.6

# BASIN B13



## Detention Basin Calculations

Note: See attached drawings of detention ponds for dimensions.

### Basin B13

Pond 1			Pond 2			Pond 3			Pond 4			Pond 5		
Depth (ft)	Area	Storage	Depth (ft)	Area	Storage	Depth (ft)	Area	Storage	Depth (ft)	Area	Storage	Depth (ft)	Area	Storage
0	0.092		0	0.563		0	0.159		0	0.944		0	0.116	
2	0.738	0.73	3	0.991	2.30	3	0.566	1.02	3	1.34	3.41	1	0.187	0.15
4	1.03	1.35	6	1.44	3.63	6	0.762	1.98	5	1.74	3.07	4	0.33	0.77
<b>Total (ac ft)</b>		2.08			5.93			3.01			6.48			0.92

All Ponds = 2.08 + 5.93 + 3.01 + 6.48 + 0.92 = 18.4 ac ft.

Park Basin

BASIN 515

X 1211.5

X 1211.9

1210

X 12

Area 1 = 1.44 acre

X 1200.2

X 1206.6

X 1212.3

Area 2 = 0.991 acre

X 1208.9

X 1207.2

Area 3 = 0.563 acre

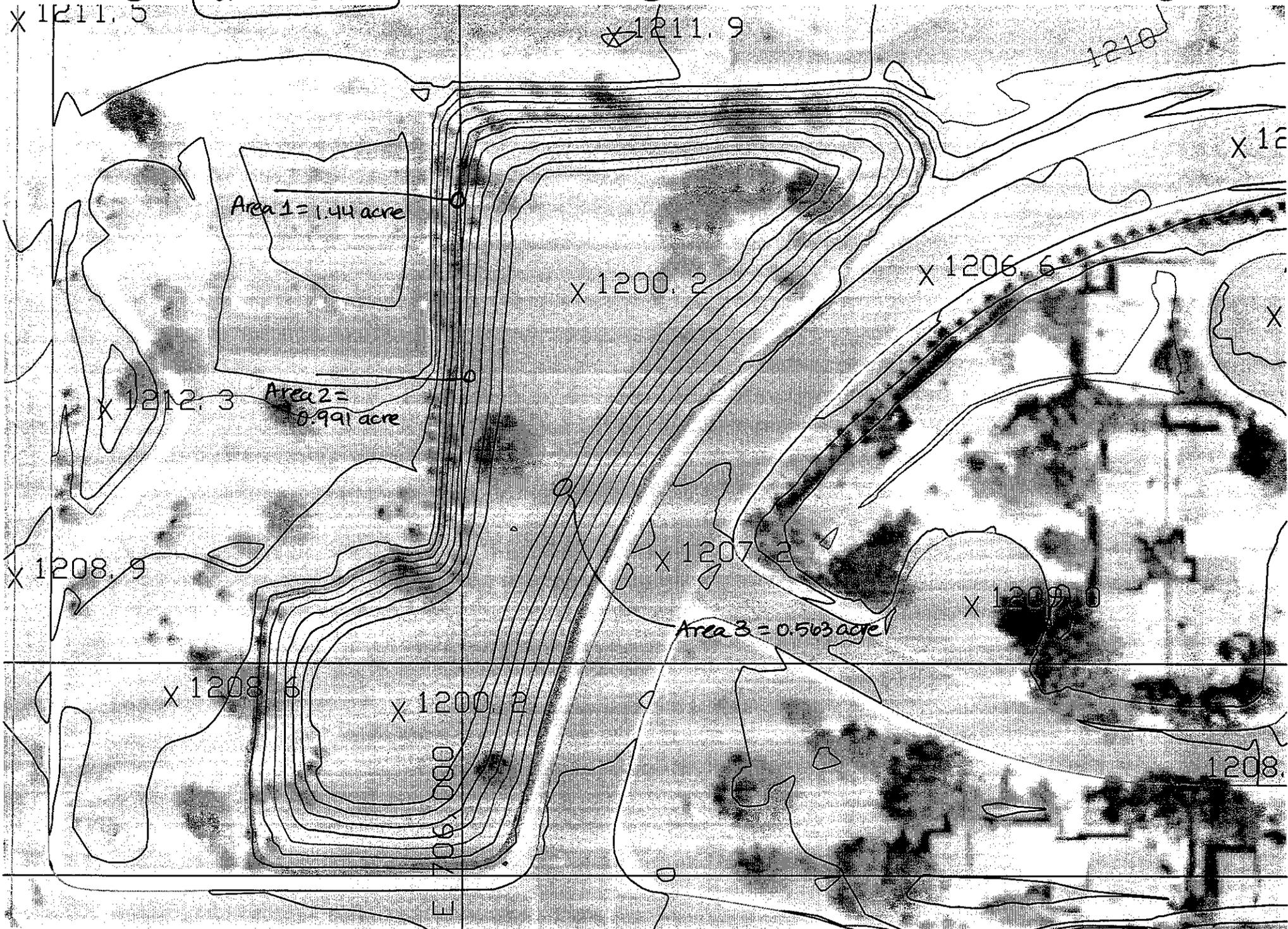
X 1209.0

X 1208.6

X 1200.2

1206.000  
1206.900

1208



BASIN B13

91.2

X 1190.4

Pima Basin

X 1192.5

Area 1 = 1.74 acre

X 1183.6

Area 2 = 1.34 acre

X 1192.1

X 1183.5

X 1190.0

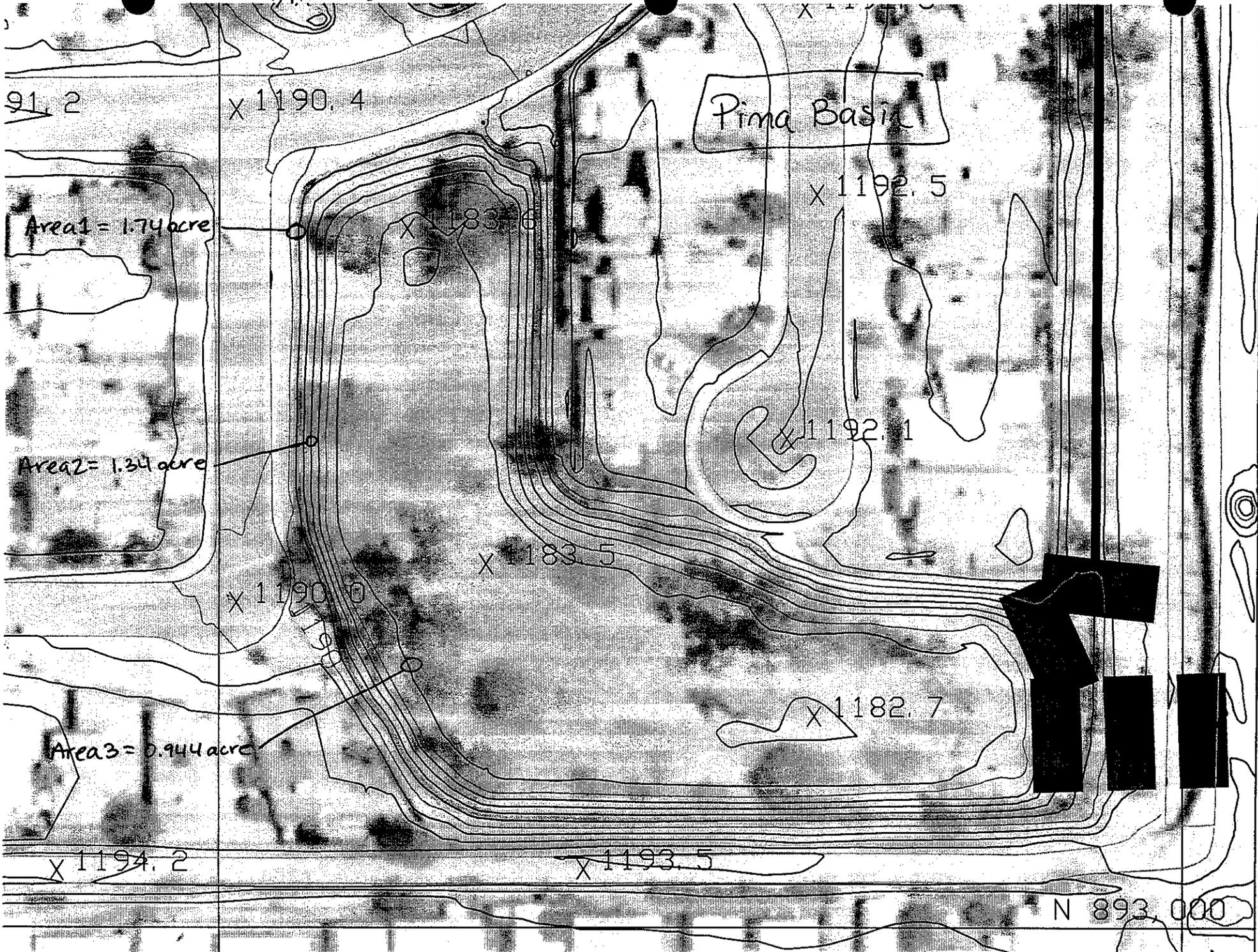
Area 3 = 0.944 acre

X 1182.7

X 1194.2

X 1193.5

N 893.000



BASIN BB

X

202.4

Roosevelt Basin

X 1201.7

Area 1 = 0.762 acre

X 1199.9

X 1186.7

Area 2 = 0.566

Area 3 = 0.159 acre

894,500

E 707,

5013

E 707, 500

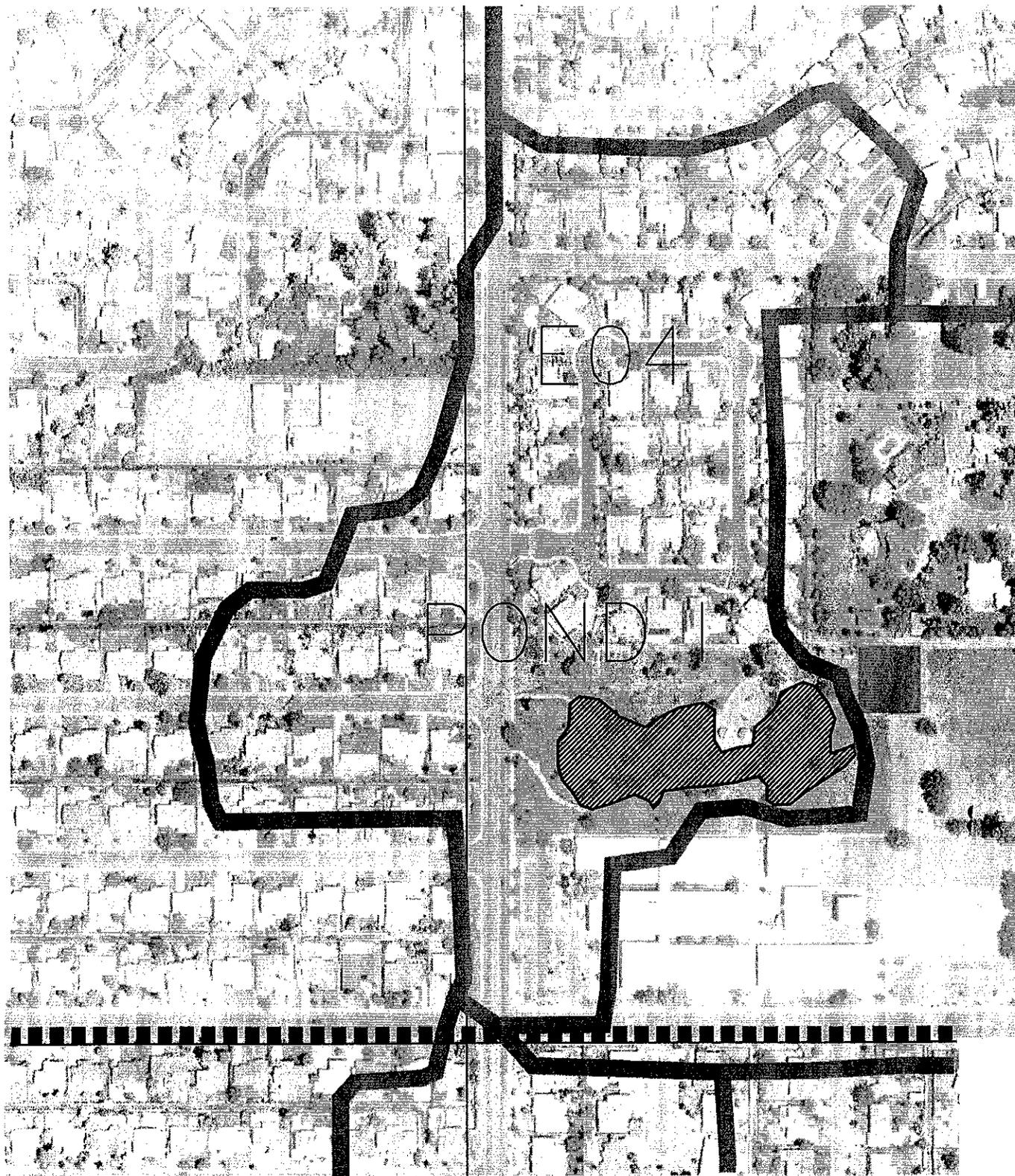
5013

200/0

1196.9

1101.0

# BASIN E04



## Detention Basin Calculations

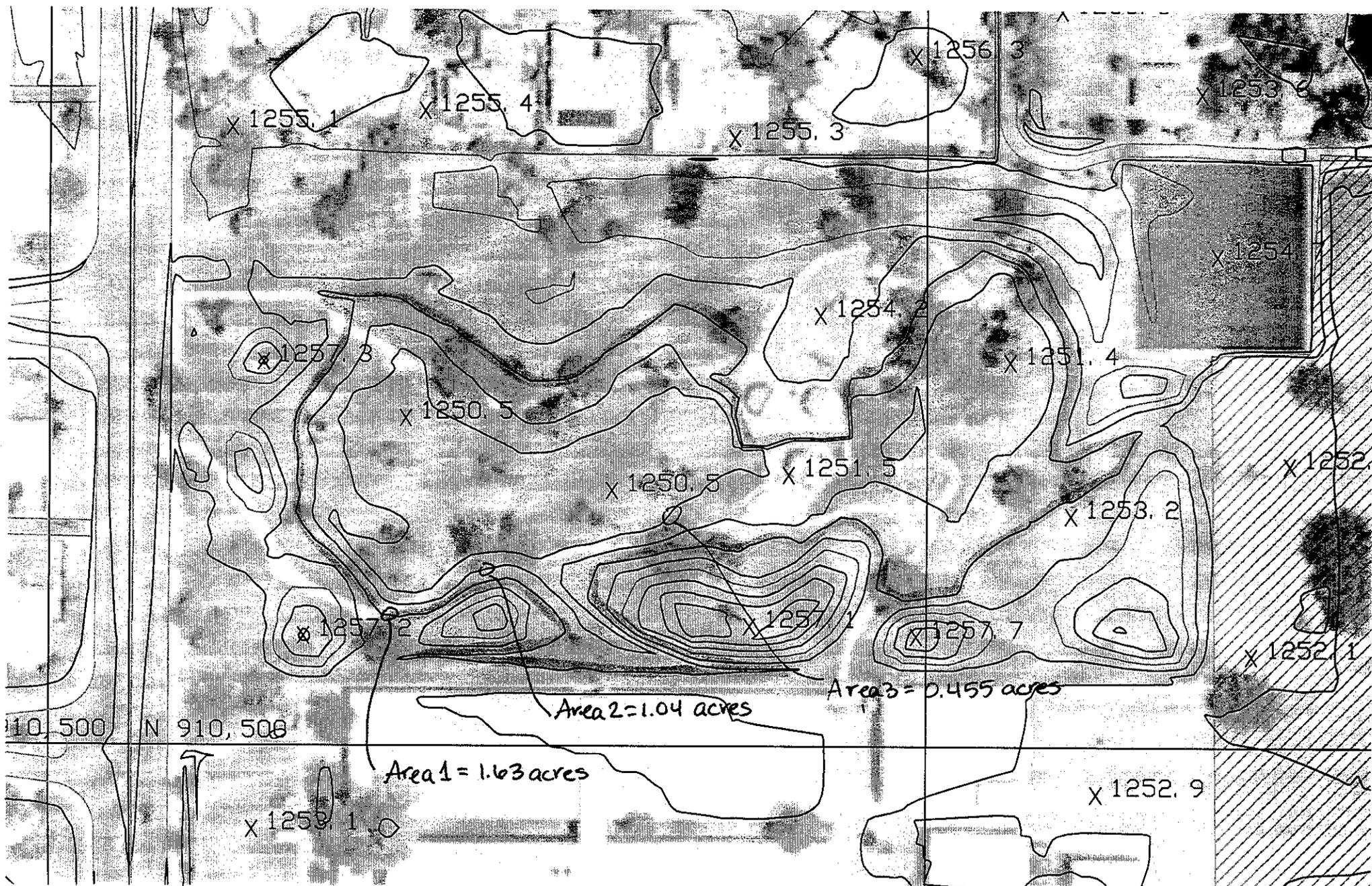
Note: See attached drawings of detention ponds for dimensions.

### Basin E04

Pond 1		
Depth (ft)	Area	Storage
0	0.455	
1	1.04	0.73
2	1.63	1.32
Total (ac ft)		2.05

Camel back Basin

BASIN E04



# BASIN F06



## Detention Basin Calculations

Note: See attached drawings of detention ponds for dimensions.

### Basin F06

Pond 1		
Depth (ft)	Area	Storage
0	0.96	
3.35	1.605	4.25
Total (ac ft)		4.25

### SRP-MIC FUTURE DETENTION

Basin ID	Area (sq mi)	Storage *(1) (ac ft)	80%
A01	0.507	44	35.2
B14	0.149	13	10.4
C13	0.176	14	11.2
D09	0.216	17	13.6
E09	0.223	17	13.6
F07	0.25	19	15.2
G06	0.127	10	8

Notes: 1) The Storage area was calculated from a HEC-1 model that used the 100-year, 2-hour rainfall. (See HEC-1 model - attached.)

\*\*\* \*\*

33 KK \*\*\*\*\*  
 \* \*  
 \* A01 \* BASIN  
 \* \*  
 \*\*\*\*\*

SUBBASIN RUNOFF DATA

34 BA SUBBASIN CHARACTERISTICS  
 TAREA 0.51 SUBBASIN AREA

35 LG GREEN AND AMPT LOSS RATE  
 STRTL 0.10 STARTING LOSS  
 DTH 0.25 MOISTURE DEFICIT  
 PSIF 5.40 WETTING FRONT SUCTION  
 XKSAT 0.32 HYDRAULIC CONDUCTIVITY  
 RTIMP 70.00 PERCENT IMPERVIOUS AREA

36 UC CLARK UNITGRAPH  
 TC 0.54 TIME OF CONCENTRATION  
 R 0.24 STORAGE COEFFICIENT

37 UA ACCUMULATED-AREA VS. TIME, 11 ORDINATES  
 0.0 5.0 16.0 30.0 65.0 77.0 84.0 90.0 94.0 97.0  
 100.0

\*\*\*

UNIT HYDROGRAPH PARAMETERS  
 CLARK TC= 0.54 HR, R= 0.24 HR  
 SNYDER TP= 0.27 HR, CP= 0.56

UNIT HYDROGRAPH  
 46 END-OF-PERIOD ORDINATES

20.	66.	129.	208.	298.	463.	634.	692.	679.	647.
613.	575.	534.	492.	451.	416.	376.	330.	287.	249.
217.	188.	164.	142.	124.	108.	93.	81.	71.	61.
53.	46.	40.	35.	30.	26.	23.	20.	17.	15.
13.	11.	10.	9.	7.	7.				

\*\*\* \*\*

INTERPOLATED HYDROGRAPH AT A01

PEAK FLOW (CFS)	TIME (HR)	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	66.63-HR
640.	4.20	88.	22.	8.	8.
		(INCHES) 1.615	1.624	1.624	1.624
		(AC-FT) 44.	44.	44.	44.

CUMULATIVE AREA = 0.51 SQ MI

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39 KK \* B14 \* BASIN

SUBBASIN RUNOFF DATA

40 BA SUBBASIN CHARACTERISTICS  
 TAREA 0.15 SUBBASIN AREA

41 LG GREEN AND AMPT LOSS RATE  
 STRTL 0.10 STARTING LOSS  
 DTH 0.25 MOISTURE DEFICIT  
 PSIF 4.60 WETTING FRONT SUCTION  
 XKSAT 0.48 HYDRAULIC CONDUCTIVITY  
 RTIMP 70.00 PERCENT IMPERVIOUS AREA

42 UC CLARK UNITGRAPH  
 TC 0.62 TIME OF CONCENTRATION  
 R 0.64 STORAGE COEFFICIENT

43 UA ACCUMULATED-AREA VS. TIME, 11 ORDINATES  
 0.0 5.0 16.0 30.0 65.0 77.0 84.0 90.0 94.0 97.0  
 100.0

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UNIT HYDROGRAPH PARAMETERS

CLARK TC= 0.62 HR, R= 0.64 HR  
 SNYDER TP= 0.40 HR, CP= 0.40

UNIT HYDROGRAPH

110 END-OF-PERIOD ORDINATES

2.	6.	12.	21.	30.	43.	63.	82.	92.	95.
96.	97.	96.	95.	94.	92.	89.	87.	85.	81.
77.	73.	70.	66.	63.	60.	57.	54.	51.	48.
46.	44.	41.	39.	37.	35.	34.	32.	30.	29.
27.	26.	25.	23.	22.	21.	20.	19.	18.	17.
16.	15.	15.	14.	13.	13.	12.	11.	11.	10.
10.	9.	9.	8.	8.	7.	7.	7.	6.	6.
6.	5.	5.	5.	5.	4.	4.	4.	4.	4.
3.	3.	3.	3.	3.	3.	3.	2.	2.	2.
2.	2.	2.	2.	2.	2.	1.	1.	1.	1.
1.	1.	1.	1.	1.	1.	1.	1.	1.	1.

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INTERPOLATED HYDROGRAPH AT B14

PEAK FLOW + (CFS)	TIME (HR)	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	66.63-HR
110.	4.30	25.	6.	2.	2.
		(INCHES) 1.566	1.598	1.598	1.598
		(AC-FT) 12.	13.	13.	13.

CUMULATIVE AREA = 0.15 SQ MI

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 \* \*  
 45 KK \* D09 \* BASIN  
 \* \*  
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SUBBASIN RUNOFF DATA

46 BA SUBBASIN CHARACTERISTICS  
 TAREA 0.22 SUBBASIN AREA

47 LG GREEN AND AMPT LOSS RATE  
 STRTL 0.10 STARTING LOSS  
 DTH 0.29 MOISTURE DEFICIT  
 PSIF 2.65 WETTING FRONT SUCTION  
 XKSAT 1.70 HYDRAULIC CONDUCTIVITY  
 RTIMP 70.00 PERCENT IMPERVIOUS AREA

48 UC CLARK UNITGRAPH  
 TC 0.48 TIME OF CONCENTRATION  
 R 0.39 STORAGE COEFFICIENT

49 UA ACCUMULATED-AREA VS. TIME, 11 ORDINATES  
 0.0 5.0 16.0 30.0 65.0 77.0 84.0 90.0 94.0 97.0  
 100.0

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UNIT HYDROGRAPH PARAMETERS

CLARK TC= 0.48 HR, R= 0.39 HR  
 SNYDER TP= 0.27 HR, CP= 0.41

UNIT HYDROGRAPH

69 END-OF-PERIOD ORDINATES

6.	21.	43.	69.	114.	173.	207.	214.	214.	211.
206.	198.	190.	182.	172.	159.	146.	134.	123.	113.
104.	96.	88.	81.	74.	68.	63.	58.	53.	49.
45.	41.	38.	35.	32.	29.	27.	25.	23.	21.
19.	18.	16.	15.	14.	13.	12.	11.	10.	9.
8.	8.	7.	6.	6.	5.	5.	5.	4.	4.
4.	3.	3.	3.	3.	2.	2.	2.	2.	

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INTERPOLATED HYDROGRAPH AT D09

PEAK FLOW + (CFS)	TIME (HR)	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	66.63-HR
191.	4.20	34.	8.	3.	3.
		(INCHES) 1.447	1.461	1.461	1.461
		(AC-FT) 17.	17.	17.	17.

CUMULATIVE AREA = 0.22 SQ MI

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 \* \*  
 \* E09 \* BASIN  
 \* \*  
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SUBBASIN RUNOFF DATA

52 BA SUBBASIN CHARACTERISTICS  
 TAREA 0.22 SUBBASIN AREA

53 LG GREEN AND AMPT LOSS RATE  
 STRTL 0.10 STARTING LOSS  
 DTH 0.29 MOISTURE DEFICIT  
 PSIF 2.65 WETTING FRONT SUCTION  
 XKSAT 1.70 HYDRAULIC CONDUCTIVITY  
 RTIMP 70.00 PERCENT IMPERVIOUS AREA

54 UC CLARK UNITGRAPH  
 TC 0.57 TIME OF CONCENTRATION  
 R 0.47 STORAGE COEFFICIENT

55 UA ACCUMULATED-AREA VS. TIME, 11 ORDINATES  
 0.0 5.0 16.0 30.0 65.0 77.0 84.0 90.0 94.0 97.0  
 100.0

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UNIT HYDROGRAPH PARAMETERS

CLARK TC= 0.57 HR, R= 0.47 HR  
 SNYDER TP= 0.33 HR, CP= 0.42

UNIT HYDROGRAPH

82 END-OF-PERIOD ORDINATES

4.	14.	29.	48.	68.	105.	153.	180.	186.	188.
186.	184.	180.	174.	168.	162.	156.	148.	137.	128.
119.	111.	103.	96.	89.	83.	77.	72.	67.	62.
58.	54.	50.	47.	44.	41.	38.	35.	33.	30.
28.	26.	25.	23.	21.	20.	18.	17.	16.	15.
14.	13.	12.	11.	10.	10.	9.	8.	8.	7.
7.	6.	6.	5.	5.	5.	4.	4.	4.	4.
3.	3.	3.	3.	2.	2.	2.	2.	2.	2.
2.	1.								

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INTERPOLATED HYDROGRAPH AT E09

PEAK FLOW (CFS)	TIME (HR)	MAXIMUM AVERAGE FLOW				
		6-HR	24-HR	72-HR	66.63-HR	
177.	4.23	35.	9.	3.	3.	
		(INCHES)	1.444	1.461	1.461	1.461
		(AC-FT)	17.	17.	17.	17.

CUMULATIVE AREA = 0.22 SQ MI

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 \* \*  
 57 KK \* F07 \* BASIN  
 \* \*  
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SUBBASIN RUNOFF DATA

58 BA SUBBASIN CHARACTERISTICS  
 TAREA 0.25 SUBBASIN AREA

59 LG GREEN AND AMPT LOSS RATE  
 STRTL 0.10 STARTING LOSS  
 DTH 0.29 MOISTURE DEFICIT  
 PSIF 2.65 WETTING FRONT SUCTION  
 XKSAT 1.70 HYDRAULIC CONDUCTIVITY  
 RTIMP 70.00 PERCENT IMPERVIOUS AREA

60 UC CLARK UNITGRAPH  
 TC 0.56 TIME OF CONCENTRATION  
 R 0.43 STORAGE COEFFICIENT

61 UA ACCUMULATED-AREA VS. TIME, 11 ORDINATES  
 0.0 5.0 16.0 30.0 65.0 77.0 84.0 90.0 94.0 97.0  
 100.0

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UNIT HYDROGRAPH PARAMETERS

CLARK TC= 0.56 HR, R= 0.43 HR  
 SNYDER TP= 0.31 HR, CP= 0.42

UNIT HYDROGRAPH

76 END-OF-PERIOD ORDINATES

5.	18.	36.	59.	83.	129.	187.	216.	222.	223.
220.	216.	210.	203.	194.	186.	178.	167.	155.	143.
133.	123.	114.	105.	97.	90.	84.	77.	72.	66.
61.	57.	53.	49.	45.	42.	39.	36.	33.	31.
28.	26.	24.	22.	21.	19.	18.	17.	15.	14.
13.	12.	11.	10.	10.	9.	8.	8.	7.	7.
6.	6.	5.	5.	4.	4.	4.	4.	3.	3.
3.	3.	2.	2.	2.	2.				

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INTERPOLATED HYDROGRAPH AT F07

PEAK FLOW + (CFS)	TIME (HR)	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	66.63-HR
206.	4.23	39.	10.	4.	4.
		(INCHES) 1.445	1.460	1.460	1.460
		(AC-FT) 19.	19.	19.	19.

CUMULATIVE AREA = 0.25 SQ MI

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 \* \* \* \* \*  
 63 KK \* G06 \* BASIN  
 \* \* \* \* \*  
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SUBBASIN RUNOFF DATA

64 BA SUBBASIN CHARACTERISTICS  
 TAREA 0.13 SUBBASIN AREA

65 LG GREEN AND AMPT LOSS RATE  
 STRTL 0.10 STARTING LOSS  
 DTH 0.35 MOISTURE DEFICIT  
 PSIF 2.65 WETTING FRONT SUCTION  
 XKSAT 1.70 HYDRAULIC CONDUCTIVITY  
 RTIMP 70.00 PERCENT IMPERVIOUS AREA

66 UC CLARK UNITGRAPH  
 TC 0.26 TIME OF CONCENTRATION  
 R 0.14 STORAGE COEFFICIENT

67 UA ACCUMULATED-AREA VS. TIME, 11 ORDINATES  
 0.0 5.0 16.0 30.0 65.0 77.0 84.0 90.0 94.0 97.0  
 100.0

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UNIT HYDROGRAPH PARAMETERS

CLARK TC= 0.26 HR, R= 0.14 HR  
 SNYDER TP= 0.13 HR, CP= 0.50

UNIT HYDROGRAPH

26 END-OF-PERIOD ORDINATES

21.	79.	198.	296.	301.	276.	245.	212.	175.	138.
109.	86.	68.	54.	42.	33.	26.	21.	17.	13.
10.	8.	6.	5.	4.	3.				

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INTERPOLATED HYDROGRAPH AT G06

PEAK FLOW (CFS)	TIME (HR)	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	66.63-HR
180.	4.07	20.	5.	2.	2.
		(INCHES) 1.442	1.447	1.447	1.447
		(AC-FT) 10.	10.	10.	10.

CUMULATIVE AREA = 0.13 SQ MI

## D.5 Flow Splits and Diversions Data



SHEET 1 OF 25

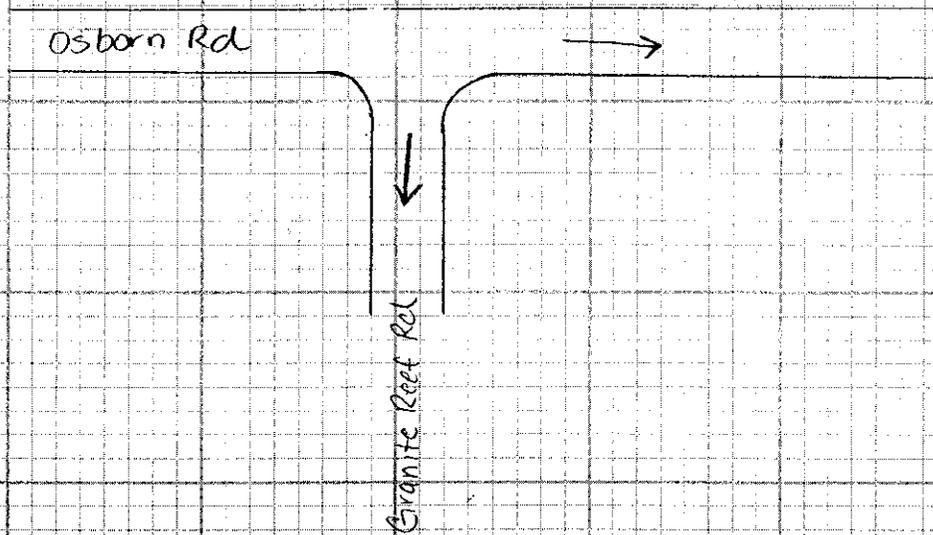
BY AMG DATE 4/17/02

CHECK PAW DATE 4/18/02

CLIENT Flood Control District of Maricopa County

JOB NAME Granite Reef Drainage Master Plan JOB NO. 310.021

Diversion DDOS at Granite Reef Rd and Osborn Rd



See split flow calculation table diversion DDOS  
and DDOS #2

**Normal Depth Split Flow Calculations  
Diversion DD05**

(1) Main Path South on Granite Reef Road (Route RD05S)

(2) Diverted Flow east along Osborn Road (RD05E)

Depth	Width (1)	Width (2)	Manning's	Slope (1)	Slope (2)	Side Slope (1)	Side Slope (2)	Q <sub>(1)</sub>	Q <sub>(2)</sub>	Q <sub>total</sub>
0.1	40	48	0.02	0.00158	0.00109	0	0	2.5	2.5	5
0.2	40	48	0.02	0.00158	0.00109	0	0	8	8	16
0.3	40	48	0.02	0.00158	0.00109	0	0	16	16	32
0.4	40	48	0.02	0.00158	0.00109	0	0	25	25	50
0.5	40	48	0.02	0.00158	0.00109	0	0	<b>37</b>	<b>37</b>	74
0.6	40	48	0.02	0.00158	0.00109	25	100	40	40	80
0.7	40	48	0.02	0.00158	0.00109	25	100	46	47	93
0.8	40	48	0.02	0.00158	0.00109	25	100	54	58	112
0.9	40	48	0.02	0.00158	0.00109	25	100	65	74	139
1	40	48	0.02	0.00158	0.00109	25	100	79	95	174
1.1	40	48	0.02	0.00158	0.00109	25	100	96	121	217
1.2	40	48	0.02	0.00158	0.00109	25	100	116	154	270
1.3	40	48	0.02	0.00158	0.00109	25	100	117	194	311

Notes: (a) Assumed 6 inch vertical curbs.



SHEET 3 OF 25

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CLIENT Flood Control District of Maricopa County

JOB NAME Granite Reef Drainage Master Plan

JOB NO. 310.021

Diversion DDO5 #2 at Mulberry Rd and Osborn Rd

Osborn Rd.



Mulberry Rd. ↓

See split flow calculation table DDO5 #2 and DDO5



SHEET 4 OF 25

BY AMG DATE 4/17/02

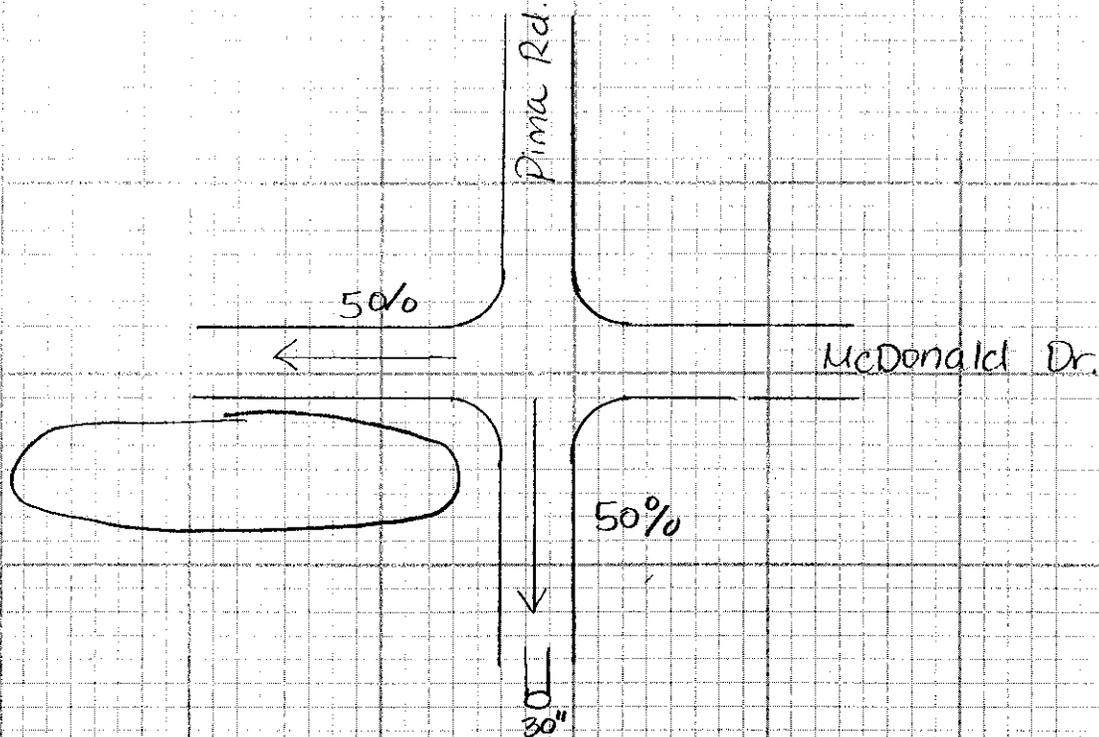
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CLIENT Flood Control District of Maricopa County

JOB NAME Granite Reef Drainage Master Plan

JOB NO. 310.021

Diversion DE06 at Pima Rd & McDonald Dr.



Assumed 50% of flow from basin G06 flows into detention basin, remaining flow is overland south.



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SHEET 5 OF 25

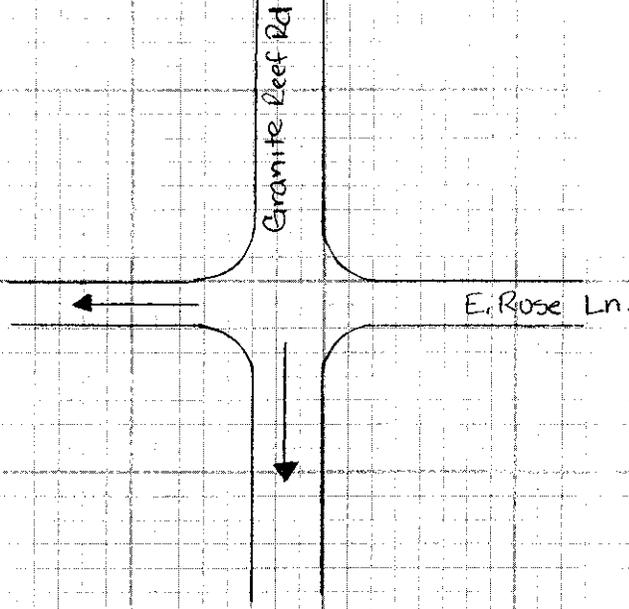
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CLIENT Flood Control District of Maricopa County

JOB NAME Granite Reef Drainage Master Plan JOB NO. 310.02-1

Diversion D6103 at Granite Reef Rd & E. Rose Ln.



See Split Flow Calculation Table.

**Normal Depth Split Flow Calculations  
Diversion DG03**

(1) Main Path South on Granite Reef Road (Route RG03)

(2) Diverted Flow west along Rose Lane

Depth	Width (1)	Width (2)	Manning's	Slope (1)	Slope (2)	Side Slope (1)	Side Slope (2)	Q <sub>(1)</sub>	Q <sub>(2)</sub>	Q <sub>total</sub>
0.1	44	38	0.02	0.0024	0.00335	0	0	3.5	3.5	7
0.2	44	38	0.02	0.0024	0.00335	0	0	11	11	22
0.3	44	38	0.02	0.0024	0.00335	0	0	21	22	43
0.4	44	38	0.02	0.0024	0.00335	0	0	35	35	70
0.5	44	38	0.02	0.0024	0.00335	0	0	50	51	101
0.6	44	38	0.02	0.0024	0.00335	20	30	54	55	109
0.7	44	38	0.02	0.0024	0.00335	20	30	61	63	124
0.8	44	38	0.02	0.0024	0.00335	20	30	73	75	148
0.9	44	38	0.02	0.0024	0.00335	20	30	88	92	180
1	44	38	0.02	0.0024	0.00335	20	30	105	112	217
1.1	44	38	0.02	0.0024	0.00335	20	30	127	137	264
1.2	44	38	0.02	0.0024	0.00335	20	30	151	166	317

Notes: (a) Assumed 6 inch vertical curbs.



SHEET 7 OF 25

BY AMB DATE 4/17/02

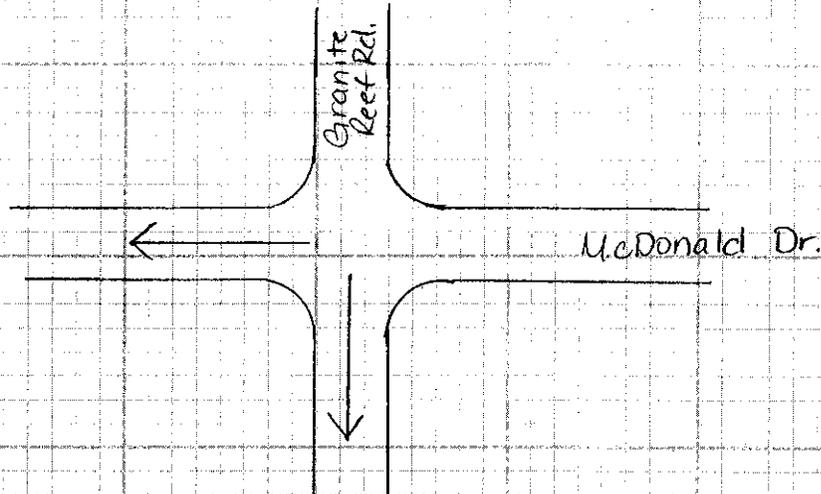
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CLIENT Flood Control District of Maricopa County

JOB NAME Granite Reef Drainage Master Plan

JOB NO. 310.021

Diversion DG01 at Granite Reef Rd & McDonald Dr.



See split flow calculation table

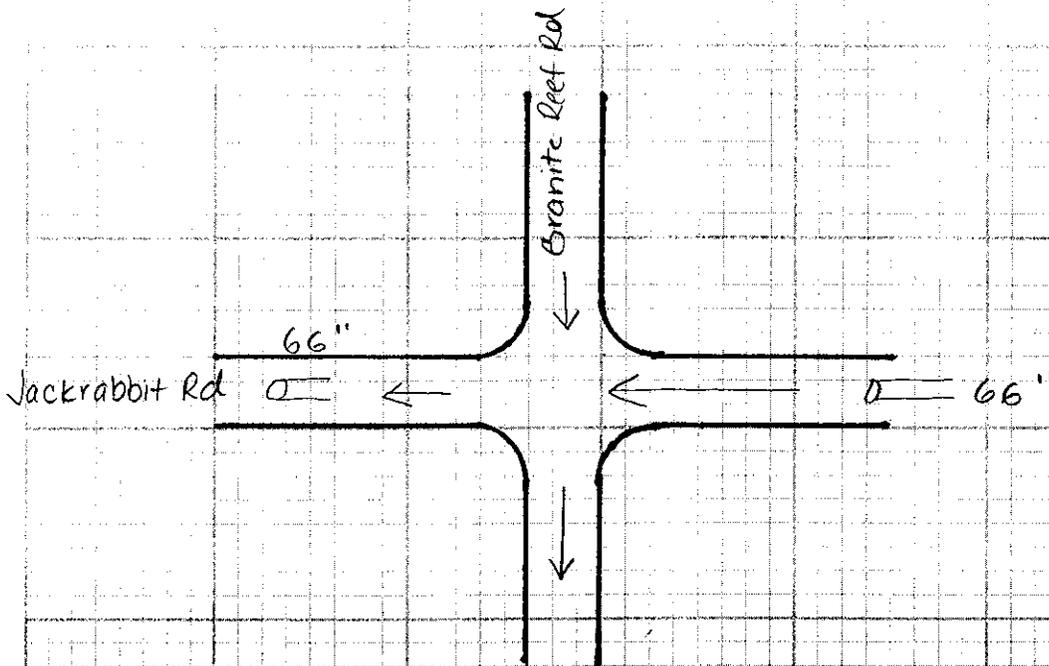
**Normal Depth Split Flow Calculations  
Diversion DG01**

(1) Main Path South on Granite Reef Road (Route RG01)

(2) Diverted Flow west along McDonald Drive

Depth	Width <sub>(1)</sub>	Width <sub>(2)</sub>	Manning's	Slope <sub>(1)</sub>	Slope <sub>(2)</sub>	Side Slope <sub>(1)</sub>	Side Slope <sub>(2)</sub>	Q <sub>(1)</sub>	Q <sub>(2)</sub>	Q <sub>total</sub>
0.1	44	70	0.02	0.0014	0.00348	0	0	3	7	10
0.2	44	70	0.02	0.0014	0.00348	0	0	8	21	29
0.3	44	70	0.02	0.0014	0.00348	0	0	16	41	57
0.4	44	70	0.02	0.0014	0.00348	0	0	26	66	92
0.5	44	70	0.02	0.0014	0.00348	0	0	<b>38</b>	<b>96</b>	134
0.6	44	70	0.02	0.0014	0.00348	20	20	41	103	144
0.7	44	70	0.02	0.0014	0.00348	20	20	47	117	164
0.8	44	70	0.02	0.0014	0.00348	20	20	55	139	194
0.9	44	70	0.02	0.0014	0.00348	20	20	67	166	233
1	44	70	0.02	0.0014	0.00348	20	20	80	198	278
1.1	44	70	0.02	0.0014	0.00348	20	20	97	236	333
1.2	44	70	0.02	0.0014	0.00348	20	20	115	280	395

Notes: (a) Assumed 6 inch vertical curbs.

CLIENT Flood Control District of Maricopa CountyJOB NAME Granite Reef Drainage Master Plan JOB NO. 310.021Diversion DFO4 @ Granite Reef Rd & Jackrabbit Rd

Flow West is storm drain only. Maximum Capacity is 185 cfs (see storm drain calculations)  
120 cfs is collected here.

**Normal Depth Split Flow Calculations  
Diversion DF04**

(1) Main Path South on Granite Reef Road (Route RF04)

(2) Diverted Flow west along Jackrabbit Road and the storm drain.

Depth	Width <sub>(1)</sub>	Width <sub>(2)</sub>	Manning's	Slope <sub>(1)</sub>	Slope <sub>(2)</sub>	Side Slope <sub>(1)</sub>	Side Slope <sub>(2)</sub>	Q <sub>(1)</sub>	Q <sub>(2)</sub>	Q <sub>(storm drain)</sub>	Q <sub>total</sub>
0.1	44	34	0.02	0.0038	0.0054	30	20	4	4	1	9
0.2	44	34	0.02	0.0038	0.0054	30	20	14	13	3	30
0.3	44	34	0.02	0.0038	0.0054	30	20	27	25	5	57
0.4	44	34	0.02	0.0038	0.0054	30	20	43	40	10	93
0.5	44	34	0.02	0.0038	0.0054	30	20	63	58	15	136
0.6	44	34	0.02	0.0038	0.0054	30	20	68	62	20	150
0.7	44	34	0.02	0.0038	0.0054	30	20	78	71	20	169
0.8	44	34	0.02	0.0038	0.0054	30	20	93	85	20	198
0.9	44	34	0.02	0.0038	0.0054	30	20	112	103	20	235
1	44	34	0.02	0.0038	0.0054	30	20	136	124	20	280
1.1	44	34	0.02	0.0038	0.0054	30	20	166	150	20	336
1.2	44	34	0.02	0.0038	0.0054	30	20	200	180	20	400

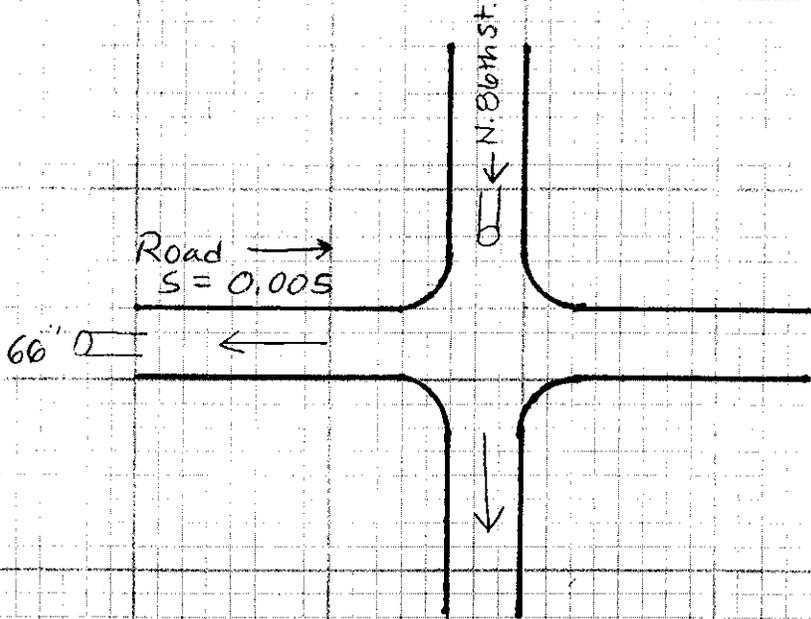
Notes: (a) Assumed 6 inch vertical curbs.

(b) The stormdrain capacity is 20 cfs at this location (see stormdrain capacity calculations.)



CLIENT Flood Control District of Maricopa County  
JOB NAME Granite Reef Drainage Master Plan JOB NO. 310.021

Diversion DFOS @ N. 86th St & Jackrabbit Rd.



FLOW WEST IS STORM DRAIN ONLY.

Maximum Capacity is 185 cfs. (See storm drain calculations) 60 cfs collected at this location.



SHEET 12 OF 25

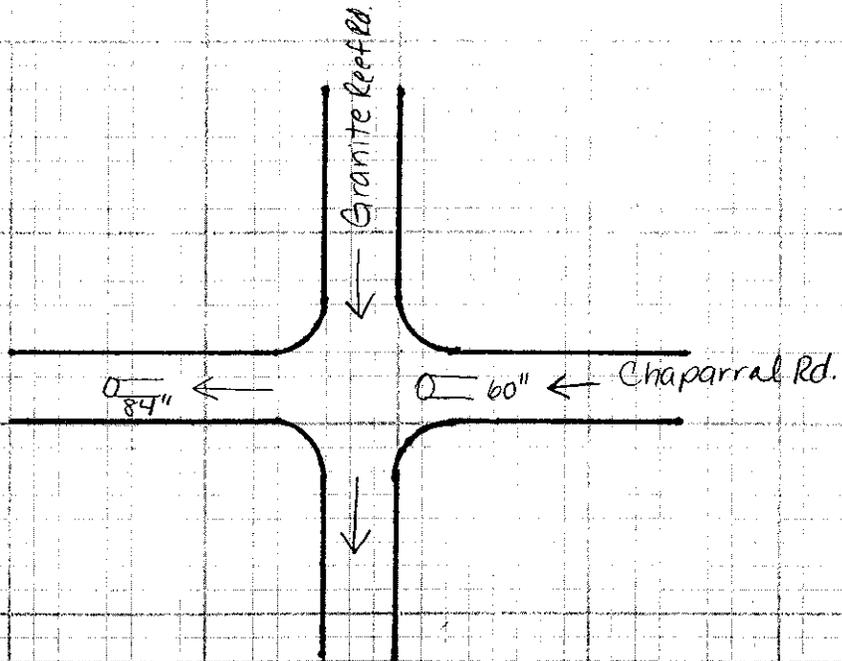
BY AMG DATE 4/17/02

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CLIENT Flood Control District of Maricopa County

JOB NAME Granite Reef Drainage Master Plan JOB NO. 310.021

Diversion DFO1 @ Granite Reef Rd & Chaparral Rd.



See Split Flow Calculation Sheet.

**Normal Depth Split Flow Calculations  
Diversion DF01**

(1) Main path west along Chapparal Road and the storm drain.

(2) Diverted flow south on Granite Reef Road (Route RF01S)

Depth	Width <sub>(1)</sub>	Width <sub>(2)</sub>	Manning's	Slope <sub>(1)</sub>	Slope <sub>(2)</sub>	Side Slope <sub>(1)</sub>	Side Slope <sub>(2)</sub>	Q <sub>(1)</sub>	Q <sub>(2)</sub>	Q <sub>(storm drain)</sub>	Q <sub>total</sub>
0.1	56	46	0.02	0.00436	0.0059	0	0	6	0	1	7
0.2	56	46	0.02	0.00436	0.0059	0	0	19	0	2	21
0.3	56	46	0.02	0.00436	0.0059	0	0	37	0	5	42
0.4	56	46	0.02	0.00436	0.0059	0	0	60	0	15	75
0.5	56	46	0.02	0.00436	0.0059	0	0	86	0	25	111
0.6	56	46	0.02	0.00436	0.0059	20	30	98	6	35	139
0.7	56	46	0.02	0.00436	0.0059	20	30	125	18	45	188
0.8	56	46	0.02	0.00436	0.0059	20	30	164	35	55	254
0.9	56	46	0.02	0.00436	0.0059	20	30	213	57	65	335

Notes: (a) Assumed 6 inch vertical curbs.

(b) The main Route RF01S is 1/2 foot lower than the diversion.

(c) The stormdrain capacity is 128 cfs at this location (see stormdrain capacity calculations.)



SHEET 14 OF 25

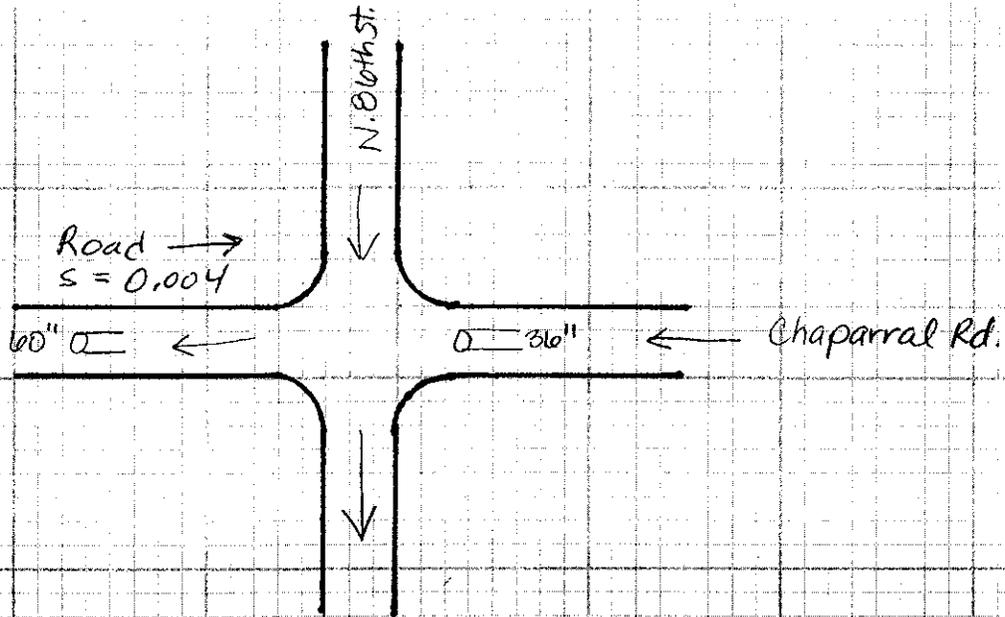
BY AMG DATE 4/17/02

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CLIENT Flood Control District of Maricopa County

JOB NAME Granite Reef Drainage Master Plan JOB NO. 310 021

Diversion DFO2 @ N. 86th St. & Chaparral Rd.



FLOW WEST IS STORM DRAIN ONLY  
Maximum Capacity is 63 cfs (see storm  
Drain Calculations) 47 cfs is collected  
Here.



SHEET 15 OF 25

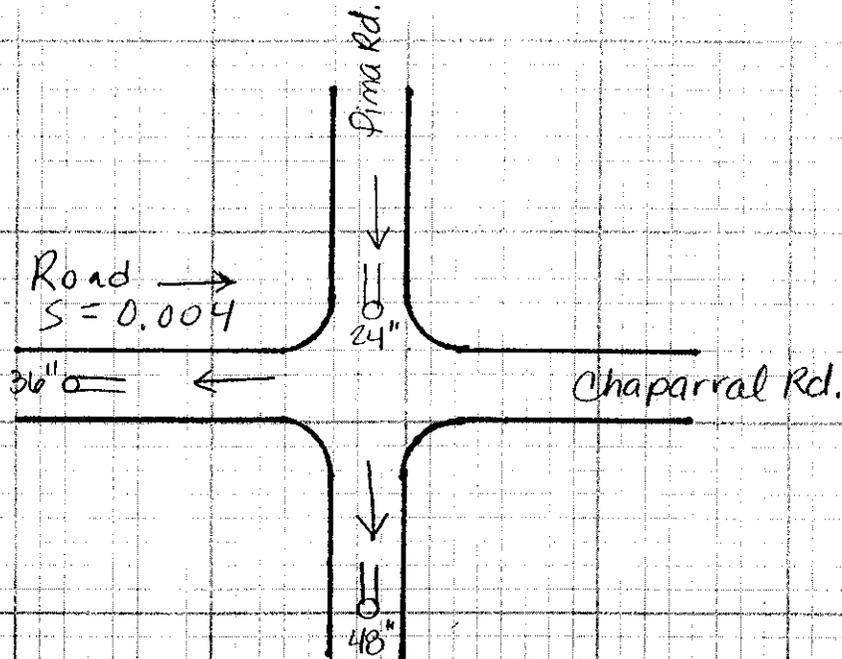
BY AMG DATE 4/17/02

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CLIENT Flood Control District of Maricopa County

JOB NAME Granite Reef Drainage Master Plan JOB NO. 310.021

Diversion DF07 @ Pima Rd + Chaparral Rd.



FLOW WEST IS STORM DRAIN ONLY.  
Maximum Capacity is 16 cfs. (see storm  
Drain calculations.)

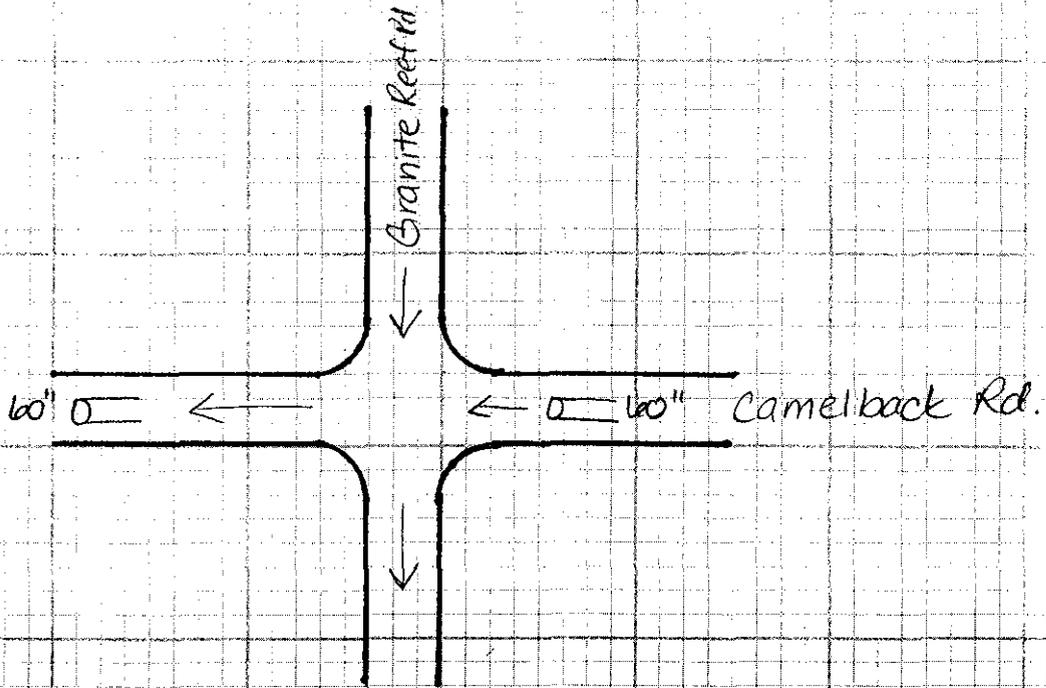


SHEET 16 OF 25  
BY AMG DATE 4/17/02  
CHECK PAW DATE 4/18/02

CLIENT Flood Control District of Manicopa County

JOB NAME Granite Reef Drainage Master Plan JOB NO. 310.021

Diversion DE04 @ Granite Reef Rd + Camelback Rd.



See Split Flow calculation sheet.

**Normal Depth Split Flow Calculations  
Diversion DE04**

(1) Main path west along Camelback Road (with storm drain) rest in street. (Route RE04W)

(2) Diverted flow south on Granite Reef Road (Route RE04S)

Depth	Width (1)	Width (2)	Manning's	Slope (1)	Slope (2)	Side Slope (1)	Side Slope (2)	Q <sub>(1)</sub>	Q <sub>(2)</sub>	Q <sub>(storm drain)</sub>	Q <sub>total</sub>
0.1	48	48	0.02	0.0015	0.0028	0	0	0	3	0	3
0.2	48	48	0.02	0.0015	0.0028	0	0	0	12	3	15
0.3	48	48	0.02	0.0015	0.0028	0	0	0	23	8	31
0.4	48	48	0.02	0.0015	0.0028	0	0	0	37	12	49
0.5	48	48	0.02	0.0015	0.0028	0	0	0	43	16	59
0.6	48	48	0.02	0.0015	0.0028	30	30	0	47	20	67
0.7	48	48	0.02	0.0015	0.0028	30	30	0	55	24	79
0.8	48	48	0.02	0.0015	0.0028	30	30	0	67	24	91
0.9	48	48	0.02	0.0015	0.0028	30	30	0	83	24	107
1	48	48	0.02	0.0015	0.0028	30	30	0	102	24	126
1.1	48	48	0.02	0.0015	0.0028	30	30	3	124	24	151
1.2	48	48	0.02	0.0015	0.0028	30	30	9	151	24	184
1.3	48	48	0.02	0.0015	0.0028	30	30	18	180	24	222
1.4	48	48	0.02	0.0015	0.0028	30	30	30	213	24	267

Notes: (a) Assumed 6 inch vertical curbs.

(b) The main Route RE04W is 1 foot higher than the diversion.

(c) The stormdrain capacity is 92 cfs at this location (see stormdrain capacity calculations.)

The total flow is divided by basin E05, E04, and the basin west of the project area.



SHEET 18 OF 25

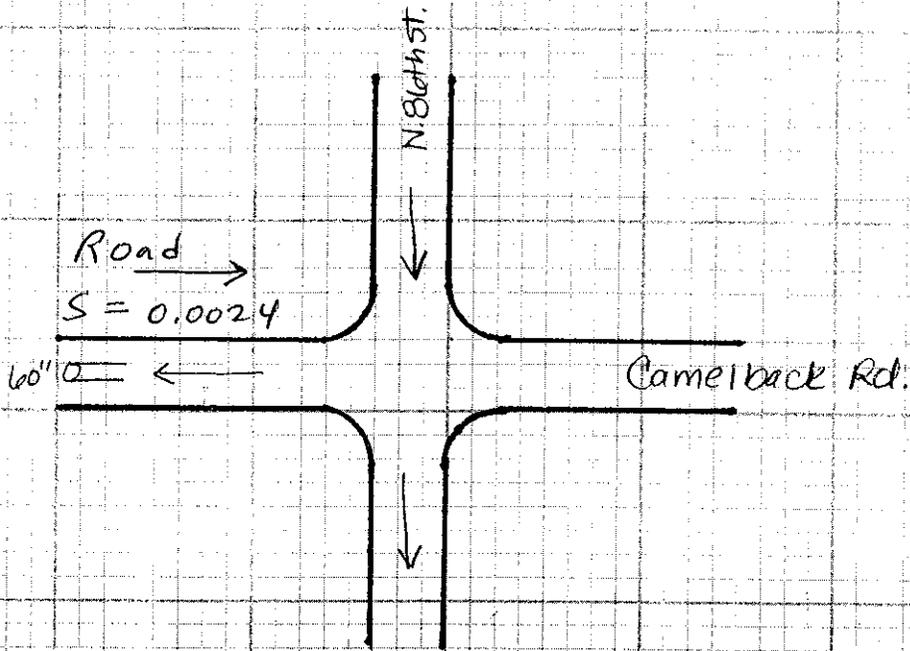
BY AMG DATE 4/17/02

CHECK PAW DATE 4/18/02

CLIENT Flood Control District of Maricopa County

JOB NAME Granite Reef Drainage Master Plan JOB NO. 310.021

Diversion DEOS at N. 86th St. & Camelback Rd.



FLOW WEST IS STORM DRAIN ONLY.

Maximum capacity is 92 cfs (see storm drain calculations) 30 cfs is taken here.



SHEET 19 OF 25

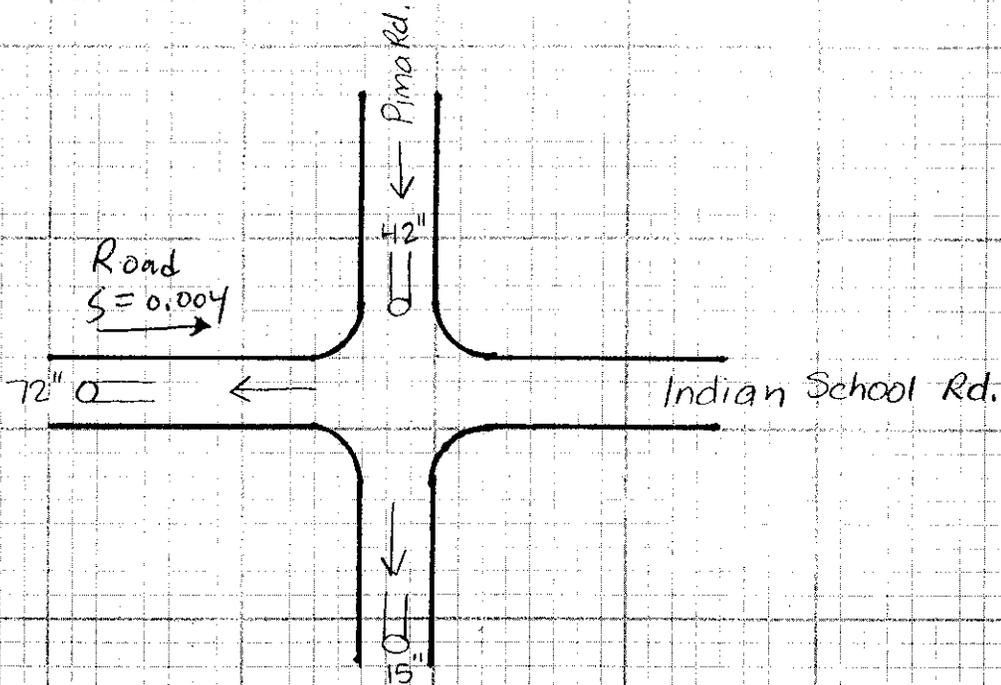
BY AMG DATE 4/17/02

CHECK PAW DATE 4/18/02

CLIENT Flood Control District of Maricopa County

JOB NAME Granite Reef Drainage Master Plan JOB NO. 310.021

Diversion DE09 at Indian School Rd & Pima Rd.



FLDW WEST IS STORM DRAIN ONLY.

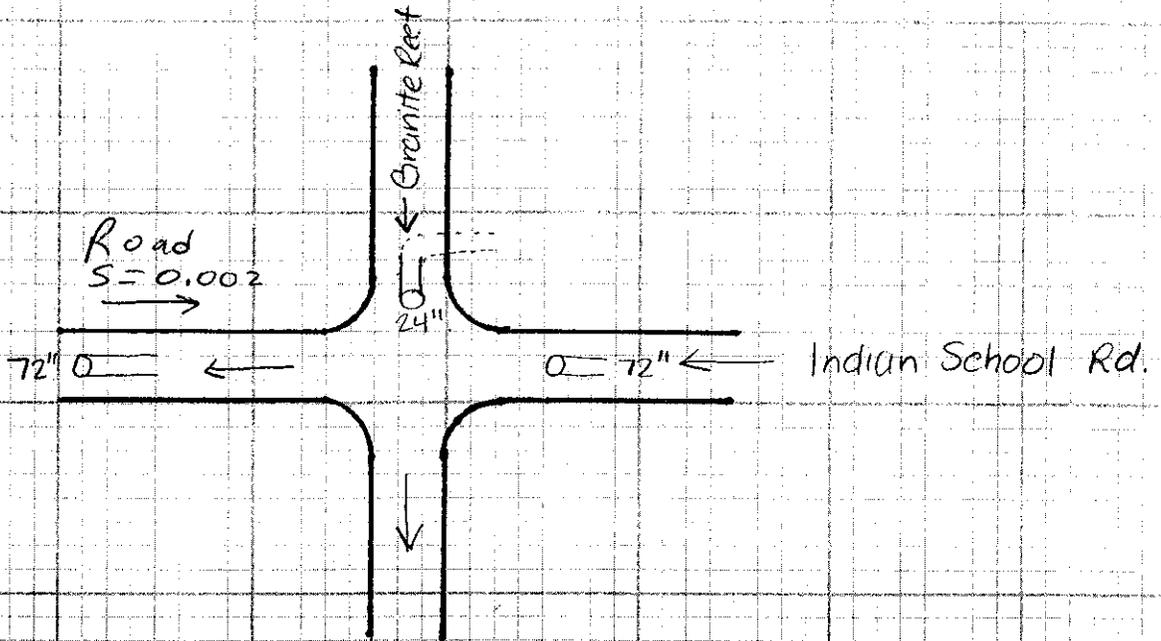
Maximum Capacity is 96 cfs (see storm drain calculations.) 20 cfs is taken at this location.



CLIENT Flood Control District Mancoska County

JOB NAME Granite Reef Drainage Master Plan JOB NO. 310.021

Diversion DE01 @ Granite Reef Rd + Indian School Rd.



FLOW WEST IS STORM DRAIN ONLY

Maximum Capacity is 96cfs (see storm drain calculations.)



SHEET 21 OF 25

BY AMG DATE 4/17/02

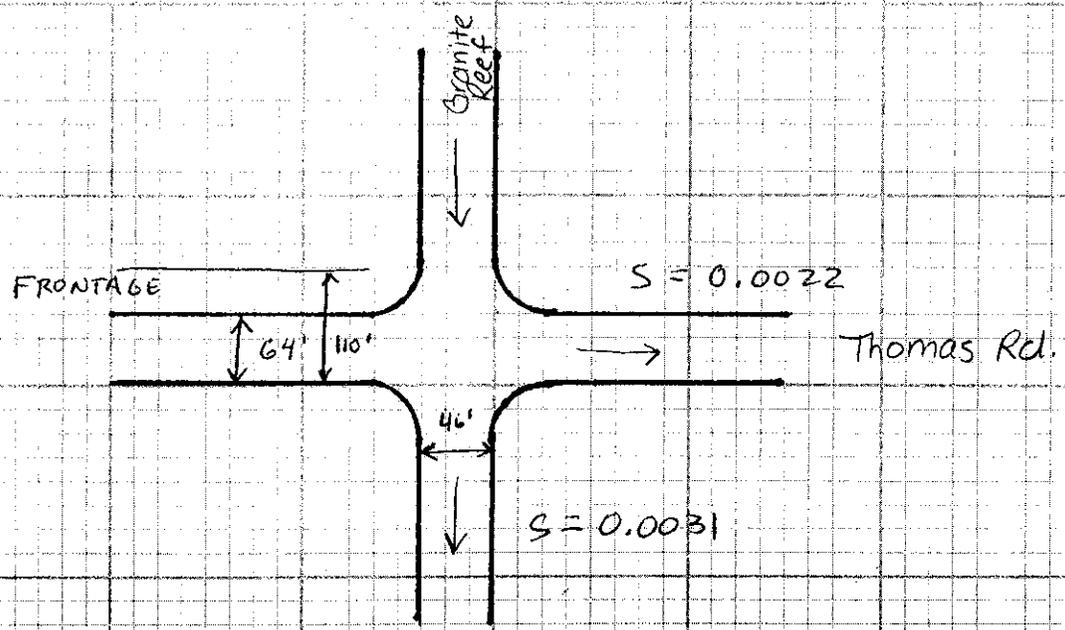
CHECK PAW DATE 4/18/02

CLIENT Flood Control District Maricopa County

JOB NAME Granite Reef Drainage Master Plan

JOB NO. 310021

Diversion D001 @ Granite Reef Rd & Thomas Rd.



See Split Flow calculation sheet

**Normal Depth Split Flow Calculations  
Diversion DD01**

(1) Main path east along Thomas Road. (Route RD01E)

(2) Diverted flow south on Granite Reef Road (Route RD01S)

Depth	Width (1)	Width (2)	Manning's	Slope (1)	Slope (2)	Side Slope (1)	Side Slope (2)	Q <sub>(1)</sub>	Q <sub>(2)</sub>	Q <sub>total</sub>
0.1	64	46	0.02	0.00216	0.0031	0	0	5	4	9
0.2	64	46	0.02	0.00216	0.0031	0	0	15	13	28
0.3	64	46	0.02	0.00216	0.0031	0	0	30	25	55
0.4	64	46	0.02	0.00216	0.0031	0	0	48	41	89
0.5	64	46	0.02	0.00216	0.0031	0	0	69	59	128
0.6	110	46	0.02	0.00216	0.0031	25	25	77	63	140
0.7	110	46	0.02	0.00216	0.0031	25	25	95	73	168
0.8	110	46	0.02	0.00216	0.0031	25	25	121	86	207
0.9	110	46	0.02	0.00216	0.0031	25	25	155	104	259
1	110	46	0.02	0.00216	0.0031	25	25	194	126	320
1.1	110	46	0.02	0.00216	0.0031	25	25	240	152	392
1.2	110	46	0.02	0.00216	0.0031	25	25	293	182	475
1.3	110	46	0.02	0.00216	0.0031	25	25	351	217	568
1.4	110	46	0.02	0.00216	0.0031	25	25	416	256	672

Notes: (a) Assumed 6 inch vertical curbs.

(b) Thomas Road has north and south frontage roads that were considered at flow depths of greater than 6 inches.



SHEET 23 OF 25

BY AMG DATE 4/17/02

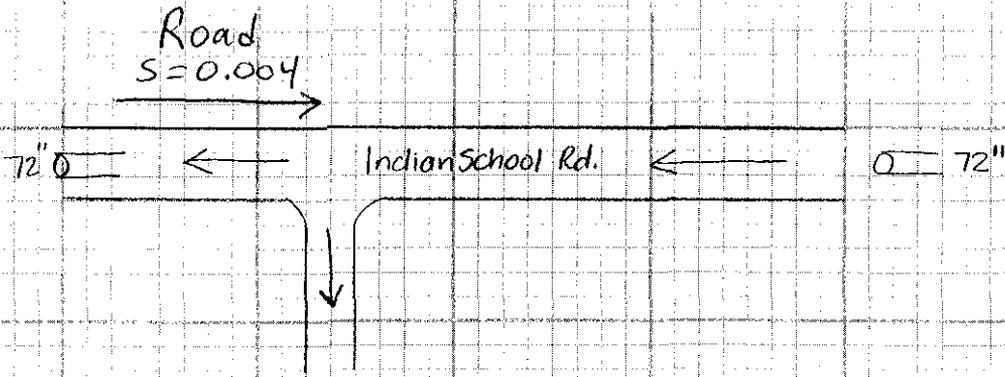
CHECK PAW DATE 4/18/02

CLIENT Flood Control District Maricopa County

JOB NAME Granite Reef Drainage Master Plan

JOB NO. 310.021

Diversion DE02 at Indian School Rd between Granite Reef Rd and N. 86th St.



FLOW WEST IS STORM DRAIN ONLY.  
STORM DRAIN MAXIMUM CAPACITY IS 96 cfs.  
60 cfs IS DIVERTED AT THIS LOCATION.



SHEET 24 OF 25

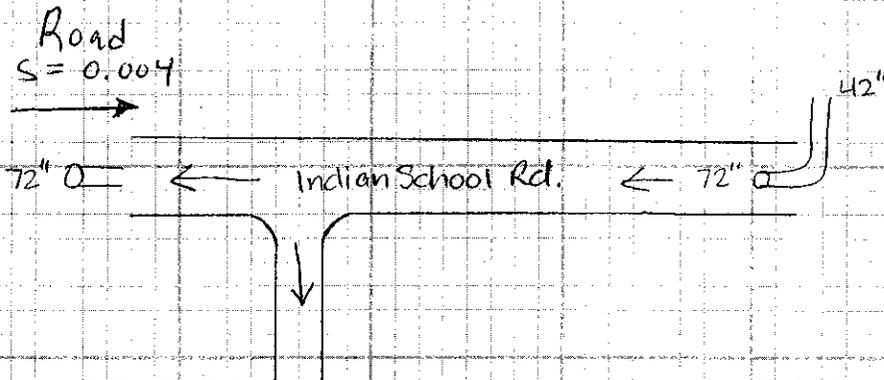
BY AMG DATE 4/17/02

CHECK PAW DATE 4/18/02

CLIENT Flood Control District Maricopa County

JOB NAME Granite Reef Drainage Master Plan JOB NO. 310.021

Diversion DE03 at Indian School Rd between  
N. 86th St. and Pima Rd.



FLOW WEST IS STORM DRAIN ONLY.  
STORM DRAIN MAXIMUM CAPACITY IS 96 cfs,  
(see storm drain calculations) Half is Diverted  
at this Diversion.



SHEET 25 OF 25

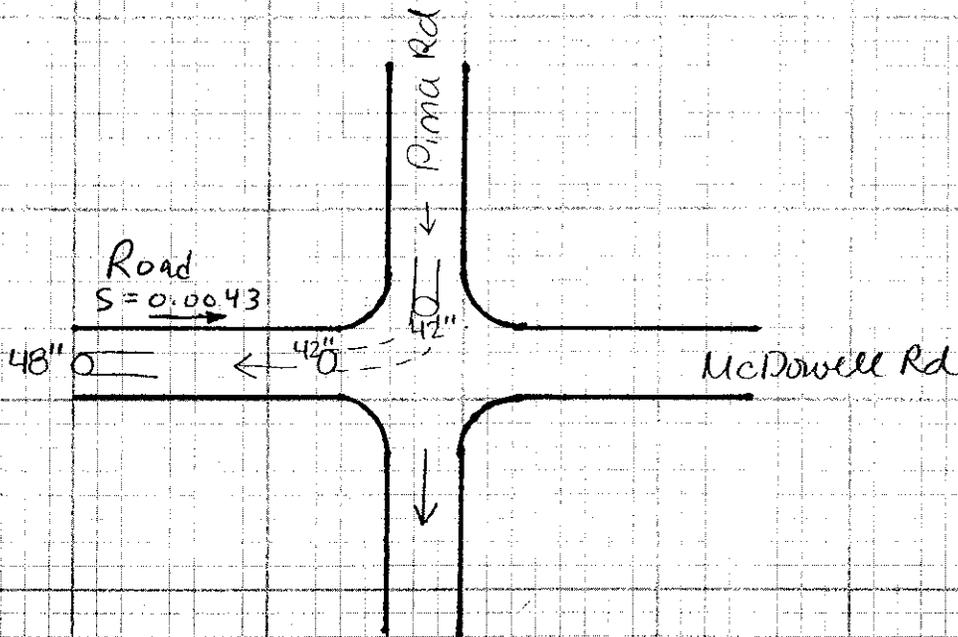
BY AMG DATE 4/17/02

CHECK PAW DATE 4/18/02

CLIENT Flood Control District Maricopa County

JOB NAME Granite Reef Drainage Master Plan JOB NO. 310021

Diversion DC13 at Pima Rd and McDowell Rd



FLOW WEST IS STORM DRAIN ONLY.  
STORM DRAIN CAPACITY IS 51CFS.  
(see storm drain capacity calculations.)

APPENDIX E HYDRAULIC ANALYSIS SUPPORTING DOCUMENTATION



**STORM DRAIN ANALYSIS**

**MCFCFD - GRANITE REEF ADMP PROJECT  
FLOW RATE ANALYSIS**

**Manning's Formula:**

$$Q = (1.486/n) * (R^{0.667}) * (S^{0.5}) * A$$

Where

Q = Flow Rate in cubic feet per sec

n = Manning's Coefficient from **Attachment 1** depending on pipe material

R = Hydraulic Radius (Area/Wetted Perimeter; in case of pipe flowing full  $R = d/4$  for circular pipe) in feet

S = Slope in feet per foot

A = Area of pipe in square feet

d = Diameter of pipe in feet

Pipe I.D.s have been taken from **Attachments 2 & 3**.

	Pipe Mat.	n	Pipe Dia (in.)	Pipe Dia (ft)	From El.	To El.	Length (ft)	Slope (S)	Hyd. Rad. (R)	Area (A)	Flow Rate (Q)
<b>Segment 1(Pipe I.D.:02-48)</b>											
Pipe 1	RGRCP	0.02	48	4.00	1207.90	1201.00	1785	0.0039	1.00	12.56	68.40
<b>Segment 2 (Pipe I.D.s: 23-15, 22-18, 21-24, 20-30, 19-54, 18-48, 10-48)</b>											
Pipe 1	RGRCP	0.02	15	1.25	1241.00	1238.45	243	0.0105	0.31	1.23	5.05
Pipe 2	RGRCP	0.02	18	1.50	1238.45	1236.98	598	0.0025	0.38	1.77	3.98
Pipe 3	RGRCP	0.02	24	2.00	1236.98	1233.80	1242	0.0026	0.50	3.14	8.75
Pipe 4	RGRCP	0.02	30	2.50	1233.80	1226.10	1770	0.0044	0.63	4.91	20.69
Pipe 5	RGRCP	0.02	54	4.50	1226.10	1224.93	1190	0.0010	1.13	15.90	47.24
Pipe 6	RGRCP	0.02	48	4.00	1224.93	1224.00	120	0.0078	1.00	12.56	96.85
Pipe 7	RGRCP	0.02	48	4.00	1220.59	1209.85	5127	0.0021	1.00	12.56	50.35

	Pipe Mat.	n	Pipe Dia (in.)	Pipe Dia (ft)	From El.	To El.	Length (ft)	Slope (S)	Hyd. Rad. (R)	Area (A)	Flow Rate (Q)
<b>Segment 3 (Pipe I.D.s: 09-24, 08-42, 06-48, 05-42, 04-48)</b>											
Pipe 1	CIP	0.02	24	2.00	1222.10	1219.89	414	0.0053	0.50	3.14	14.31
Pipe 2	CIP	0.02	42	3.50	1219.89	1218.65	428	0.0029	0.88	9.62	46.98
Pipe 3	RGRCP	0.02	48	4.00			70	0.0090	1.00	12.56	104.37
Pipe 4	RGRCP	0.02	42	3.50	1219.92	1215.63	982	0.0044	0.88	9.62	50.91
Pipe 5	RGRCP	0.02	48	4.00	1215.63	1210.83	905	0.0053	1.00	12.56	80.12
<b>Segment 4 (Pipe I.D.s: 03-18)</b>											
Pipe 1	RGRCP	0.02	18	1.50	1221.50	1214.00	3227	0.0023	0.38	1.77	3.87
<b>Segment 5 (Pipe I.D.s: 25-72)</b>											
Pipe 1	RGRCP	0.02	72	6.00	1240.50	1236.00	5175	0.0009	1.50	28.26	95.77
<b>Segment 6 (Pipe I.D.s: 28-60)</b>											
Pipe 1	RGRCP	0.02	60	5.00	1248.06	1240.50	3576	0.0021	1.25	19.63	91.78
<b>Segment 7 (Pipe I.D.s: 41-18, 40-24, 39-30, 38-36, 37-42)</b>											
Pipe 1	RGRCP	0.02	18	1.50	1254.71	1251.62	1048	0.0029	0.38	1.77	4.35
Pipe 2	RGRCP	0.02	24	2.00	1251.62	1250.10	724	0.0021	0.50	3.14	7.92
Pipe 3	RGRCP	0.02	30	2.50	1250.05	1245.20	1340	0.0036	0.63	4.91	18.87
Pipe 4	RGRCP	0.02	36	3.00	1245.20	1244.12	584	0.0018	0.75	7.07	21.95
Pipe 5	RGRCP	0.02	42	3.50	1244.12	1241.60	1239	0.0020	0.88	9.62	34.73
<b>Segment 8 (Pipe I.D.s: 34-36, 33-60, 32-84, 31-90, 30-96)</b>											
Pipe 1	RGRCP	0.02	36	3.00		1255.30	610	0.0010	0.75	7.07	16.14
Pipe 2	RGRCP	0.02	60	5.00	1255.30	1255.60	652	0.0010	1.25	19.63	63.12
Pipe 3	RGRCP	0.02	84	7.00	1255.60	1254.28	1925	0.0007	1.75	38.47	128.36
Pipe 4	RGRCP	0.02	90	7.50	1254.28	1252.94	694	0.0019	1.88	44.16	258.95
Pipe 5	RGRCP	0.02	96	8.00	1252.94	1249.70	576	0.0056	2.00	50.24	525.11

	Pipe Mat.	n	Pipe Dia (in.)	Pipe Dia (ft)	From El.	To El.	Length (ft)	Slope (S)	Hyd. Rad. (R)	Area (A)	Flow Rate (Q)
<b>Segment 9 (Pipe I.D.s: 55-18)</b>											
Pipe 1	RGRCP	0.02		0.00	1264.20	1260.00	3685	0.0011	0.00	0.00	0.00
<b>Segment 10 (Pipe I.D.s: 46-24)</b>											
Pipe 1	HDPE	0.01	24	2.00	1269.00	1255.10	5138	0.0027	0.50	3.14	12.74
<b>Segment 11 (Pipe I.D.s: 53-24, 52-30)</b>											
Pipe 1	RGRCP	0.02	24	2.00	1283.94	1271.75	2051	0.0059	0.50	3.14	13.33
Pipe 2	RGRCP	0.02	30	2.50	1271.75	1269.00	632	0.0044	0.63	4.91	20.69

The friction coefficients  $C_h$  and  $n$  are assumed to be independent of Reynolds number. Even though this is not correct, the uncertainty involved in obtaining a reliable value of the friction coefficients can introduce an error as large as the variation with Reynolds number.

Engineers in the waterworks field generally use the Manning or the Hazen-Williams equation. This choice may have been based on simplifications of their solution due to the availability of special slide rules, tables, nomographs, and charts. The availability of modern hand calculators and computers makes the solution of any of the equations so simple that this is no longer a factor. Even so, it is unlikely that there will be any significant change in the preferred equation by the various user groups. It is therefore advisable to be familiar with all three equations.

Manning's  $n$  values are listed in Table 1.2. Extreme values of  $n$  range from 0.01 for new pipe in excellent condition to 0.035 for an extremely rough pipe. Typical values used in design range from 0.011 to 0.017. The same caution should be exercised using these values as was suggested for the tabulated  $f$  values in Fig. 1.4. Whenever possible, obtain information from the manufacturer. Suggested values of Hazen-Williams  $C_h$  are listed in Table 1.3. Values of  $C_h$  range from 140 for new pipe in excellent condition to less than 100 for old pipe in poor condition. Typical values would be between 120 and 130.

TABLE 1.2 Values of Manning's Coefficient  $n$

Kind of Pipe	Variation		Use in Designing	
	From	To	From	To
Clean uncoated cast-iron pipe	0.011	0.015	0.013	0.015
Clean coated cast-iron pipe	0.010	0.014	0.012	0.014
Dirty or tuberculated cast-iron pipe	0.015	0.035		
Riveted steel pipe	0.013	0.017	0.015	0.017
Lock-bar and welded pipe	0.010	0.013	0.012	0.013
Galvanized-iron pipe	0.012	0.017	0.015	0.017
Brass and glass pipe	0.009	0.013		
Wood-stave pipe	0.010	0.014		
Wood-stave pipe, small diameter			0.011	0.012
Wood-stave pipe, large diameter			0.012	0.013
Concrete pipe	0.010	0.017		
Concrete pipe with rough joints			0.016	0.017
Concrete pipe, "dry mix," rough forms			0.015	0.016
Concrete pipe, "wet mix," steel forms			0.012	0.014
Concrete pipe, very smooth			0.011	0.012
Vitrified sewer pipe	0.010	0.017	0.013	0.015
Common clay drainage tile	0.011	0.017	0.012	0.014

Source: R. Manning, "Flow of Water Open Channels and Pipes," *Trans. Inst. Civil Engrs. (Ireland)*, vol. 20, 1890.



CLIENT FCD of Maricopa Co.  
JOB NAME GRANITE REEF WASH

JOB NO. 310021

S.D.#	S.D. SIZE		AS-BUILT SET NO
01	54	@ ROOSEVELT & GRANITE REEF	(46)
02	48	N. OF ROOSEVELT, E. OF GRANITE REEF	-
03	18	HAYDEN TO GRANITE REEF ON MCDOWELL	-
04	48	GRANITE REEF TO PIMA ON MCDOWELL	(43)
05	42	" " " "	"
06	48	TURNS TO THE NORTH @ END (PIMA RD.)	"
07	42	N. ON PIMA FROM MCDOWELL	(36)
09	24	" " " " "	"
10	48	FROM MCDOWELL N. TO THOMAS RD ALONG 87TH ST.	(53)
18	48	FROM THOMAS N. TO INDIAN SCHOOL RD. ON PIMA RD.	(37)
19	54		
20	30		
21	24		
22	18		
23	15		
25	72	FROM HAYDEN TO PIMA RD. ON INDIAN SCHOOL RD. (Assumed to be 24 RCP from Det.; Also missing 1 sht. for exact lengths)	(48)
28	60	E. OF HAYDEN RD. +/- 523' TO 86TH ST ON CAMELBACK RD. (Assumed 26 RCP)	(51)
30	96	Just E. of HAYDEN RD. TO PIMA RD	(54)
31	90	ON CHAPARRAL RD.	
32	84		
33	60		
34	36		

see pg. 2. for 07-

9 {

10 {

Made and Assumed for length 06-43  
Made from center cut 06-43



SHEET 2 OF 2  
 BY KAB DATE 2/5/02  
 CHECK PAW DATE 4/15/02

CLIENT FCD of Maricopa Co

JOB NAME GRANITE REEF WASH

JOB NO. 310021

S.D. S.D.  
 NO SIZE

AS-BUILT  
 SET NO

137-42	FROM INDIAN SCHOOL RD. N. TO	(34)
138-36	CHAPARRAL RD. ON PIMA RD.	}
139-30	↓	
140-24		
141-18		
07-24		100LF PIPE RUNS E-W. APPROX. 1395 FT. N. OF MCKELLIPS ON PIMA RD.
Seg 10 ✓ 46-24	FROM CHAPARRAL RD. TO McDONALD DR. ON PIMA RD.	(35) HDP (4-7-02)
Seg 11 [ 52-30 53-24	FROM McDONALD DR. TO THE AZ. CANAL ON PIMA RD	(38)
Seg 9 ( 55-18	FROM WASH E. of HAYDEN TO 86TH ST. ON JACKRABBIT RD	( )



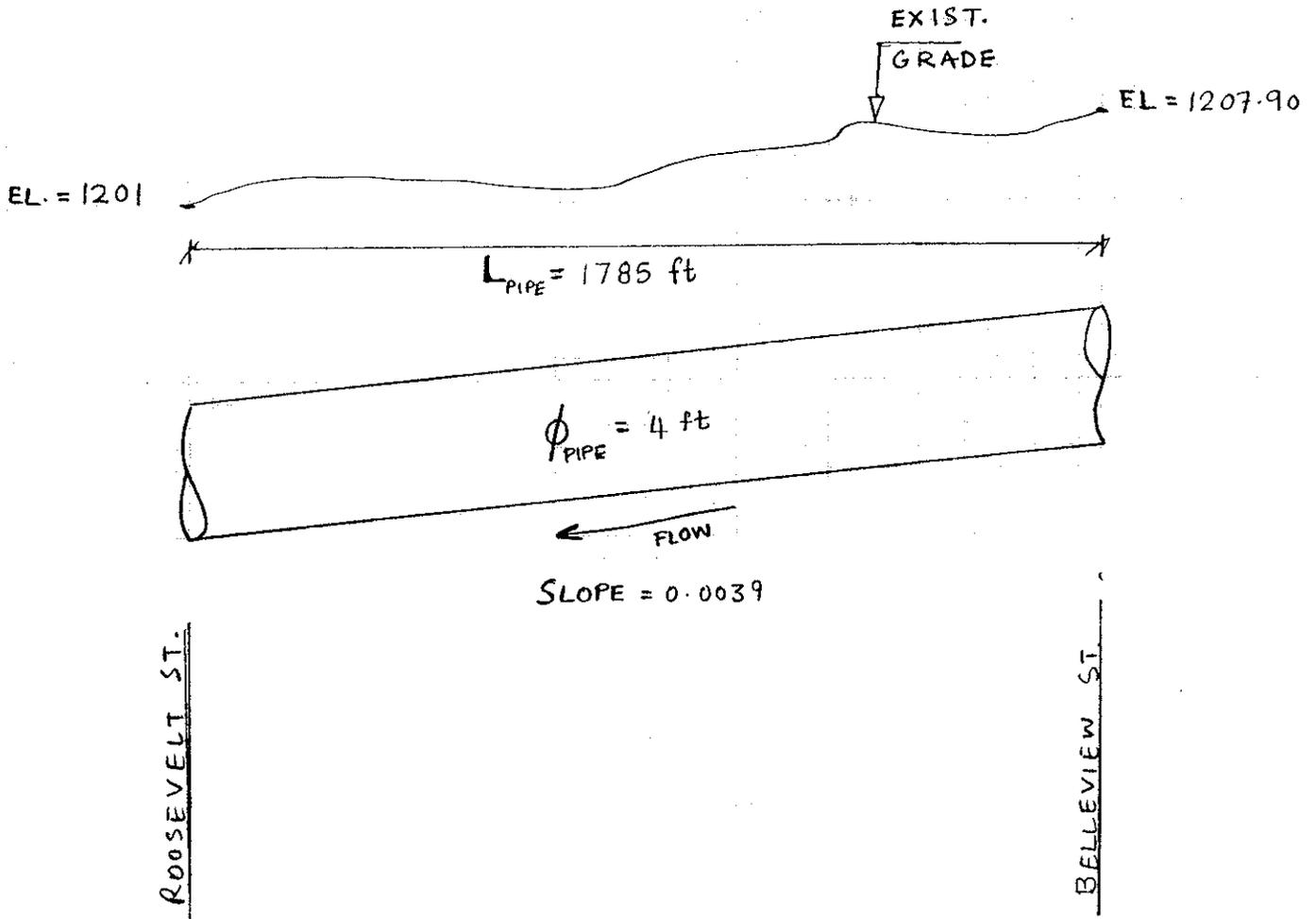
SHEET 1 OF 1  
BY SR DATE 02/05/02  
CHECK PAW DATE 4/17/02

CLIENT FCD of MARICOPA COUNTY

JOB NAME GRANITE REEF WASH

JOB NO. 310.021

PIPE ID: 02-48  
84<sup>th</sup> PLACE



SEGMENT : 1

PIPE # : 1



ATTACHMENT - 4.2

SHEET 1 OF 7  
BY SR DATE 02/05/02  
CHECK PAW DATE 4/17/02

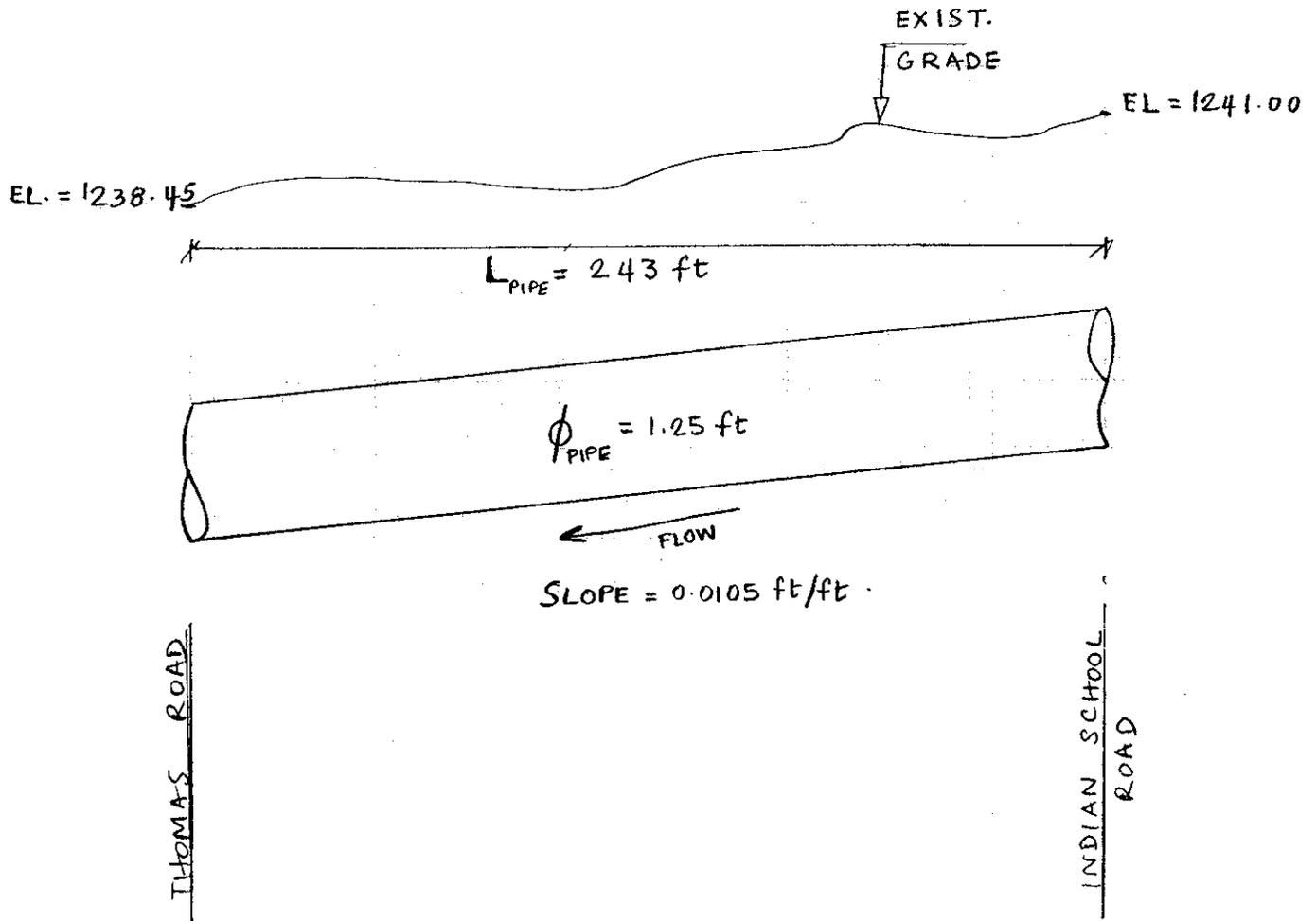
CLIENT FCD of MARICOPA COUNTY

JOB NAME GRANITE REEF WASH

JOB NO. 310.021

PIPE I.D. : 23-15

PIMA ROAD



SEGMENT : 2

PIPE # : 1

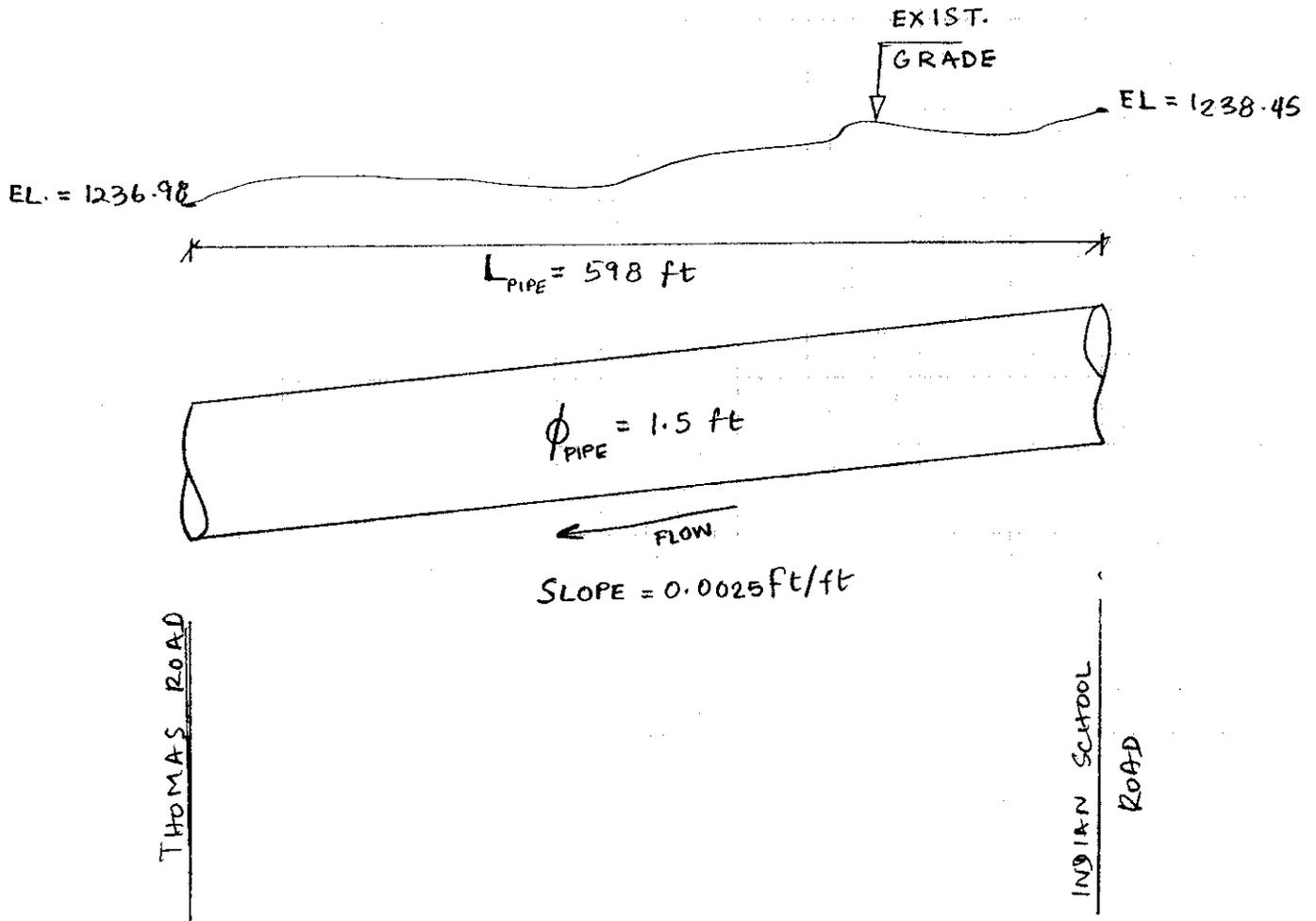


SHEET 2 OF 7  
BY SR DATE 02/05/02  
CHECK PAW DATE 4/17/02

CLIENT FCD of MARICOPA COUNTY  
JOB NAME GRANITE REEF WASH

JOB NO. 310.021

PIPE I.D. = 22-18  
PIMA ROAD



SEGMENT : 2

PIPE # : 2



SHEET 3 OF 7  
BY SR DATE 02/05/02  
CHECK PAW DATE 4/17/02

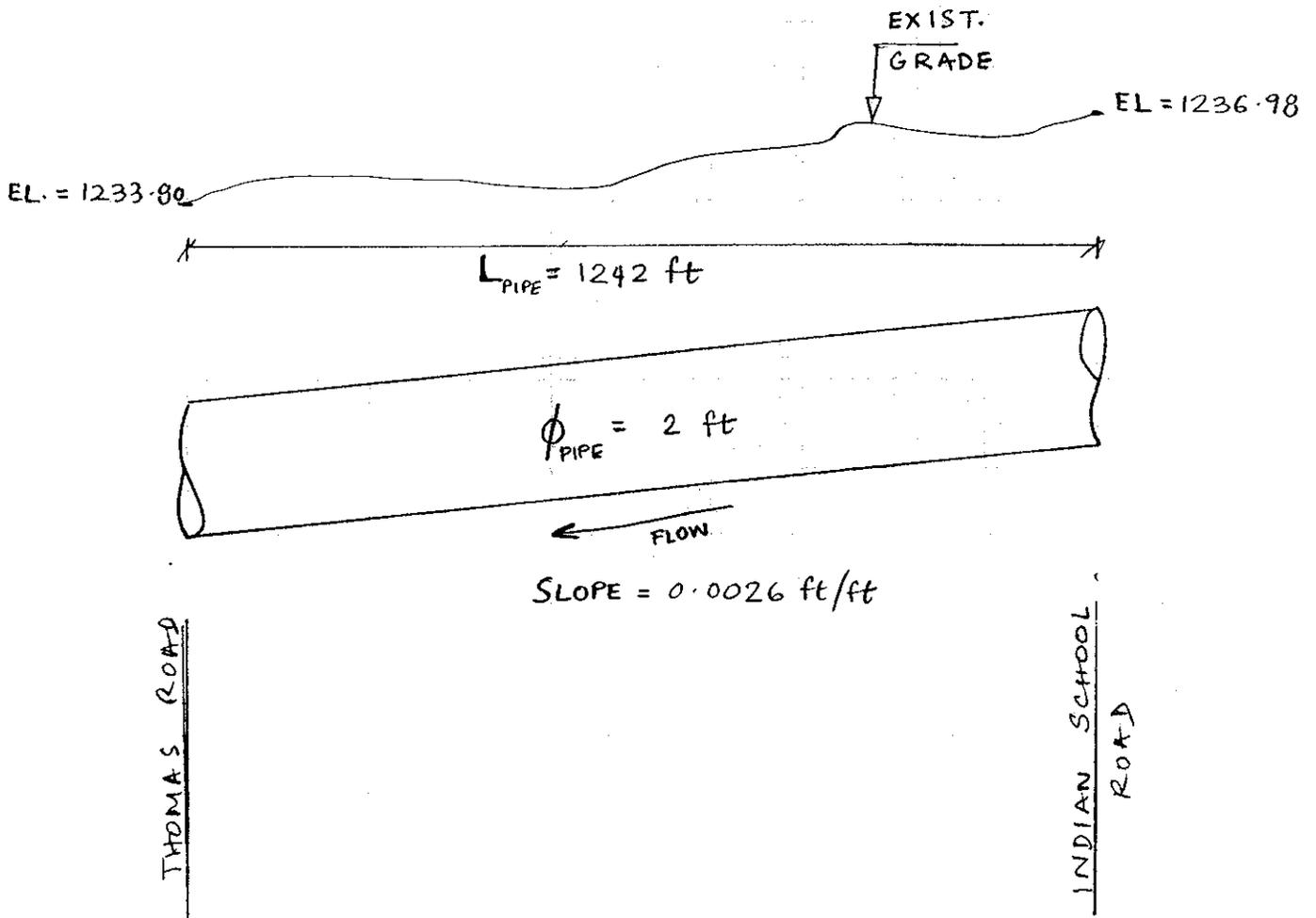
CLIENT FCD of MARICOPA COUNTY

JOB NAME GRANITE REEF WASH

JOB NO. 310.021

PIPE ID. : 21-24

PIMA ROAD



SEGMENT : 2

PIPE # : 3



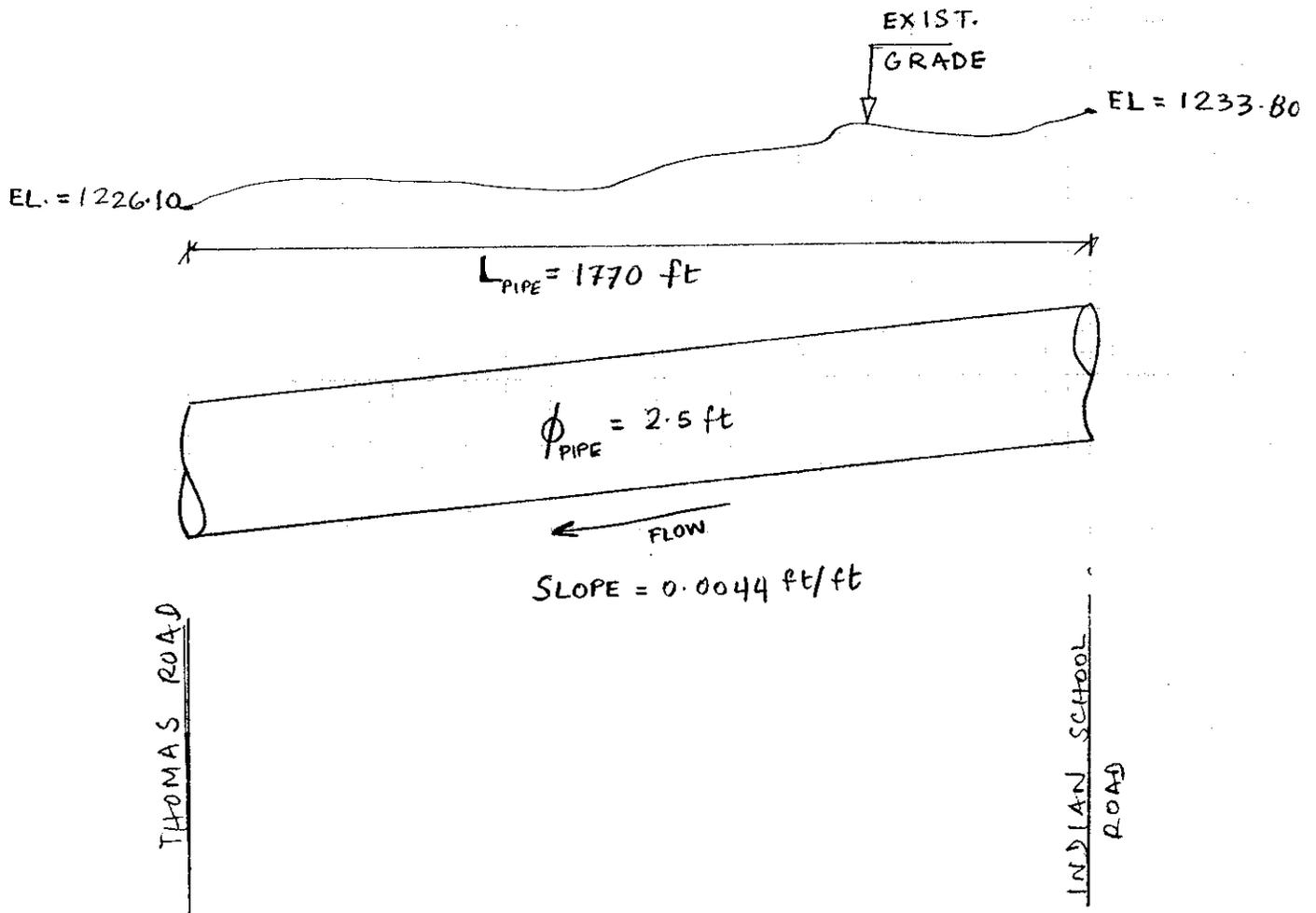
SHEET 4 OF 7  
BY SR DATE 02/05/02  
CHECK PAW DATE 4/17/02

CLIENT FCD of MARICOPA COUNTY

JOB NAME GRANITE REEF WASH

JOB NO. 310.021

PIPE I.D. : 20-30  
PIMA ROAD



SEGMENT : 2

PIPE # : 4



SHEET 5 OF 7  
BY SR DATE 02/05/02  
CHECK PAW DATE 4/17/02

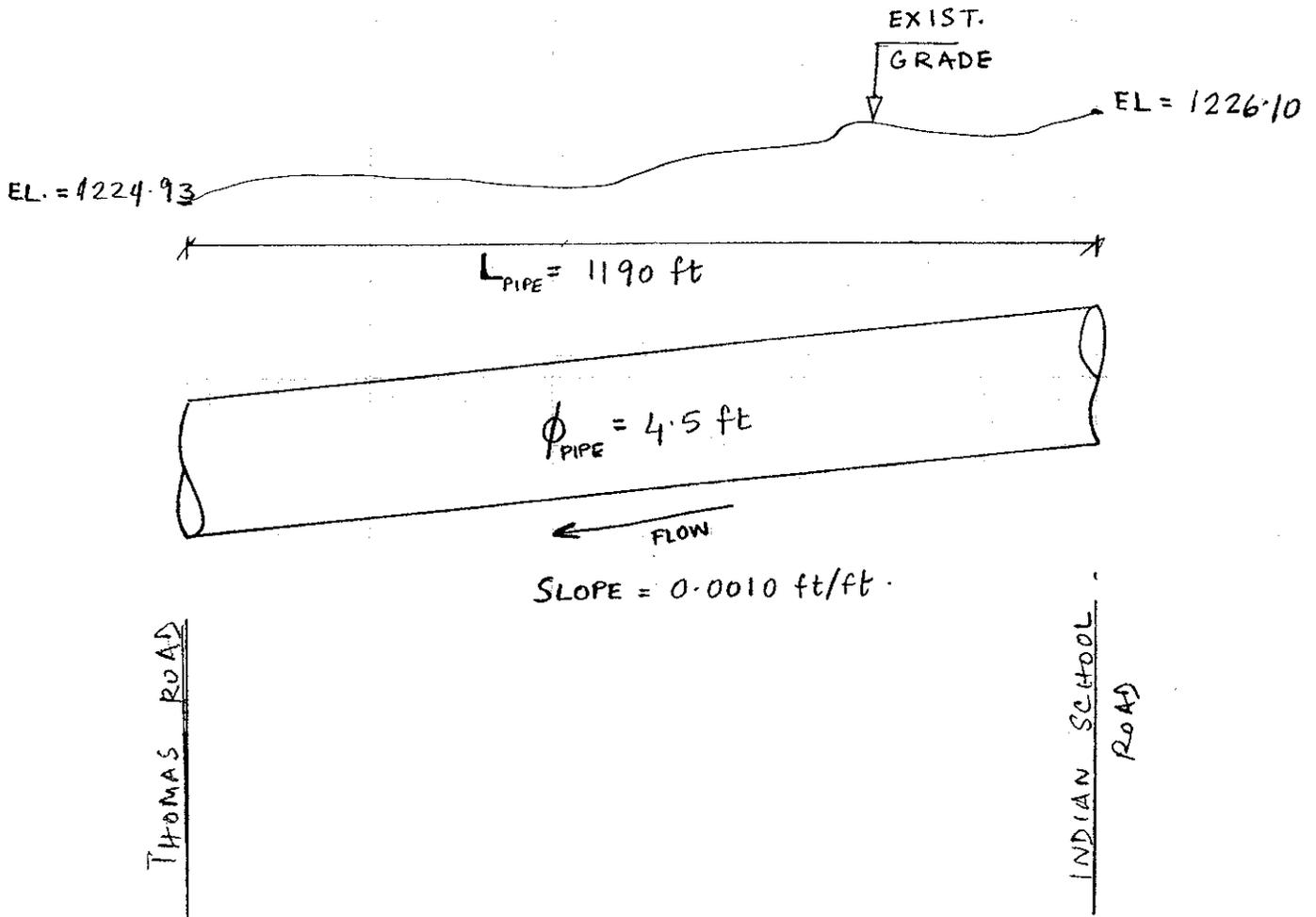
CLIENT FCD of MARICOPA COUNTY

JOB NAME GRANITE REEF WASH

JOB NO. 310.021

PIPE I.D. : 19-54

PIMA ROAD



SEGMENT : 1

PIPE # : 5



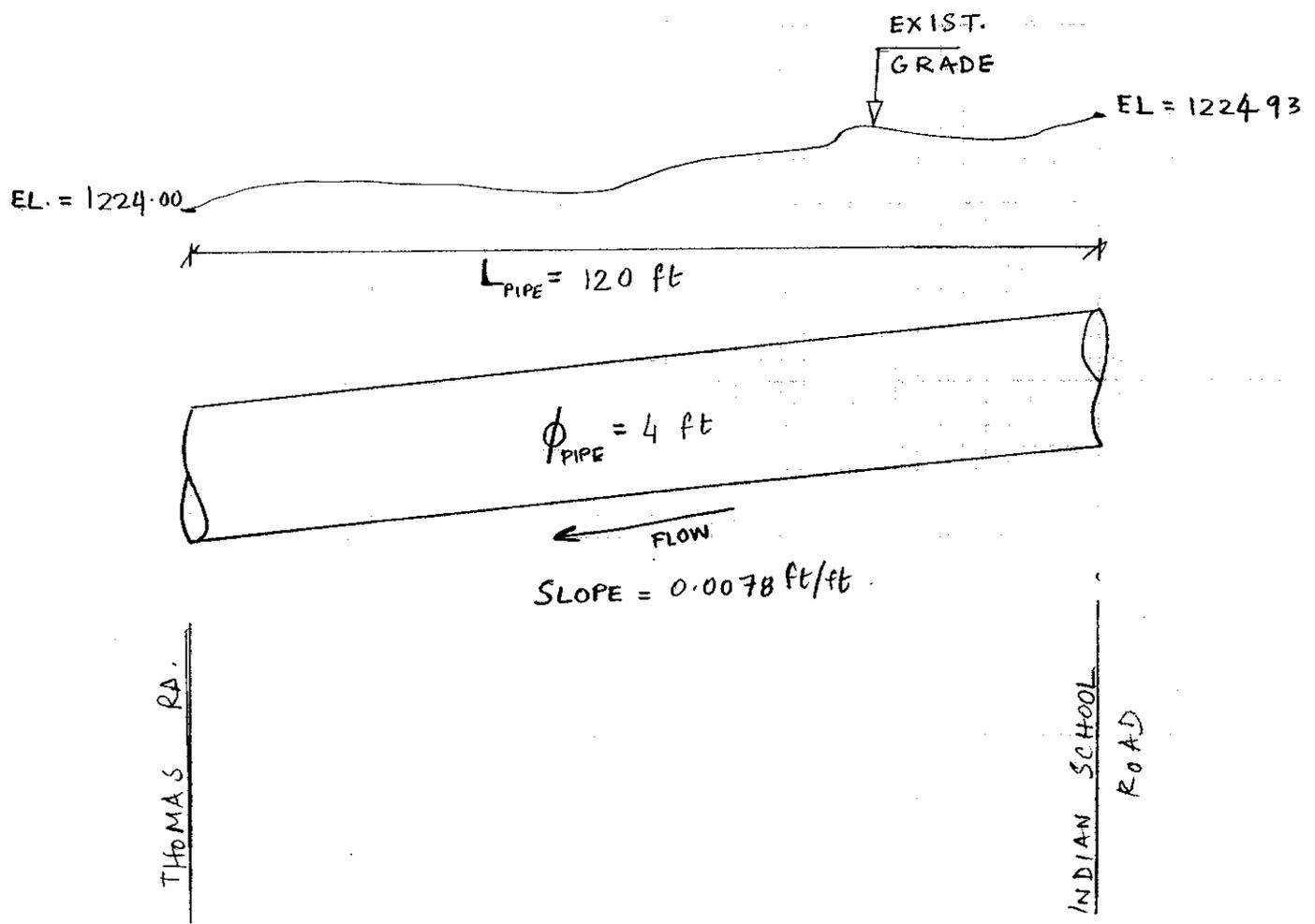
SHEET 6 OF 7  
BY SR DATE 02/05/02  
CHECK PAW DATE 4/17/02

CLIENT FCD of MARICOPA COUNTY

JOB NAME GRANITE REEF WASH

JOB NO. 310.021

PIPE I.D. = 18-48  
PIMA ROAD



SEGMENT : 2  
PIPE # : 6



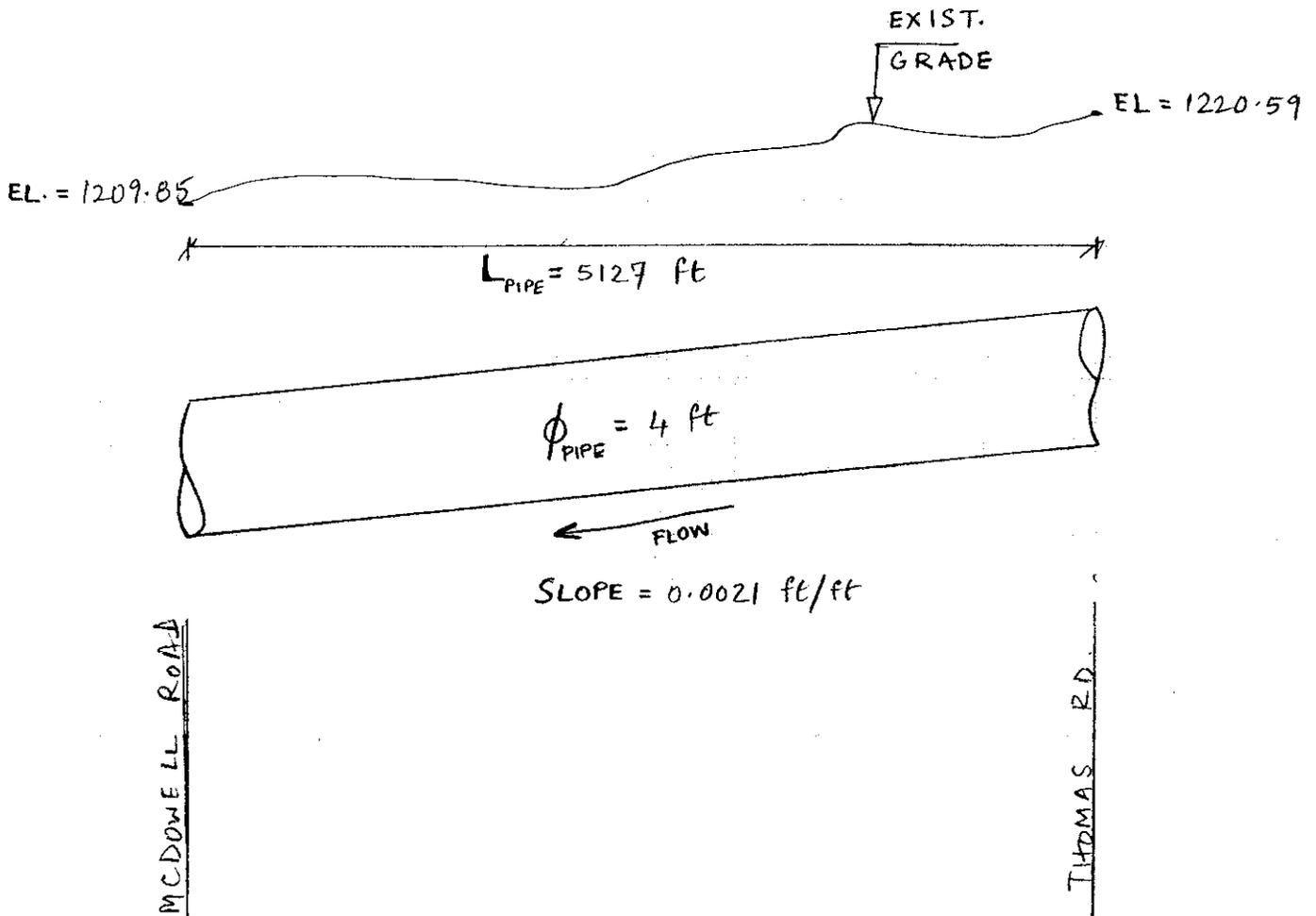
SHEET 7 OF 7  
BY SR DATE 02/05/02  
CHECK PAW DATE 4/17/02

CLIENT FCD of MARICOPA COUNTY

JOB NAME GRANITE REEF WASH

JOB NO. 310.021

PIPE I.D. = 10-48  
87<sup>th</sup> STREET



SEGMENT : 2

PIPE # : 7



ATTACHMENT - 4.3

SHEET 1 OF 5  
BY SR DATE 02/05/02  
CHECK PAW DATE 4/17/02

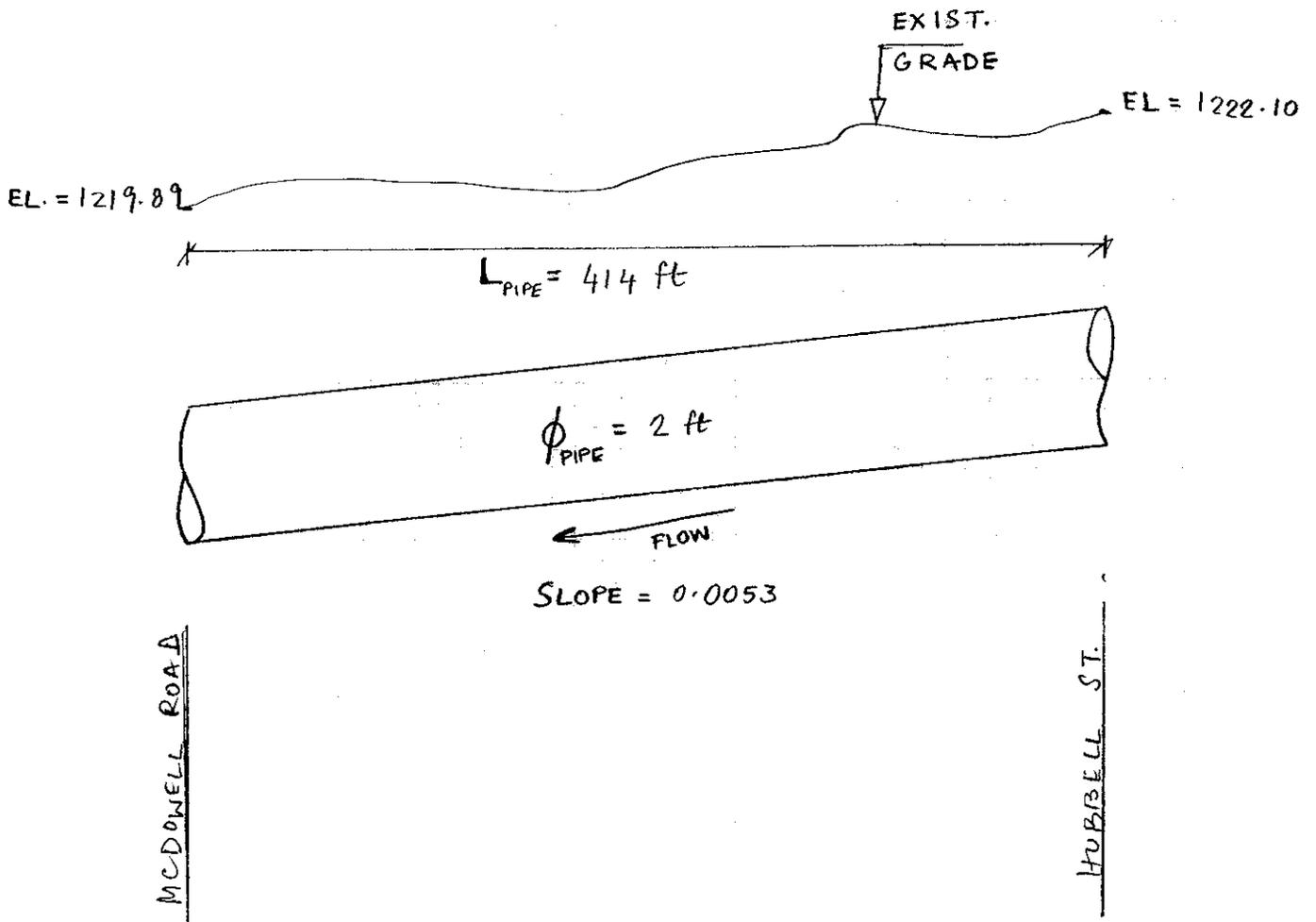
CLIENT FCD of MARICOPA COUNTY

JOB NAME GRANITE REEF WASH

JOB NO. 310.021

PIPE I.D. = 09-24

PIMA ROAD



SEGMENT : 3

PIPE # : 1



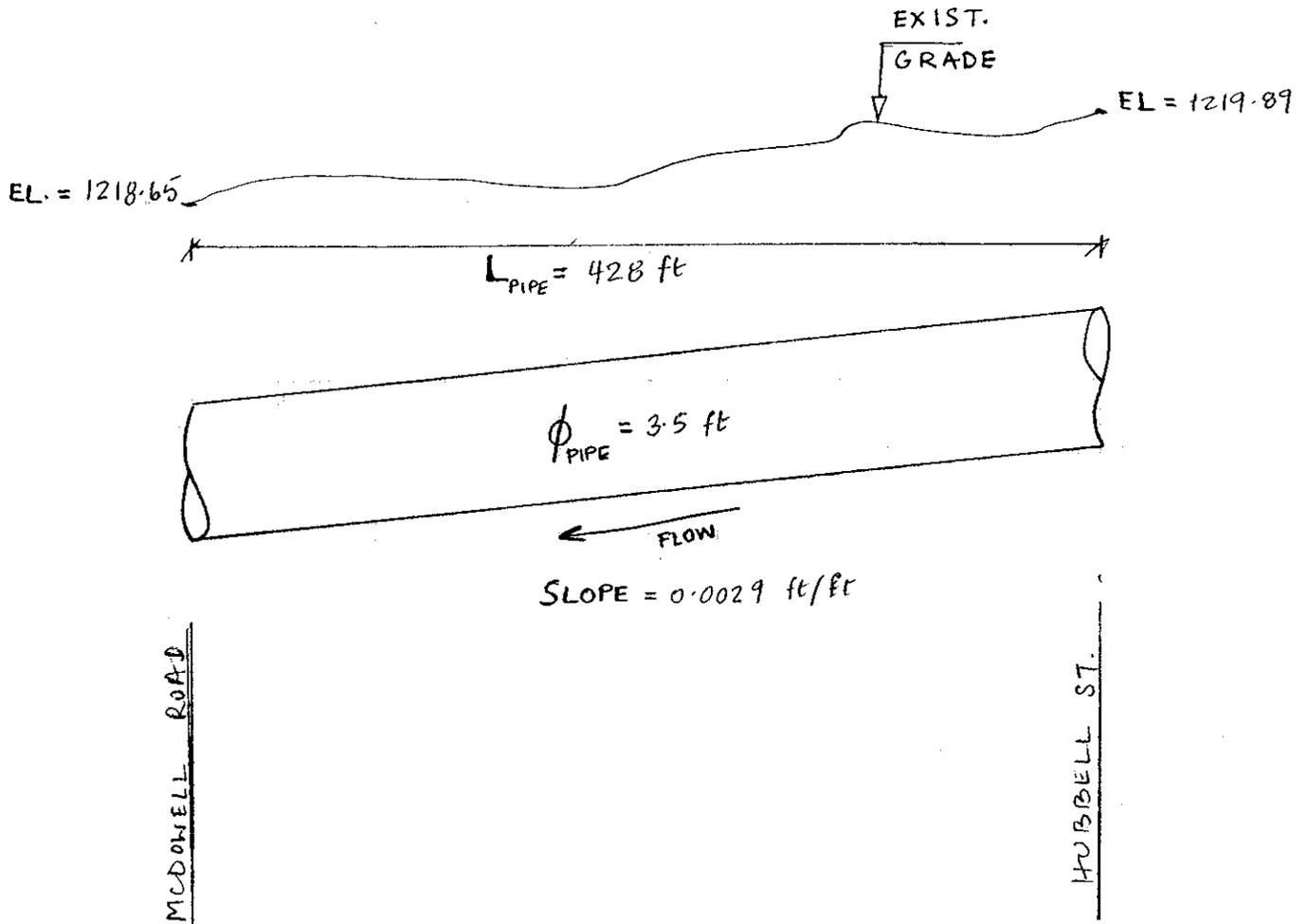
SHEET 2 OF 5  
BY SR DATE 02/05/02  
CHECK PAW DATE 4/17/02

CLIENT FCD of MARICOPA COUNTY

JOB NAME GRANITE REEF WASH

JOB NO. 310.021

PIPE I.D. : 08-42  
PIMA ROAD



SEGMENT : 3

PIPE # : 2



SHEET 3 OF 5  
BY SR DATE 02/05/02  
CHECK PAW DATE 4/17/02

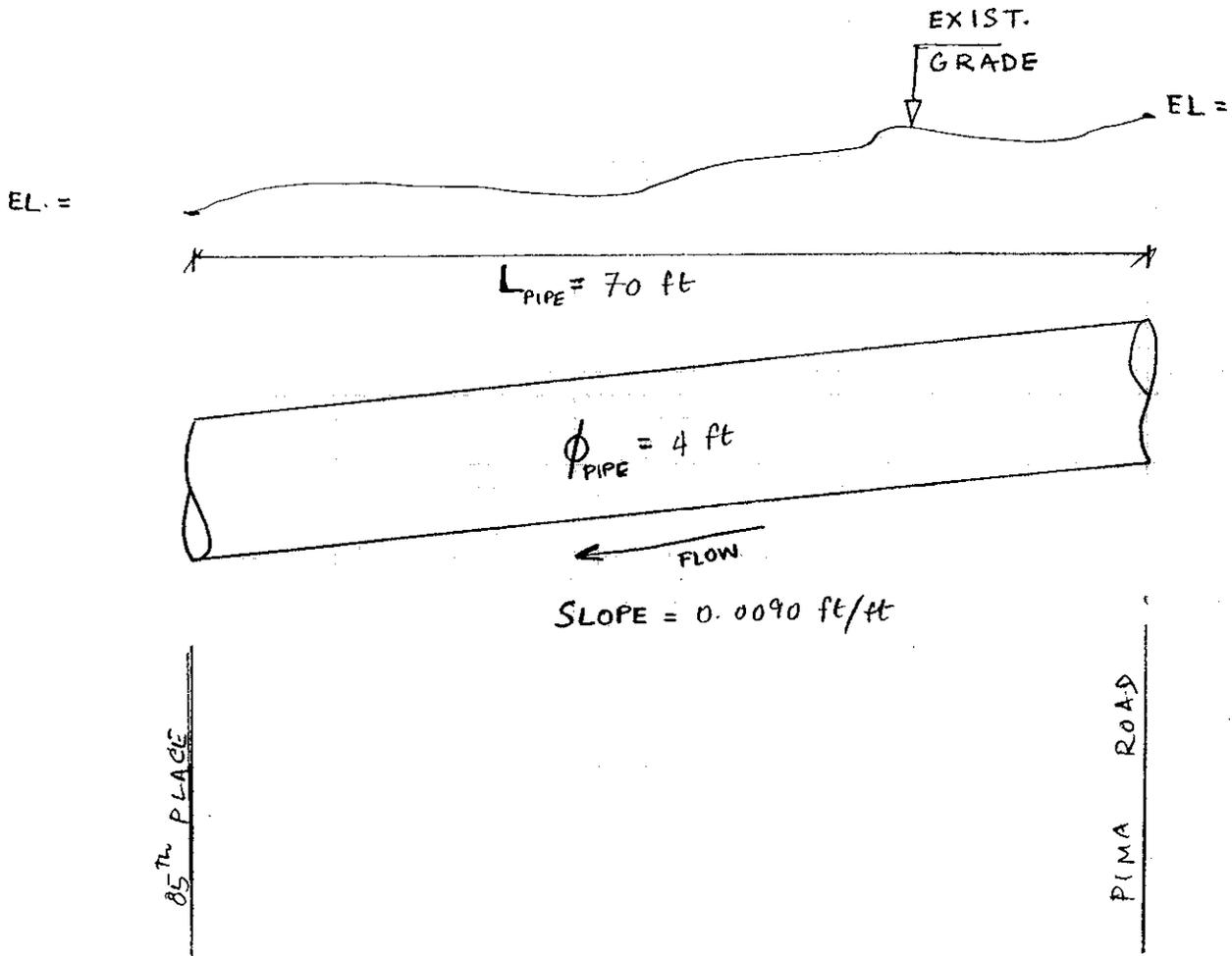
CLIENT FCD of MARICOPA COUNTY

JOB NAME GRANITE REEF WASH

JOB NO. 310.021

PIPE ID. : 06-48

MCDOWELL ROAD



SEGMENT : 3

PIPE # : 3

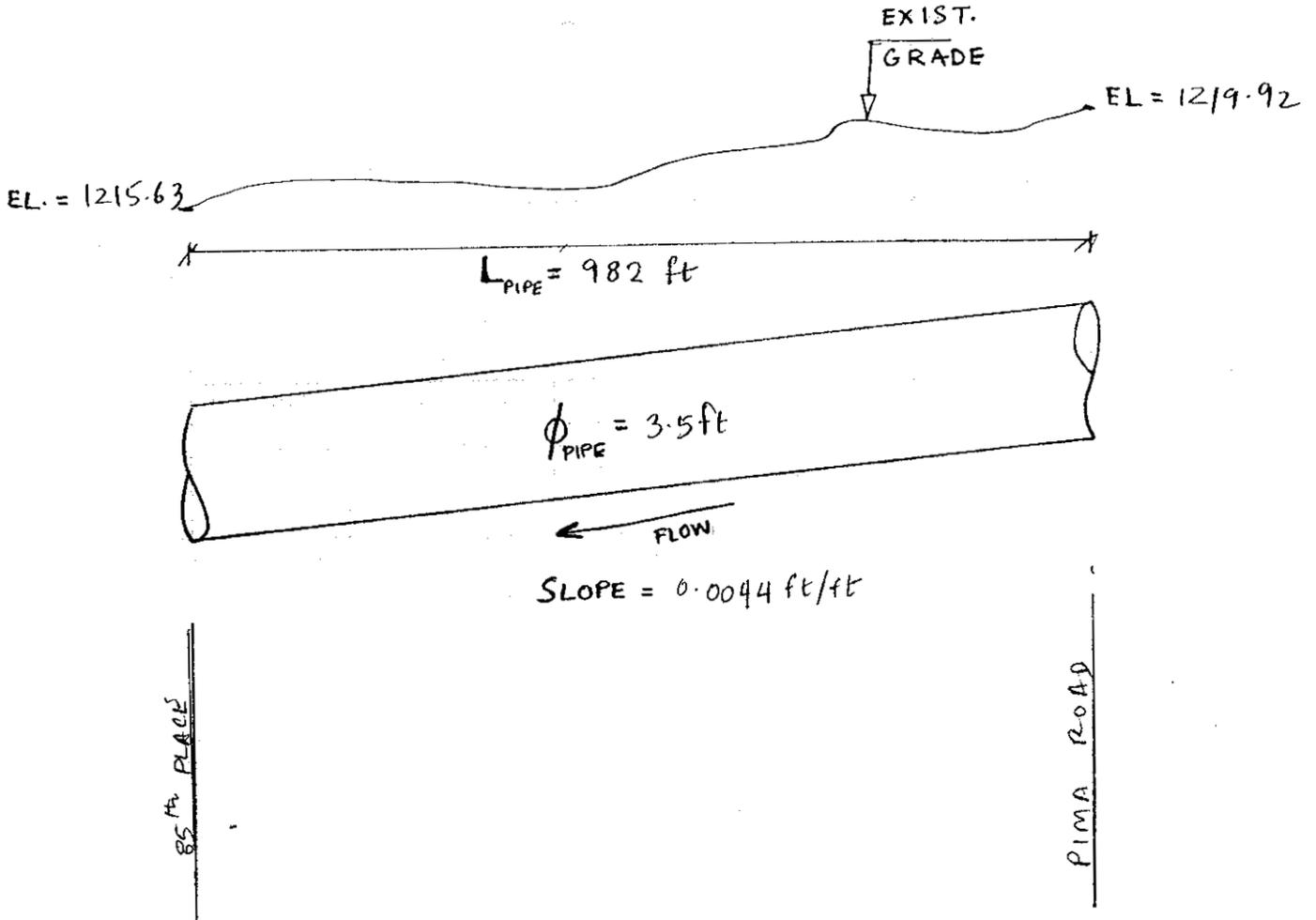


SHEET 4 OF 5  
BY SR DATE 02/05/02  
CHECK PAW DATE 4/17/02

CLIENT FCD of MARICOPA COUNTY  
JOB NAME GRANITE REEF WASH

JOB NO. 310.021

PIPE ID : 05-42  
MCDOWELL ROAD



SEGMENT : 3

PIPE # : 4



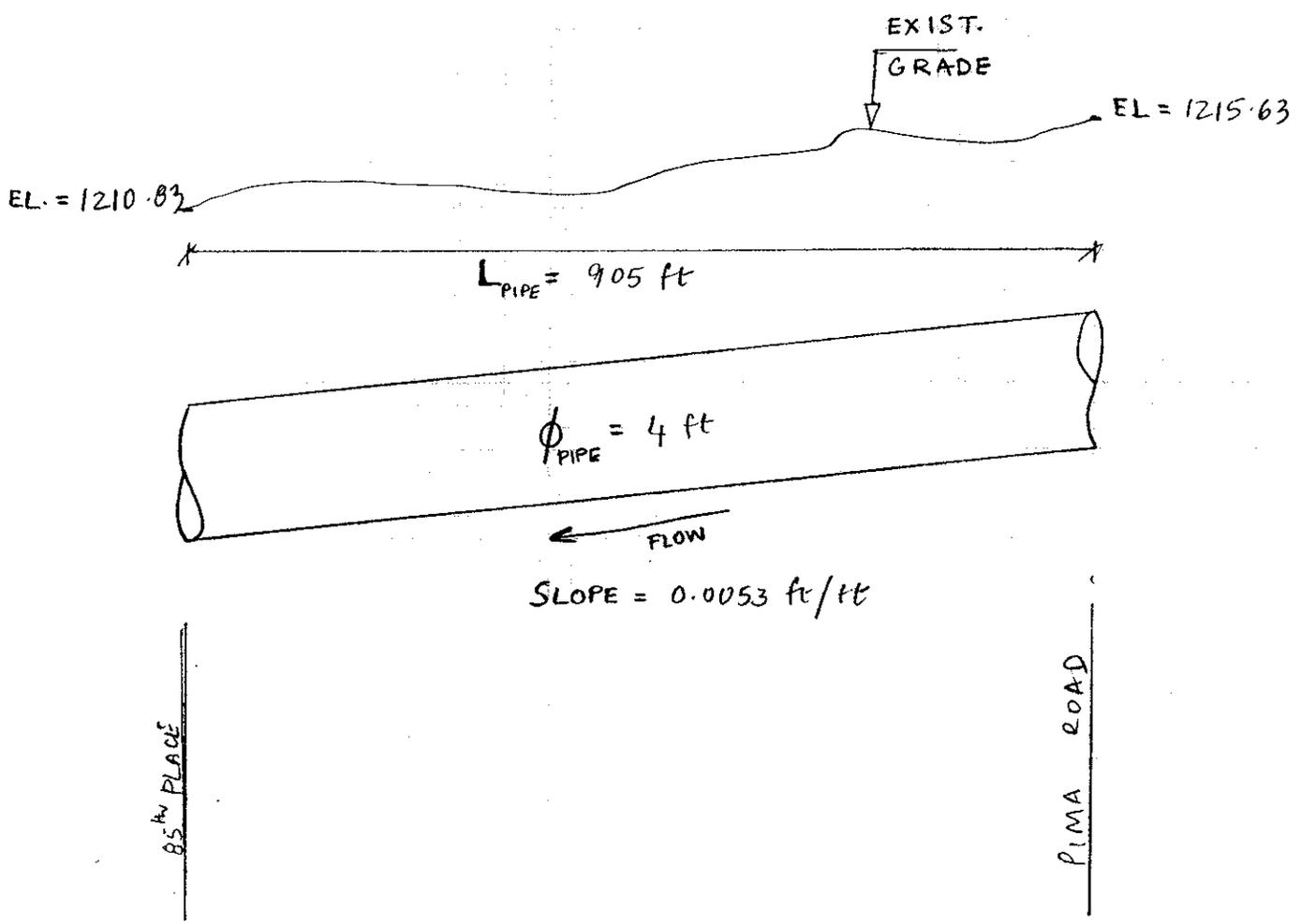
SHEET 5 OF 5  
BY SR DATE 02/05/02  
CHECK PAW DATE 4/17/02

CLIENT FCD of MARICOPA COUNTY

JOB NAME GRANITE REEF WASH

JOB NO. 310.021

PIPE ID. = 04-48  
MCDOWELL ROAD



SEGMENT : 3  
PIPE # : 5



ATTACHMENT -4.4

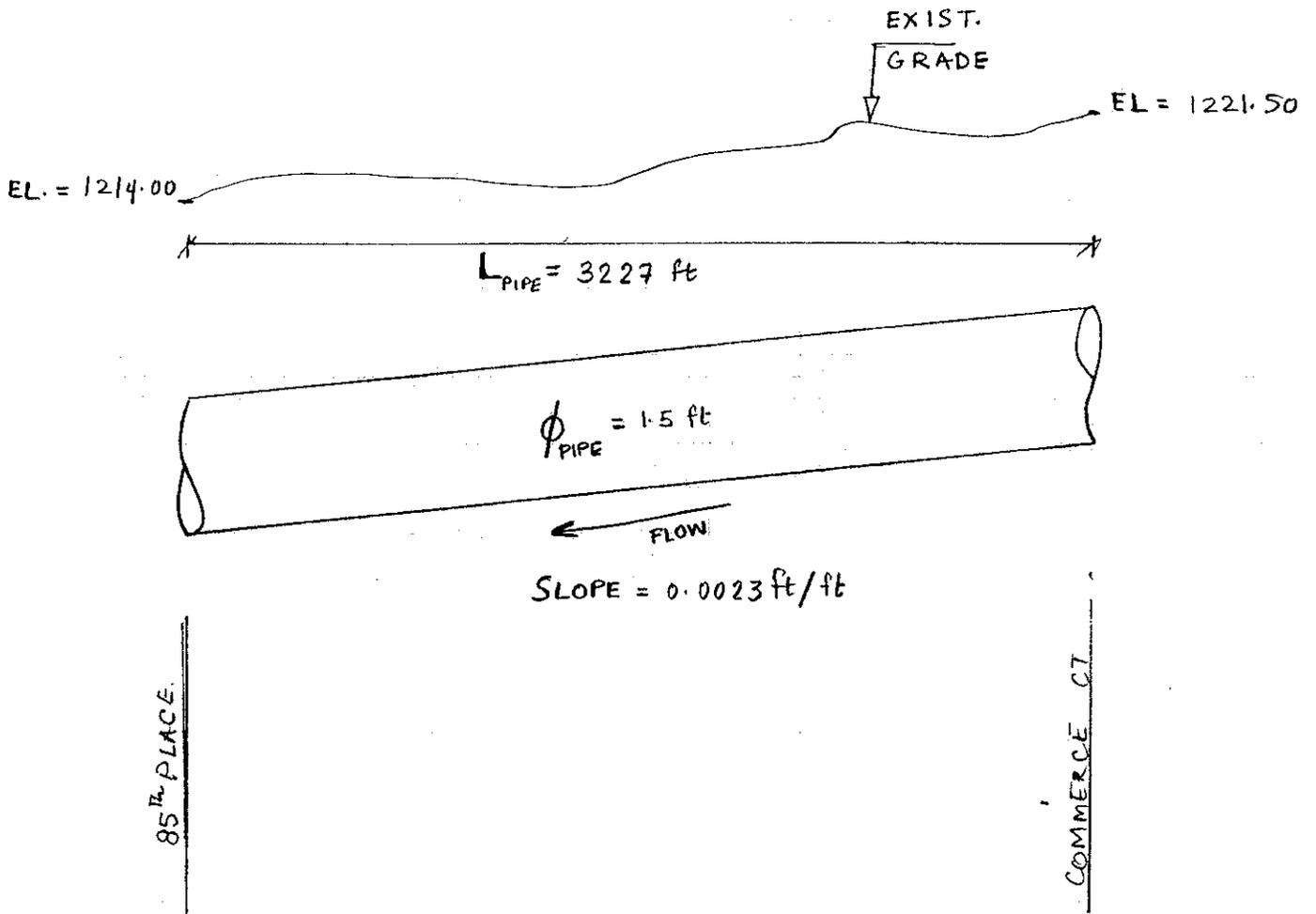
SHEET 1 OF 1  
BY SR DATE 02/05/02  
CHECK PAW DATE 4/17/02

CLIENT FCD of MARICOPA COUNTY

JOB NAME GRANITE REEF WASH

JOB NO. 310.021

PIPE ID. : 03-18  
MCDOWELL ROAD



SEGMENT : 4

PIPE # : 1



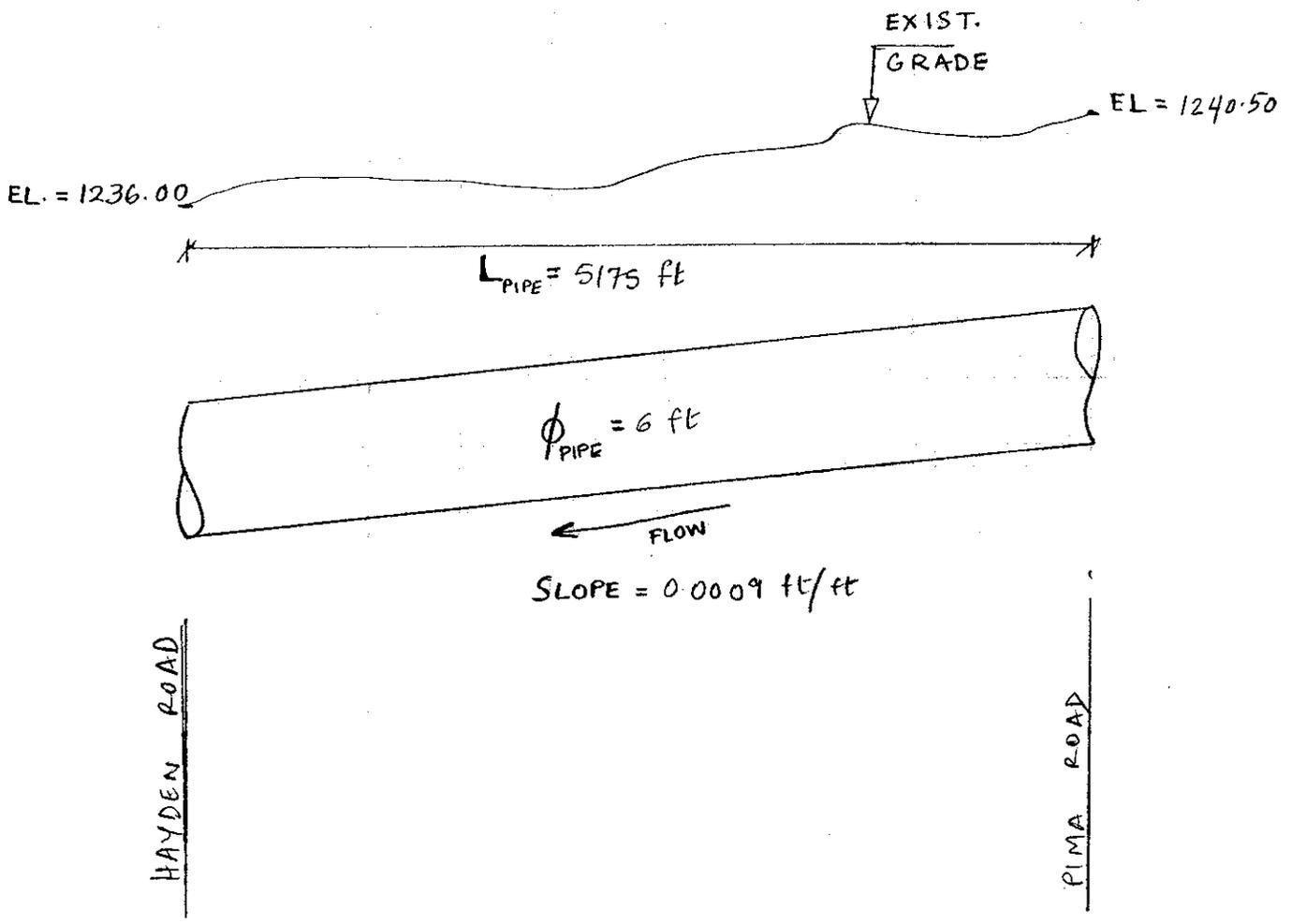
SHEET 1 OF 1  
BY SR DATE 02/05/02  
CHECK PAW DATE 4/17/02

CLIENT FCD of MARICOPA COUNTY

JOB NAME GRANITE REEF WASH

JOB NO. 310.021

PIPE I.D. # 25-72  
INDIAN SCHOOL ROAD



SEGMENT : 5

PIPE # : 1



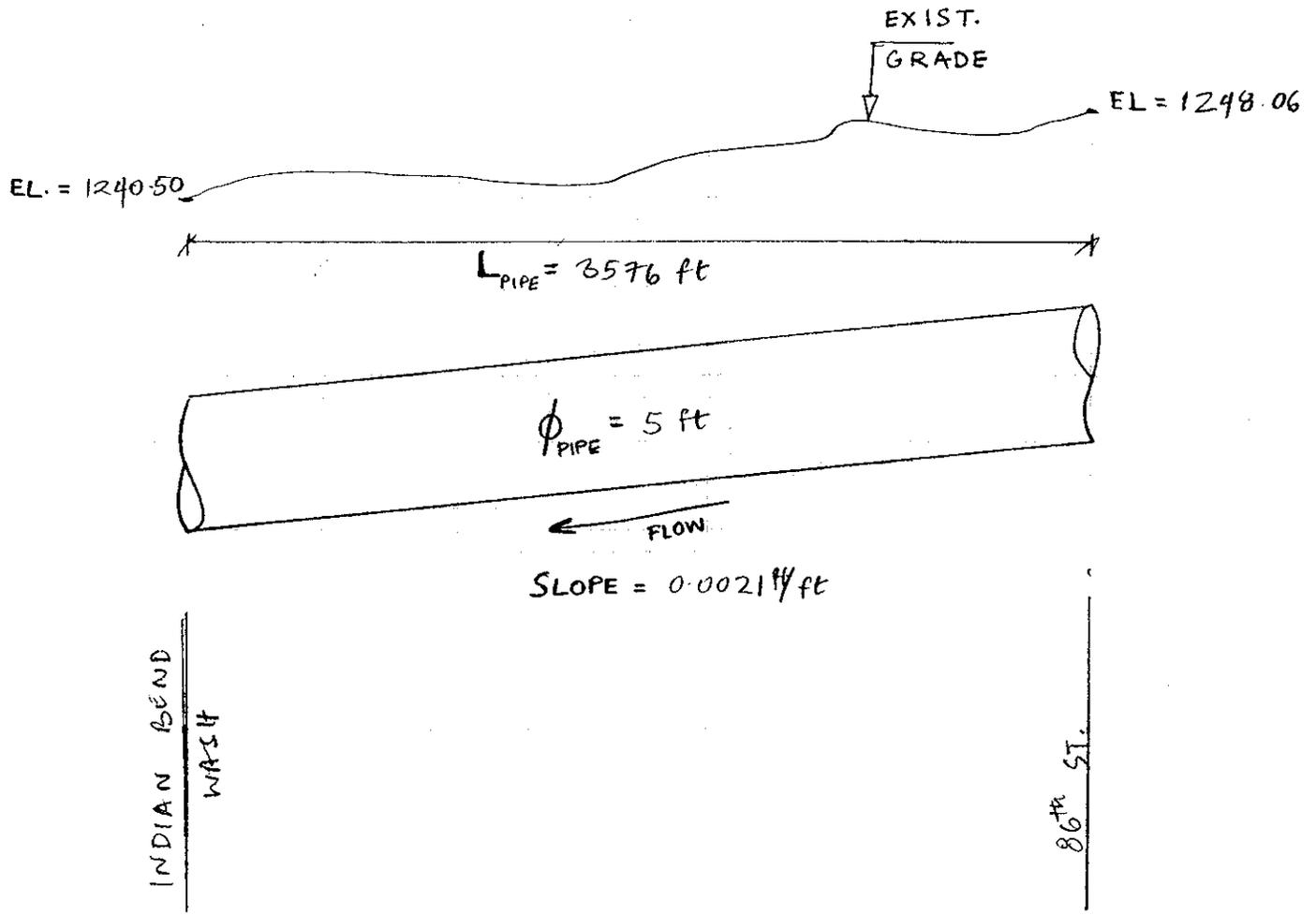
ATTACHMENT - 4.6

SHEET 1 OF 1  
BY SR DATE 02/05/02  
CHECK PAW DATE 4/17/02

CLIENT FCD of MARICOPA COUNTY  
JOB NAME GRANITE REEF WASH

JOB NO. 310.021

PIPE I.D. = 28-60



SEGMENT : 6

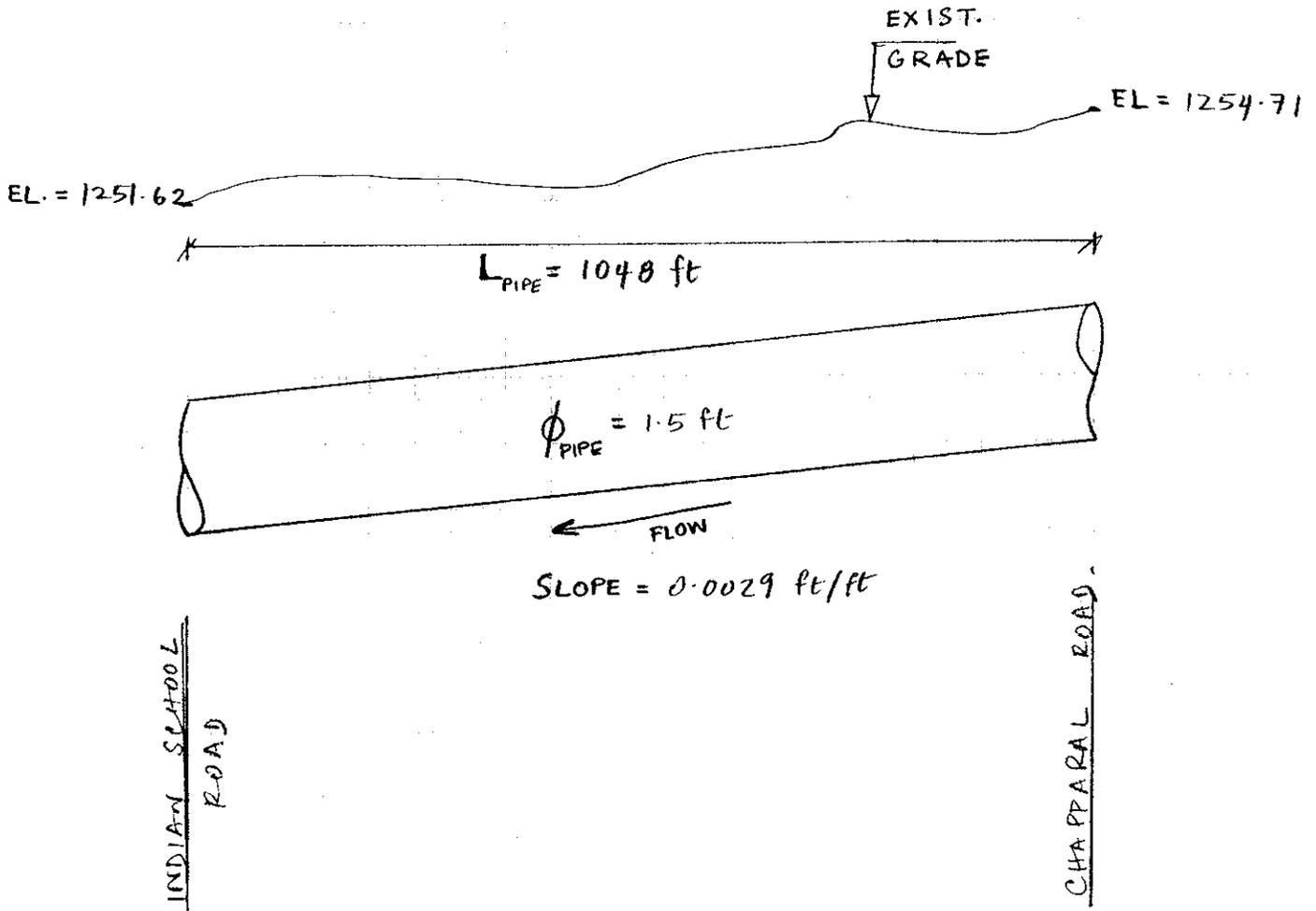
PIPE # : 1



CLIENT FCD of MARICOPA COUNTY  
JOB NAME GRANITE REEF WASH

JOB NO. 310.021

PIPE I.D. : 41-18  
PIMA ROAD



SEGMENT : 7

PIPE # : 1

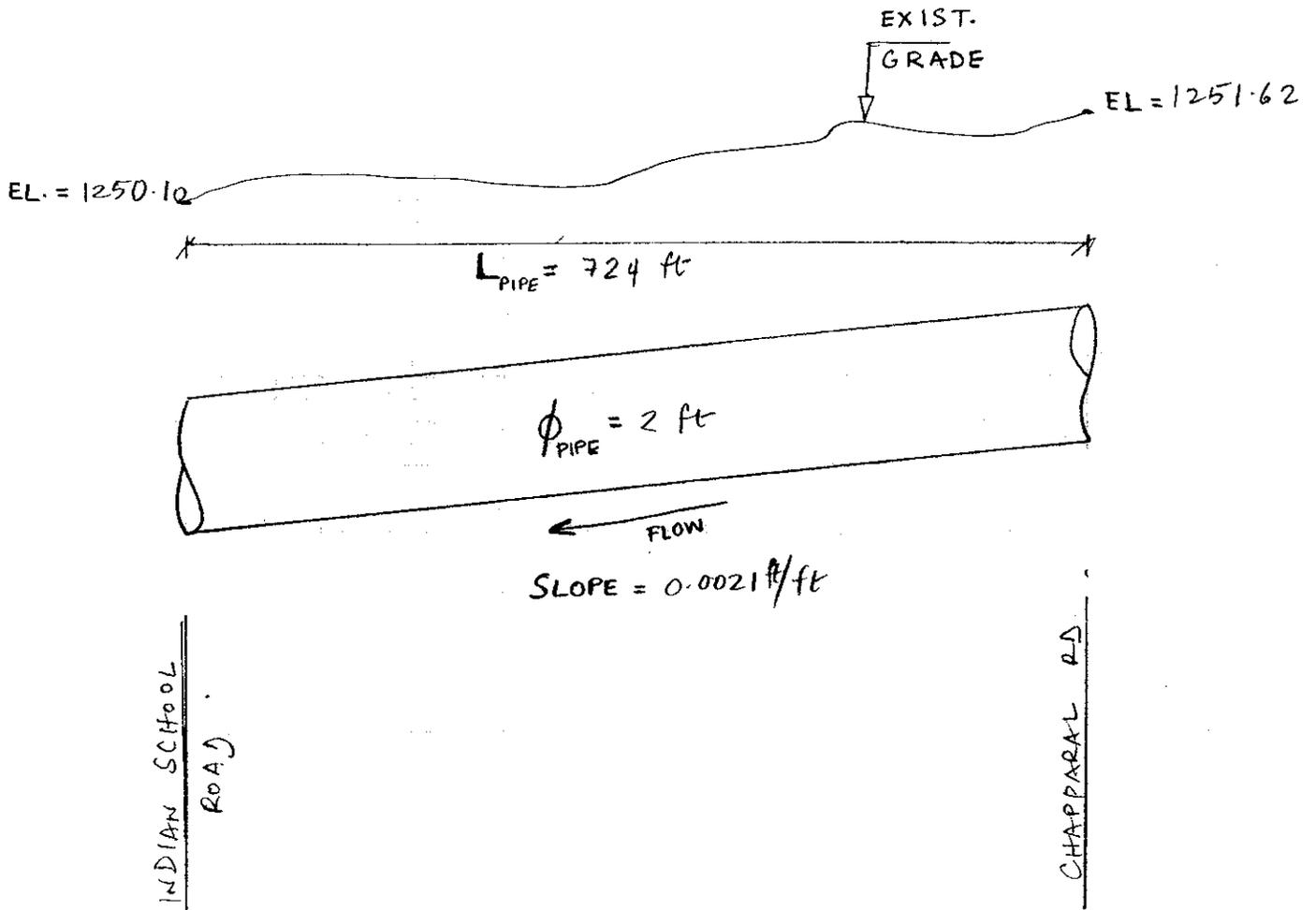


SHEET 2 OF 5  
BY SR DATE 02/05/02  
CHECK PAW DATE 4/17/02

CLIENT FCD of MARICOPA COUNTY  
JOB NAME GRANITE REEF WASH

JOB NO. 310.021

PIPE I.D. : 40-24  
PIMA ROAD



SEGMENT : 7

PIPE # : 2



SHEET 3 OF 5

BY SR DATE 02/05/02

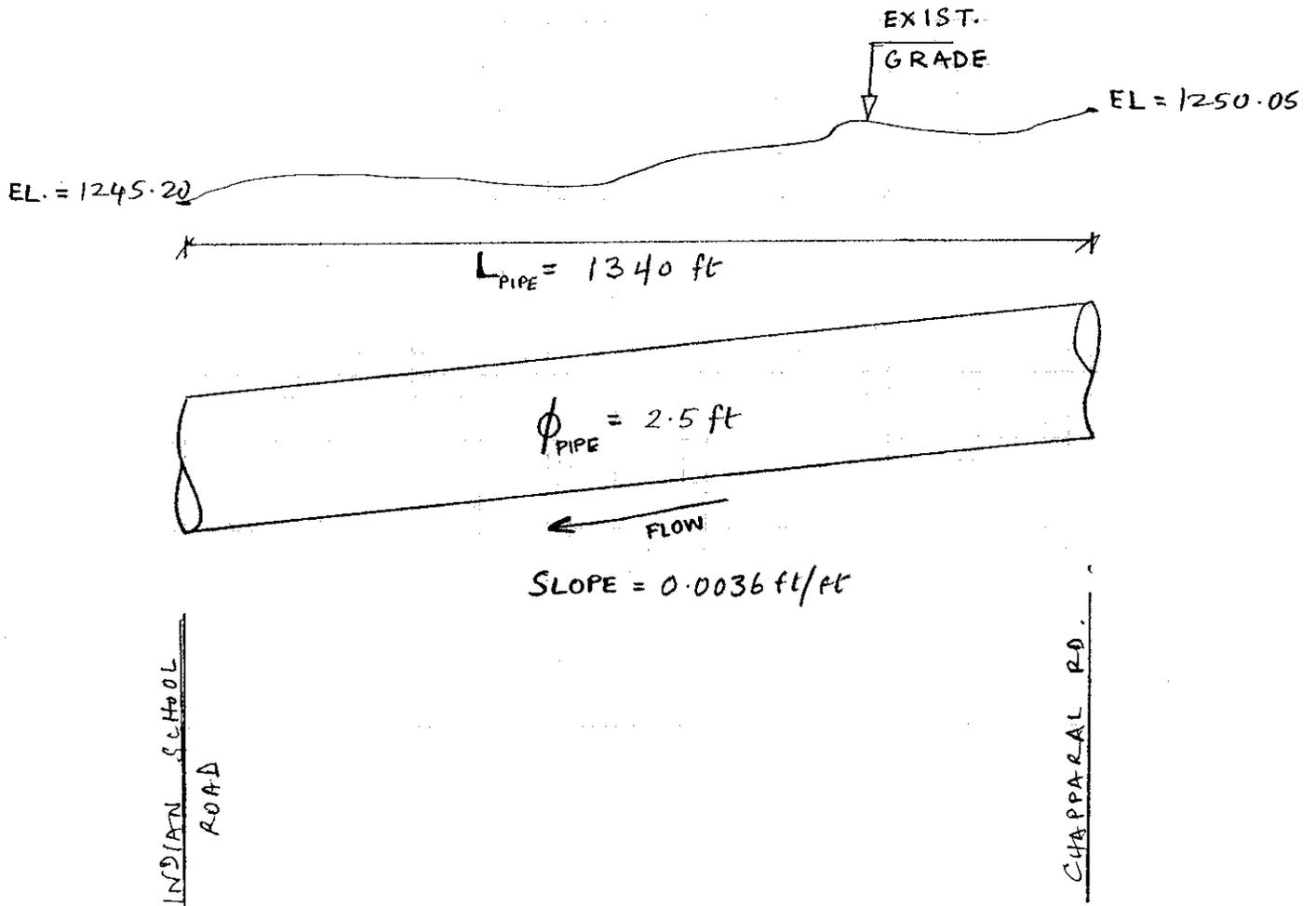
CHECK PAW DATE 4/17/02

CLIENT FCD of MARICOPA COUNTY

JOB NAME GRANITE REEF WASH

JOB NO. 310.021

PIPE ID. # 39-30  
PIMA ROAD



SEGMENT : 7

PIPE # : 3



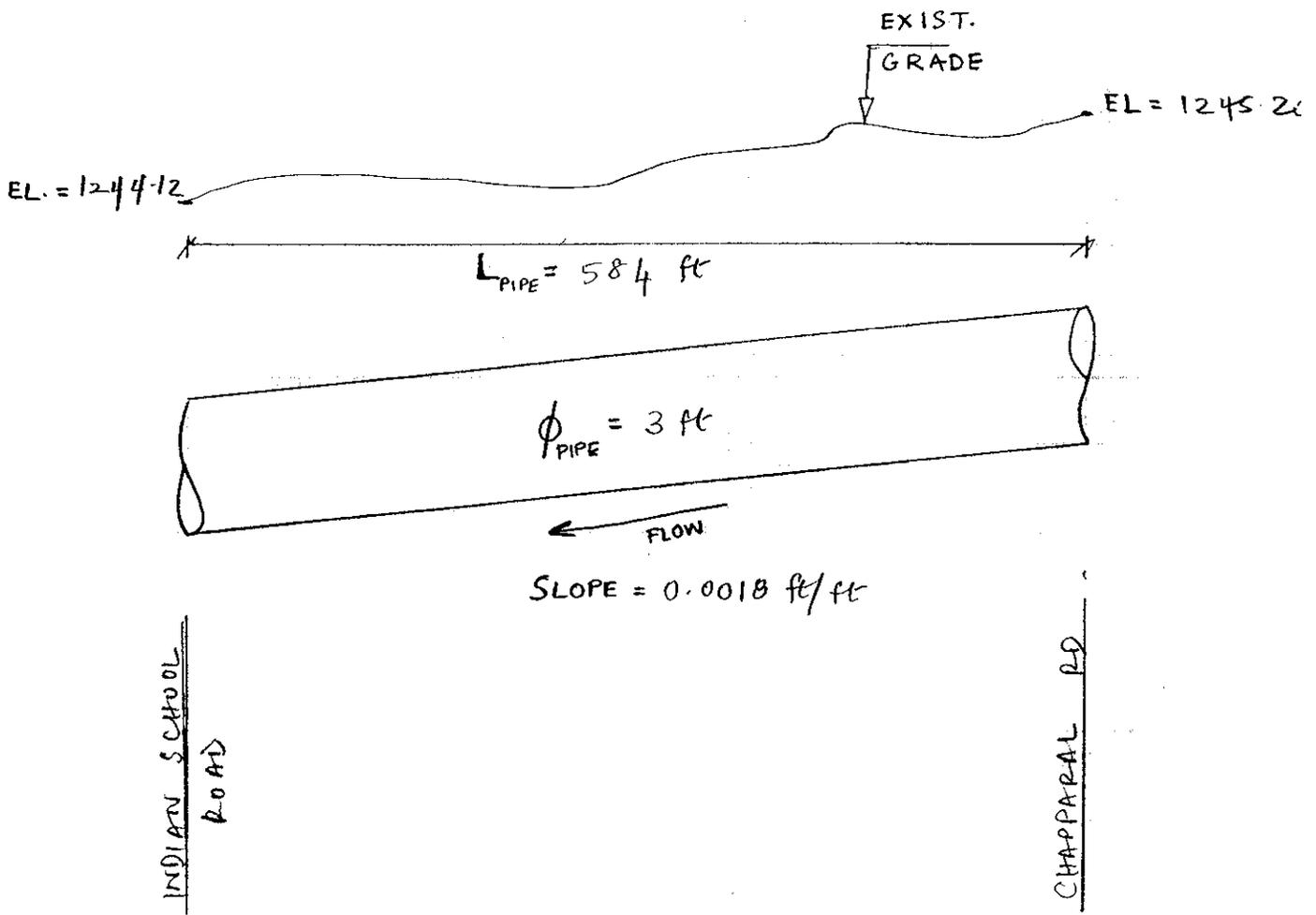
SHEET 4 OF 5  
BY SR DATE 02/05/02  
CHECK PAW DATE 4/17/02

CLIENT FCD of MARICOPA COUNTY

JOB NAME GRANITE REEF WASH

JOB NO. 310.021

PIPE I.D. & 38-36  
PIMA ROAD



SEGMENT : 7

PIPE # : 4



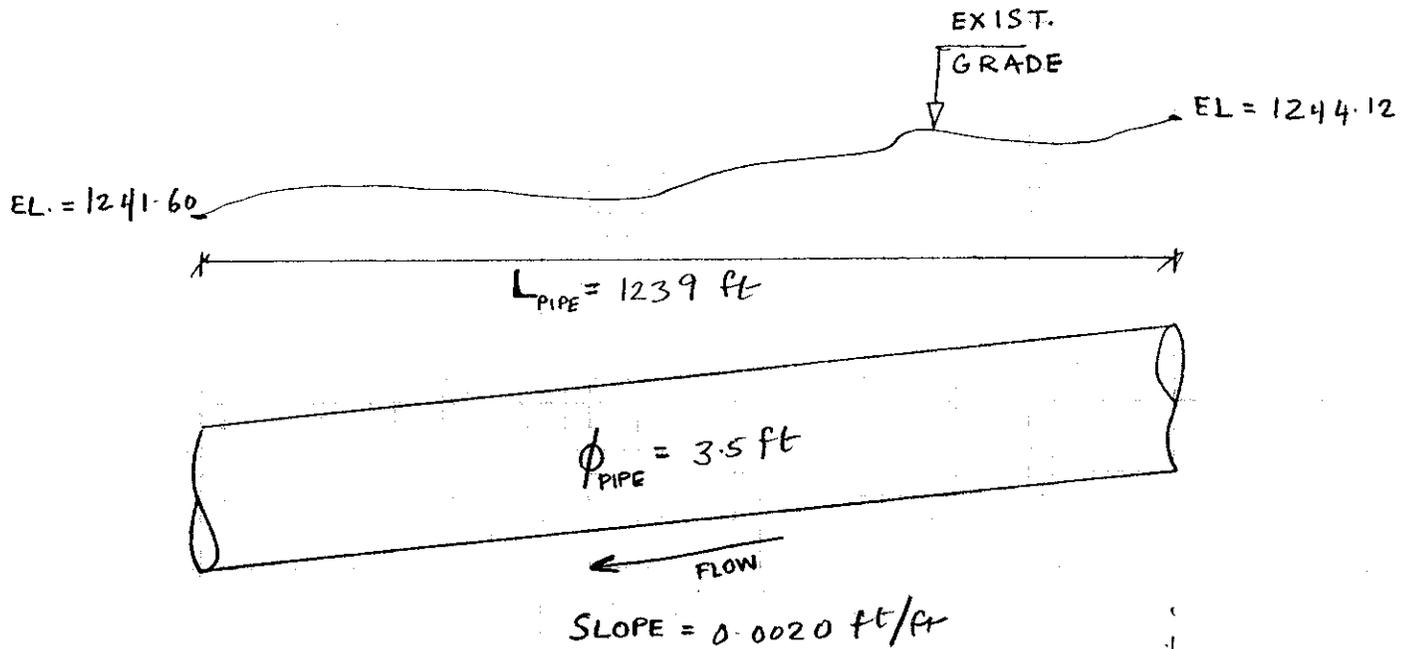
SHEET 5 OF 5  
BY SR DATE 02/05/02  
CHECK PAW DATE 4/17/02

CLIENT FCD of MARICOPA COUNTY

JOB NAME GRANITE REEF WASH

JOB NO. 310.021

PIPE. I.D. = 37-42  
PIMA ROAD



SEGMENT : 7

PIPE # : 5



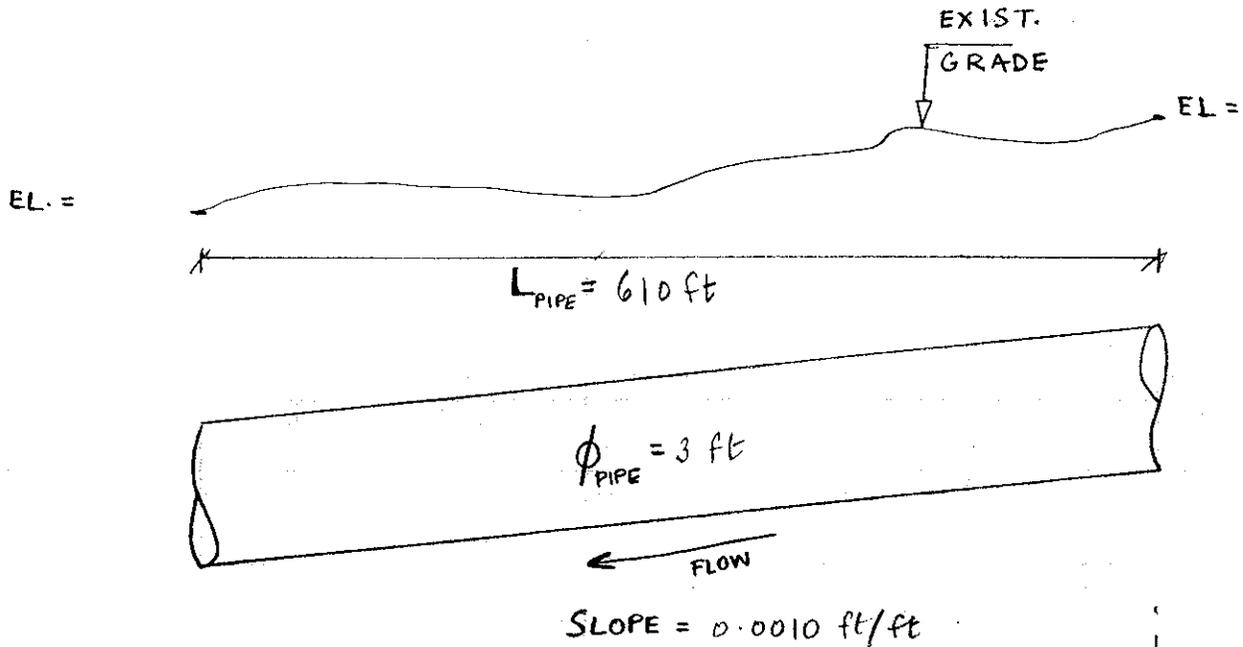
ATTACHMENT - 4.8

SHEET 1 OF 5  
BY SR DATE 02/05/02  
CHECK PAW DATE 4/17/02

CLIENT FCD of MARICOPA COUNTY  
JOB NAME GRANITE REEF WASH

JOB NO. 310.021

PIPE I.D. = 34-36  
CHAPPARAL ROAD



INDIAN GEND  
WASH

PIMA P.D.

SEGMENT : 8

PIPE # : 1



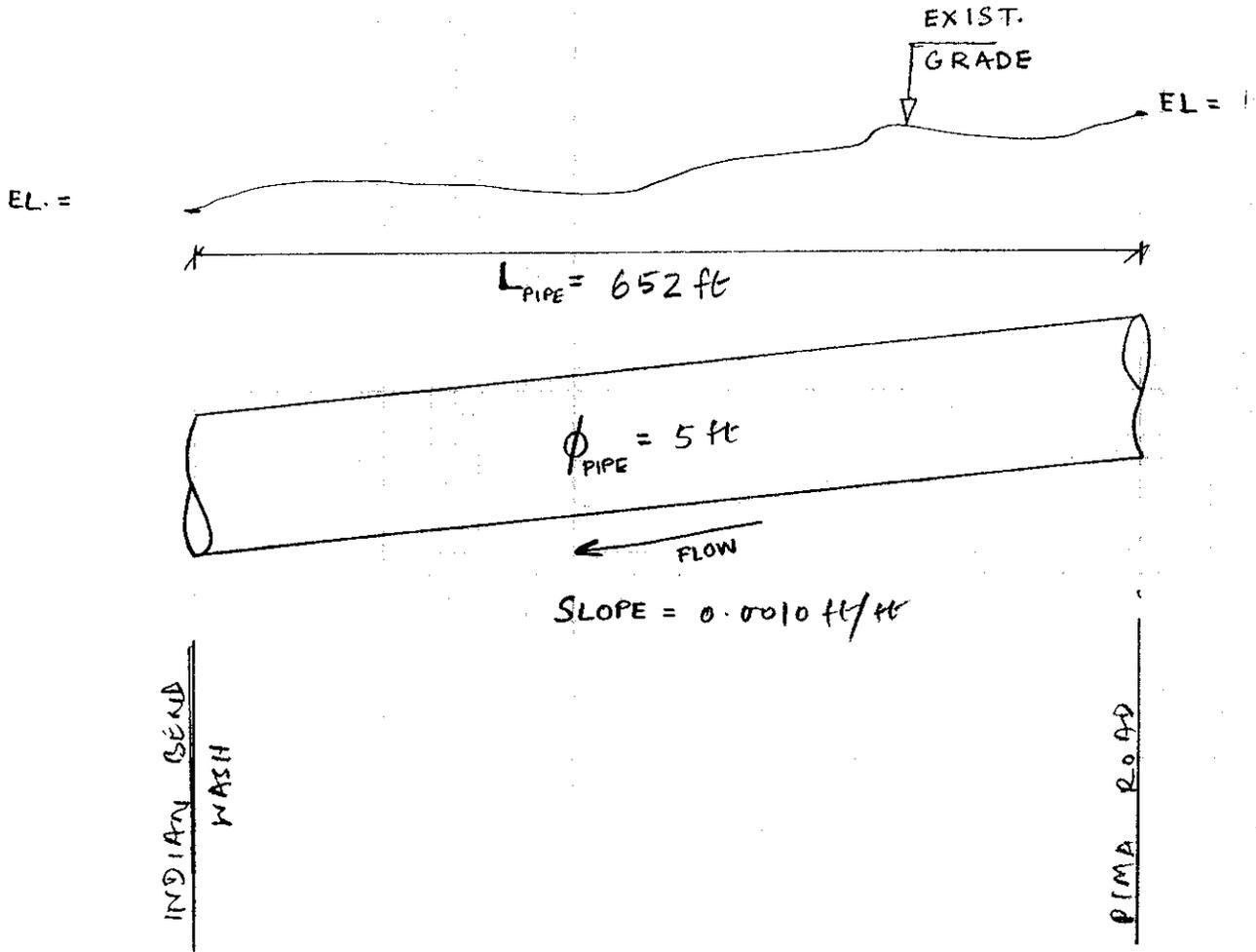
SHEET 2 OF 5  
BY SR DATE 02/05/02  
CHECK PAW DATE 4/17/02

CLIENT FCD of MARICOPA COUNTY

JOB NAME GRANITE REEF WASH

JOB NO. 310.021

PIPE I.D. = 33-60  
CHAPARRAL ROAD



SEGMENT : 8

PIPE # : 2



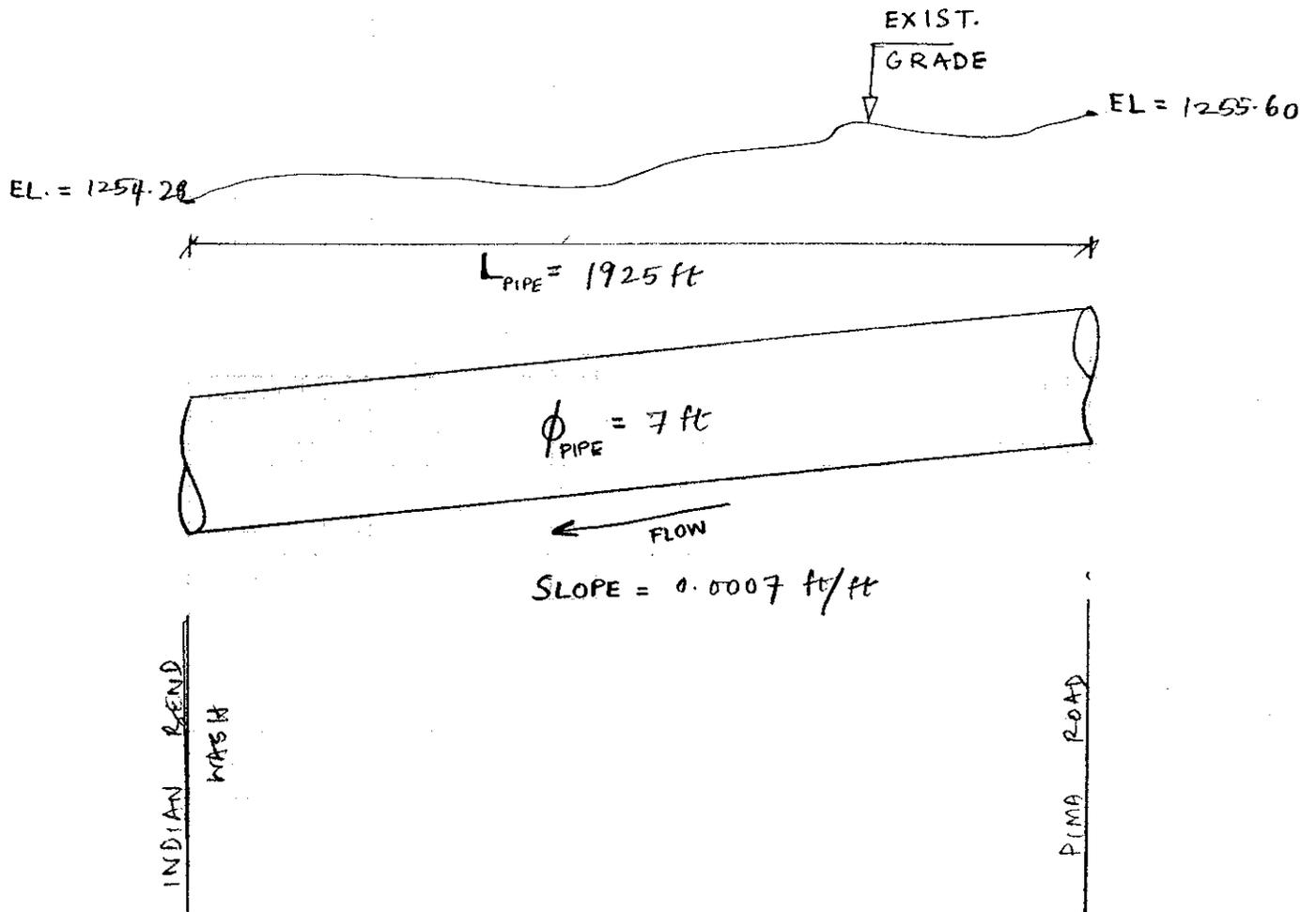
SHEET 3 OF 5  
BY SR DATE 02/05/02  
CHECK PAW DATE 4/17/02

CLIENT FCD of MARICOPA COUNTY

JOB NAME GRANITE REEF WASH

JOB NO. 310.021

PIPE I.D. = 32-84  
CHAPPARAL RD.



SEGMENT : 0

PIPE # : 3

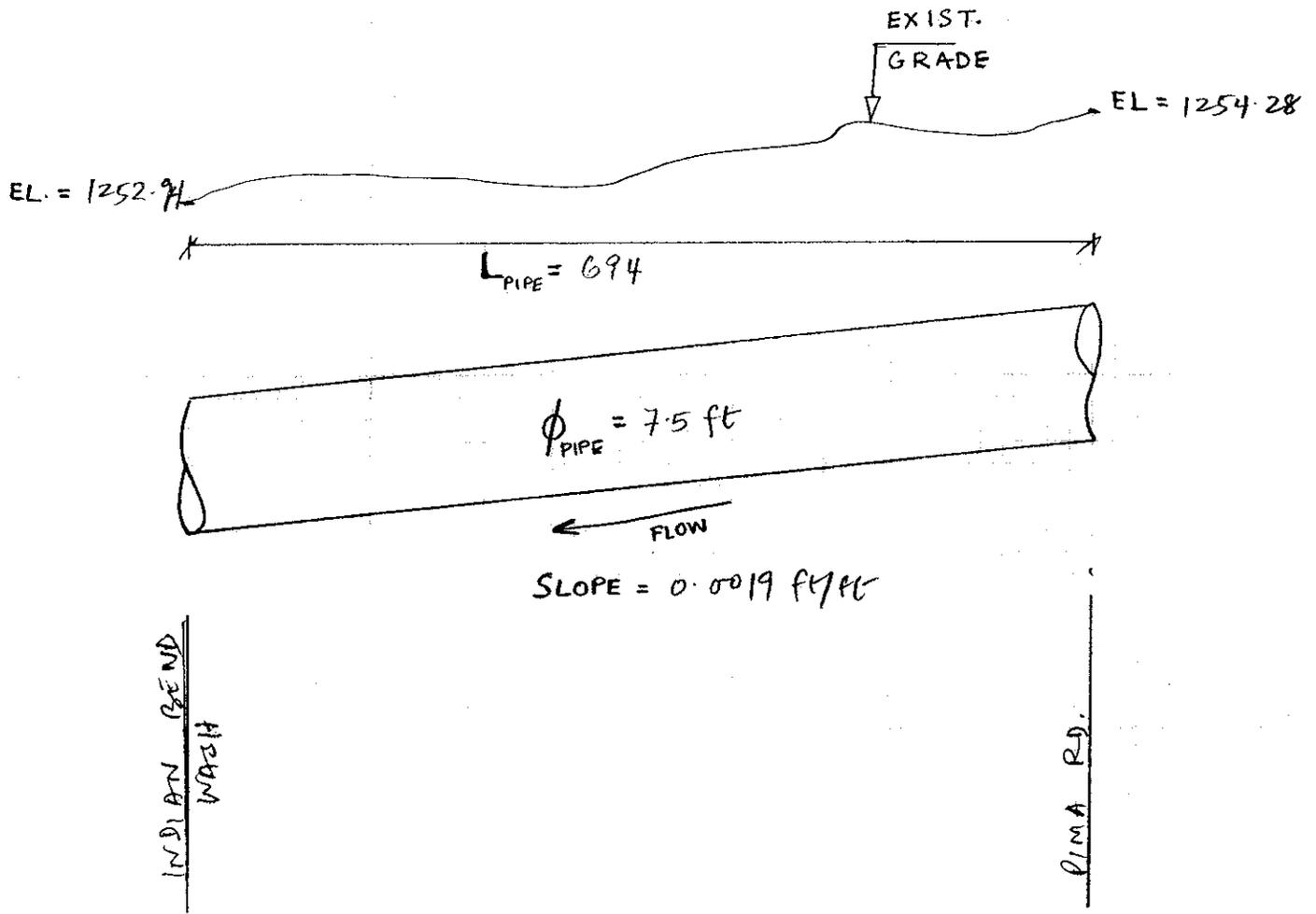


SHEET 4 OF 5  
BY SR DATE 02/05/02  
CHECK PAW DATE 4/17/02

CLIENT FCD of MARICOPA COUNTY  
JOB NAME GRANITE REEF WASH

JOB NO. 310.021

PIPE I.D. = 31-90  
CHAPPARAL ROAD



SEGMENT : 8

PIPE # : 4

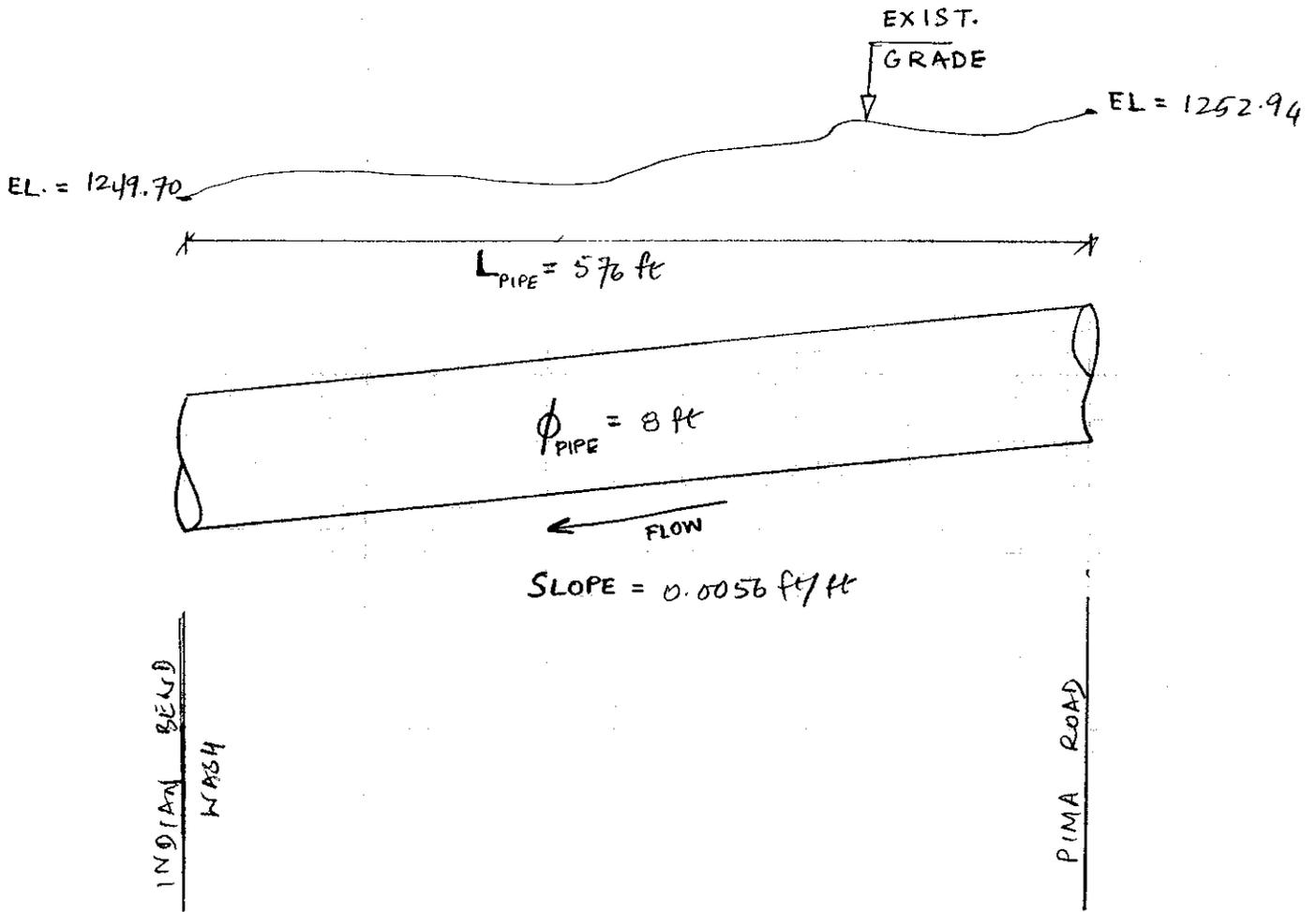


SHEET 5 OF 5  
BY SR DATE 02/05/02  
CHECK PAW DATE 4/17/02

CLIENT FCD of MARICOPA COUNTY  
JOB NAME GRANITE REEF WASH

JOB NO. 310.021

PIPE I.D. : 30-96  
CHAPPARAL ROAD



SEGMENT : 8

PIPE # : 5



ATTACHMENT - 4.9

SHEET 1 OF 1  
BY SR DATE 02/05/02  
CHECK PAW DATE 4/17/02

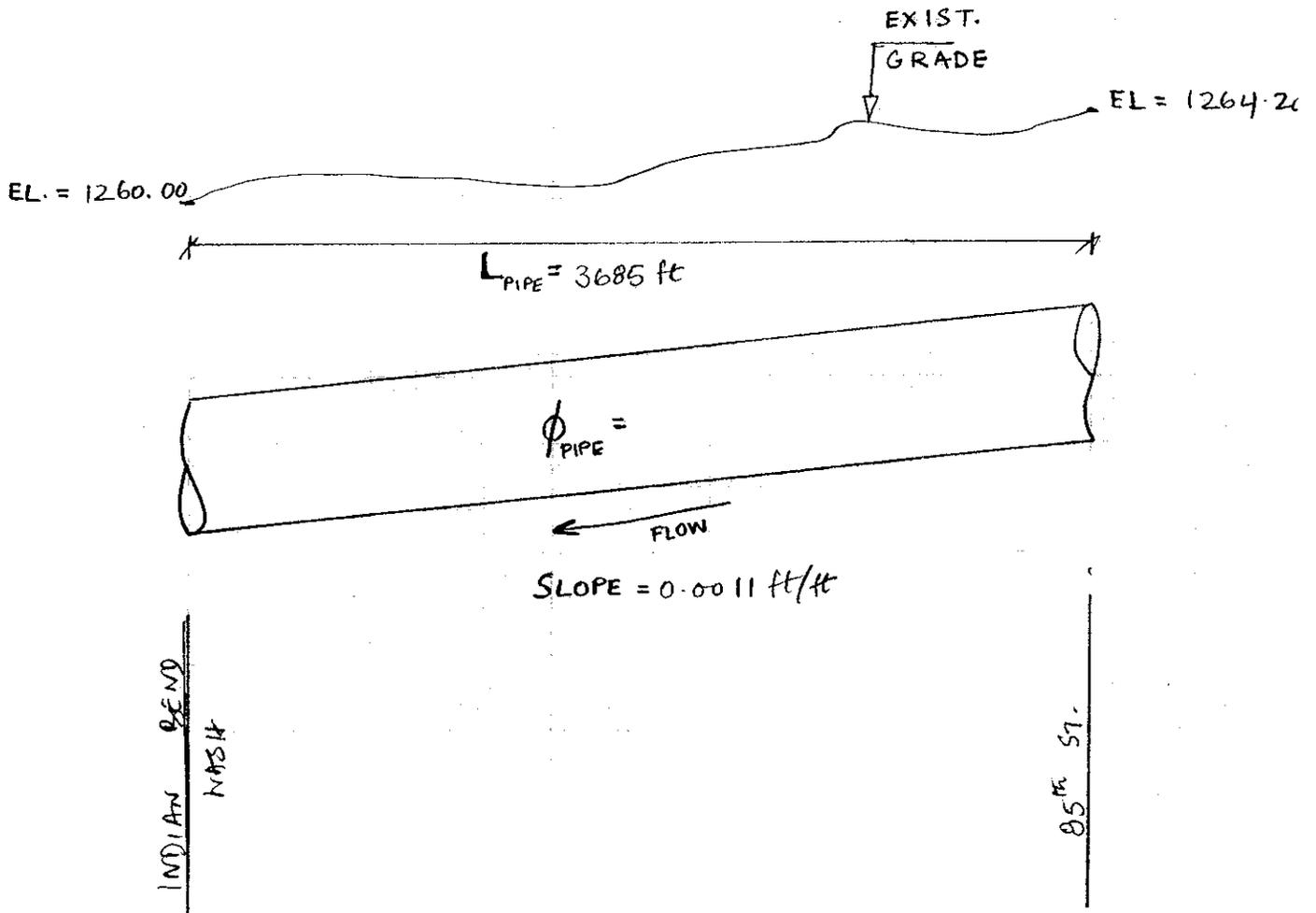
CLIENT FCD of MARICOPA COUNTY

JOB NAME GRANITE REEF WASH

JOB NO. 310.021

PIPE I.D. : 55 -

JACK RABBIT ROAD



SEGMENT : 9

PIPE # : 1



ATTACHMENT - 4-10

SHEET 1 OF 1  
BY SR DATE 02/05/02  
CHECK PAW DATE 4/17/02

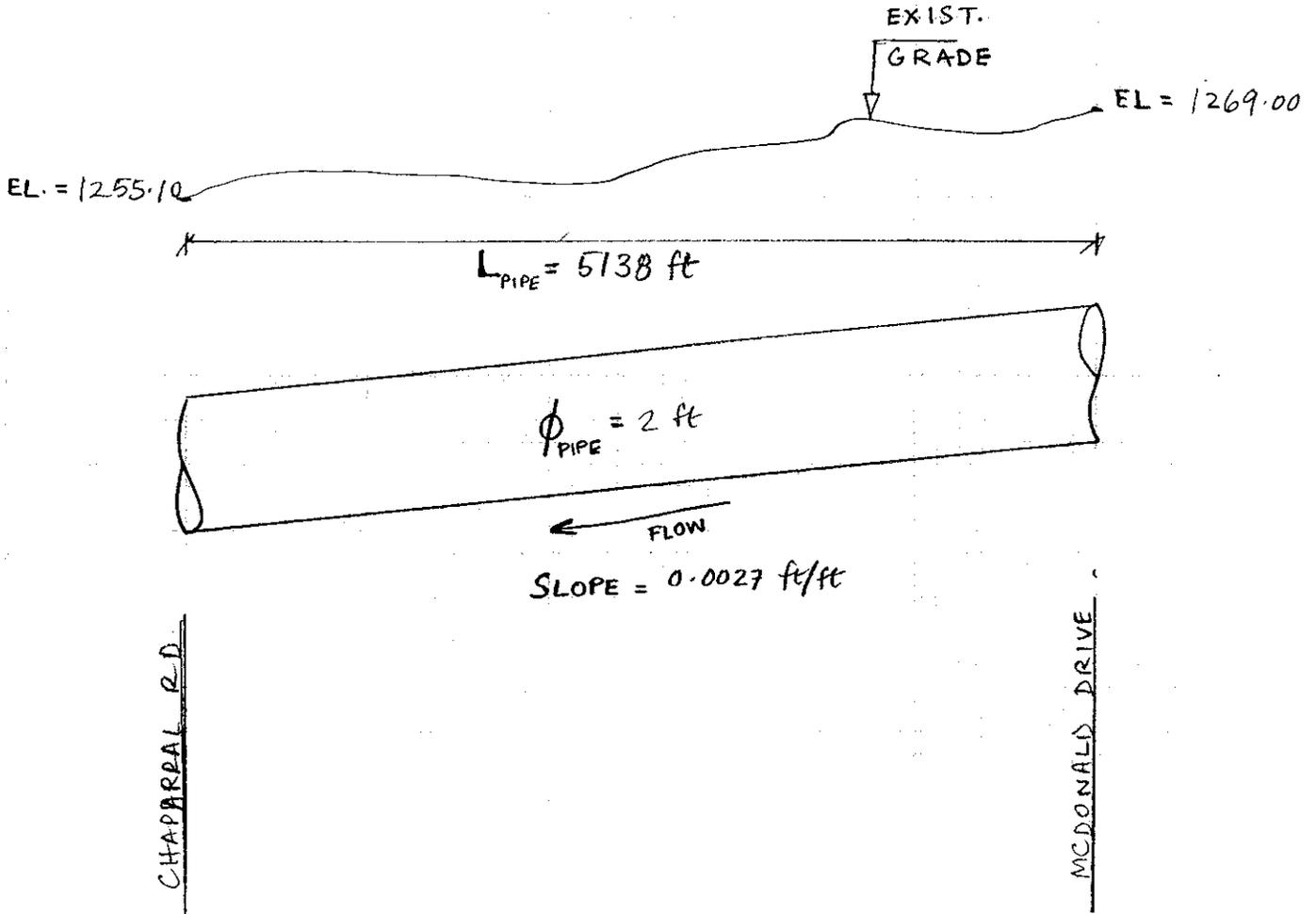
CLIENT FCD of MARICOPA COUNTY

JOB NAME GRANITE REEF WASH

JOB NO. 310.021

PIPE ID: 46-24

PIMA ROAD



SEGMENT : 10

PIPE # : 1



ATTACHMENT - 4.11

SHEET 1 OF 2  
BY SR DATE 02/05/02  
CHECK PAW DATE 4/17/02

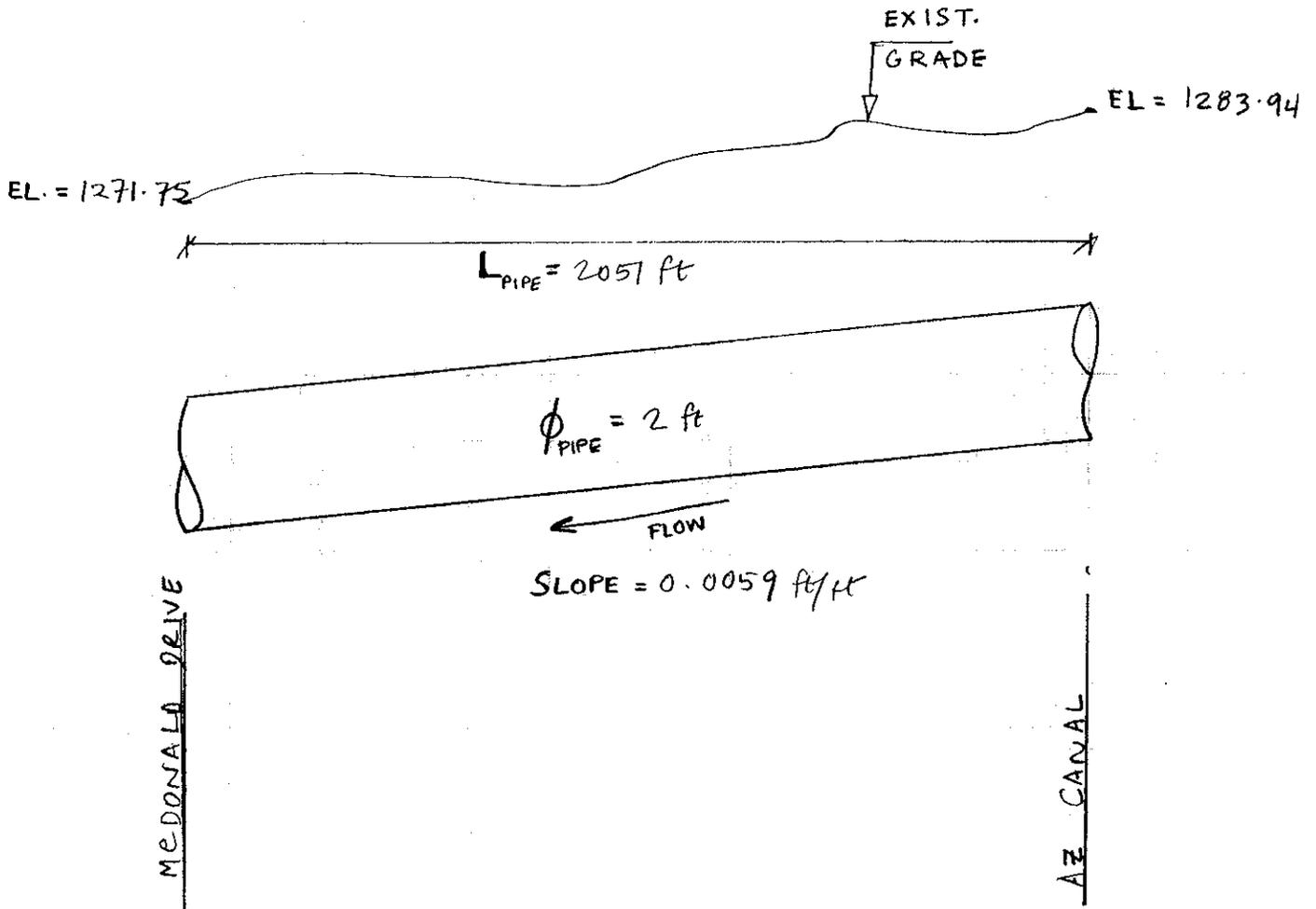
CLIENT FCD of MARICOPA COUNTY

JOB NAME GRANITE REEF WASH

JOB NO. 310.021

PIPE ID. 253-24

PIMA ROAD



SEGMENT : 11

PIPE # : 1

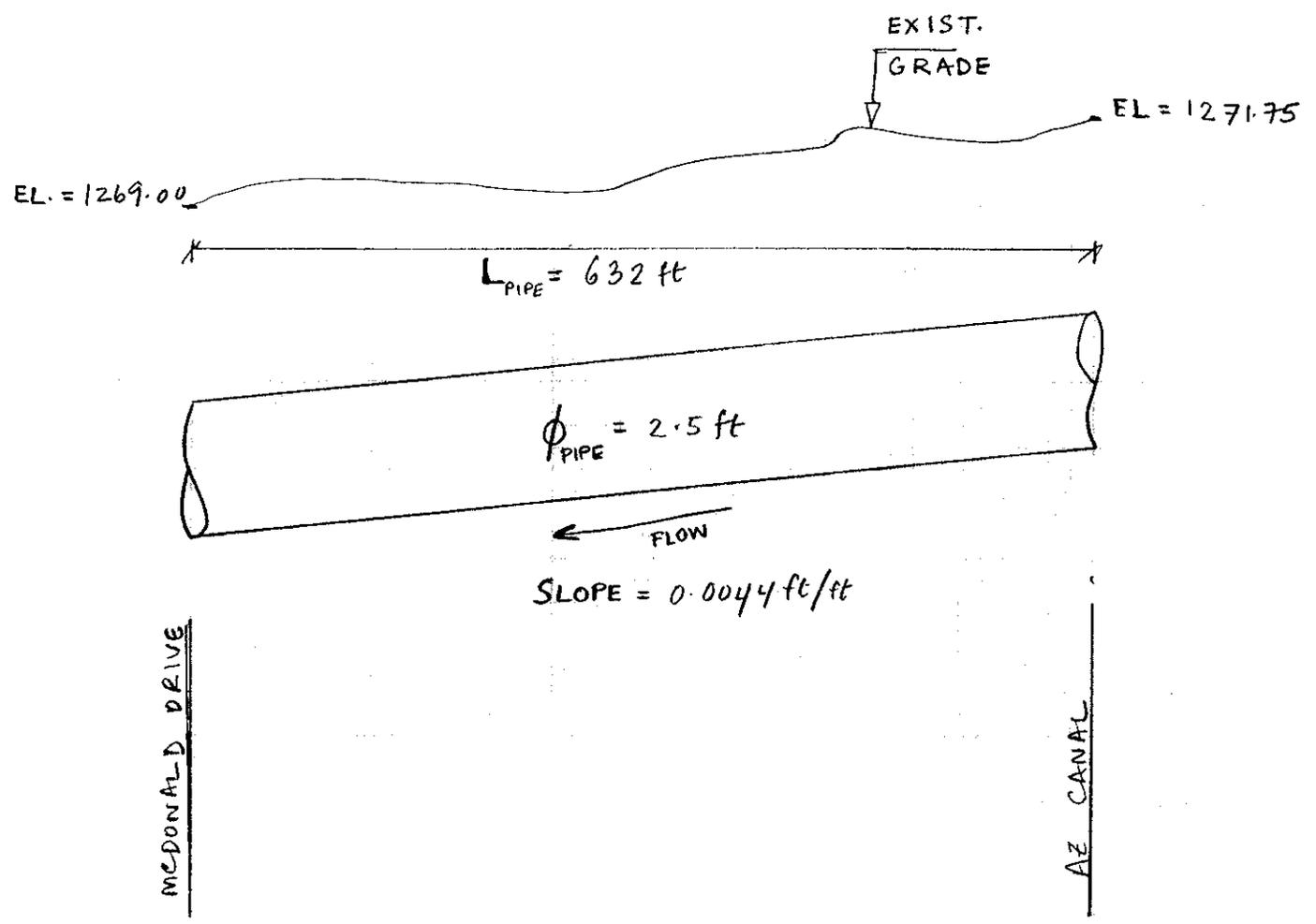


SHEET 2 OF 2  
BY SR DATE 02/05/02  
CHECK PAW DATE 4/17/02

CLIENT FCD of MARICOPA COUNTY  
JOB NAME GRANITE REEF WASH

JOB NO. 310.021

PIPE I.D. = 52-30  
PIMA ROAD



SEGMENT : 11  
PIPE # : 2

**APPENDIX F 100-year, 6-hour future conditions HEC-1 SUMMARY**



```

1*****
*
* FLOOD HYDROGRAPH PACKAGE (HEC-1)
*   MAY 1991
*   VERSION 4.0.1E
*   Lahey F77L-EM/32 version 5.01
*   Dodson & Associates, Inc.
*   RUN DATE 04/30/02 TIME 10:05:11
*****

```

```

*****
*
* U.S. ARMY CORPS OF ENGINEERS
* HYDROLOGIC ENGINEERING CENTER
* 609 SECOND STREET
* DAVIS, CALIFORNIA 95616
* (916) 551-1748
*
*****

```

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X X XXXXXXX XXXXX X
X X X X X XX
X X X X X X
XXXXXXXX XXXX X XXXXX X
X X X X X X
X X X X X X
X X XXXXXXX XXXXX XXX

```

THIS PROGRAM REPLACES ALL PREVIOUS VERSIONS OF HEC-1 KNOWN AS HEC1 (JAN 73), HEC1GS, HEC1DB, AND HEC1KW.

THE DEFINITIONS OF VARIABLES -RTIMP- AND -RTIOR- HAVE CHANGED FROM THOSE USED WITH THE 1973-STYLE INPUT STRUCTURE. THE DEFINITION OF -AMSKK- ON RM-CARD WAS CHANGED WITH REVISIONS DATED 28 SEP 81. THIS IS THE FORTRAN77 VERSION

NEW OPTIONS: DAMBREAK OUTFLOW SUBMERGENCE , SINGLE EVENT DAMAGE CALCULATION, DSS:WRITE STAGE FREQUENCY, DSS:READ TIME SERIES AT DESIRED CALCULATION INTERVAL LOSS RATE:GREEN AND AMPT INFILTRATION

KINEMATIC WAVE: NEW FINITE DIFFERENCE ALGORITHM

1

HEC-1 INPUT

```

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10
1 ID Project ID: SCOTTSDALE - Major Basin: 01 - Return Period: 100
2 ID *****
3 ID **
4 ID ** Granite Reef ADMP **
5 ID **
6 ID *****
7 ID *****
8 ID PROJECT: Granite Reef DMP
9 ID CLIENT: Flood Control District of Maricopa County
10 ID PREPARED BY: Entellus, Inc.
11 ID PROJECT No: FCD Entellus 310.021
12 ID FILE NAME: FUT-6.DAT CREATED DATE: Mar 28, 2002
13 ID MODIFIED:
14 ID STORM: 100-year 6-hour Storm
15 ID DEVELOPMENT CONDITIONS: Existing Conditions
16 ID
17 ID
18 ID
19 ID
20 ID DDM MCUHP1
*
*
21 ID
22 IT 5 1000
23 IO 5
*
*DIAGRAM
*
24 IN 15
25 JD 3.20 0.01
26 PC 0.000 0.008 0.016 0.025 0.033 0.041 0.050 0.058 0.066 0.074
27 PC 0.087 0.099 0.118 0.138 0.216 0.377 0.834 0.911 0.931 0.950
28 PC 0.962 0.972 0.983 0.991 1.000
29 JD 3.179 0.50
30 JD 3.120 2.80
31 PC 0.000 0.009 0.016 0.025 0.034 0.042 0.051 0.059 0.067 0.076
32 PC 0.087 0.100 0.120 0.163 0.252 0.451 0.694 0.837 0.900 0.938
33 PC 0.950 0.963 0.975 0.988 1.000
34 JD 2.950 16.0
35 PC 0.000 0.009 0.020 0.030 0.048 0.063 0.076 0.090 0.105 0.119
36 PC 0.135 0.152 0.175 0.222 0.304 0.472 0.670 0.796 0.868 0.912
37 PC 0.946 0.960 0.973 0.987 1.000
38 JD 2.598 90.0
39 PC 0.000 0.021 0.035 0.051 0.071 0.087 0.105 0.125 0.143 0.160
40 PC 0.179 0.201 0.232 0.281 0.364 0.500 0.658 0.773 0.841 0.888
41 PC 0.927 0.945 0.964 0.982 1.000
42 JD 1.824 500.0
43 PC 0.000 0.024 0.043 0.059 0.078 0.098 0.119 0.141 0.162 0.186
44 PC 0.212 0.239 0.271 0.321 0.408 0.515 0.627 0.735 0.814 0.864
45 PC 0.907 0.930 0.954 0.977 1.000

```

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

46	KK	G03	BASIN								
47	BA	0.093									
48	LG	0.23	0.25	5.60	0.26	55					
49	UC	0.367	0.275								
50	UA	0	5.0	16.0	30.0	65.0	77.0	84.0	90.0	94.0	97.0
51	UA	100									

\*  
\*  
\* \*\*\* DIVERSION DG03 \*\*\*\*\*  
\*  
\* See Diversion Calculation in Appendix D of Hydrology Report.  
\*  
\* DDM \*\*\*\*\* Preserved \*\*\*\*\*

52	KK	DG03									
53	DT	DG03I									
54	DI	0	7	22	43	70	101	124	180	217	264
55	DQ	0	3.5	11	22	35	51	63	92	112	137

\*  
\* Route is a Minor Arterial Road with a median.  
\*

56	KK	RG03S									
57	RS	1	FLOW	0							
58	RC	.035	0.015	.035	1322	0.0024					
59	RX	0	1	40	41	105	106	146	147		
60	RY	8	2	1	.5	.5	1	2	8		

61	KK	G01	BASIN								
62	BA	0.049									
63	LG	0.21	0.25	4.80	0.38	57					
64	UC	0.588	0.598								
65	UA	0	5.0	16.0	30.0	65.0	77.0	84.0	90.0	94.0	97.0
66	UA	100									

\*  
\*  
\* THE CONTRIBUTING AREA WAS HARD-CODED TO ACCOUNT FOR THE DIVERSION DG03.  
\* THE HARD-CODE VALUE IS 50% OF CG03'S CONTRIBUTING AREA.  
\* SEE DIVERSION DG03.  
\*  
\* DDM \*\*\*\*\* Preserved \*\*\*\*\*

67	KK	CG01									
68	HC	2	0.096								

\*  
\* \*\*\* DIVERSION DG01 \*\*\*\*\*  
\*  
\* See Diversion Calculation in Appendix D of Hydrology Report.  
\*  
\* DDM \*\*\*\*\* Preserved \*\*\*\*\*

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

69	KK	DG01									
70	DT	DG01I									
71	DI	0	10	29	57	92	134	144	194	233	278
72	DQ	0	7	21	41	66	96	103	139	166	198

\*  
\* Route is a Minor Arterial Road.  
\*

73	KK	RG01S									
74	RS	16	FLOW	0							
75	RC	0.035	0.015	0.035	2625	0.0014					
76	RX	0	1	40	41	105	106	146	147		
77	RY	8	2	1	.5	.5	1	2	8		

78 KK F04 BASIN  
 79 KM  
 80 KM This basin's original area was 0.073 sq mi. The infield area is bermed  
 81 KM so it is self-detained. The infield area (0.013 sq mi) was subtracted  
 82 KM from the subbasin calculations to account for the storage.  
 83 KM  
 84 BA 0.060  
 85 LG 0.15 0.25 4.80 0.42 68  
 86 UC 0.567 0.659  
 87 UA 0 5.0 16.0 30.0 65.0 77.0 84.0 90.0 94.0 97.0  
 88 UA 100  
 \*  
 \*

89 KK G04 BASIN  
 90 BA 0.045  
 91 LG 0.25 0.25 4.80 0.35 51  
 92 UC 0.296 0.261  
 93 UA 0 5.0 16.0 30.0 65.0 77.0 84.0 90.0 94.0 97.0  
 94 UA 100  
 \*  
 \*  
 \* Route is a Subdivision Road.  
 \*

95 KK RG04  
 96 RS 4 FLOW 0  
 97 RC .035 0.015 0.035 1336 0.0033  
 98 RX 0 1 28 29 57 58 86 87  
 99 RY 8 2 1 0.5 0.5 1 2 8  
 \*

100 KK G05 BASIN  
 101 BA 0.045  
 102 LG 0.24 0.15 7.00 0.14 53  
 103 UC 0.317 0.283  
 104 UA 0 5.0 16.0 30.0 65.0 77.0 84.0 90.0 94.0 97.0  
 105 UA 100  
 \*  
 \*  
 \* Route is a Subdivision Road.  
 \*

HEC-1 INPUT

PAGE

1  
4

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

106 KK RG05  
 107 RS 7 FLOW 0  
 108 RC 0.035 0.015 0.035 1994 0.0026  
 109 RX 0 1 28 29 57 58 86 87  
 110 RY 8 2 1 0.5 0.5 1 2 8  
 \*

111 KK G06 BASIN  
 112 BA 0.127  
 113 LG 0.10 0.35 2.65 1.70 80  
 114 UC 0.254 0.140  
 115 UA 0 5.0 16.0 30.0 65.0 77.0 84.0 90.0 94.0 97.0  
 116 UA 100  
 \*  
 \*  
 \*  
 \* \*\*\* STORAGE ROUTING LG06 \*\*\*\*\*  
 \*  
 \* See the detention calculations in the Appendix.  
 \* DDM \*\*\*\*\* Preserved \*\*\*\*\*

117 KK LG06  
 118 DT LG06D 8.0  
 119 DI 0 100 1000 10000  
 120 DQ 0 100 1000 10000  
 \*  
 \*  
 \* \*\*\* DIVERSION DG06 \*\*\*\*\*  
 \*  
 \* THIS DIVESION IS DUE TO THE DETENTION BASIN.  
 \* See Diversion Calculation in Appendix D of Hydrology Report.  
 \* DDM \*\*\*\*\* Preserved \*\*\*\*\*

121 KK DG06  
 122 DT DG06I  
 123 DI 0 50 100 200 300 500 700 900 1000  
 124 DQ 0 25 49 75 132 195 230 259 287

\*  
 \*  
 \* \*\*\* STORAGE ROUTING LG06A \*\*\*\*\*  
 \* THE STORAGE CALCULATIONS CAN BE FOUND IN THE APPENDIX.  
 \*  
 \* DDM \*\*\*\*\* Preserved \*\*\*\*\*

125 KK LG06A  
 126 DT LG06AD 4.25  
 127 DI 0 100 1000 10000  
 128 DQ 0 100 1000 10000

HEC-1 INPUT

PAGE

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

129 KK G02 BASIN  
 130 BA 0.075  
 131 LG 0.21 0.25 4.80 0.38 52  
 132 UC 0.592 0.517  
 133 UA 0 5.0 16.0 30.0 65.0 77.0 84.0 90.0 94.0 97.0  
 134 UA 100

\*  
 \* THE CONTRIBUTING AREA WAS HARD-CODED TO ACCOUNT FOR THE DIVERSION DG06.  
 \* THE HARD-CODE VALUE IS 50% OF CG06'S CONTRIBUTING AREA.  
 \* SEE DIVERSION DG06.  
 \*  
 \* DDM \*\*\*\*\* Preserved \*\*\*\*\*

135 KK CG02  
 136 HC 4 0.229

\*  
 \*  
 \* Route is a Minor Arterial Road.  
 \*

137 KK RG02  
 138 RS 4 FLOW 0  
 139 RC 0.035 0.015 0.035 1358 0.0018  
 140 RX 0 1 26 27 62 63 88 89  
 141 RY 8 2 1 0.5 0.5 1 2 8

142 KK F06 BASIN  
 143 BA 0.103  
 144 LG 0.23 0.25 4.80 0.36 49  
 145 UC 0.604 0.427  
 146 UA 0 5.0 16.0 30.0 65.0 77.0 84.0 90.0 94.0 97.0  
 147 UA 100

\*  
 \* DDM \*\*\*\*\* Preserved \*\*\*\*\*

148 KK CF06  
 149 HC 2

\*  
 \*  
 \* Route is a Minor Arterial Road.  
 \*

150 KK RF06  
 151 RS 3 FLOW 0  
 152 RC 0.035 0.015 0.035 1299 0.002  
 153 RX 0 1 26 27 62 63 88 89  
 154 RY 8 2 1 0.5 0.5 1 2 8

HEC-1 INPUT

PAGE

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

155 KK F05 BASIN

```

156      BA    0.088
157      LG    0.24    0.25    4.80    0.36    52
158      UC    0.554    0.401
159      UA    0        5.0    16.0    30.0    65.0    77.0    84.0    90.0    94.0    97.0
160      UA    100
*
* DDM ***** Preserved *****

161      KK    CF05
162      HC    2
*
* THERE IS A DIVERSION SOUTH FOR WHATEVER THE STORM DRAIN DOES NOT COLLECT.
* ALL EXCESS OVERLAND AMOUNT FLOWS SOUTH.
*
*
* STORM DRAIN ANALYSIS SHOWS A CAPACITY OF 185 CFS, 1/3 IS INTERCEPTED AT
* THIS LOCATION.(61 cfs) 20% Clogging Factor means 49 cfs here.
* (61 x .8 = 49cfs)
*
* *** DIVERSION DF05 *****
*
* See Diversion Calculation in Appendix D of Hydrology Report.
*
* DDM ***** Preserved *****

163      KK    DF05
164      DT    DF05I
165      DI    0        1        50       100      200      300      400      500      1000     2000
166      DQ    0        .9       10       51       151      251      351      451      951      1951
*
*
* THE INFORMATION FOR THE STORM DRAIN WAS MEASURED IN THE FIELD.
* THE PIPE IS A 66" RCP, SLOPE WAS ESTIMATED TO BE 0.004 FT/FT.
*

167      KK    RF05W
168      KO    3
169      RK    1302    0.001    0.02          CIRC      3
*
*
* THE CONTRIBUTING AREA WAS HARD-CODED TO ACCOUNT FOR DIVERSION DF05 & DG01.
* THE HARD-CODE VALUE IS 80% OF CG01 AND 10% OF CF05'S CONTRIBUTING AREA.
* SEE DIVERSION DG01 AND DF05.
*
* DDM ***** Preserved *****

170      KK    CF04
171      HC    3        0.17
*
* THERE IS A DIVERSION SOUTH FOR WHATEVER THE STORM DRAIN DOES NOT COLLECT.
*
*
* *** DIVERSION DF04 *****
*
* See Diversion Calculation in Appendix D of Hydrology Report.
*
* DDM ***** Preserved *****

172      KK    DF04
173      DT    DF04I
174      DI    0        9        30       57       93       136      150      198      336      400
175      DQ    0        5        16       30       50       73       82       105      170      200
*
*
* DIVERSION RECOVER D05
*

176      KK    F05R
177      DR    DF05I
*
* Route is a Minor Arterial Road.
*

178      KK    RF05S
179      RS    4        FLOW      0
180      RC    0.035    0.015    0.035    2167    0.0032

```

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LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10





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LINE      ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

234      KK   RF02W
235      KO     3
236      RK   1310   0.001   0.02           CIRC     5
*
*
* DIVERSION RECOVER F04
*
237      KK   F04R
238      DR   DF04I
*
*
* Route is a Minor Arterial Road.
*
*
239      KK   RF04S
240      RS   11     FLOW     0
241      RC   0.035  0.013  0.035  2652  0.0023
242      RX   0       1       26     27     62     63     88     89
243      RY   8       3       2       0       0       2       3       8
*
*
* THE WEST SIDE OF THE ROAD IS DIVERTED WEST AND THE EAST SIDE OF THE ROAD
* FLOWS SOUTH INTO CF01.  THE TOTAL FLOW DOES NOT EXCEED THE CROWN OF THE ROAD
* SO THE SPLIT IS 50-50.
*
* *** DIVERSION DRF04 *****
*
* See Diversion Calculation in Appendix D of Hydrology Report.
*
* DDM ***** Preserved *****

244      KK   DRF04
245      DT   DRF04I
246      DI   0       1       20     50     100    200    300
247      DQ   0       .5     10     25     50     100    150
*
*
248      KK   F01   BASIN
249      BA   0.030
250      LG   0.22   0.25   4.80   0.39   53
251      UC   0.467  0.490
252      UA   0       5.0   16.0   30.0   65.0   77.0   84.0   90.0   94.0   97.0
253      UA   100
*
*
* THE CONTRIBUTING AREA WAS HARD-CODED TO ACCOUNT FOR DIVERSIONS DF04 & DF02.
* THE HARD-CODE VALUE IS 25% OF CF04 AND 10% OF CF02'S CONTRIBUTING AREA.
* SEE DIVERSION DF04 AND DF02.
*
* DDM ***** Preserved *****

```

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```

LINE      ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

254      KK   CF01
255      HC     3     .101
*
*
* THE DIVERSION AT THIS LOCATION IS SOUTH (SEE SPLITFLOW ANALYSIS)
*
* *** DIVERSION DF01 *****
*
* See Diversion Calculation in Appendix D of Hydrology Report.
*
* DDM ***** Preserved *****

256      KK   DF01
257      DT   DF01I
258      DI   0       7       21     42     75     111    139    188    254    335
259      DQ   0       0       0       0       0       0       6       18     35     57
*
*
* DIVERSION RECOVER DF02

```

```

*
260      KK   DRF02
261      DR   DF02I
*
*
* Route is a Minor Arterial Road.
*
262      KK   RF02S
263      RS   3      FLOW      0
264      RC   0.035  0.015  0.035  1338  0.0022
265      RX   0      1      26      27      62      63      88      89
266      RY   8      2      1      0.5    0.5    1      2      8
*
*
267      KK   E07   BASIN
268      BA   0.084
269      LG   0.25  0.25  4.80  0.36  49
270      UC   0.442  0.305
271      UA   0      5.0   16.0  30.0  65.0  77.0  84.0  90.0  94.0  97.0
272      UA   100
*
*
* THE CONTRIBUTING AREA WAS HARD-CODED TO ACCOUNT FOR DIVERSION DF02.
* THE HARD-CODE VALUE IS 90% OF CF02'S CONTRIBUTING AREA.
* SEE DIVERSION DF02.
*
* DDM ***** Preserved *****

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LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

```

273      KK   CE07
274      HC   2      .661
*
*
* Route is a Minor Arterial Road.
*
275      KK   RE07
276      RS   1      FLOW      0
277      RC   0.035  0.015  0.035  1335  0.0025
278      RX   0      1      26      27      62      63      88      89
279      RY   8      2      1      0.5    0.5    1      2      8
*
*
280      KK   E05   BASIN
281      KM
282      KM   This basin's original area was 0.060 sq mi. The infield area is bermed
283      KM   so it is self-detained. The infield area (0.013 sq mi) was subtracted
284      KM   from the subbasin calculations to account for the storage.
285      KM
286      BA   0.047
287      LG   0.21  0.25  4.80  0.37  57
288      UC   0.375  0.292
289      UA   0      5.0   16.0  30.0  65.0  77.0  84.0  90.0  94.0  97.0
290      UA   100
*
* DDM ***** Preserved *****
291      KK   CE05
292      HC   2
*
*
* THERE IS A DIVERSION SOUTH FOR WHATEVER THE STORM DRAIN DOES NOT INTERCEPT.
* THE STORM DRAIN ANALYSIS SHOWS THAT THE MAXIMUM FLOW IN THE DRAIN IS 92 CFS.
* THE TOTAL FLOW WAS DIVIDED BY E05, E04, AND THE BASIN WEST OF THE PROJECT
* AREA, SO 31 CFS AT THIS LOCATION.
* 20% clogging factor means 25cfs at this location (31cfs * 0.8 = 25cfs).
* ALL OVERLAND FLOW CONTINUES SOUTH.
*
* *** DIVERSION DE05 *****
*
* See Diversion Calculation in Appendix D of Hydrology Report.
*
* DDM ***** Preserved *****

```

293 KK DE05  
 294 DT DE05I  
 295 DI 0 1 50 100 200 300 400 500 1000 2000  
 296 DQ 0 1 25 75 175 275 375 475 975 1975

\*  
 \*  
 \*  
 \* ROUTE IS A 5FT RCP WITH A SLOPE OF 0.0021 FT/FT.  
 \*

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LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

297 KK RE05W  
 298 RK 1490 0.0021 0.02 CIRC 5  
 \*  
 299 KK E04 BASIN  
 300 BA 0.038  
 301 LG 0.22 0.25 4.80 0.39 47  
 302 UC 0.621 0.655  
 303 UA 0 5.0 16.0 30.0 65.0 77.0 84.0 90.0 94.0 97.0  
 304 UA 100  
 \*  
 \*

\* \*\*\* STORAGE ROUTING LE04 \*\*\*\*\*  
 \*  
 \* DDM \*\*\*\*\* Preserved \*\*\*\*\*

305 KK LE04  
 306 DT LE04D 1.5  
 307 DI 0 100 1000 10000  
 308 DQ 0 100 1000 10000  
 \*  
 \* DIVERSION RECOVER DF01  
 \*

309 KK DRF01I  
 310 DR DF01I  
 \*  
 \*  
 \*  
 \* Route is a Minor Arterial Road.  
 \*

311 KK RF01S  
 312 RS 24 FLOW 0  
 313 RC 0.035 0.015 0.035 2619 0.0031  
 314 RX 0 1 26 27 62 63 88 89  
 315 RY 8 2 1 0.5 0.5 1 2 8  
 \*  
 \*

\* THE CONTRIBUTING AREA WAS HARD-CODED TO ACCOUNT FOR DIVERSIONS DF01 & DE05.  
 \* THE HARD-CODE VALUE IS 5% OF CF01 AND 5% OF CE05'S CONTRIBUTING AREA.  
 \* SEE DIVERSION DF01 AND DE05.  
 \*  
 \* DDM \*\*\*\*\* Preserved \*\*\*\*\*

316 KK CE04  
 317 HC 3 0.071  
 \*  
 \*  
 \* THERE IS A DIVERSION SOUTH (SEE SPLITFLOW CALCULATIONS)  
 \*  
 \* \*\*\* DIVERSION DE04 \*\*\*\*\*  
 \*  
 \* See Diversion Calculation in Appendix D of Hydrology Report.  
 \*  
 \* DDM \*\*\*\*\* Preserved \*\*\*\*\*

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LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

318 KK DE04  
 319 DT DE04I  
 320 DI 0 3 31 59 91 126 151 184 222 267

```

321      DQ      0      0      0      0      0      0      3      9      18      30
*
*
* DDM ***** Preserved *****

322      KK      DUMMY
323      HC      3
*

324      KK      E08      BASIN
325      BA      0.031
326      LG      0.24      0.25      4.80      0.35      51
327      UC      0.488      0.531
328      UA      0      5.0      16.0      30.0      65.0      77.0      84.0      90.0      94.0      97.0
329      UA      100
*
*
* Route is a Minor Arterial Road with a wall on the west side.
*

330      KK      RE08
331      RS      5      FLOW      0
332      RC      0.035      0.015      0.035      1326      0.0029
333      RX      0      1      19      25      30      49      79      80
334      RY      10      1.5      0      0      0      0      1.5      8
*

335      KK      E06      BASIN
336      BA      0.036
337      LG      0.24      0.25      4.80      0.36      53
338      UC      0.500      0.548
339      UA      0      5.0      16.0      30.0      65.0      77.0      84.0      90.0      94.0      97.0
340      UA      100
*
* DDM ***** Preserved *****

341      KK      CE06
342      HC      2
*
*
* Route is a Minor Arterial Road with a wall on the west side.
*

343      KK      RE06
344      RS      13      FLOW      0
345      RC      0.035      0.015      0.035      3333      0.0014
346      RX      0      1      19      25      30      49      79      80
347      RY      10      1.5      0      0      0      0      1.5      8
*

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LINE      ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

348      KK      E03      BASIN
349      BA      0.070
350      LG      0.23      0.25      4.80      0.36      53
351      UC      0.917      1.373
352      UA      0      5.0      16.0      30.0      65.0      77.0      84.0      90.0      94.0      97.0
353      UA      100
*
*
* DIVERSION RECALL DF07
*

354      KK      DRF07
355      DR      DF07I
*
* Route is a minor arterial along Pima Rd.
*

356      KK      RF07S
357      RS      7      FLOW      0
358      RC      0.035      0.015      0.035      5280      0.0023
359      RX      0      1      40      41      104      105      145      146
360      RY      8      3      1.5      .75      0.75      1.5      3      8
*
*

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*
361      KK      E09      BASIN
362      BA      0.223
363      LG      0.10      0.29      2.65      1.70      80
364      UC      0.546      0.446
365      UA      0          5.0      16.0      30.0      65.0      77.0      84.0      90.0      94.0      97.0
366      UA      100

```

```

*
*
* *** STORAGE ROUTING LE09 *****
*
* See the detention calculations in the Appendix.
* DDM ***** Preserved *****

```

```

367      KK      LE09
368      DT      LE09D      13.6
369      DI      0          100      1000      10000
370      DQ      0          100      1000      10000

```

```

*
* THE CONTRIBUTING AREA WAS HARD-CODED TO ACCOUNT FOR DIVERSIONS DF07.
* THE HARD-CODE VALUE IS 90% OF CF07'S CONTRIBUTING AREA.
* SEE DIVERSION DF07.
*
* DDM ***** Preserved *****

```

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LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

```

371      KK      CE09
372      HC      2          .511

```

```

*
* THERE IS A DIVERSION SOUTH FOR WHATEVER THE STORM DRAIN DOES NOT INTERCEPT.
* THE STORM DRAIN ANALYSIS SHOWS THAT THE MAXIMUM FLOW IN THE DRAIN IS 96 CFS.
* THIS AMOUNT IS DIVIDED BETWEEN DIVERSIONS DE01, DE02, DE03, DE09, AND THE
* BASIN EAST OF THE PROJECT AREA, WHICH IS 19 CFS AT THIS LOCATION.
*
* 20% clogging factor means 15cfs, (19cfs * 0.8 = 15cfs).
* ALL OVERLAND FLOW CONTINUES SOUTH AT THIS DIVERSION LOCATION.

```

```

* *** DIVERSION DE09 *****
*
* See Diversion Calculation in Appendix D of Hydrology Report.
*
* DDM ***** Preserved *****

```

```

373      KK      DE09
374      DT      DE09I
375      DI      0          1          50      100      200      300      400      500      1000      2000
376      DQ      0          1          35      85      185      285      385      485      985      1985

```

```

*
* ROUTE IS A 6FT RCP WITH A SLOPE OF 0.0009 FT/FT.
*

```

```

377      KK      RE09W
378      KO      3
379      RK      779      0.0009      0.013      CIRC      6

```

```

*
* DIVERSION RECOVER DE05
*

```

```

380      KK      DRE05I
381      DR      DE05I

```

```

*
* Route is a Minor Arterial Road.
*

```

```

382      KK      RE05S
383      RS      6          FLOW      0
384      RC      0.035      0.015      0.035      3053      0.0021
385      RX      0          1          26      27      62      63      88      89

```

386 RY 8 2 1 0.5 0.5 1 2 8  
 \*  
 \*  
 \* THE CONTRIBUTING AREA IS HARD-CODED TO ACCOUNT FOR DIVERSIONS DE05 & DE09.  
 \* THE HARD-CODE VALUE IS 95% OF CE05 AND 10% OF CE09'S CONTRIBUTING AREA.  
 \* SEE DIVERSION DE05 AND DE09.  
 \*  
 \* DDM \*\*\*\*\* Preserved \*\*\*\*\*

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17

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

387 KK CE03  
 388 HC 4 .858

\*  
 \*  
 \* THERE IS A DIVERSION SOUTH FOR WHATEVER THE STORM DRAIN DOES NOT INTERCEPT.  
 \*  
 \* THE STORM DRAIN ANALYSIS SHOWS THAT THE MAXIMUM FLOW IN THE DRAIN IS 96 CFS.  
 \* THIS AMOUNT IS DIVIDED BETWEEN DIVERSIONS DE01, DE02, DE03, DE09, AND THE  
 \* BASIN EAST OF THE PROJECT AREA, WHICH IS 38cfs AT THIS LOCATION.  
 \*  
 \* 20% clogging factor means that 31cfs (38.4 cfs \* 0.8 = 31cfs)  
 \* ALL OVERLAND FLOWS CONTINUE SOUTH.  
 \*  
 \* \*\*\* DIVERSION DE03 \*\*\*\*\*  
 \*  
 \* See Diversion Calculation in Appendix D of Hydrology Report.  
 \*  
 \* DDM \*\*\*\*\* Preserved \*\*\*\*\*

389 KK DE03  
 390 DT DE03I  
 391 DI 0 1 50 100 200 300 400 500 1000 2000  
 392 DQ 0 1 19 69 169 269 369 469 969 1969

\*  
 \*  
 \* ROUTE IS A 6FT RCP WITH A SLOPE OF 0.0009 FT/FT.  
 \*

393 KK RE03W  
 394 RK 1504 0.0009 0.02 CIRC 6

395 KK E02 BASIN  
 396 BA 0.149  
 397 LG 0.24 0.25 4.80 0.37 50  
 398 UC 0.683 0.539  
 399 UA 0 5.0 16.0 30.0 65.0 77.0 84.0 90.0 94.0 97.0  
 400 UA 100

\*  
 \* THE CONTRIBUTING AREA IS HARD-CODED DUE TO DIVERSION DE03.  
 \* THE HARD-CODE VALUE IS 5% OF CE03'S CONTRIBUTING AREA.  
 \* SEE DIVERSION DE03.  
 \*  
 \* DDM \*\*\*\*\* Preserved \*\*\*\*\*

401 KK CE02  
 402 HC 2 .183

\*  
 \*  
 \* THERE IS A DIVERSION SOUTH FOR WHATEVER THE STORM DRAIN DOES NOT INTERCEPT.  
 \*  
 \* THE STORM DRAIN ANALYSIS SHOWS THAT THE MAXIMUM FLOW IN THE DRAIN IS 96 CFS.  
 \* THIS AMOUNT IS DIVIDED BETWEEN DIVERSIONS DE01, DE02, DE03, DE09, AND THE  
 \* BASIN EAST OF THE PROJECT AREA, 58cfs AT THIS LOCATION.  
 \*  
 \* 20% clogging factor means 46cfs (58cfs \* 0.8 = 46cfs).  
 \* ALL OVERLAND FLOW CONTINUES SOUTH.  
 \*  
 \* \*\*\* DIVERSION DE02 \*\*\*\*\*  
 \*  
 \* See Diversion Calculation in Appendix D of Hydrology Report.  
 \*  
 \* DDM \*\*\*\*\* Preserved \*\*\*\*\*

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LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

403 KK DE02  
 404 DT DE02I  
 405 DI 0 1 50 100 200 300 400 500 1000 2000  
 406 DQ 0 1 20 60 154 254 354 454 954 1954

\*  
 \*  
 \* ROUTE IS A 6FT RCP WITH A SLOPE OF 0.0009 FT/FT.  
 \*

407 KK RE02W  
 408 KO 3  
 409 RD 355 0.0009 0.02 CIRC 6

410 KK E01 BASIN  
 411 BA 0.058  
 412 LG 0.20 0.25 4.80 0.38 60  
 413 UC 0.492 0.494  
 414 UA 0 5.0 16.0 30.0 65.0 77.0 84.0 90.0 94.0 97.0  
 415 UA 100

\*  
 \* DIVERSION RECOVER DE04  
 \*

416 KK DRE04I  
 417 DR DE04I  
 \*  
 \* Route is a Minor Arterial Road.  
 \*

418 KK RE04S  
 419 RS 1 FLOW 0  
 420 RC 0.035 0.015 0.035 2636 0.0026  
 421 RX 0 1 26 27 62 63 88 89  
 422 RY 8 2 1 0.5 0.5 1 2 8

\*  
 \* THE CONTRIBUTING AREA IS HARD-CODED TO ACCOUNT FOR DIVERSIONS DE04 & DE02.  
 \* THE HARD-CODE VALUE IS 100% OF CE04 AND 25% OF CE02'S CONTRIBUTING AREA.  
 \* SEE DIVERSION DE04 AND DE02.  
 \*  
 \* DDM \*\*\*\*\* Preserved \*\*\*\*\*

423 KK CE01  
 424 HC 3 .1656

\*  
 \* THERE IS A DIVERSION SOUTH FOR WHATEVER THE STORM DRAIN DOES NOT INTERCEPT.  
 \*  
 \* THE STORM DRAIN ANALYSIS SHOWS THAT THE MAXIMUM FLOW IN THE DRAIN IS 96 CFS.  
 \* THIS AMOUNT IS DIVIDED BETWEEN DIVERSIONS DE01, DE02, DE03, DE09, AND THE  
 \* BASIN EAST OF THE PROJECT AREA, 76.8 cfs AT THIS LOCATION.  
 \*  
 \* 20% clogging factor means 61cfs (76.8cfs \* 0.8 = 61.4 cfs)  
 \* THIS DIVERSION IS STORMDRAIN ONLY, ALL OVERLAND FLOW IS SOUTH.  
 \*  
 \* \*\*\* DIVERSION DE01 \*\*\*\*\*  
 \*  
 \* See Diversion Calculation in Appendix D of Hydrology Report.  
 \*  
 \* DDM \*\*\*\*\* Preserved \*\*\*\*\*

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LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

425 KK DE01  
 426 DT DE01I  
 427 DI 0 1 50 100 200 300 400 500 1000 2000  
 428 DQ 0 1 20 40 139 239 339 439 939 1939

\*  
 \* DIVERSION RECALL E01  
 \*

429 KK DRE01

```

430      DR  DE01I
      *
      *
      * Route is a Subdivision Road.
      *

431      KK  RE01S
432      RS   9    FLOW      0
433      RC  0.035  0.015  0.035  2567  0.0032
434      RX   0     1     28     29     57     58     86     87
435      RY   8     2     1     0.5   0.5     1     2     8
      *

436      KK  D05  BASIN
437      KM
438      KM  This basin's original area was 0.168 sq mi. The infield area is bermed
439      KM  so it is self-detained. The infield area (0.01 sq mi) was subtracted
440      KM  from the subbasin calculations to account for the storage.
441      KM
442      BA  0.158
443      LG  0.23  0.25  4.80  0.38  51
444      UC  0.692  0.540
445      UA   0     5.0  16.0  30.0  65.0  77.0  84.0  90.0  94.0  97.0
446      UA  100
      *
      *
      * THE CONTRIBUTING AREA WAS HARD-CODED TO ACCOUNT FOR DIVERSION DE01.
      * THE HARD-CODE VALUE IS 70% OF CE01'S CONTRIBUTING AREA.
      * SEE DIVERSION DE01.
      *
      * DDM ***** Preserved *****

447      KK  CD05
448      HC   2    0.284
      *
      * *** DIVERSION DD05 *****
      *
      * THIS DIVERSION IS AT GRANITE REEF ROAD AND OSBORN ROAD.
      * THE MAIN PATH IS SOUTH ON GRANITE REEF ROAD.
      * See Diversion Calculation in Appendix D of Hydrology Report.
      *
      * DDM ***** Preserved *****

```

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LINE  ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

449      KK  DD05
450      DT  DD05I
451      DI   0     5     16     50     93     112     174     217     270     311
452      DQ   0     2     8     25     47     58     95     121     154     194
      *
      *
      * Route is a Minor Arterial Road.
      *

453      KK  RD05S
454      RS   8    FLOW      0
455      RC  0.035  0.015  0.035  2669  0.0026
456      RX   0     1     26     27     62     63     88     89
457      RY   8     2     1     0.5   0.5     1     2     8
      *

458      KK  D04  BASIN
459      BA  0.060
460      LG  0.23  0.25  4.80  0.36  53
461      UC  0.638  0.762
462      UA   0     5.0  16.0  30.0  65.0  77.0  84.0  90.0  94.0  97.0
463      UA  100
      *
      *
      * Route is a Major Arterial Road.
      *

464      KK  RD04
465      RS   2    FLOW      0
466      RC  0.035  0.015  0.035  1084  0.0018
467      RX   0     1     40     41     104     105     145     146
468      RY   8     3     1.5   .75   0.75   1.5     3     8
      *

```

469 KK D01 BASIN  
 470 BA 0.082  
 471 LG 0.24 0.25 4.80 0.36 53  
 472 UC 0.667 0.622  
 473 UA 0 5.0 16.0 30.0 65.0 77.0 84.0 90.0 94.0 97.0  
 474 UA 100

\*  
 \* DDM \*\*\*\*\* Preserved \*\*\*\*\*

475 KK CD01  
 476 HC 3 .2556

\*  
 \*  
 \*  
 \* \*\*\* DIVERSION DD01 \*\*\*\*\*  
 \*  
 \* See Diversion Calculation in Appendix D of Hydrology Report.  
 \*  
 \* DDM \*\*\*\*\* Preserved \*\*\*\*\*

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LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

477 KK DD01  
 478 DT DD01I  
 479 DI 0 9 28 89 168 259 392 475 568 672  
 480 DQ 0 4 13 41 73 104 152 182 217 256

\*  
 \*  
 \* Route is a Major Arterial Road.  
 \*

481 KK RD01E  
 482 RS 1 FLOW 0  
 483 RC 0.035 0.015 0.035 1315 0.0024  
 484 RX 0 1 40 41 104 105 145 146  
 485 RY 8 3 1.5 .75 0.75 1.5 3 8

\*  
 \* DIVERSION RECALL D05  
 \*

486 KK DRD05  
 487 DR DD05I

\*  
 \*  
 \* Route is a Minor Arterial Road.  
 \*  
 \*

488 KK RD05E  
 489 RS 2 FLOW 0  
 490 RC 0.035 0.015 0.035 560 0.0018  
 491 RX 0 1 26 27 62 63 88 89  
 492 RY 8 3 2 0 0 2 3 8

\*  
 \* \*\*\* DIVERSION DD052 \*\*\*\*\*  
 \*  
 \* THIS DIVERSION IS AT GRANITE REEF ROAD AND MULBERRY.  
 \* THE MAIN PATH IS SOUTH ON MULBERRY.  
 \* SEE APPENDIX D FOR DIVERSION CALCULATIONS.  
 \*  
 \* DDM \*\*\*\*\* Preserved \*\*\*\*\*

493 KK DD052  
 494 DT DD052I  
 495 DI 0 37 44 59 80 110 147 178 222 281  
 496 DQ 0 37 40 47 58 74 95 122 155 195

\*  
 \*  
 \* Route is a Subdivision Road. (MULBERRY)  
 \*

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 2

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

497 KK RD05S2  
 498 RS 1 FLOW 0  
 499 RC 0.035 0.015 0.035 3035 0.0030  
 500 RX 0 1 28 29 57 58 86 87  
 501 RY 8 2 1 0.5 0.5 1 2 8  
 \*  
 \*

502 KK D02 BASIN  
 503 BA 0.139  
 504 LG 0.23 0.25 4.80 0.36 51  
 505 UC 0.592 0.457  
 506 UA 0 5.0 16.0 30.0 65.0 77.0 84.0 90.0 94.0 97.0  
 507 UA 100  
 \*  
 \*

\* THE CONTRIBUTING AREA WAS HARD-CODED BECAUSE OF DIVERSION DD01 & DD052.  
 \* THE HARD-CODE VALUE IS 60% OF CD01 AND 40% OF DD052'S CONTRIBUTING AREA.  
 \* SEE DIVERSION DD01 AND DD052.  
 \*  
 \* DDM \*\*\*\*\* Preserved \*\*\*\*\*

508 KK CD02  
 509 HC 3 0.361  
 \*  
 \*  
 \* Route is a Major Arterial Road.  
 \*

510 KK RD02  
 511 RS 2 FLOW 0  
 512 RC 0.035 0.015 0.035 1063 0.0045  
 513 RX 0 1 40 41 104 105 145 146  
 514 RY 8 3 1.5 .75 0.75 1.5 3 8  
 \*  
 \*  
 \* DIVERSION RECALL E02  
 \*

515 KK DRE02  
 516 DR DE02I  
 \*  
 \*  
 \* Route is a Subdivision Road.  
 \*

517 KK RE02S  
 518 RS 6 FLOW 0  
 519 RC 0.035 0.015 0.035 2617 0.0028  
 520 RX 0 1 28 29 57 58 86 87  
 521 RY 8 2 1 0.5 0.5 1 2 8  
 \*  
 \*  
 \* DIVERSION RECALL D052  
 \*

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LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

522 KK DRD052  
 523 DR DD052I  
 \*  
 \*  
 \* Route is a Minor Arterial Road.  
 \*  
 \*

524 KK RD05E2  
 525 RS 1 FLOW 0  
 526 RC 0.035 0.015 0.035 767 0.0013  
 527 RX 0 1 26 27 62 63 88 89  
 528 RY 8 3 2 0 0 2 3 8  
 \*  
 \*

529 KK D06 BASIN  
 530 BA 0.109  
 531 LG 0.24 0.25 4.80 0.35 52

532 UC 0.671 0.602  
 533 UA 0 5.0 16.0 30.0 65.0 77.0 84.0 90.0 94.0 97.0  
 534 UA 100

\*  
 \* THE CUMMALATIVE AREA WAS HARD-CODED TO ACCOUNT FOR DIVERSION DE02 & DD052.  
 \* THE HARD-CODE VALUE IS 80% OF CE02 AND 60% OF DD052'S CONTRIBUTING AREA.  
 \* SEE DIVERSION DE02 AND DD052.  
 \*  
 \* DDM \*\*\*\*\* Preserved \*\*\*\*\*

535 KK CD06  
 536 HC 3 0.358

\*  
 \*  
 \* Route is a Minor Arterial Road.  
 \*  
 \*

537 KK RD06  
 538 RS 1 FLOW 0  
 539 RC 0.035 0.015 0.035 609 0.0033  
 540 RX 0 1 26 27 62 63 88 89  
 541 RY 8 3 2 0 0 2 3 8

542 KK D07 BASIN  
 543 BA 0.049  
 544 LG 0.25 0.25 4.80 0.35 51  
 545 UC 0.508 0.576  
 546 UA 0 5.0 16.0 30.0 65.0 77.0 84.0 90.0 94.0 97.0  
 547 UA 100

\*  
 \*  
 \* DDM \*\*\*\*\* Preserved \*\*\*\*\*

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LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

548 KK CD07  
 549 HC 2

\*  
 \*  
 \* Route is a Minor Arterial Road.  
 \*

550 KK RD07  
 551 RS 8 FLOW 0  
 552 RC 0.035 0.015 0.035 3657 0.003  
 553 RX 0 1 26 27 62 63 88 89  
 554 RY 8 2 1 0.5 0.5 1 2 8

\*  
 \* DIVERSION RECALL DE03  
 \*

555 KK DRE03  
 556 DR DE03I

\*  
 \*  
 \* Route is a Subdivision Road.  
 \*

557 KK RE03S  
 558 RS 6 FLOW 0  
 559 RC 0.035 0.015 0.035 3444 0.0032  
 560 RX 0 1 28 29 57 58 86 87  
 561 RY 8 2 1 0.5 0.5 1 2 8

562 KK D08 BASIN  
 563 BA 0.054  
 564 LG 0.24 0.25 4.80 0.36 53  
 565 UC 0.658 0.955  
 566 UA 0 5.0 16.0 30.0 65.0 77.0 84.0 90.0 94.0 97.0  
 567 UA 100

\*  
 \*  
 \* THE CUMMALATIVE AREA WAS HARD-CODED TO ACCOUNT FOR DIVERSION DE03.

\* THE HARD-CODE VALUE IS 95% OF CE03'S CONTRIBUTING AREA.  
 \* SEE DIVERSION DE03.

\*  
 \* DDM \*\*\*\*\* Preserved \*\*\*\*\*

568 KK CD08  
 569 HC 2 0.877

\*  
 \* Route is a Minor Arterial Road with a wall on the west side.  
 \*

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LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

570 KK RD08  
 571 RS 4 FLOW 0  
 572 RC 0.035 0.015 0.035 2751 0.0037  
 573 RX 0 1 19 25 30 49 79 80  
 574 RY 10 1.5 0 0 0 0 1.5 8

575 KK D03 BASIN  
 576 BA 0.103  
 577 LG 0.20 0.25 4.50 0.45 52  
 578 UC 0.613 0.608  
 579 UA 0 5.0 16.0 30.0 65.0 77.0 84.0 90.0 94.0 97.0  
 580 UA 100

\*  
 \* DIVERSION RECOVER DE09  
 \*

581 KK DRE09I  
 582 DR DE09I

\*  
 \* Route is a Minor Arterial Road along Pima Rd.  
 \*

583 KK RE09S  
 584 RS 7 FLOW 0  
 585 RC 0.035 0.015 0.035 5501 0.0037  
 586 RX 0 1 26 27 62 63 88 89  
 587 RY 8 2 1 0.5 0.5 1 2 8

588 KK D09 BASIN  
 589 BA 0.216  
 590 LG 0.10 0.29 2.65 1.70 80  
 591 UC 0.463 0.379  
 592 UA 0 5.0 16.0 30.0 65.0 77.0 84.0 90.0 94.0 97.0  
 593 UA 100

\*  
 \* \*\*\* STORAGE ROUTING LD09 \*\*\*\*\*  
 \*  
 \* See the detention calculations in the Appendix.  
 \* DDM \*\*\*\*\* Preserved \*\*\*\*\*

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LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

594 KK LD09  
 595 DT LD09D 13.6  
 596 DI 0 100 1000 10000  
 597 DQ 0 100 1000 10000

\*  
 \* THE CONTRIBUTING AREA IS HARD-CODED TO ACCOUNT FOR DIVERSION DE09.  
 \* THE HARD-CODE VALUE IS 90% OF CE09'S CONTRIBUTING AREA.  
 \* SEE DIVERSION DE09.

\*  
\* DDM \*\*\*\*\* Preserved \*\*\*\*\*

598 KK CD09  
599 HC 2 0.686

\*  
\*  
\*  
\*  
\* DDM \*\*\*\*\* Preserved \*\*\*\*\*

600 KK CD03  
601 HC 5

\*  
\*  
\* Route is a Minor Arterial Road.  
\*

602 KK RD03  
603 RS 1 FLOW 0  
604 RC 0.035 0.015 0.035 1307 0.004  
605 RX 0 1 26 27 62 63 88 89  
606 RY 8 2 1 0.5 0.5 1 2 8

607 KK C11 BASIN  
608 BA 0.050  
609 LG 0.25 0.25 4.40 0.45 49  
610 UC 0.358 0.322  
611 UA 0 5.0 16.0 30.0 65.0 77.0 84.0 90.0 94.0 97.0  
612 UA 100

613 KK C10 BASIN  
614 BA 0.054  
615 LG 0.25 0.25 4.65 0.38 50  
616 UC 0.433 0.409  
617 UA 0 5.0 16.0 30.0 65.0 77.0 84.0 90.0 94.0 97.0  
618 UA 100

\*  
\*  
\* Route is a Minor Arterial Road.  
\*

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LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

619 KK RC10  
620 RS 1 FLOW 0  
621 RC 0.035 0.015 0.035 688 0.009  
622 RX 0 1 26 27 62 63 88 89  
623 RY 8 2 1 0.5 0.5 1 2 8

\*  
\*  
\* DDM \*\*\*\*\* Preserved \*\*\*\*\*

624 KK CC11  
625 HC 3

\*  
\*  
\* Route is a Minor Arterial Road.  
\*

626 KK RC11  
627 RS 1 FLOW 0  
628 RC 0.035 0.015 0.035 1334 0.0015  
629 RX 0 1 26 27 62 63 88 89  
630 RY 8 2 1 0.5 0.5 1 2 8

631 KK C08 BASIN  
632 BA 0.118  
633 LG 0.25 0.25 4.40 0.45 51  
634 UC 0.588 0.434  
635 UA 0 5.0 16.0 30.0 65.0 77.0 84.0 90.0 94.0 97.0  
636 UA 100

\*  
\*

\* DDM \*\*\*\*\* Preserved \*\*\*\*\*

637 KK CC08  
638 HC 2  
\*  
\*  
\* Route is a Minor Arterial Road.  
\*

639 KK RC08  
640 RS 4 FLOW 0  
641 RC 0.035 0.015 0.035 2205 0.0026  
642 RX 0 1 26 27 62 63 88 89  
643 RY 8 2 1 0.5 0.5 1 2 8  
\*

644 KK C06 BASIN  
645 KM  
646 KM This basin's original area was 0.162 sq mi. The infield area is bermed  
647 KM so it is self-detained. The infield area (0.0095 sq mi) was subtracted  
648 KM from the subbasin calculations to account for the storage.  
649 KM

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LINE	ID	1	2	3	4	5	6	7	8	9	10
650	BA	0.153									
651	LG	0.23	0.25	4.30	0.49	52					
652	UC	0.554	0.435								
653	UA	0	5.0	16.0	30.0	65.0	77.0	84.0	90.0	94.0	97.0
654	UA	100									

\* DDM \*\*\*\*\* Preserved \*\*\*\*\*

655 KK CC06  
656 HC 2  
\*  
\*  
\* This is a channel.  
\*

657 KK RC06  
658 RS 1 FLOW 0  
659 RC 0.04 0.035 0.04 1271 0.0056  
660 RX 0 1 30 40 70 80 110 111  
661 RY 20 10 8 0 0 8 10 20  
\*

\* DIVERSION RECALL D01

662 KK DRD01  
663 DR DD01I  
\*  
\*  
\* Route is a Minor Arterial Road.  
\*

664 KK RD01S  
665 RS 8 FLOW 0  
666 RC 0.035 0.015 0.035 2611 0.0023  
667 RX 0 1 26 27 62 63 88 89  
668 RY 8 2 1 0.5 0.5 1 2 8  
\*

669 KK C07 BASIN  
670 BA 0.072  
671 LG 0.25 0.25 4.80 0.35 50  
672 UC 0.667 0.699  
673 UA 0 5.0 16.0 30.0 65.0 77.0 84.0 90.0 94.0 97.0  
674 UA 100  
\*

\* THE CONTRIBUTING AREA WAS HARD-CODED TO ACCOUNT FOR THE DIVERSION DD01.  
\* THE HARD-CODE VALUE IS 40% OF CD01'S CONTRIBUTING AREA.  
\* SEE DIVERSION DD01.

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29

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

675 KK CC07  
676 HC 2 .174

\*  
\*  
\*  
\* Route is a Minor Arterial Road.  
\*

677 KK RC07  
678 RS 6 FLOW 0  
679 RC 0.035 0.015 0.035 2638 0.003  
680 RX 0 1 26 27 62 63 88 89  
681 RY 8 2 1 0.5 0.5 1 2 8

\*  
\*

682 KK C09 BASIN  
683 BA 0.057  
684 LG 0.25 0.25 4.80 0.36 50  
685 UC 0.596 0.646  
686 UA 0 5.0 16.0 30.0 65.0 77.0 84.0 90.0 94.0 97.0  
687 UA 100

\*  
\*  
\* Route is a Minor Arterial Road.  
\*

688 KK RC09  
689 RS 1 FLOW 0  
690 RC 0.035 0.015 0.035 3368 0.003  
691 RX 0 1 26 27 62 63 88 89  
692 RY 8 2 1 0 0 1 2 8

\*

693 KK C05 BASIN  
694 BA 0.137  
695 LG 0.23 0.25 4.70 0.37 54  
696 UC 0.754 0.675  
697 UA 0 5.0 16.0 30.0 65.0 77.0 84.0 90.0 94.0 97.0  
698 UA 100

\*

699 KK C01 BASIN  
700 BA 0.032  
701 LG 0.22 0.25 4.80 0.37 57  
702 UC 0.546 0.711  
703 UA 0 5.0 16.0 30.0 65.0 77.0 84.0 90.0 94.0 97.0  
704 UA 100

\*  
\*  
\*  
\* Route is a Major Arterial Road.  
\*

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30

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

705 KK RC01  
706 RS 8 FLOW 0  
707 RC 0.035 0.015 0.035 1217 0.0013  
708 RX 0 1 40 41 104 105 145 146  
709 RY 8 3 1.5 .75 0.75 1.5 3 8

\*

710 KK C02 BASIN  
711 BA 0.036  
712 LG 0.15 0.25 4.80 0.40 70  
713 UC 0.554 0.780  
714 UA 0 5.0 16.0 30.0 65.0 77.0 84.0 90.0 94.0 97.0  
715 UA 100

\*

\* DDM \*\*\*\*\* Preserved \*\*\*\*\*

716 KK CC02  
717 HC 5 .436

\*  
\* Route is a Major Arterial Road.  
\*

718 KK RC02  
719 RS 1 FLOW 0  
720 RC 0.035 0.015 0.035 632 0.0063  
721 RX 0 1 40 45 100 105 145 146  
722 RY 8 3 2 0 0 2 3 8

723 KK C03 BASIN  
724 BA 0.040  
725 LG 0.18 0.25 4.10 0.61 62  
726 UC 0.333 0.388  
727 UA 0 5.0 16.0 30.0 65.0 77.0 84.0 90.0 94.0 97.0  
728 UA 100

729 KK C12 BASIN  
730 BA 0.010  
731 LG 0.19 0.25 4.10 0.58 64  
732 UC 1.500 4.505  
733 UA 0 5.0 16.0 30.0 65.0 77.0 84.0 90.0 94.0 97.0  
734 UA 100

\*  
\* Route is a Minor Arterial Road with a wall on the west side.  
\*

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LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

735 KK RC12  
736 RS 22 FLOW 0  
737 RC 0.035 0.015 0.035 2693 0.0007  
738 RX 0 1 19 25 30 49 79 80  
739 RY 10 1.5 0 0 0 0 1.5 8

740 KK C13 BASIN  
741 BA 0.176  
742 LG 0.10 0.35 2.65 1.70 80  
743 UC 0.546 0.508  
744 UA 0 5.0 16.0 30.0 65.0 77.0 84.0 90.0 94.0 97.0  
745 UA 100

\* \*\*\* STORAGE ROUTING LC13 \*\*\*\*\*

\* See the detention calculations in the Appendix.  
\* DDM \*\*\*\*\* Preserved \*\*\*\*\*

746 KK LC13  
747 DT LC13D 11.2  
748 DI 0 100 1000 10000  
749 DQ 0 100 1000 10000

\* DDM \*\*\*\*\* Preserved \*\*\*\*\*

750 KK CC13  
751 HC 2

\*  
\* \*\*\* DIVERSION DC13 \*\*\*\*\*  
\* THE STORM DRAIN ANALYSIS SHOWS THAT THE MAXIMUM FLOW IN THE DRAIN IS 51 CFS.  
\* ALL OVERLAND FLOW CONTINUES SOUTH.  
\* 20% clogging factor means 41cfs, (51cfs \* 0.8 = 41cfs).  
\*  
\* See Diversion Calculation in Appendix D of Hydrology Report.

\*  
\* DDM \*\*\*\*\* Preserved \*\*\*\*\*

752 KK DC13  
753 DT DC13I  
754 DI 0 10 50 100 200 300 400 500  
755 DQ 0 4 30 59 159 259 359 459

\*  
\* ROUTE IS A 42" RCP WITH A SLOPE OF 0.0044 FT/FT.  
\*

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LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

756 KK RC13W  
757 RS 5 FLOW -1  
758 RD 2052 0.005 0.012 CIRC 3.5

759 KK C04 BASIN  
760 BA 0.044  
761 LG 0.18 0.28 4.20 0.51 44  
762 UC 0.650 0.830  
763 UA 0 5.0 16.0 30.0 65.0 77.0 84.0 90.0 94.0 97.0  
764 UA 100

\*  
\* Route is a Major Arterial Road.  
\*

765 KK RC04  
766 RS 5 FLOW 0  
767 RC 0.035 0.015 0.035 1563 0.0106  
768 RX 0 1 40 41 104 105 145 146  
769 RY 8 3 1.5 0 0 1.5 3 8

\* THE CONTRIBUTING AREA IS HARD-CODED TO ACCOUNT FOR DIVERSION D13.  
\* THE HARD-CODE VALUE IS 20% OF CC13'S CONTRIBUTING AREA.  
\* SEE DIVERSION D13.

\* DDM \*\*\*\*\* Preserved \*\*\*\*\*

770 KK CC03  
771 HC 5 3.37

\*  
\* This route is a channel.  
\*

772 KK RC03  
773 RS 2 FLOW 0  
774 RC 0.04 0.03 0.04 1097 0.0009  
775 RX 0 1 30 40 70 80 110 111  
776 RY 20 10 8 0 0 8 10 20

777 KK B07 BASIN  
778 BA 0.016  
779 LG 0.13 0.28 4.35 0.51 49  
780 UC 0.192 0.234  
781 UA 0 5.0 16.0 30.0 65.0 77.0 84.0 90.0 94.0 97.0  
782 UA 100

\* DDM \*\*\*\*\* Preserved \*\*\*\*\*

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LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

783 KK CB07  
784 HC 2

\*  
\* Route is a Minor Arterial Road.

```

*
785      KK      RB07
786      RS      2      FLOW      0
787      RC      0.035  0.015  0.035  1525  0.0026
788      RX      0      1      40      41      104      105      145      146
789      RY      8      3      1.5      .75  0.75  1.5      3      8
*

```

```

790      KK      B05      BASIN
791      BA      0.088
792      LG      0.22      0.26      4.15      0.58      44
793      UC      0.583  0.685
794      UA      0      5.0      16.0      30.0      65.0      77.0      84.0      90.0      94.0      97.0
795      UA      100
*

```

\* \*\*\* STORAGE ROUTING LB05 \*\*\*\*\*

\* DDM \*\*\*\*\* Preserved \*\*\*\*\*

```

796      KK      LB05
797      DT      LB05D  0.16
798      DI      0      100      1000  10000
799      DQ      0      100      1000  10000
*

```

```

800      KK      B06      BASIN
801      BA      0.015
802      LG      0.13      0.26      4.00      0.64      60
803      UC      0.304  0.589
804      UA      0      5.0      16.0      30.0      65.0      77.0      84.0      90.0      94.0      97.0
805      UA      100
*

```

\* DDM \*\*\*\*\* Preserved \*\*\*\*\*

```

806      KK      CB05
807      HC      3
*

```

\* Route is a Minor Arterial Road.  
\* Normal Depth Routing did not work for this Route.  
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LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

```

808      KK      RB05
809      RD
810      RC      0.035  0.02  0.035  500  0.0052
811      RX      0      1      40      51      94      105      145      146
812      RY      8      3      2      0      0      2      3      8
*

```

```

813      KK      B04      BASIN
814      BA      0.077
815      LG      0.15      0.25      4.45      0.50      71
816      UC      0.271  0.232
817      UA      0      5.0      16.0      30.0      65.0      77.0      84.0      90.0      94.0      97.0
818      UA      100
*

```

\* \*\*\* STORAGE ROUTING LB04 \*\*\*\*\*

\* MOTOROLA DETENTION BASINS (SEE CALCULATIONS)

\* DDM \*\*\*\*\* Preserved \*\*\*\*\*

```

819      KK      LB04
820      DT      LB04D  12.95
821      DI      0      100      1000  10000
822      DQ      0      100      1000  10000
*

```

```

823      KK      B02      BASIN
824      BA      0.101
825      LG      0.15      0.25      4.70      0.41      68
826      UC      0.363  0.335
827      UA      0      5.0      16.0      30.0      65.0      77.0      84.0      90.0      94.0      97.0

```

828 UA 100  
 \*  
 \*  
 \* \*\*\* STORAGE ROUTING LB02 \*\*\*\*\*  
 \*  
 \* DDM \*\*\*\*\* Preserved \*\*\*\*\*

829 KK LB02  
 830 DT LB02D 2.64  
 831 DI 0 100 1000 10000  
 832 DQ 0 100 1000 10000  
 \*  
 \*  
 \*  
 \* Route is a Minor Arterial Road.  
 \* Normal Depth Routing did not work for this Route.

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LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

833 KK RB02  
 834 RD  
 835 RC 0.035 0.015 0.035 548 0.0186  
 836 RX 0 1 40 41 104 105 145 146  
 837 RY 8 3 1.5 .75 0.75 1.5 3 8  
 \*

838 KK B03 BASIN  
 839 BA 0.007  
 840 LG 0.25 0.35 4.65 0.41 47  
 841 UC 0.271 0.474  
 842 UA 0 5.0 16.0 30.0 65.0 77.0 84.0 90.0 94.0 97.0  
 843 UA 100  
 \*  
 \*

\* DDM \*\*\*\*\* Preserved \*\*\*\*\*

844 KK CB03  
 845 HC 4 3.66  
 \*  
 \*  
 \*  
 \* Route is a Minor Arterial Road.  
 \*

846 KK RB03  
 847 KO 21  
 848 RS 2 FLOW 0  
 849 RC 0.035 0.015 0.035 1348 0.003  
 850 RX 0 1 40 41 104 105 145 146  
 851 RY 8 3 1.5 .75 0.75 1.5 3 8  
 \*

852 KK B01 BASIN  
 853 BA 0.071  
 854 LG 0.25 0.25 4.30 0.54 44  
 855 UC 0.267 0.188  
 856 UA 0 5.0 16.0 30.0 65.0 77.0 84.0 90.0 94.0 97.0  
 857 UA 100  
 \*  
 \*

\* \*\*\* STORAGE ROUTING LB01 \*\*\*\*\*

\* DDM \*\*\*\*\* Preserved \*\*\*\*\*

858 KK LB01  
 859 DT LB01D 1.15  
 860 DI 0 100 1000 10000  
 861 DQ 0 100 1000 10000  
 \*  
 \*  
 \*

\* DDM \*\*\*\*\* Preserved \*\*\*\*\*

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LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

862 KK CB01  
 863 HC 2  
 \*  
 \*  
 \* Route is a channel.  
 \*

864 KK RB01  
 865 KO 21  
 866 RS 1 FLOW 0  
 867 RC 0.035 0.015 0.035 1320 0.0019  
 868 RX 0 1 34 40 50 56 80 81  
 869 RY 12 6 4 0 0 4 6 12  
 \*

870 KK B12 BASIN  
 871 BA 0.119  
 872 LG 0.25 0.25 4.80 0.38 39  
 873 UC 0.442 0.233  
 874 UA 0 5.0 16.0 30.0 65.0 77.0 84.0 90.0 94.0 97.0  
 875 UA 100  
 \*

876 KK B11 BASIN  
 877 BA 0.060  
 878 LG 0.24 0.34 4.80 0.39 37  
 879 UC 0.400 0.277  
 880 UA 0 5.0 16.0 30.0 65.0 77.0 84.0 90.0 94.0 97.0  
 881 UA 100  
 \*

\* \*\*\* STORAGE ROUTING LB11 \*\*\*\*\*  
 \* Pond was assumed to have 1 foot of freeboard.  
 \* DDM \*\*\*\*\* Preserved \*\*\*\*\*

882 KK LB11  
 883 DT LB11D 2.87  
 884 DI 0 100 1000 10000  
 885 DQ 0 100 1000 10000  
 \*

\* DIVERSION RECALL DC13I  
 \*

886 KK DRDC13  
 887 DR DC13I  
 \*  
 \*  
 \* Route is overland flow. (SLOPE ESTIMATED.)  
 \*

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LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

888 KK RC13S  
 889 RS 33 FLOW 0  
 890 RC 0.035 0.035 0.035 5197 .0058  
 891 RX 0 200 300 400 500 600 700 900  
 892 RY 5 4 3 0 0 3 4 5  
 \*

893 KK B13 BASIN  
 894 BA 0.222  
 895 LG 0.22 0.26 4.55 0.42 44  
 896 UC 0.375 0.232  
 897 UA 0 5.0 16.0 30.0 65.0 77.0 84.0 90.0 94.0 97.0  
 898 UA 100  
 \*

\* \*\*\* STORAGE ROUTING LB13 \*\*\*\*\*  
 \*  
 \* DDM \*\*\*\*\* Preserved \*\*\*\*\*

899 KK LB13  
 900 DT LB13D 18.4  
 901 DI 0 100 1000 10000  
 902 DQ 0 100 1000 10000  
 \*

\* Route is shallow flow. (SLOPE WAS ESTIMATED.)  
 \*

903 KK RB13  
 904 RS 7 FLOW 0  
 905 RC 0.035 0.035 0.035 1269 0.004  
 906 RX 0 200 300 400 500 600 700 900  
 907 RY 5 4 3 0 0 3 4 5  
 \*

908 KK B14 BASIN  
 909 BA 0.149  
 910 LG 0.10 0.25 4.60 0.48 80  
 911 UC 0.608 0.628  
 912 UA 0 5.0 16.0 30.0 65.0 77.0 84.0 90.0 94.0 97.0  
 913 UA 100  
 \*

\* \*\*\* STORAGE ROUTING LB14 \*\*\*\*\*  
 \*  
 \* See the detention calculations in the Appendix.  
 \* DDM \*\*\*\*\* Preserved \*\*\*\*\*

914 KK LB14  
 915 DT LB14D 10.4  
 916 DI 0 100 1000 10000  
 917 DQ 0 100 1000 10000  
 \*

\* DDM \*\*\*\*\* Preserved \*\*\*\*\*

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LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

918 KK CB14  
 919 HC 3 0.524  
 \*

\* Route is a MAJOR ARTERIAL. (SLOPE WAS ESTIMATED.)  
 \*

920 KK RB14  
 921 RS 4 FLOW 0  
 922 RC 0.035 0.015 0.035 2746 0.004  
 923 RX 0 1 30 40 100 110 145 300  
 924 RY 8 4 3 0 0 3 4 5  
 \*

\* DDM \*\*\*\*\* Preserved \*\*\*\*\*

925 KK CB12  
 926 HC 4  
 \*

\* Route is a channel. (SLOPE ESTIMATED FROM UPSTREAM CHANNEL'S SLOPE.)  
 \*

927 KK RB12  
 928 KO 3  
 929 RS 6 FLOW 0  
 930 RC 0.035 .035 0.035 4395 0.0019  
 931 RX 0 200 330 340 370 380 410 600  
 932 RY 9 8 6 0 0 6 8 9  
 \*

933 KK A01 BASIN  
 934 BA 0.507  
 935 LG 0.10 0.25 5.40 0.32 80  
 936 UC 0.542 0.238  
 937 UA 0 5.0 16.0 30.0 65.0 77.0 84.0 90.0 94.0 97.0  
 938 UA 100  
 \*

\* \*\*\* STORAGE ROUTING LA01 \*\*\*\*\*  
 \*  
 \* See the detention calculations in the Appendix.  
 \* DDM \*\*\*\*\* Preserved \*\*\*\*\*

939 KK LA01  
 940 DT LA01D 35.2  
 941 DI 0 100 1000 10000  
 942 DQ 0 100 1000 10000  
 \*

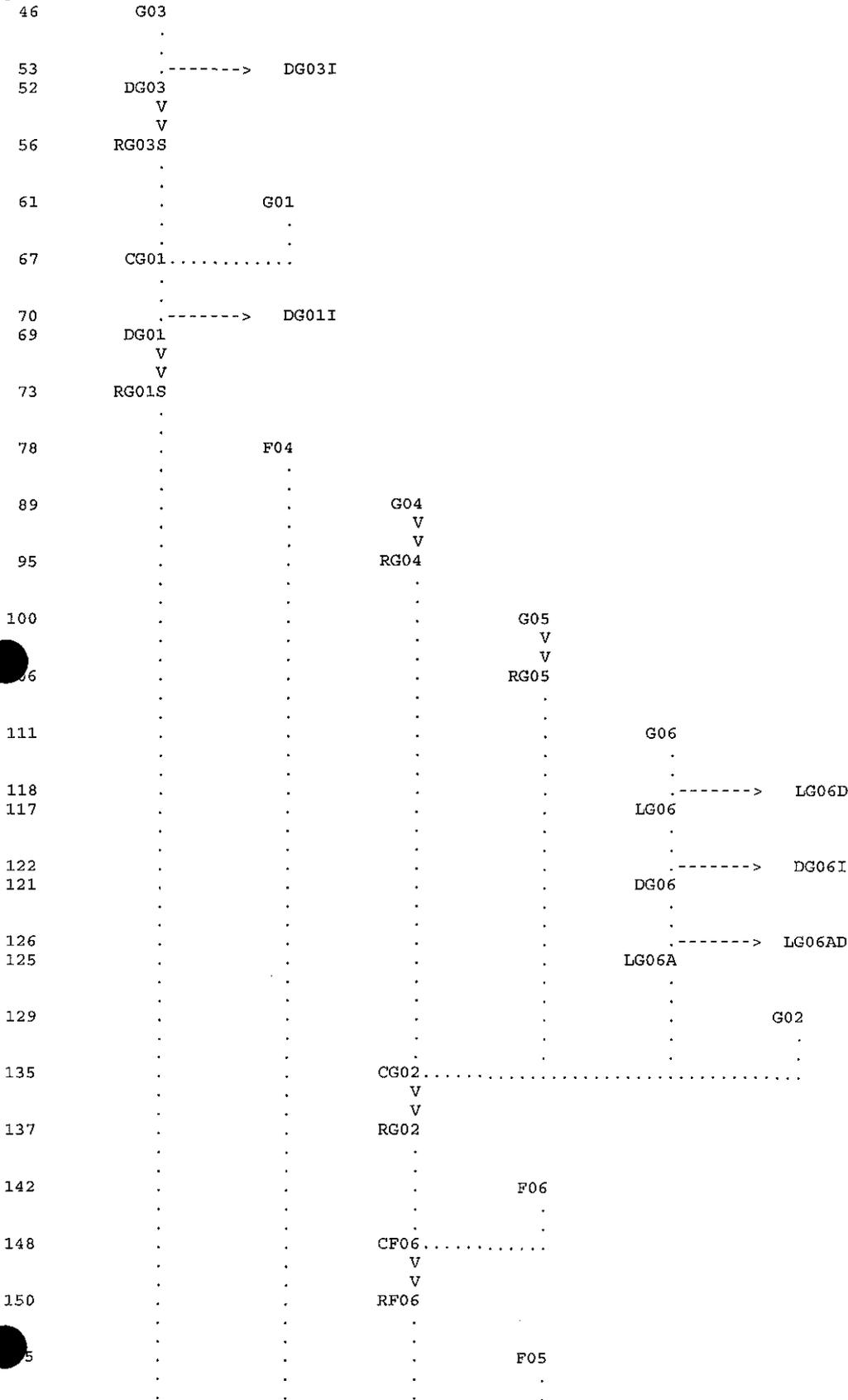
\* DDM \*\*\*\*\* Preserved \*\*\*\*\*

LINE	ID	1	2	3	4	5	6	7	8	9	10
943	KK	CA01									
944	HC	2									
	*										
945	KK	B10	BASIN								
946	BA	0.110									
947	LG	0.21	0.25	4.60	0.49	28					
948	UC	0.742	0.765								
949	UA	0	5.0	16.0	30.0	65.0	77.0	84.0	90.0	94.0	97.0
950	UA	100									
	*										
	*	*** STORAGE ROUTING LB10 *****									
	*										
	*	DDM ***** Preserved *****									
951	KK	LB10									
952	DT	LB10D	10.5								
953	DI	0	100	1000	10000						
954	DQ	0	100	1000	10000						
	*										
955	ZZ										

SCHEMATIC DIAGRAM OF STREAM NETWORK

INPUT  
LINE

(V) ROUTING                    (--->) DIVERSION OR PUMP FLOW  
(.) CONNECTOR                (<---) RETURN OF DIVERTED OR PUMPED FLOW



```

161 . . . . . CF05.....
. . . . .
. . . . .
164 . . . . . <-----> DF05I
163 . . . . . DF05
. . . . . V
. . . . . V
167 . . . . . RF05W
. . . . .
. . . . .
170 CF04.....
. . . . .
. . . . .
173 <-----> DF04I
172 DF04
. . . . .
. . . . .
177 . . . . . <-----> DF05I
176 . . . . . F05R
. . . . . V
. . . . . V
178 . . . . . RF05S
. . . . .
. . . . .
183 . . . . . F03
. . . . .
. . . . .
189 . . . . . CF03.....
. . . . . V
. . . . . V
191 . . . . . RF03
. . . . .
. . . . .
197 . . . . . <-----> DG06I
196 . . . . . G06R
. . . . . V
. . . . . V
198 . . . . . RG06S
. . . . .
. . . . .
203 . . . . . F07
. . . . .
. . . . . <-----> LF07D
210 . . . . .
209 . . . . . LF07
. . . . .
. . . . .
213 . . . . . CF07.....
. . . . .
. . . . .
216 . . . . . <-----> DF07I
215 . . . . . DF07
. . . . . V
. . . . . V
219 . . . . . RF07W
. . . . .
. . . . .
222 . . . . . F02
. . . . .
. . . . .
228 . . . . . CF02.....
. . . . .
. . . . .
231 . . . . . <-----> DF02I
230 . . . . . DF02
. . . . . V
. . . . . V
234 . . . . . RF02W
. . . . .
. . . . .
238 . . . . . <-----> DF04I
237 . . . . . F04R
. . . . . V
. . . . . V
239 . . . . . RF04S
. . . . .
. . . . .
. . . . . <-----> DRF04I
. . . . . DRF04
. . . . .
. . . . .

```

```

248 . . . . . F01
. . . . .
254 CF01 .....
. . . . .
257 -----> DF01I
256 DF01
. . . . .
261 . . . . . <----- DF02I
260 DRF02
. . . . . V
. . . . . V
262 RF02S
. . . . .
267 . . . . . E07
. . . . .
273 CE07 .....
. . . . . V
. . . . . V
275 RE07
. . . . .
280 . . . . . E05
. . . . .
291 CE05 .....
. . . . .
294 -----> DE05I
293 DE05
. . . . . V
. . . . . V
297 RE05W
. . . . .
299 . . . . . E04
. . . . .
306 -----> LE04D
305 LE04
. . . . .
310 . . . . . <----- DF01I
309 . . . . . DRF01I
. . . . . V
. . . . . V
311 RF01S
. . . . .
316 CE04 .....
. . . . .
319 -----> DE04I
318 DE04
. . . . .
322 DUMMY .....
. . . . .
324 E08
. . . . . V
. . . . . V
330 RE08
. . . . .
335 . . . . . E06
. . . . .
341 CE06 .....
. . . . . V
. . . . . V
343 RE06
. . . . .
. . . . . E03
. . . . .

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```

355 . . . . . <----- DF07I
354 . . . . . DRF07
. . . . . V
. . . . . V
356 . . . . . RF07S
. . . . .
. . . . . E09
361 . . . . .
. . . . .
. . . . . <-----> LE09D
368 . . . . .
367 . . . . . LE09
. . . . .
. . . . .
371 . . . . . CE09.....
. . . . .
. . . . . <-----> DE09I
374 . . . . .
373 . . . . . DE09
. . . . . V
. . . . . V
377 . . . . . RE09W
. . . . .
. . . . . <-----> DE05I
381 . . . . . DRE05I
380 . . . . . V
. . . . . V
382 . . . . . RE05S
. . . . .
. . . . .
387 . . . . . CE03.....
. . . . .
. . . . . <-----> DE03I
390 . . . . .
389 . . . . . DE03
. . . . . V
. . . . . V
393 . . . . . RE03W
. . . . .
. . . . . E02
. . . . .
. . . . .
401 . . . . . CE02.....
. . . . .
. . . . . <-----> DE02I
404 . . . . .
403 . . . . . DE02
. . . . . V
. . . . . V
407 . . . . . RE02W
. . . . .
. . . . . E01
. . . . .
. . . . . <-----> DE04I
417 . . . . . DRE04I
416 . . . . . V
. . . . . V
418 . . . . . RE04S
. . . . .
. . . . .
423 . . . . . CE01.....
. . . . .
. . . . . <-----> DE01I
426 . . . . .
425 . . . . . DE01
. . . . .
. . . . . <-----> DE01I
430 . . . . . DRE01
429 . . . . . V
. . . . . V
431 . . . . . RE01S
. . . . .
. . . . .
. . . . . D05
. . . . .
. . . . .

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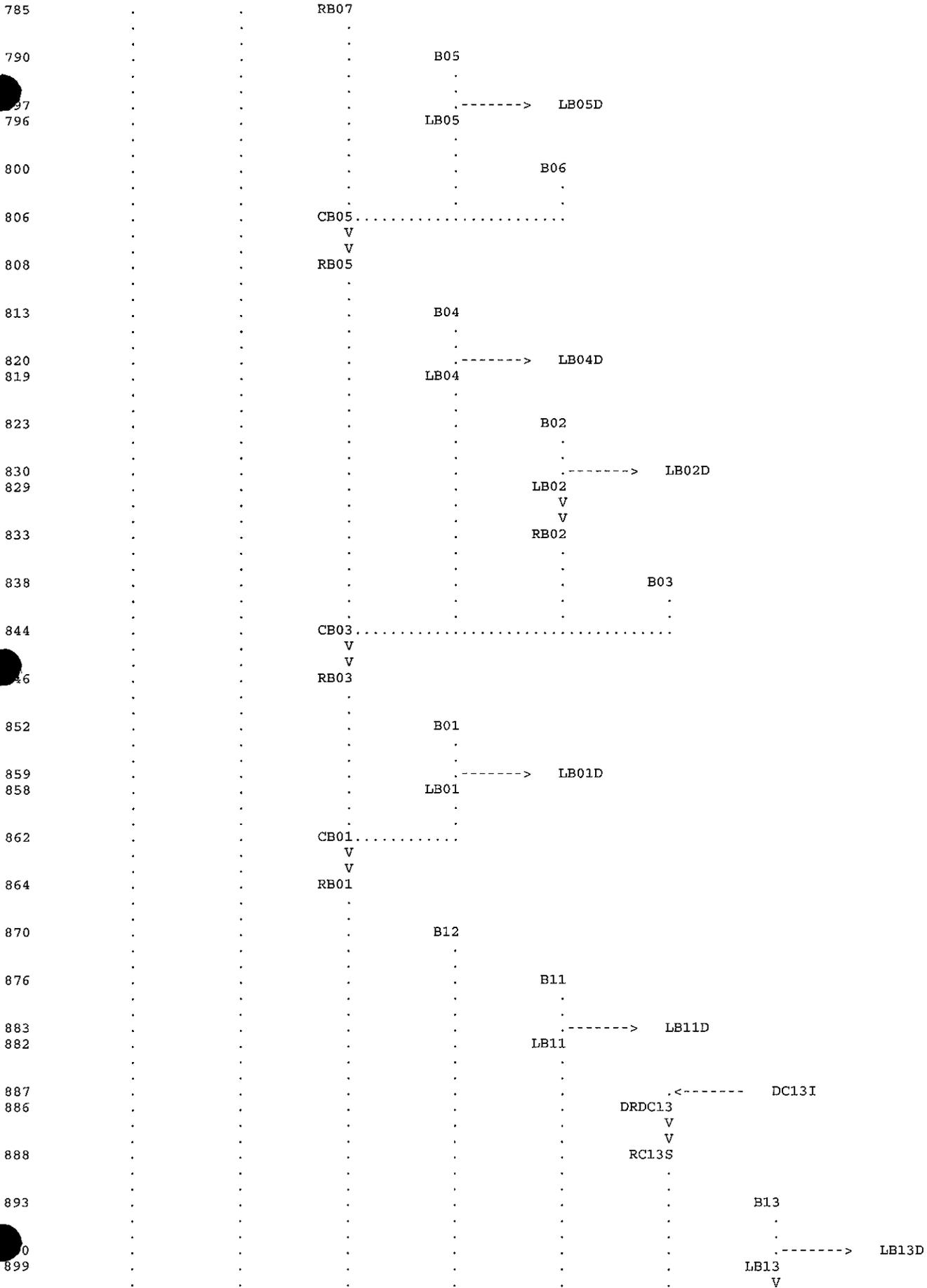




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664 . . . . . V
RD01S
. . . . .
669 . . . . . C07
. . . . .
675 . . . . . CC07.....
. . . . . V
677 . . . . . V
RC07
. . . . .
682 . . . . . C09
. . . . . V
688 . . . . . V
RC09
. . . . .
693 . . . . . C05
. . . . .
699 . . . . . C01
. . . . . V
705 . . . . . V
RC01
. . . . .
710 . . . . . C02
. . . . .
716 . . . . . CC02.....
. . . . . V
718 . . . . . V
RC02
. . . . .
723 . . . . . C03
. . . . .
729 . . . . . C12
. . . . . V
735 . . . . . V
RC12
. . . . .
740 . . . . . C13
. . . . .
747 . . . . . LC13D
746 . . . . . LC13----->
. . . . .
750 . . . . . CC13.....
. . . . .
753 . . . . . DC13I
752 . . . . . DC13----->
. . . . . V
756 . . . . . V
RC13W
. . . . .
759 . . . . . C04
. . . . . V
765 . . . . . V
RC04
. . . . .
770 . . . . . CC03.....
. . . . . V
772 . . . . . V
RC03
. . . . .
777 . . . . . B07
. . . . .
. . . . . CB07.....
. . . . . V
. . . . . V

```



```

903      . . . . . V
          . . . . . RB13
          . . . . .
908      . . . . . B14
          . . . . .
915      . . . . .
914      . . . . . -----> LB14D
          . . . . . LB14
          . . . . .
918      . . . . . CB14.....
          . . . . . V
          . . . . . V
920      . . . . . RB14
          . . . . .
925      . . . . . CB12.....
          . . . . . V
          . . . . . V
927      . . . . . RB12
          . . . . .
933      . . . . . A01
          . . . . .
940      . . . . . -----> LA01D
939      . . . . . LA01
          . . . . .
943      . . . . . CA01.....
          . . . . .
945      . . . . . B10
          . . . . .
952      . . . . . -----> LB10D
951      . . . . . LB10

```

(\*\*\*) RUNOFF ALSO COMPUTED AT THIS LOCATION

RUNOFF SUMMARY  
FLOW IN CUBIC FEET PER SECOND  
TIME IN HOURS, AREA IN SQUARE MILES

OPERATION	STATION	PEAK FLOW	TIME OF PEAK	AVERAGE FLOW FOR MAXIMUM PERIOD			BASIN AREA	MAXIMUM STAGE
				6-HOUR	24-HOUR	72-HOUR		
HYDROGRAPH AT								
	G03	188.	4.17	25.	6.	2.	0.09	
DIVERSION TO								
	DG03I	96.	4.17	13.	3.	1.	0.09	
HYDROGRAPH AT								
	DG03	92.	4.17	12.	3.	1.	0.09	
ROUTED TO								
	RG03S	84.	4.25	12.	3.	1.	0.09	
HYDROGRAPH AT								
	G01	62.	4.25	13.	3.	1.	0.05	
2 COMBINED AT								
	CG01	146.	4.25	25.	6.	2.	0.10	
DIVERSION TO								
	DG01I	104.	4.25	18.	5.	2.	0.10	
HYDROGRAPH AT								
	DG01	42.	4.25	7.	2.	1.	0.10	
ROUTED TO								
	RG01S	39.	4.67	7.	2.	1.	0.10	
HYDROGRAPH AT								
	F04	74.	4.25	17.	4.	1.	0.06	
HYDROGRAPH AT								
	G04	94.	4.08	11.	3.	1.	0.05	
ROUTED TO								
	RG04	92.	4.17	11.	3.	1.	0.05	
HYDROGRAPH AT								
	G05	96.	4.08	13.	3.	1.	0.05	
ROUTED TO								
	RG05	95.	4.25	13.	3.	1.	0.05	
HYDROGRAPH AT								
	G06	333.	4.08	37.	9.	3.	0.13	
DIVERSION TO								
	LG06D	226.	3.92	16.	4.	1.	0.13	
HYDROGRAPH AT								
	LG06	333.	4.08	21.	5.	2.	0.13	
DIVERSION TO								
	DG06I	142.	4.08	9.	2.	1.	0.13	
HYDROGRAPH AT								
	DG06	191.	4.08	12.	3.	1.	0.13	
DIVERSION TO								
	LG06AD	191.	4.08	9.	2.	1.	0.13	
HYDROGRAPH AT								
	LG06A	43.	4.42	3.	1.	0.	0.13	
HYDROGRAPH AT								
	G02	100.	4.25	19.	5.	2.	0.08	
4 COMBINED AT								
	CG02	283.	4.25	46.	12.	4.	0.23	
ROUTED TO								
	RG02	283.	4.33	46.	12.	4.	0.23	

	OPERATION	STATION	PEAK FLOW	TIME OF PEAK	AVERAGE FLOW FOR MAXIMUM PERIOD			BASIN AREA	MAXIMUM STAGE
					6-HOUR	24-HOUR	72-HOUR		
+	HYDROGRAPH AT	F06	151.	4.25	26.	6.	2.	0.10	
+	2 COMBINED AT	CF06	427.	4.33	71.	18.	6.	0.33	
+	ROUTED TO	RF06	425.	4.33	71.	18.	6.	0.33	
+	HYDROGRAPH AT	F05	137.	4.25	22.	6.	2.	0.09	
+	2 COMBINED AT	CF05	552.	4.33	94.	24.	8.	0.42	
+	DIVERSION TO	DF05I	503.	4.33	68.	17.	6.	0.42	
+	HYDROGRAPH AT	DF05	49.	3.92	25.	6.	2.	0.42	
+	ROUTED TO	RF05W	49.	4.08	25.	6.	2.	0.42	
+	3 COMBINED AT	CF04	147.	4.50	49.	12.	4.	0.17	
+	DIVERSION TO	DF04I	80.	4.50	26.	7.	2.	0.17	
+	HYDROGRAPH AT	DF04	67.	4.50	23.	6.	2.	0.17	
+	HYDROGRAPH AT	F05R	503.	4.33	68.	17.	6.	0.42	
+	ROUTED TO	RF05S	492.	4.42	68.	17.	6.	0.42	
+	HYDROGRAPH AT	F03	243.	4.25	45.	11.	4.	0.17	
+	2 COMBINED AT	CF03	705.	4.42	112.	28.	9.	0.56	
+	ROUTED TO	RF03	700.	4.42	112.	28.	9.	0.56	
+	HYDROGRAPH AT	G06R	142.	4.08	9.	2.	1.	0.13	
+	ROUTED TO	RG06S	127.	4.42	9.	2.	1.	0.13	
+	HYDROGRAPH AT	F07	392.	4.25	72.	18.	6.	0.25	
+	DIVERSION TO	LF07D	381.	4.17	31.	8.	3.	0.25	
+	HYDROGRAPH AT	LF07	370.	4.33	42.	10.	3.	0.25	
+	2 COMBINED AT	CF07	462.	4.42	51.	13.	4.	0.31	
+	DIVERSION TO	DF07I	449.	4.42	45.	11.	4.	0.31	
+	HYDROGRAPH AT	DF07	16.	6.33	6.	2.	1.	0.31	
+	ROUTED TO	RF07W	16.	6.33	6.	2.	1.	0.31	
	HYDROGRAPH AT								

100-year, 6-hour HEC-1 (Future Conditions)

	OPERATION	STATION	PEAK FLOW	TIME OF PEAK	AVERAGE FLOW FOR MAXIMUM PERIOD			BASIN AREA	MAXIMUM STAGE
					6-HOUR	24-HOUR	72-HOUR		
+		F02	62.	4.25	12.	3.	1.	0.05	
	3 COMBINED AT								
+		CF02	751.	4.42	129.	33.	11.	0.63	
	DIVERSION TO								
+		DF02I	701.	4.42	102.	26.	9.	0.63	
	HYDROGRAPH AT								
+		DF02	50.	3.92	27.	7.	2.	0.63	
	ROUTED TO								
+		RF02W	50.	4.00	27.	7.	2.	0.63	
	HYDROGRAPH AT								
+		F04R	80.	4.50	26.	7.	2.	0.17	
	ROUTED TO								
+		RF04S	80.	4.67	26.	7.	2.	0.17	
	DIVERSION TO								
+		DRF04I	40.	4.67	13.	3.	1.	0.17	
	HYDROGRAPH AT								
+		DRF04	40.	4.67	13.	3.	1.	0.17	
	HYDROGRAPH AT								
+		F01	42.	4.25	8.	2.	1.	0.03	
	3 COMBINED AT								
+		CF01	127.	4.25	48.	12.	4.	0.10	
	DIVERSION TO								
+		DF01I	3.	4.25	0.	0.	0.	0.10	
	HYDROGRAPH AT								
+		DF01	123.	4.25	48.	12.	4.	0.10	
	HYDROGRAPH AT								
+		DRF02	701.	4.42	102.	26.	9.	0.63	
	ROUTED TO								
+		RF02S	699.	4.50	102.	26.	9.	0.63	
	HYDROGRAPH AT								
+		E07	153.	4.17	21.	5.	2.	0.08	
	2 COMBINED AT								
+		CE07	773.	4.42	122.	31.	10.	0.66	
	ROUTED TO								
+		RE07	771.	4.50	122.	31.	10.	0.66	
	HYDROGRAPH AT								
+		E05	91.	4.17	12.	3.	1.	0.05	
	2 COMBINED AT								
+		CE05	804.	4.50	133.	33.	11.	0.71	
	DIVERSION TO								
+		DE05I	779.	4.50	120.	30.	10.	0.71	
	HYDROGRAPH AT								
+		DE05	25.	3.83	13.	3.	1.	0.71	
	ROUTED TO								
+		RE05W	25.	3.92	13.	3.	1.	0.71	
	HYDROGRAPH AT								
+		E04	42.	4.25	9.	2.	1.	0.04	
	DIVERSION TO								
+		LE04D	40.	4.25	3.	1.	0.	0.04	
	HYDROGRAPH AT								
+		LE04	42.	4.33	6.	2.	1.	0.04	

100-year, 6-hour HEC-1 (Future Conditions)

OPERATION	STATION	PEAK FLOW	TIME OF PEAK	AVERAGE FLOW FOR MAXIMUM PERIOD			BASIN AREA	MAXIMUM STAGE
				6-HOUR	24-HOUR	72-HOUR		
HYDROGRAPH AT	DRF01I	3.	4.25	0.	0.	0.	0.10	
ROUTED TO	RF01S	3.	4.58	0.	0.	0.	0.10	
3 COMBINED AT	CE04	68.	4.33	19.	5.	2.	0.07	
DIVERSION TO	DE04I	0.	0.08	0.	0.	0.	0.07	
HYDROGRAPH AT	DE04	68.	4.33	19.	5.	2.	0.07	
3 COMBINED AT	DUMMY	255.	4.33	89.	23.	8.	0.34	
HYDROGRAPH AT	E08	42.	4.25	8.	2.	1.	0.03	
ROUTED TO	RE08	41.	4.33	8.	2.	1.	0.03	
HYDROGRAPH AT	E06	48.	4.25	9.	2.	1.	0.04	
2 COMBINED AT	CE06	87.	4.33	17.	4.	1.	0.07	
ROUTED TO	RE06	85.	4.58	17.	4.	1.	0.07	
HYDROGRAPH AT	E03	47.	4.58	17.	5.	2.	0.07	
HYDROGRAPH AT	DRF07	449.	4.42	45.	11.	4.	0.31	
ROUTED TO	RF07S	404.	4.67	45.	11.	4.	0.31	
HYDROGRAPH AT	E09	338.	4.25	64.	16.	5.	0.22	
DIVERSION TO	LE09D	325.	4.17	27.	7.	2.	0.22	
HYDROGRAPH AT	LE09	321.	4.33	37.	9.	3.	0.22	
2 COMBINED AT	CE09	596.	4.67	82.	20.	7.	0.51	
DIVERSION TO	DE09I	581.	4.67	76.	19.	6.	0.51	
HYDROGRAPH AT	DE09	15.	4.33	6.	1.	0.	0.51	
ROUTED TO	RE09W	15.	4.42	6.	1.	0.	0.51	
HYDROGRAPH AT	DRE05I	779.	4.50	120.	30.	10.	0.71	
ROUTED TO	RE05S	766.	4.58	120.	30.	10.	0.71	
4 COMBINED AT	CE03	877.	4.58	157.	40.	13.	0.86	
DIVERSION TO	DE03I	846.	4.58	138.	35.	12.	0.86	
HYDROGRAPH AT								

100-year, 6-hour HEC-1 (Future Conditions)

	OPERATION	STATION	PEAK FLOW	TIME OF PEAK	AVERAGE FLOW FOR MAXIMUM PERIOD			BASIN AREA	MAXIMUM STAGE
					6-HOUR	24-HOUR	72-HOUR		
+		DE03	31.	4.00	18.	5.	2.	0.86	
	ROUTED TO								
+		RE03W	31.	4.17	18.	5.	2.	0.86	
	HYDROGRAPH AT								
+		E02	186.	4.33	37.	9.	3.	0.15	
	2 COMBINED AT								
+		CE02	217.	4.33	55.	14.	5.	0.18	
	DIVERSION TO								
+		DE02I	171.	4.33	34.	9.	3.	0.18	
	HYDROGRAPH AT								
+		DE02	46.	4.25	21.	5.	2.	0.18	
	ROUTED TO								
+		RE02W	46.	4.42	21.	5.	2.	0.18	
	HYDROGRAPH AT								
+		E01	84.	4.25	15.	4.	1.	0.06	
	HYDROGRAPH AT								
+		DRE04I	0.	0.08	0.	0.	0.	0.07	
	ROUTED TO								
+		RE04S	0.	0.08	0.	0.	0.	0.07	
	3 COMBINED AT								
+		CE01	130.	4.25	36.	9.	3.	0.17	
	DIVERSION TO								
+		DE01I	69.	4.25	16.	4.	1.	0.17	
	HYDROGRAPH AT								
+		DE01	60.	4.25	21.	5.	2.	0.17	
	HYDROGRAPH AT								
+		DRE01	69.	4.25	16.	4.	1.	0.17	
	ROUTED TO								
+		RE01S	68.	4.33	16.	4.	1.	0.17	
	HYDROGRAPH AT								
+		D05	197.	4.33	39.	10.	3.	0.16	
	2 COMBINED AT								
+		CD05	265.	4.33	55.	14.	5.	0.28	
	DIVERSION TO								
+		DD05I	151.	4.33	29.	7.	2.	0.28	
	HYDROGRAPH AT								
+		DD05	114.	4.33	26.	7.	2.	0.28	
	ROUTED TO								
+		RD05S	114.	4.50	26.	7.	2.	0.28	
	HYDROGRAPH AT								
+		D04	63.	4.33	15.	4.	1.	0.06	
	ROUTED TO								
+		RD04	63.	4.42	15.	4.	1.	0.06	
	HYDROGRAPH AT								
+		D01	97.	4.33	21.	5.	2.	0.08	
	3 COMBINED AT								
+		CD01	266.	4.42	62.	16.	5.	0.26	
	DIVERSION TO								
+		DD01I	106.	4.42	27.	7.	2.	0.26	
	HYDROGRAPH AT								
+		DD01	159.	4.42	35.	9.	3.	0.26	

100-year, 6-hour HEC-1 (Future Conditions)

	OPERATION	STATION	PEAK FLOW	TIME OF PEAK	AVERAGE FLOW FOR MAXIMUM PERIOD			BASIN AREA	MAXIMUM STAGE
					6-HOUR	24-HOUR	72-HOUR		
+	ROUTED TO	RD01E	156.	4.50	35.	9.	3.	0.26	
	HYDROGRAPH AT	DRD05	151.	4.33	29.	7.	2.	0.28	
+	ROUTED TO	RD05E	151.	4.33	29.	7.	2.	0.28	
	DIVERSION TO	DD052I	98.	4.33	23.	6.	2.	0.28	
+	HYDROGRAPH AT	DD052	52.	4.33	6.	2.	1.	0.28	
+	ROUTED TO	RD05S2	42.	4.58	6.	2.	1.	0.28	
	HYDROGRAPH AT	D02	199.	4.25	35.	9.	3.	0.14	
+	3 COMBINED AT	CD02	358.	4.42	76.	19.	6.	0.36	
+	ROUTED TO	RD02	356.	4.42	76.	19.	6.	0.36	
	HYDROGRAPH AT	DRE02	171.	4.33	34.	9.	3.	0.18	
+	ROUTED TO	RE02S	168.	4.50	34.	9.	3.	0.18	
	HYDROGRAPH AT	DRD052	98.	4.33	23.	6.	2.	0.28	
+	ROUTED TO	RD05E2	97.	4.42	23.	6.	2.	0.28	
	HYDROGRAPH AT	D06	130.	4.33	28.	7.	2.	0.11	
+	3 COMBINED AT	CD06	391.	4.42	84.	22.	7.	0.36	
+	ROUTED TO	RD06	389.	4.42	84.	22.	7.	0.36	
	HYDROGRAPH AT	D07	63.	4.25	12.	3.	1.	0.05	
+	2 COMBINED AT	CD07	446.	4.42	96.	25.	8.	0.41	
+	ROUTED TO	RD07	439.	4.58	96.	25.	8.	0.41	
	HYDROGRAPH AT	DRE03	846.	4.58	138.	35.	12.	0.86	
+	ROUTED TO	RE03S	844.	4.75	138.	35.	12.	0.86	
	HYDROGRAPH AT	D08	49.	4.33	13.	3.	1.	0.05	
+	2 COMBINED AT	CD08	878.	4.75	151.	38.	13.	0.88	
+	ROUTED TO	RD08	874.	4.83	151.	38.	13.	0.88	
	HYDROGRAPH AT	D03	122.	4.25	25.	6.	2.	0.10	
+	HYDROGRAPH AT								

100-year, 6-hour HEC-1 (Future Conditions)

OPERATION	STATION	PEAK FLOW	TIME OF PEAK	AVERAGE FLOW FOR MAXIMUM PERIOD			BASIN AREA	MAXIMUM STAGE
				6-HOUR	24-HOUR	72-HOUR		
+ DRE09I		581.	4.67	76.	19.	6.	0.51	
ROUTED TO								
+ RE09S		518.	4.92	76.	19.	6.	0.51	
HYDROGRAPH AT								
+ D09		360.	4.17	62.	16.	5.	0.22	
DIVERSION TO								
+ LD09D		346.	4.17	27.	7.	2.	0.22	
HYDROGRAPH AT								
+ LD09		352.	4.25	35.	9.	3.	0.22	
2 COMBINED AT								
+ CD09		591.	4.92	108.	27.	9.	0.69	
5 COMBINED AT								
+ CD03		1767.	4.83	416.	106.	35.	2.43	
ROUTED TO								
+ RD03		1762.	4.83	416.	106.	35.	2.43	
HYDROGRAPH AT								
+ C11		90.	4.17	12.	3.	1.	0.05	
HYDROGRAPH AT								
+ C10		85.	4.17	13.	3.	1.	0.05	
ROUTED TO								
+ RC10		85.	4.17	13.	3.	1.	0.05	
3 COMBINED AT								
+ CC11		1799.	4.83	437.	111.	37.	2.54	
ROUTED TO								
+ RC11		1797.	4.92	437.	111.	37.	2.54	
HYDROGRAPH AT								
+ C08		170.	4.25	29.	7.	2.	0.12	
2 COMBINED AT								
+ CC08		1843.	4.92	462.	118.	39.	2.66	
ROUTED TO								
+ RC08		1837.	4.92	462.	118.	39.	2.66	
HYDROGRAPH AT								
+ C06		222.	4.25	38.	9.	3.	0.15	
2 COMBINED AT								
+ CC06		1895.	4.92	493.	126.	42.	2.81	
ROUTED TO								
+ RC06		1894.	5.00	493.	126.	42.	2.81	
HYDROGRAPH AT								
+ DRD01		106.	4.42	27.	7.	2.	0.26	
ROUTED TO								
+ RD01S		106.	4.58	27.	7.	2.	0.26	
HYDROGRAPH AT								
+ C07		78.	4.33	18.	5.	2.	0.07	
2 COMBINED AT								
+ CC07		175.	4.50	44.	11.	4.	0.17	
ROUTED TO								
+ RC07		174.	4.67	44.	11.	4.	0.17	
HYDROGRAPH AT								
+ C09		66.	4.25	14.	4.	1.	0.06	
ROUTED TO								
+ RC09		56.	4.50	14.	4.	1.	0.06	

100-year, 6-hour HEC-1 (Future Conditions)

	OPERATION	STATION	PEAK FLOW	TIME OF PEAK	AVERAGE FLOW FOR MAXIMUM PERIOD			BASIN AREA	MAXIMUM STAGE
					6-HOUR	24-HOUR	72-HOUR		
+	HYDROGRAPH AT	C05	151.	4.33	35.	9.	3.	0.14	
+	HYDROGRAPH AT	C01	37.	4.25	8.	2.	1.	0.03	
+	ROUTED TO	RC01	36.	4.50	8.	2.	1.	0.03	
+	HYDROGRAPH AT	C02	41.	4.25	10.	3.	1.	0.04	
+	5 COMBINED AT	CC02	436.	4.50	111.	28.	9.	0.44	
+	ROUTED TO	RC02	435.	4.50	111.	28.	9.	0.44	
+	HYDROGRAPH AT	C03	68.	4.17	11.	3.	1.	0.04	
+	HYDROGRAPH AT	C12	3.	5.25	2.	1.	0.	0.01	
+	ROUTED TO	RC12	3.	5.75	2.	1.	0.	0.01	
+	HYDROGRAPH AT	C13	248.	4.25	50.	13.	4.	0.18	
+	DIVERSION TO	LC13D	248.	4.25	23.	6.	2.	0.18	
+	HYDROGRAPH AT	LC13	224.	4.42	28.	7.	2.	0.18	
+	2 COMBINED AT	CC13	224.	4.42	30.	8.	3.	0.19	
+	DIVERSION TO	DC13I	183.	4.42	20.	5.	2.	0.19	
+	HYDROGRAPH AT	DC13	41.	4.33	10.	3.	1.	0.19	
+	ROUTED TO	RC13W	41.	4.58	10.	3.	1.	0.19	
+	HYDROGRAPH AT	C04	40.	4.33	10.	3.	1.	0.04	
+	ROUTED TO	RC04	40.	4.42	10.	3.	1.	0.04	
+	5 COMBINED AT	CC03	2204.	5.00	615.	158.	53.	3.37	
+	ROUTED TO	RC03	2203.	5.00	615.	158.	53.	3.37	
+	HYDROGRAPH AT	B07	35.	4.08	4.	1.	0.	0.02	
+	2 COMBINED AT	CB07	2204.	5.00	618.	158.	53.	3.39	
+	ROUTED TO	RB07	2199.	5.08	618.	158.	53.	3.39	
+	HYDROGRAPH AT	B05	91.	4.25	20.	5.	2.	0.09	
+	DIVERSION TO	LB05D	2.	1.50	0.	0.	0.	0.09	
	HYDROGRAPH AT								

	OPERATION	STATION	PEAK FLOW	TIME OF PEAK	AVERAGE FLOW FOR MAXIMUM PERIOD			BASIN AREA	MAXIMUM STAGE
					6-HOUR	24-HOUR	72-HOUR		
+		LB05	91.	4.25	20.	5.	2.	0.09	
	HYDROGRAPH AT								
+		B06	20.	4.17	4.	1.	0.	0.01	
	3 COMBINED AT								
+		CB05	2230.	5.08	636.	163.	54.	3.49	
	ROUTED TO								
+		RB05	2227.	5.08	636.	163.	54.	3.49	
	HYDROGRAPH AT								
+		B04	178.	4.08	22.	6.	2.	0.08	
	DIVERSION TO								
+		LB04D	178.	4.08	22.	6.	2.	0.08	
	HYDROGRAPH AT								
+		LB04	0.	0.08	0.	0.	0.	0.08	
	HYDROGRAPH AT								
+		B02	191.	4.17	28.	7.	2.	0.10	
	DIVERSION TO								
+		LB02D	50.	3.75	5.	1.	0.	0.10	
	HYDROGRAPH AT								
+		LB02	191.	4.17	23.	6.	2.	0.10	
	ROUTED TO								
+		RB02	189.	4.17	23.	6.	2.	0.10	
	HYDROGRAPH AT								
+		B03	10.	4.17	2.	0.	0.	0.01	
	4 COMBINED AT								
+		CB03	2247.	5.08	656.	168.	56.	3.66	
	ROUTED TO								
+		RB03	2243.	5.08	656.	168.	56.	3.66	
	HYDROGRAPH AT								
+		B01	162.	4.08	16.	4.	1.	0.07	
	DIVERSION TO								
+		LB01D	25.	3.67	2.	1.	0.	0.07	
	HYDROGRAPH AT								
+		LB01	162.	4.08	14.	3.	1.	0.07	
	2 COMBINED AT								
+		CB01	2244.	5.08	667.	171.	57.	3.73	
	ROUTED TO								
+		RB01	2235.	5.17	667.	171.	57.	3.73	
	HYDROGRAPH AT								
+		B12	233.	4.17	27.	7.	2.	0.12	
	HYDROGRAPH AT								
+		B11	107.	4.17	13.	3.	1.	0.06	
	DIVERSION TO								
+		LB11D	103.	4.08	6.	1.	0.	0.06	
	HYDROGRAPH AT								
+		LB11	95.	4.25	7.	2.	1.	0.06	
	HYDROGRAPH AT								
+		DRDC13	183.	4.42	20.	5.	2.	0.19	
	ROUTED TO								
+		RC13S	166.	5.08	20.	5.	2.	0.19	
	HYDROGRAPH AT								
+		B13	445.	4.08	52.	13.	4.	0.22	

100-year, 6-hour HEC-1 (Future Conditions)

OPERATION	STATION	PEAK FLOW	TIME OF PEAK	AVERAGE FLOW FOR MAXIMUM PERIOD			BASIN AREA	MAXIMUM STAGE
				6-HOUR	24-HOUR	72-HOUR		
+ DIVERSION TO	LB13D	445.	4.08	37.	9.	3.	0.22	
+ HYDROGRAPH AT	LB13	224.	4.42	15.	4.	1.	0.22	
+ ROUTED TO	RB13	216.	4.58	15.	4.	1.	0.22	
+ HYDROGRAPH AT	B14	195.	4.25	44.	11.	4.	0.15	
+ DIVERSION TO	LB14D	195.	4.25	21.	5.	2.	0.15	
+ HYDROGRAPH AT	LB14	174.	4.50	24.	6.	2.	0.15	
+ 3 COMBINED AT	CB14	367.	4.58	59.	15.	5.	0.52	
+ ROUTED TO	RB14	336.	4.67	59.	15.	5.	0.52	
+ 4 COMBINED AT	CB12	2349.	5.08	731.	187.	62.	4.43	
+ ROUTED TO	RB12	2315.	5.42	731.	187.	62.	4.43	
+ HYDROGRAPH AT	A01	1075.	4.17	154.	39.	13.	0.51	
+ DIVERSION TO	LA01D	953.	4.08	71.	18.	6.	0.51	
+ HYDROGRAPH AT	LA01	1041.	4.25	84.	21.	7.	0.51	
+ 2 COMBINED AT	CA01	2349.	5.42	799.	204.	68.	4.94	
+ HYDROGRAPH AT	B10	95.	4.33	22.	6.	2.	0.11	
+ DIVERSION TO	LB10D	95.	4.33	21.	5.	2.	0.11	
+ HYDROGRAPH AT	LB10	7.	6.75	1.	0.	0.	0.11	

\*\*\* NORMAL END OF HEC-1 \*\*\*

**APPENDIX G 10-year, 6-hour future conditions HEC-1 SUMMARY**





44 PC 0.212 0.239 0.271 0.321 0.408 0.515 0.627 0.735 0.814 0.864  
 45 PC 0.907 0.930 0.954 0.977 1.000

\*  
\*

HEC-1 INPUT

PAGE

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

46 KK G03 BASIN  
 47 BA .093  
 48 LG 0.23 0.25 5.60 0.26 55  
 49 UC 0.488 0.378  
 50 UA 0 5.0 16.0 30.0 65.0 77.0 84.0 90.0 94.0 97.0  
 51 UA 100

\*  
\*  
\* \*\*\* DIVERSION DG03 \*\*\*\*\*  
\*  
\* See Diversion Calculation in Appendix D of Hydrology Report.  
\*  
\* DDM \*\*\*\*\* Preserved \*\*\*\*\*

52 KK DG03  
 53 DT DG03I  
 54 DI 0 7 22 43 70 101 124 180 217 264  
 55 DQ 0 3.5 11 22 35 51 63 92 112 137

\*  
\* Route is a Minor Arterial Road with a median.  
\*

56 KK RG03S  
 57 RS 7 FLOW 0  
 58 RC .035 0.015 .035 1322 0.0024  
 59 RX 0 1 40 41 105 106 146 147  
 60 RY 8 2 1 .5 .5 1 2 8

61 KK G01 BASIN  
 62 BA .049  
 63 LG 0.21 0.25 4.80 0.38 57  
 64 UC 0.792 0.832  
 65 UA 0 5.0 16.0 30.0 65.0 77.0 84.0 90.0 94.0 97.0  
 66 UA 100

\*  
\*  
\* THE CONTRIBUTING AREA WAS HARD-CODED TO ACCOUNT FOR THE DIVERSION DG03.  
\* THE HARD-CODE VALUE IS 50% OF CG03'S CONTRIBUTING AREA.  
\* SEE DIVERSION DG03.  
\*  
\* DDM \*\*\*\*\* Preserved \*\*\*\*\*

67 KK CG01  
 68 HC 2 0.096

\*  
\*  
\* \*\*\* DIVERSION DG01 \*\*\*\*\*  
\*  
\* See Diversion Calculation in Appendix D of Hydrology Report.  
\*  
\* DDM \*\*\*\*\* Preserved \*\*\*\*\*

HEC-1 INPUT

PAGE

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

69 KK DG01  
 70 DT DG01I  
 71 DI 0 10 29 57 92 134 164 194 233 333  
 72 DQ 0 7 21 41 66 96 117 139 166 236

\*  
\*  
\* Route is a Minor Arterial Road.  
\*

73 KK RG01S  
 74 RS 1 FLOW 0  
 75 RC 0.035 0.015 0.035 2625 0.0014  
 76 RX 0 1 40 41 105 106 146 147

1  
2

1  
3

```

77      RY      8      2      1      .5      .5      1      2      8
      *
78      KK      F04      BASIN
79      KM
80      KM      This basin's original area was 0.073 sq mi. The infield area is bermed
81      KM      so it is self-detained. The infield area (0.013 sq mi) was subtracted
82      KM      from the subbasin calculations to account for the storage.
83      KM
84      BA      .060
85      LG      0.15      0.25      4.80      0.42      68
86      UC      0.750      0.899
87      UA      0      5.0      16.0      30.0      65.0      77.0      84.0      90.0      94.0      97.0
88      UA      100
      *
      *
89      KK      G04      BASIN
90      BA      .045
91      LG      0.25      0.25      4.80      0.35      51
92      UC      0.396      0.360
93      UA      0      5.0      16.0      30.0      65.0      77.0      84.0      90.0      94.0      97.0
94      UA      100
      *
      *
      * Route is a Subdivision Road.
      * N steps Calculation didn't work for this Route.
      *
95      KK      RG04
96      RS      1      FLOW      0
97      RC      .035      0.015      0.035      1336      0.0033
98      RX      0      1      28      29      57      58      86      87
99      RY      8      2      1      0.5      0.5      1      2      8
      *

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HEC-1 INPUT

PAGE

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

```

100     KK      G05      BASIN
101     BA      .045
102     LG      0.24      0.15      7.00      0.14      53
103     UC      0.404      0.370
104     UA      0      5.0      16.0      30.0      65.0      77.0      84.0      90.0      94.0      97.0
105     UA      100
      *
      *
      * Route is a Subdivision Road.
      * N steps Calculation didn't work for this Route.
      *
106     KK      RG05
107     RS      1      FLOW      0
108     RC      0.035      0.015      0.035      1994      0.0026
109     RX      0      1      28      29      57      58      86      87
110     RY      8      2      1      0.5      0.5      1      2      8
      *
111     KK      G06      BASIN
112     BA      .127
113     LG      0.10      0.29      2.65      1.70      80
114     UC      0.329      0.186
115     UA      0      5.0      16.0      30.0      65.0      77.0      84.0      90.0      94.0      97.0
116     UA      100
      *
      *
      *
      * *** STORAGE ROUTING LG06 *****
      *
      * See the detention calculations in the Appendix.
      * DDM ***** Preserved *****
117     KK      LG06
118     DT      LG06D      8.0
119     DI      0      100      1000      10000
120     DQ      0      100      1000      10000
      *
      *
      * *** DIVERSION DG06 *****

```

\*  
 \* THIS DIVERSION IS DUE TO THE DETENTION BASIN.  
 \* See Diversion Calculation in Appendix D of Hydrology Report.  
 \*  
 \* DDM \*\*\*\*\* Preserved \*\*\*\*\*

121	KK	DG06								
122	DT	DG06I								
123	DI	0	50	100	200	300	500	700	900	1000
124	DQ	0	25	49	75	132	195	230	259	287

\*  
 \* \*\*\* STORAGE ROUTING LG06A \*\*\*\*\*  
 \*  
 \* THE STORAGE CALCULATIONS CAN BE FOUND IN THE APPENDIX.  
 \*  
 \* DDM \*\*\*\*\* Preserved \*\*\*\*\*

1  
 5  
 HEC-1 INPUT PAGE

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

125	KK	LG06A								
126	DT	LG06AD	4.25							
127	DI	0	100	1000	10000					
128	DQ	0	100	1000	10000					

129	KK	G02	BASIN							
130	BA	.075								
131	LG	0.21	0.25	4.80	0.38	52				
132	UC	1.558	1.512							
133	UA	0	5.0	16.0	30.0	65.0	77.0	84.0	90.0	94.0
134	UA	100								

\*  
 \* THE CONTRIBUTING AREA WAS HARD-CODED TO ACCOUNT FOR THE DIVERSION DG06.  
 \* THE HARD-CODE VALUE IS 50% OF CG06'S CONTRIBUTING AREA.  
 \* SEE DIVERSION DG06.  
 \*  
 \* DDM \*\*\*\*\* Preserved \*\*\*\*\*

135	KK	CG02								
136	HC	4	0.229							

\*  
 \* Route is a Minor Arterial Road.  
 \* N steps calculation did not work for this route.

137	KK	RG02								
138	RS	1	FLOW	0						
139	RC	0.035	0.015	0.035	1358	0.0018				
140	RX	0	1	26	27	62	63	88	89	
141	RY	8	2	1	0.5	0.5	1	2	8	

142	KK	F06	BASIN							
143	BA	.103								
144	LG	0.23	0.25	4.80	0.36	49				
145	UC	0.833	0.610							
146	UA	0	5.0	16.0	30.0	65.0	77.0	84.0	90.0	94.0
147	UA	100								

\* DDM \*\*\*\*\* Preserved \*\*\*\*\*

148	KK	CF06								
149	HC	2								

\*  
 \* Route is a Minor Arterial Road.  
 \*

1  
 6  
 HEC-1 INPUT PAGE

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

150	KK	RF06								
151	RS	4	FLOW	0						

152 RC 0.035 0.015 0.035 1299 0.002  
 153 RX 0 1 26 27 62 63 88 89  
 154 RY 8 2 1 0.5 0.5 1 2 8  
 \*  
 155 KK F05 BASIN  
 156 BA .088  
 157 LG 0.24 0.25 4.80 0.36 52  
 158 UC 0.758 0.568  
 159 UA 0 5.0 16.0 30.0 65.0 77.0 84.0 90.0 94.0 97.0  
 160 UA 100  
 \*

\* DDM \*\*\*\*\* Preserved \*\*\*\*\*

161 KK CF05  
 162 HC 2  
 \*  
 \* THERE IS A DIVERSION SOUTH FOR WHATEVER THE STORM DRAIN DOES NOT INTERCEPT.  
 \* ALL EXCESS OVERLAND AMOUNT FLOWS SOUTH.  
 \*  
 \*  
 \* STORM DRAIN ANALYSIS SHOWS A CAPACITY OF 185 CFS, 1/3 IS CARRIED AT  
 \* THIS LOCATION. (61 cfs) 20% Clogging Factor means 49 cfs here.  
 \* (61 x .8 = 49cfs)  
 \*

\* \*\*\* DIVERSION DF05 \*\*\*\*\*  
 \*  
 \* See Diversion Calculation in Appendix D of Hydrology Report.  
 \*  
 \* DDM \*\*\*\*\* Preserved \*\*\*\*\*

163 KK DF05  
 164 DT DF05I  
 165 DI 0 1 50 100 200 300 400 500 1000 2000  
 166 DQ 0 .9 10 51 151 251 351 451 951 1951  
 \*

\* THE INFORMATION FOR THE STORM DRAIN WAS MEASURED IN THE FIELD.  
 \* THE PIPE IS A 66" RCP, SLOPE WAS ESTIMATED TO BE 0.004 FT/FT.  
 \*

167 KK RF05W  
 168 KO 3  
 169 RK 1302 0.001 0.02 CIRC 3  
 \*

\* THE CONTRIBUTING AREA WAS HARD-CODED TO ACCOUNT FOR DIVERSION DF05 & DG01.  
 \* THE HARD-CODE VALUE IS 20% OF CG01 AND 20% OF CF05'S CONTRIBUTING AREA.  
 \* SEE DIVERSION DG01 AND DF05.  
 \*  
 \* DDM \*\*\*\*\* Preserved \*\*\*\*\*

HEC-1 INPUT

PAGE

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

170 KK CF04  
 171 HC 3 0.162  
 \*  
 \* THERE IS A DIVERSION SOUTH FOR WHATEVER THE STORM DRAIN DOES NOT INTERCEPT.  
 \*  
 \* SEE SPLIT FLOW CALCULATIONS IN APPENDIX.  
 \*  
 \*  
 \* \*\*\* DIVERSION DF04 \*\*\*\*\*  
 \*  
 \* See Diversion Calculation in Appendix D of Hydrology Report.  
 \*  
 \* DDM \*\*\*\*\* Preserved \*\*\*\*\*

172 KK DF04  
 173 DT DF04I  
 174 DI 0 9 30 57 93 136 150 198 336 400  
 175 DQ 0 5 16 30 50 73 82 105 170 200  
 \*

\* DIVERSION RECOVER D05  
 \*

176 KK F05R

1  
7

177 DR DF05I  
 \*  
 \* Route is a Minor Arterial Road.  
 \* N steps calculation did not work for this route.

178 KK RF05S  
 179 RS 1 FLOW 0  
 180 RC 0.035 0.015 0.035 2167 0.0032  
 181 RX 0 1 26 27 62 63 88 89  
 182 RY 8 2 1 0.5 0.5 1 2 8  
 \*  
 \*

183 KK F03 BASIN  
 184 BA .170  
 185 LG 0.21 0.25 4.80 0.37 58  
 186 UC 0.921 0.619  
 187 UA 0 5.0 16.0 30.0 65.0 77.0 84.0 90.0 94.0 97.0  
 188 UA 100  
 \*  
 \* THE CONTRIBUTING AREA WAS HARD-CODED TO ACCOUNT FOR DIVERSION DF05.  
 \* THE HARD-CODE VALUE IS 80% OF CF05'S CONTRIBUTING AREA.  
 \* SEE DIVERSION DF05.  
 \*  
 \* DDM \*\*\*\*\* Preserved \*\*\*\*\*

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LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

189 KK CF03  
 190 HC 2 0.51  
 \*  
 \*  
 \* Route is a Minor Arterial Road.  
 \*  
 \*

191 KK RF03  
 192 RS 1 FLOW 0  
 193 RC 0.035 0.015 0.035 482 0.0035  
 194 RX 0 1 26 27 62 63 88 89  
 195 RY 8 3 2 0 0 2 3 8  
 \*  
 \*  
 \* DIVERSION RECOVER G06  
 \*

196 KK G06R  
 197 DR DG06I  
 \*  
 \*  
 \* Route is a Minor Arterial Road along Pima Road.  
 \*  
 \*

198 KK RG06S  
 199 RS 29 FLOW 0  
 200 RC 0.035 0.015 0.035 5286 0.0028  
 201 RX 0 1 26 27 62 63 88 89  
 202 RY 8 3 2 0 0 2 3 8  
 \*

203 KK F07 BASIN  
 204 BA .250  
 205 LG 0.10 0.29 2.65 1.70 80  
 206 UC 0.704 0.559  
 207 UA 0 5.0 16.0 30.0 65.0 77.0 84.0 90.0 94.0 97.0  
 208 UA 100  
 \*  
 \*  
 \* \*\*\* STORAGE ROUTING LF07 \*\*\*\*\*  
 \*  
 \* See the detention calculations in the Appendix.  
 \* DDM \*\*\*\*\* Preserved \*\*\*\*\*

209 KK LF07

210 DT LF07D 15.2  
 211 DI 0 100 1000 10000  
 212 DQ 0 100 1000 10000

\*  
 \*  
 \* THE CONTRIBUTING AREA WAS HARD-CODED TO ACCOUNT FOR DIVERSION DG06.  
 \* THE HARD-CODE VALUE IS 50% OF CG06'S CONTRIBUTING AREA.  
 \* SEE DIVERSION DG06.  
 \*  
 \* DDM \*\*\*\*\* Preserved \*\*\*\*\*

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LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

213 KK CF07  
 214 HC 2 0.314

\*  
 \* THERE IS A DIVERSION SOUTH FOR WHATEVER THE STORM DRAIN DOES NOT INTERCEPT.  
 \* THE STORM DRAIN ANALYSIS SHOWS THE MAXIMUM FLOW IN THE STORM DRAIN AT THIS  
 \* LOCATION IS 16 CFS. 20% clogging factor means 13 cfs (16cfs \* .8 = 13 cfs).  
 \* ALL OVERLAND FLOW CONTINUES SOUTH.  
 \*  
 \*  
 \* \*\*\* DIVERSION DF07 \*\*\*\*\*  
 \*  
 \* See Diversion Calculation in Appendix D of Hydrology Report.  
 \*  
 \* DDM \*\*\*\*\* Preserved \*\*\*\*\*

215 KK DF07  
 216 DT DF07I  
 217 DI 0 1 13 100 200 400 600 800 1000 2000  
 218 DQ 0 0 0 87 187 387 587 787 987 1987

\*  
 \* Route is a Storm Drain 36" RCP with a Slope of 0.001 ft/ft  
 \*  
 \*

219 KK RF07W  
 220 KO 3  
 221 RD 1315 0.001 0.013 CIRC 3

222 KK F02 BASIN  
 223 BA .046  
 224 LG 0.23 0.25 4.80 0.36 55  
 225 UC 0.708 0.756  
 226 UA 0 5.0 16.0 30.0 65.0 77.0 84.0 90.0 94.0 97.0  
 227 UA 100

\*  
 \* THE CONTRIBUTING AREA WAS HARD-CODED TO ACCOUNT FOR DIVERSION DF07.  
 \* THE HARD-CODE VALUE IS 10% OF CF07'S CONTRIBUTING AREA.  
 \* SEE DIVERSION DF07.  
 \*  
 \* DDM \*\*\*\*\* Preserved \*\*\*\*\*

228 KK CF02  
 229 HC 3 0.586

\*  
 \* THERE IS A DIVERSION SOUTH FOR WHATEVER THE STORM DRAIN DOES NOT INTERCEPT.  
 \* THE STORM DRAIN ANALYSIS SHOWS THAT THE MAXIMUM FLOW IN THIS STORM DRAIN  
 \* AT THIS LOCATION IS 63 CFS AND 16 CFS IS IN THE STORMDRAIN FROM THE EAST.  
 \* 20% clogging factor means 50 cfs (63cfs \* 0.8 = 50cfs).  
 \*  
 \* ALL OVERLAND FLOW CONTINUES SOUTH.  
 \*  
 \* \*\*\* DIVERSION DF02 \*\*\*\*\*  
 \*  
 \* See Diversion Calculation in Appendix D of Hydrology Report.  
 \*  
 \* DDM \*\*\*\*\* Preserved \*\*\*\*\*

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LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

230 KK DF02  
 231 DT DF02I  
 232 DI 0 1 50 100 200 300 400 500 1000 2000  
 233 DQ 0 1 40 60 160 260 360 460 960 1960

\*  
 \*  
 \*  
 \* ROUTE IS A 5FT RCP STORM DRAIN WITH A SLOPE OF 0.001 FT/FT  
 \*

234 KK RF02W  
 235 KO 3  
 236 RS 5 FLOW 0  
 237 RD 1310 0.001 0.02 CIRC 5

\*  
 \*  
 \* DIVERSION RECOVER F04  
 \*

238 KK F04R  
 239 DR DF04I

\*  
 \*  
 \* Route is a Minor Arterial Road.  
 \*  
 \*

240 KK RF04S  
 241 RS 12 FLOW 0  
 242 RC 0.035 0.013 0.035 2652 0.0023  
 243 RX 0 1 26 27 62 63 88 89  
 244 RY 8 3 2 0 0 2 3 8

\*  
 \*  
 \* THE WEST SIDE OF THE ROAD IS DIVERTED WEST AND THE EAST SIDE OF THE ROAD  
 \* FLOWS SOUTH INTO CF01. THE TOTAL FLOW DOES NOT EXCEED THE CROWN OF THE ROAD  
 \* SO THE SPLIT IS 50-50.  
 \*  
 \* \*\*\* DIVERSION DRF04 \*\*\*\*\*  
 \*  
 \* See Diversion Calculation in Appendix D of Hydrology Report.  
 \*  
 \* DDM \*\*\*\*\* Preserved \*\*\*\*\*

245 KK DRF04  
 246 DT DRF04I  
 247 DI 0 1 20 50 100 200 300  
 248 DQ 0 .5 10 25 50 100 150

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LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

249 KK F01 BASIN  
 250 BA .030  
 251 LG 0.22 0.25 4.80 0.39 53  
 252 UC 0.637 0.692  
 253 UA 0 5.0 16.0 30.0 65.0 77.0 84.0 90.0 94.0 97.0  
 254 UA 100

\*  
 \*  
 \* THE CONTRIBUTING AREA WAS HARD-CODED TO ACCOUNT FOR DIVERSIONS DF04 & DF02.  
 \* THE HARD-CODE VALUE IS 25% OF CF04 AND 20% OF CF02'S CONTRIBUTING AREA.  
 \* SEE DIVERSION DF04 AND DF02.  
 \*  
 \* DDM \*\*\*\*\* Preserved \*\*\*\*\*

255 KK CF01  
 256 HC 3 0.19

\*  
 \*  
 \* THE DIVERSION AT THIS LOCATION IS SOUTH (SEE SPLITFLOW ANALYSIS)  
 \*  
 \*  
 \*  
 \* \*\*\* DIVERSION DF01 \*\*\*\*\*  
 \*  
 \* See Diversion Calculation in Appendix D of Hydrology Report.

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\*  
\* DDM \*\*\*\*\* Preserved \*\*\*\*\*

257 KK DF01  
258 DT DF01I  
259 DI 0 7 21 42 75 111 139 188 254 335  
260 DQ 0 0 0 0 0 0 6 18 35 57

\*  
\* DIVERSION RECOVER DF02  
\*

261 KK DRF02  
262 DR DF02I

\*  
\* Route is a Minor Arterial Road.  
\* N steps Calculation did not work for this route.

263 KK RF02S  
264 RS 1 FLOW 0  
265 RC 0.035 0.015 0.035 1338 0.0022  
266 RX 0 1 26 27 62 63 88 89  
267 RY 8 2 1 0.5 0.5 1 2 8

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LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

268 KK E07 BASIN  
269 BA .084  
270 LG 0.25 0.25 4.80 0.36 49  
271 UC 0.600 0.429  
272 UA 0 5.0 16.0 30.0 65.0 77.0 84.0 90.0 94.0 97.0  
273 UA 100

\*  
\* THE CONTRIBUTING AREA WAS HARD-CODED TO ACCOUNT FOR DIVERSION DF02.  
\* THE HARD-CODE VALUE IS 80% OF CF02'S CONTRIBUTING AREA.  
\* SEE DIVERSION DF02.

\* DDM \*\*\*\*\* Preserved \*\*\*\*\*

274 KK CE07  
275 HC 2 0.554

\*  
\* Route is a Minor Arterial Road.  
\* N steps calculation did not work for this route.

276 KK RE07  
277 RS 1 FLOW 0  
278 RC 0.035 0.015 0.035 1335 0.0025  
279 RX 0 1 26 27 62 63 88 89  
280 RY 8 2 1 0.5 0.5 1 2 8

281 KK E05 BASIN  
282 KM  
283 KM This basin's original area was 0.060 sq mi. The infield area is bermed  
284 KM so it is self-detained. The infield area (0.013 sq mi) was subtracted  
285 KM from the subbasin calculations to account for the storage.  
286 KM  
287 BA .047  
288 LG 0.21 0.25 4.80 0.37 57  
289 UC 0.496 0.399  
290 UA 0 5.0 16.0 30.0 65.0 77.0 84.0 90.0 94.0 97.0  
291 UA 100

\* DDM \*\*\*\*\* Preserved \*\*\*\*\*

292 KK CE05  
293 HC 2

\*  
\* THERE IS A DIVERSION SOUTH FOR WHATEVER THE STORM DRAIN DOES NOT INTERCEPT.  
\* THE STORM DRAIN ANALYSIS SHOWS THAT THE MAXIMUM FLOW IN THE DRAIN IS 92 CFS.  
\* THE TOTAL FLOW WAS DIVIDED BY E05, E04, AND THE BASIN WEST OF THE PROJECT



\*  
 \* \*\*\* DIVERSION DE04 \*\*\*\*\*

\* See Diversion Calculation in Appendix D of Hydrology Report.

\* DDM \*\*\*\*\* Preserved \*\*\*\*\*

319	KK	DE04									
320	DT	DE04I									
321	DI	0	3	31	59	91	126	151	184	222	267
322	DQ	0	0	0	0	0	0	3	9	18	30

\* DDM \*\*\*\*\* Preserved \*\*\*\*\*

323	KK	DUMMY									
324	HC	3									
325	KK	E08	BASIN								
326	BA	.031									
327	LG	0.24	0.25	4.80	0.35	51					
328	UC	0.671	0.757								
329	UA	0	5.0	16.0	30.0	65.0	77.0	84.0	90.0	94.0	97.0
330	UA	100									

\* Route is a Minor Arterial Road with a wall on the west side.

331	KK	RE08									
332	RS	7	FLOW	0							
333	RC	0.035	0.015	0.035	1326	0.0029					
334	RX	0	1	19	25	30	49	79	80		
335	RY	10	1.5	0	0	0	0	1.5	8		

336	KK	E06	BASIN								
337	BA	.036									
338	LG	0.24	0.25	4.80	0.36	53					
339	UC	0.683	0.774								
340	UA	0	5.0	16.0	30.0	65.0	77.0	84.0	90.0	94.0	97.0
341	UA	100									

\* DDM \*\*\*\*\* Preserved \*\*\*\*\*

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LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

342	KK	CE06									
343	HC	2									

344	KK	RE06									
345	RS	16	FLOW	0							
346	RC	0.035	0.015	0.035	3333	0.0014					
347	RX	0	1	19	25	30	49	79	80		
348	RY	10	1.5	0	0	0	0	1.5	8		

349	KK	E03	BASIN								
350	BA	.070									
351	LG	0.23	0.25	4.80	0.36	53					
352	UC	1.267	1.965								
353	UA	0	5.0	16.0	30.0	65.0	77.0	84.0	90.0	94.0	97.0
354	UA	100									

\* DIVERSION RECALL DF07

355	KK	DRF07									
356	DR	DF07I									

\* Route is a minor arterial along Pima Rd.  
\*

357	KK	RF07S								
358	RS	23	FLOW	0						
359	RC	0.035	0.015	0.035	5280	0.0023				
360	RX	0	1	40	41	104	105	145	146	
361	RY	8	3	1.5	.75	0.75	1.5	3	8	

362	KK	E09	BASIN							
363	BA	.223								
364	LG	0.10	0.29	2.65	1.70	80				
365	UC	0.712	0.599							
366	UA	0	5.0	16.0	30.0	65.0	77.0	84.0	90.0	94.0
367	UA	100								

\* \*\* STORAGE ROUTING LE09 \*\*\*\*\*  
\*  
\* See the detention calculations in the Appendix.  
\* DDM \*\*\*\*\* Preserved \*\*\*\*\*

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LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

368	KK	LE09								
369	DT	LE09D	13.6							
370	DI	0	100	1000	10000					
371	DQ	0	100	1000	10000					

\* THE CONTRIBUTING AREA WAS HARD-CODED TO ACCOUNT FOR DIVERSIONS DF07.  
\* THE HARD-CODE VALUE IS 90% OF CF07'S CONTRIBUTING AREA.  
\* SEE DIVERSION DF07.  
\*  
\* DDM \*\*\*\*\* Preserved \*\*\*\*\*

372	KK	CE09								
373	HC	2	0.499							

\* THERE IS A DIVERSION SOUTH FOR WHATEVER THE STORM DRAIN DOES NOT INTERCEPT.  
\* THE STORM DRAIN ANALYSIS SHOWS THAT THE MAXIMUM FLOW IN THE DRAIN IS 96 CFS.  
\* THIS AMOUNT IS DIVIDED BETWEEN DIVERSIONS DE01, DE02, DE03, DE09, AND THE  
\* BASIN EAST OF THE PROJECT AREA, WHICH IS 19 CFS AT THIS LOCATION.  
\*  
\* 20% clogging factor means 15cfs, (19cfs \* 0.8 = 15cfs).  
\* ALL OVERLAND FLOW CONTINUES SOUTH AT THIS DIVERSION LOCATION.  
\*  
\* \*\* DIVERSION DE09 \*\*\*\*\*  
\*  
\* See Diversion Calculation in Appendix D of Hydrology Report.  
\*  
\* DDM \*\*\*\*\* Preserved \*\*\*\*\*

374	KK	DE09								
375	DT	DE09I								
376	DI	0	1	50	100	200	300	400	500	1000
377	DQ	0	0	35	85	185	285	385	485	985

\* ROUTE IS A 6FT RCP WITH A SLOPE OF 0.0009 FT/FT.  
\*  
\*

378	KK	RE09W								
379	KO	3								
380	RD	779	0.0009	0.013		CIRC	6			

\* DIVERSION RECOVER DE05  
\*

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LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

381 KK DRE05I  
382 DR DE05I  
\*  
\*  
\* Route is a Minor Arterial Road.  
\*

383 KK RE05S  
384 RS 8 FLOW 0  
385 RC 0.035 0.015 0.035 3053 0.0021  
386 RX 0 1 26 27 62 63 88 89  
387 RY 8 2 1 0.5 0.5 1 2 8

\* THE CONTRIBUTING AREA IS HARD-CODED TO ACCOUNT FOR DIVERSIONS DE05 & DE09.  
\* THE HARD-CODE VALUE IS 90% OF CE05 AND 10% OF CE09'S CONTRIBUTING AREA.  
\* SEE DIVERSION DE05 AND DE09.  
\*  
\* DDM \*\*\*\*\* Preserved \*\*\*\*\*

388 KK CE03  
389 HC 4 0.73

\*  
\* THERE IS A DIVERSION SOUTH FOR WHATEVER THE STORM DRAIN DOES NOT INTERCEPT.  
\*  
\* THE STORM DRAIN ANALYSIS SHOWS THAT THE MAXIMUM FLOW IN THE DRAIN IS 96 CFS.  
\* THIS AMOUNT IS DIVIDED BETWEEN DIVERSIONS DE01, DE02, DE03, DE09, AND THE  
\* BASIN EAST OF THE PROJECT AREA, WHICH IS 38cfs AT THIS LOCATION.  
\*  
\* 20% clogging factor means that 31cfs (38.4 cfs \* 0.8 = 31cfs)  
\* ALL OVERLAND FLOWS CONTINUE SOUTH.  
\*  
\* \*\*\* DIVERSION DE03 \*\*\*\*\*  
\*  
\* See Diversion Calculation in Appendix D of Hydrology Report.  
\*  
\* DDM \*\*\*\*\* Preserved \*\*\*\*\*

390 KK DE03  
391 DT DE03I  
392 DI 0 1 50 100 200 300 400 500 1000 2000  
393 DQ 0 0 19 69 169 269 369 469 969 1969

\* ROUTE IS A 6FT RCP WITH A SLOPE OF 0.0009 FT/FT.  
\*

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LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

394 KK RE03W  
395 RK 1504 0.0009 0.02 CIRC 6  
\*

396 KK E02 BASIN  
397 BA .149  
398 LG 0.24 0.25 4.80 0.37 50  
399 UC 0.933 0.762  
400 UA 0 5.0 16.0 30.0 65.0 77.0 84.0 90.0 94.0 97.0  
401 UA 100

\* THE CONTRIBUTING AREA IS HARD-CODED DUE TO DIVERSION DE03.  
\* THE HARD-CODE VALUE IS 10% OF CE03'S CONTRIBUTING AREA.  
\* SEE DIVERSION DE03.  
\*  
\* DDM \*\*\*\*\* Preserved \*\*\*\*\*

402 KK CE02  
403 HC 2 0.156

\*  
\* THERE IS A DIVERSION SOUTH FOR WHATEVER THE STORM DRAIN DOES NOT INTERCEPT.  
\*  
\* THE STORM DRAIN ANALYSIS SHOWS THAT THE MAXIMUM FLOW IN THE DRAIN IS 96 CFS.

\* THIS AMOUNT IS DIVIDED BETWEEN DIVERSIONS DE01, DE02, DE03, DE09, AND THE  
 \* BASIN EAST OF THE PROJECT AREA, 58cfs AT THIS LOCATION.  
 \*  
 \* 20% clogging factor means 46cfs (58cfs \* 0.8 = 46cfs).  
 \* ALL OVERLAND FLOW CONTINUES SOUTH.  
 \*  
 \* \*\*\* DIVERSION DE02 \*\*\*\*\*  
 \*  
 \* See Diversion Calculation in Appendix D of Hydrology Report.  
 \*  
 \* DDM \*\*\*\*\* Preserved \*\*\*\*\*

404	KK	DE02									
405	DT	DE02I									
406	DI	0	1	50	100	200	300	400	500	1000	2000
407	DQ	0	0	4	54	154	254	354	454	954	1954

\* ROUTE IS A 6FT RCP WITH A SLOPE OF 0.0009 FT/FT.  
 \*

408	KK	RE02W								
409	KO	3								
410	RD	355	0.0009	0.02		CIRC	6			

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LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

411	KK	E01	BASIN								
412	BA	.058									
413	LG	0.20	0.25	4.80	0.38	60					
414	UC	0.658	0.682								
415	UA	0	5.0	16.0	30.0	65.0	77.0	84.0	90.0	94.0	97.0
416	UA	100									

\* DIVERSION RECOVER DE04  
 \*

417	KK	DRE04I								
418	DR	DE04I								

\* Route is a Minor Arterial Road.  
 \*

419	KK	RE04S								
420	RS	1	FLOW	0						
421	RC	0.035	0.015	0.035	2636	0.0026				
422	RX	0	1	26	27	62	63	88	89	
423	RY	8	2	1	0.5	0.5	1	2	8	

\* THE CONTRIBUTING AREA IS HARD-CODED TO ACCOUNT FOR DIVERSIONS DE04 & DE02.  
 \* THE HARD-CODE VALUE IS 0% OF CE04 AND 40% OF CE02'S CONTRIBUTING AREA.  
 \* SEE DIVERSION DE04 AND DE02.  
 \*  
 \* DDM \*\*\*\*\* Preserved \*\*\*\*\*

424	KK	CE01								
425	HC	3	0.12							

\* THERE IS A DIVERSION SOUTH FOR WHATEVER THE STORM DRAIN DOES NOT INTERCEPT.  
 \*  
 \* THE STORM DRAIN ANALYSIS SHOWS THAT THE MAXIMUM FLOW IN THE DRAIN IS 96 CFS.  
 \* THIS AMOUNT IS DIVIDED BETWEEN DIVERSIONS DE01, DE02, DE03, DE09, AND THE  
 \* BASIN EAST OF THE PROJECT AREA, 76.8 cfs AT THIS LOCATION.  
 \*  
 \* 20% clogging factor means 61cfs (76.8cfs \* 0.8 = 61.4 cfs)  
 \* THIS DIVERSION IS STORMDRAIN ONLY, ALL OVERLAND FLOW IS SOUTH.  
 \*  
 \* \*\*\* DIVERSION DE01 \*\*\*\*\*

\* See Diversion Calculation in Appendix D of Hydrology Report.  
 \*  
 \* DDM \*\*\*\*\* Preserved \*\*\*\*\*

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LINE	ID	1	2	3	4	5	6	7	8	9	10
426	KK	DE01									
427	DT	DE01I									
428	DI	0	1	61	100	200	300	400	500	1000	2000
429	DQ	0	0	0	39	139	239	339	439	939	1939
	*	* DIVERSION RECALL E01									
	*										
430	KK	DRE01									
431	DR	DE01I									
	*	* Route is a Subdivision Road.									
	*										
432	KK	RE01S									
433	RS	12	FLOW	0							
434	RC	0.035	0.015	0.035	2567	0.0032					
435	RX	0	1	28	29	57	58	86	87		
436	RY	8	2	1	0.5	0.5	1	2	8		
	*										
437	KK	D05	BASIN								
438	KM	This basin's original area was 0.168 sq mi. The infield area is bermed									
439	KM	so it is self-detained. The infield area (0.01 sq mi) was subtracted									
440	KM	from the subbasin calculations to account for the storage.									
441	KM										
442	KM										
443	BA	.158									
444	LG	0.23	0.25	4.80	0.38	51					
445	UC	0.942	0.760								
446	UA	0	5.0	16.0	30.0	65.0	77.0	84.0	90.0	94.0	97.0
447	UA	100									
	*										
	*	* THE CONTRIBUTING AREA WAS HARD-CODED TO ACCOUNT FOR DIVERSION DE01.									
	*	* THE HARD-CODE VALUE IS 30% OF CE01'S CONTRIBUTING AREA.									
	*	* SEE DIVERSION DE01.									
	*										
	*	* DDM ***** Preserved *****									
448	KK	CD05									
449	HC	2	0.2								
	*	* *** DIVERSION DD05 *****									
	*	* THIS DIVERSION IS AT GRANITE REEF ROAD AND OSBORN ROAD.									
	*	* THE MAIN PATH IS SOUTH ON GRANITE REEF ROAD.									
	*	* See Diversion Calculation in Appendix D of Hydrology Report.									
	*										
	*	* DDM ***** Preserved *****									

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LINE	ID	1	2	3	4	5	6	7	8	9	10
450	KK	DD05									
451	DT	DD05I									
452	DI	0	5	16	50	93	112	174	217	270	311
453	DQ	0	2	8	25	47	58	95	121	154	194
	*	* Route is a Minor Arterial Road.									
	*	* N steps calculation did not work for this route.									
454	KK	RD05S									
455	RS	5	FLOW	0							
456	RC	0.035	0.015	0.035	2669	0.0026					
457	RX	0	1	26	27	62	63	88	89		
458	RY	8	2	1	0.5	0.5	1	2	8		
	*										
459	KK	D04	BASIN								
460	BA	.060									
461	LG	0.23	0.25	4.80	0.36	53					

462 UC 0.871 1.076  
 463 UA 0 5.0 16.0 30.0 65.0 77.0 84.0 90.0 94.0 97.0  
 464 UA 100  
 \*  
 \*  
 \* Route is a Major Arterial Road.  
 \*

465 KK RD04  
 466 RS 7 FLOW 0  
 467 RC 0.035 0.015 0.035 1084 0.0018  
 468 RX 0 1 40 41 104 105 145 146  
 469 RY 8 3 1.5 .75 0.75 1.5 3 8  
 \*

470 KK D01 BASIN  
 471 BA .082  
 472 LG 0.24 0.25 4.80 0.36 53  
 473 UC 0.904 0.872  
 474 UA 0 5.0 16.0 30.0 65.0 77.0 84.0 90.0 94.0 97.0  
 475 UA 100  
 \*  
 \* DDM \*\*\*\*\* Preserved \*\*\*\*\*

476 KK CD01  
 477 HC 3  
 \*  
 \*  
 \*  
 \* \*\*\* DIVERSION DD01 \*\*\*\*\*  
 \*  
 \* See Diversion Calculation in Appendix D of Hydrology Report.  
 \*  
 \* DDM \*\*\*\*\* Preserved \*\*\*\*\*

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LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

478 KK DD01  
 479 DT DD01I  
 480 DI 0 9 28 89 168 259 392 475 568 672  
 481 DQ 0 4 13 41 73 104 152 182 217 256  
 \*  
 \*  
 \*  
 \* Route is a Major Arterial Road.  
 \* N steps calculation did not work for this route.

482 KK RD01E  
 483 RS 1 FLOW 0  
 484 RC 0.035 0.015 0.035 1315 0.0024  
 485 RX 0 1 40 41 104 105 145 146  
 486 RY 8 3 1.5 .75 0.75 1.5 3 8  
 \*  
 \*  
 \* DIVERSION RECALL D05  
 \*

487 KK DRD05  
 488 DR DD05I  
 \*  
 \*  
 \* Route is a Minor Arterial Road.  
 \*  
 \*

489 KK RD05E  
 490 RS 2 FLOW 0  
 491 RC 0.035 0.015 0.035 560 0.0018  
 492 RX 0 1 26 27 62 63 88 89  
 493 RY 8 3 2 0 0 2 3 8  
 \*  
 \* \*\*\* DIVERSION DD052 \*\*\*\*\*  
 \*  
 \* THIS DIVERSION IS AT GRANITE REEF ROAD AND MULBERRY.  
 \* THE MAIN PATH IS SOUTH ON MULBERRY.  
 \* See Diversion Calculation in Appendix D of Hydrology Report.  
 \*  
 \* DDM \*\*\*\*\* Preserved \*\*\*\*\*

494	KK	DD052									
495	DT	DD052I									
496	DI	0	37	44	59	80	110	147	178	222	281
497	DQ	0	37	40	47	58	74	95	122	155	195

\*  
\*  
\* Route is a Subdivision Road. (MULBERRY)  
\*

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LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

498	KK	RD05S2									
499	RS	15	FLOW	0							
500	RC	0.035	0.015	0.035	3035	0.0030					
501	RX	0	1	28	29	57	58	86	87		
502	RY	8	2	1	0.5	0.5	1	2	8		

\*  
\*

503	KK	D02	BASIN								
504	BA	.139									
505	LG	0.23	0.25	4.80	0.36	51					
506	UC	0.808	0.645								
507	UA	0	5.0	16.0	30.0	65.0	77.0	84.0	90.0	94.0	97.0
508	UA	100									

\*  
\*

\* THE CONTRIBUTING AREA WAS HARD-CODED BECAUSE OF DIVERSION DD01 & DD052.  
\* THE HARD-CODE VALUE IS 60% OF CD01 AND 20% OF DD052'S CONTRIBUTING AREA.  
\* SEE DIVERSION DD01 AND DD052.

\* DDM \*\*\*\*\* Preserved \*\*\*\*\*

509	KK	CD02									
510	HC	3	0.304								

\*  
\*

\* Route is a Major Arterial Road.

511	KK	RD02									
512	RS	3	FLOW	0							
513	RC	0.035	0.015	0.035	1063	0.0045					
514	RX	0	1	40	41	104	105	145	146		
515	RY	8	3	1.5	.75	0.75	1.5	3	8		

\*  
\*

\* DIVERSION RECALL E02

516	KK	DRE02									
517	DR	DE02I									

\*  
\*

\* Route is a Subdivision Road.  
\* N steps calculation did not work for this route.

518	KK	RE02S									
519	RS	1	FLOW	0							
520	RC	0.035	0.015	0.035	2617	0.0028					
521	RX	0	1	28	29	57	58	86	87		
522	RY	8	2	1	0.5	0.5	1	2	8		

\*  
\*

\* DIVERSION RECALL D052

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LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

523	KK	DRD052									
524	DR	DD052I									

\*  
\*

\* Route is a Minor Arterial Road.

```

*
*
525      KK  RD05E2
526      RS      4      FLOW      0
527      RC  0.035  0.015  0.035   767  0.0013
528      RX      0      1      26     27     62     63     88     89
529      RY      8      3      2      0      0      2      3      8
*
*
530      KK  D06   BASIN
531      BA  .109
532      LG  0.24   0.25   4.80   0.35   52
533      UC  0.913  0.847
534      UA      0      5.0   16.0   30.0   65.0   77.0   84.0   90.0   94.0   97.0
535      UA  100

```

```

* THE CUMMALATIVE AREA WAS HARD-CODED TO ACCOUNT FOR DIVERSION DE02 & DD052.
* THE HARD-CODE VALUE IS 60% OF CE02 AND 80% OF DD052'S CONTRIBUTING AREA.
* SEE DIVERSION DE02 AND DD052.

```

```

* DDM ***** Preserved *****

```

```

536      KK  CD06
537      HC      3      0.283

```

```

*
*
* Route is a Minor Arterial Road.
*
* N steps calculation did not work for this route.

```

```

538      KK  RD06
539      RD
540      RC  0.035  0.015  0.035   609  0.0033
541      RX      0      1      26     27     62     63     88     89
542      RY      8      3      2      0      0      2      3      8
*
*
543      KK  D07   BASIN
544      BA  .049
545      LG  0.25   0.25   4.80   0.35   51
546      UC  0.696  0.817
547      UA      0      5.0   16.0   30.0   65.0   77.0   84.0   90.0   94.0   97.0
548      UA  100

```

```

* DDM ***** Preserved *****

```

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LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

```

549      KK  CD07
550      HC      2

```

```

*
*
* Route is a Minor Arterial Road.
*

```

```

551      KK  RD07
552      RS  10      FLOW      0
553      RC  0.035  0.015  0.035  3657  0.003
554      RX      0      1      26     27     62     63     88     89
555      RY      8      2      1      0.5   0.5     1      2      8

```

```

* DIVERSION RECALL DE03

```

```

556      KK  DRE03
557      DR  DE03I

```

```

*
*
* Route is a Subdivision Road.
*

```

```

558      KK  RE03S
559      RS      8      FLOW      0
560      RC  0.035  0.015  0.035  3444  0.0032

```

561 RX 0 1 28 29 57 58 86 87  
 562 RY 8 2 1 0.5 0.5 1 2 8  
 \*  
 \*  
 563 KK D08 BASIN  
 564 BA .054  
 565 LG 0.24 0.25 4.80 0.36 53  
 566 UC 0.896 1.346  
 567 UA 0 5.0 16.0 30.0 65.0 77.0 84.0 90.0 94.0 97.0  
 568 UA 100  
 \*  
 \*

\* THE CUMMULATIVE AREA WAS HARD-CODED TO ACCOUNT FOR DIVERSION DE03.  
 \* THE HARD-CODE VALUE IS 90% OF CE03'S CONTRIBUTING AREA.  
 \* SEE DIVERSION DE03.  
 \*  
 \* DDM \*\*\*\*\* Preserved \*\*\*\*\*

569 KK CD08  
 570 HC 2 0.714  
 \*  
 \*

\* Route is a Minor Arterial Road with a wall on the west side.  
 \*

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LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

571 KK RD08  
 572 RS 5 FLOW 0  
 573 RC 0.035 0.015 0.035 2751 0.0037  
 574 RX 0 1 19 25 30 49 79 80  
 575 RY 10 1.5 0 0 0 0 1.5 8  
 \*  
 \*  
 \*

576 KK D03 BASIN  
 577 BA .103  
 578 LG 0.20 0.25 4.50 0.45 52  
 579 UC 0.837 0.860  
 580 UA 0 5.0 16.0 30.0 65.0 77.0 84.0 90.0 94.0 97.0  
 581 UA 100  
 \*  
 \*  
 \*

\* DIVERSION RECOVER DE09  
 \*

582 KK DRE09I  
 583 DR DE09I  
 \*  
 \*

\* Route is a Minor Arterial Road along Pima Rd.  
 \*

584 KK RE09S  
 585 RS 10 FLOW 0  
 586 RC 0.035 0.015 0.035 5501 0.0037  
 587 RX 0 1 26 27 62 63 88 89  
 588 RY 8 2 1 0.5 0.5 1 2 8  
 \*  
 \*

589 KK D09 BASIN  
 590 BA .216  
 591 LG 0.10 0.29 2.65 1.70 80  
 592 UC 0.604 0.510  
 593 UA 0 5.0 16.0 30.0 65.0 77.0 84.0 90.0 94.0 97.0  
 594 UA 100  
 \*  
 \*

\* \*\* STORAGE ROUTING LD09 \*\*\*\*\*  
 \*  
 \* See the detention calculations in the Appendix.  
 \* DDM \*\*\*\*\* Preserved \*\*\*\*\*

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LINE	ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10
595	KK LD09
596	DT LD09D 13.6
597	DI 0 100 1000 10000
598	DQ 0 100 1000 10000
	* *
	* THE CONTRIBUTING AREA IS HARD-CODED TO ACCOUNT FOR DIVERSION DE09. * THE HARD-CODE VALUE IS 90% OF CE09'S CONTRIBUTING AREA. * SEE DIVERSION DE09. *
	* DDM ***** Preserved *****
599	KK CD09
600	HC 2 0.666
	* * * * * DDM ***** Preserved *****
601	KK CD03
602	HC 5
	* * * * Route is a Minor Arterial Road. * N steps calculation did not work for this route.
603	KK RD03
604	RS 1 FLOW 0
605	RC 0.035 0.015 0.035 1307 0.004
606	RX 0 1 26 27 62 63 88 89
607	RY 8 2 1 0.5 0.5 1 2 8
	* *
608	KK C11 BASIN
609	BA .050
610	LG 0.25 0.25 4.40 0.45 49
611	UC 0.483 0.448
612	UA 0 5.0 16.0 30.0 65.0 77.0 84.0 90.0 94.0 97.0
613	UA 100
	* *
614	KK C10 BASIN
615	BA .054
616	LG 0.25 0.25 4.65 0.38 50
617	UC 0.592 0.579
618	UA 0 5.0 16.0 30.0 65.0 77.0 84.0 90.0 94.0 97.0
619	UA 100
	* * * Route is a Minor Arterial Road. *

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LINE	ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10
620	KK RC10
621	RS 13 FLOW 0
622	RC 0.035 0.015 0.035 688 0.009
623	RX 0 1 26 27 62 63 88 89
624	RY 8 2 1 0.5 0.5 1 2 8
	* * * DDM ***** Preserved *****
625	KK CC11
626	HC 3
	* * * Route is a Minor Arterial Road. * *

```

627      KK      RC11
628      RS        3      FLOW      0
629      RC      0.035    0.015    0.035    1334    0.0015
630      RX        0        1        26        27        62        63        88        89
631      RY        8        2        1        0.5    0.5        1        2        8
*
632      KK      C08      BASIN
633      BA      .118
634      LG      0.25    0.25    4.40    0.45    51
635      UC      0.804    0.615
636      UA        0        5.0    16.0    30.0    65.0    77.0    84.0    90.0    94.0    97.0
637      UA      100
*
* DDM      ***** Preserved *****

638      KK      CC08
639      HC        2
*
*
* Route is a Minor Arterial Road.
*

640      KK      RC08
641      RS        4      FLOW      0
642      RC      0.035    0.015    0.035    2205    0.0026
643      RX        0        1        26        27        62        63        88        89
644      RY        8        2        1        0.5    0.5        1        2        8
*

645      KK      C06      BASIN
646      KM
647      KM      This basin's original area was 0.162 sq mi. The infield area is bermed
648      KM      so it is self-detained. The infield area (0.0095 sq mi) was subtracted
649      KM      from the subbasin calculations to account for the storage.
650      KM

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LINE      ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

651      BA      .153
652      LG      0.23    0.25    4.30    0.49    52
653      UC      0.758    0.616
654      UA        0        5.0    16.0    30.0    65.0    77.0    84.0    90.0    94.0    97.0
655      UA      100
*
* DDM      ***** Preserved *****

656      KK      CC06
657      HC        2
*
*
* This is a channel.
* N steps calculation did not work for htis route.

658      KK      RC06
659      RS        1      FLOW      0
660      RC      0.04    0.035    0.04    1271    0.0056
661      RX        0        1        30        40        70        80        110    111
662      RY        20       10        8         0         0         8         10        20
*
* DIVERSION RECALL D01
*

663      KK      DRD01
664      DR      DD01I
*
*
* Route is a Minor Arterial Road.
*

665      KK      RD01S
666      RS        15     FLOW      0
667      RC      0.035    0.015    0.035    2611    0.0023
668      RX        0        1        26        27        62        63        88        89
669      RY        8        2        1        0.5    0.5        1        2        8
*

```

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```

*
*
670      KK      C07      BASIN
671      BA      .072
672      LG      0.25      0.25      4.80      0.35      50
673      UC      0.913      0.990
674      UA      0          5.0      16.0      30.0      65.0      77.0      84.0      90.0      94.0      97.0
675      UA      100

```

```

*
* THE CONTRIBUTING AREA WAS HARD-CODED TO ACCOUNT FOR THE DIVERSION DD01.
* THE HARD-CODE VALUE IS 40% OF CD01'S CONTRIBUTING AREA.
* SEE DIVERSION DD01.

```

```

* DDM ***** Preserved *****

```

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```

LINE      ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

```

```

676      KK      CC07
677      HC      2
*
*
* Route is a Minor Arterial Road.
*

```

```

678      KK      RC07
679      RS      8          FLOW      0
680      RC      0.035      0.015      0.035      2638      0.003
681      RX      0          1          26          27          62          63          88          89
682      RY      8          2          1          0.5      0.5          1          2          8
*
*

```

```

683      KK      C09      BASIN
684      BA      .057
685      LG      0.25      0.25      4.80      0.36      50
686      UC      0.817      0.917
687      UA      0          5.0      16.0      30.0      65.0      77.0      84.0      90.0      94.0      97.0
688      UA      100
*
*
* Route is a Minor Arterial Road.
*

```

```

689      KK      RC09
690      RS      10         FLOW      0
691      RC      0.035      0.015      0.035      3368      0.003
692      RX      0          1          26          27          62          63          88          89
693      RY      8          2          1          0          0          1          2          8
*
*

```

```

694      KK      C05      BASIN
695      BA      .137
696      LG      0.23      0.25      4.70      0.37      54
697      UC      1.021      0.945
698      UA      0          5.0      16.0      30.0      65.0      77.0      84.0      90.0      94.0      97.0
699      UA      100
*
*

```

```

700      KK      C01      BASIN
701      BA      .032
702      LG      0.22      0.25      4.80      0.37      57
703      UC      0.742      0.999
704      UA      0          5.0      16.0      30.0      65.0      77.0      84.0      90.0      94.0      97.0
705      UA      100
*
*
* Route is a Major Arterial Road.
*

```

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```

LINE      ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

```

706 KK RC01  
 707 RS 11 FLOW 0  
 708 RC 0.035 0.015 0.035 1217 0.0013  
 709 RX 0 1 40 41 104 105 145 146  
 710 RY 8 3 1.5 .75 0.75 1.5 3 8  
 \*

711 KK C02 BASIN  
 712 BA .036  
 713 LG 0.15 0.25 4.80 0.40 70  
 714 UC 0.733 1.065  
 715 UA 0 5.0 16.0 30.0 65.0 77.0 84.0 90.0 94.0 97.0  
 716 UA 100  
 \*

\* DDM \*\*\*\*\* Preserved \*\*\*\*\*

717 KK CC02  
 718 HC 5 0.431  
 \*  
 \*  
 \* Route is a Major Arterial Road.  
 \*  
 \*

719 KK RC02  
 720 RS 1 FLOW 0  
 721 RC 0.035 0.015 0.035 632 0.0063  
 722 RX 0 1 40 45 100 105 145 146  
 723 RY 8 3 2 0 0 2 3 8  
 \*

724 KK C03 BASIN  
 725 BA .040  
 726 LG 0.18 0.25 4.10 0.61 62  
 727 UC 0.438 0.526  
 728 UA 0 5.0 16.0 30.0 65.0 77.0 84.0 90.0 94.0 97.0  
 729 UA 100  
 \*  
 \*

730 KK C12 BASIN  
 731 BA .010  
 732 LG 0.19 0.25 4.10 0.58 64  
 733 UC 1.500 4.505  
 734 UA 0 5.0 16.0 30.0 65.0 77.0 84.0 90.0 94.0 97.0  
 735 UA 100  
 \*  
 \*

\* Route is a Minor Arterial Road with a wall on the west side.  
 \*

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LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

736 KK RC12  
 737 RS 45 FLOW 0  
 738 RC 0.035 0.015 0.035 2693 0.0007  
 739 RX 0 1 19 25 30 49 79 80  
 740 RY 10 1.5 0 0 0 0 1.5 8  
 \*  
 \*

741 KK C13 BASIN  
 742 BA .176  
 743 LG 0.10 0.29 2.65 1.70 80  
 744 UC 0.712 0.682  
 745 UA 0 5.0 16.0 30.0 65.0 77.0 84.0 90.0 94.0 97.0  
 746 UA 100  
 \*

\* \*\*\* STORAGE ROUTING LC13 \*\*\*\*\*

\* See the detention calculations in the Appendix.  
 \* DDM \*\*\*\*\* Preserved \*\*\*\*\*

747 KK LC13  
 748 DT LC13D 11.2  
 749 DI 0 100 1000 10000  
 750 DQ 0 100 1000 10000

\*  
\*  
\* DDM \*\*\*\*\* Preserved \*\*\*\*\*

751 KK CC13  
752 HC 2

\*  
\* \*\*\* DIVERSION DC13 \*\*\*\*\*  
\* THE STORM DRAIN ANALYSIS SHOWS THAT THE MAXIMUM FLOW IN THE DRAIN IS 51 CFS.  
\* ALL OVERLAND FLOW CONTINUES SOUTH.  
\* 20% clogging factor means 41cfs, (51cfs \* 0.8 = 41cfs).  
\* See Diversion Calculation in Appendix D of Hydrology Report.  
\*  
\* DDM \*\*\*\*\* Preserved \*\*\*\*\*

753 KK DC13  
754 DT DC13I  
755 DI 0 10 41 100 200 300 400 500  
756 DQ 0 0 0 59 159 259 359 459

\* ROUTE IS A 42" RCP WITH A SLOPE OF 0.0044 FT/FT.  
\*

757 KK RC13W  
758 RK 2052 0.005 0.012 CIRC 3.5

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LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

759 KK C04 BASIN  
760 BA .044  
761 LG 0.18 0.28 4.20 0.51 44  
762 UC 0.900 1.191  
763 UA 0 5.0 16.0 30.0 65.0 77.0 84.0 90.0 94.0 97.0  
764 UA 100

\*  
\*  
\* Route is a Major Arterial Road.  
\*

765 KK RC04  
766 RS 8 FLOW 0  
767 RC 0.035 0.015 0.035 1563 0.0106  
768 RX 0 1 40 41 104 105 145 146  
769 RY 8 3 1.5 0 0 1.5 3 8

\*  
\*  
\* THE CONTRIBUTING AREA IS HARD-CODED TO ACCOUNT FOR DIVERSION D13.  
\* THE HARD-CODE VALUE IS 40% OF CC13'S CONTRIBUTING AREA.  
\* SEE DIVERSION D13.  
\*  
\* DDM \*\*\*\*\* Preserved \*\*\*\*\*

770 KK CC03  
771 HC 5 3.085

\*  
\*  
\* This route is a channel.  
\*

772 KK RC03  
773 RS 3 FLOW 0  
774 RC 0.04 0.03 0.04 1097 0.0009  
775 RX 0 1 30 40 70 80 110 111  
776 RY 20 10 8 0 0 8 10 20

777 KK B07 BASIN  
778 BA .016  
779 LG 0.13 0.28 4.35 0.51 49  
780 UC 0.237 0.295  
781 UA 0 5.0 16.0 30.0 65.0 77.0 84.0 90.0 94.0 97.0  
782 UA 100

\* DDM \*\*\*\*\* Preserved \*\*\*\*\*

LINE	ID	1	2	3	4	5	6	7	8	9	10
783	KK	CB07									
784	HC	2									
	*										
	*										
	*	Route is a Minor Arterial Road.									
	*										
785	KK	RB07									
786	RS	3	FLOW	0							
787	RC	0.035	0.015	0.035	1525	0.0026					
788	RX	0	1	40	41	104	105	145	146		
789	RY	8	3	1.5	.75	0.75	1.5	3	8		
	*										
790	KK	B05	BASIN								
791	BA	.088									
792	LG	0.22	0.26	4.15	0.58	44					
793	UC	0.808	0.984								
794	UA	0	5.0	16.0	30.0	65.0	77.0	84.0	90.0	94.0	97.0
795	UA	100									
	*										
	*	*** STORAGE ROUTING LB05 *****									
	*										
	*	DDM	*****	Preserved	*****						
	*										
796	KK	LB05									
797	DT	LB05D	0.16								
798	DI	0	100	1000	10000						
799	DQ	0	100	1000	10000						
	*										
800	KK	B06	BASIN								
801	BA	.015									
802	LG	0.13	0.26	4.00	0.64	60					
803	UC	0.404	0.807								
804	UA	0	5.0	16.0	30.0	65.0	77.0	84.0	90.0	94.0	97.0
805	UA	100									
	*										
	*										
	*	DDM	*****	Preserved	*****						
	*										
806	KK	CB05									
807	HC	3									
	*										
	*										
	*	Route is a Minor Arterial Road.									
	*	N steps did not work for this route.									

LINE	ID	1	2	3	4	5	6	7	8	9	10
808	KK	RB05									
809	RD										
810	RC	0.035	0.02	0.035	500	0.0052					
811	RX	0	1	40	51	94	105	145	146		
812	RY	8	3	2	0	0	2	3	8		
	*										
813	KK	B04	BASIN								
814	BA	.077									
815	LG	0.15	0.25	4.45	0.50	71					
816	UC	0.350	0.308								
817	UA	0	5.0	16.0	30.0	65.0	77.0	84.0	90.0	94.0	97.0
818	UA	100									
	*										
	*										
	*	*** STORAGE ROUTING LB04 *****									
	*										
	*	MOTOROLA DETENTION BASINS (SEE CALCULATIONS)									
	*										
	*	DDM	*****	Preserved	*****						

819 KK LB04  
 820 DT LB04D 12.95  
 821 DI 0 100 1000 10000  
 822 DQ 0 100 1000 10000  
 \*

823 KK B02 BASIN  
 824 BA .101  
 825 LG 0.15 0.25 4.70 0.41 68  
 826 UC 0.475 0.451  
 827 UA 0 5.0 16.0 30.0 65.0 77.0 84.0 90.0 94.0 97.0  
 828 UA 100  
 \*

\* \*\*\* STORAGE ROUTING LB02 \*\*\*\*\*

\* DDM \*\*\*\*\* Preserved \*\*\*\*\*

829 KK LB02  
 830 DT LB02D 2.64  
 831 DI 0 100 1000 10000  
 832 DQ 0 100 1000 10000  
 \*

\* Route is a Minor Arterial Road.  
 \* N steps did not work for this route.

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LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

833 KK RB02  
 834 RD  
 835 RC 0.035 0.015 0.035 548 0.0186  
 836 RX 0 1 40 41 104 105 145 146  
 837 RY 8 3 1.5 .75 0.75 1.5 3 8  
 \*

838 KK B03 BASIN  
 839 BA .007  
 840 LG 0.25 0.25 4.65 0.41 47  
 841 UC 0.358 0.645  
 842 UA 0 5.0 16.0 30.0 65.0 77.0 84.0 90.0 94.0 97.0  
 843 UA 100  
 \*

\* DDM \*\*\*\*\* Preserved \*\*\*\*\*

844 KK CB03  
 845 HC 4 3.39  
 \*

\* Route is a Minor Arterial Road.  
 \*

846 KK RB03  
 847 KO 21  
 848 RS 3 FLOW 0  
 849 RC 0.035 0.015 0.035 1348 0.003  
 850 RX 0 1 40 41 104 105 145 146  
 851 RY 8 3 1.5 .75 0.75 1.5 3 8  
 \*

852 KK B01 BASIN  
 853 BA .071  
 854 LG 0.25 0.25 4.30 0.54 44  
 855 UC 0.363 0.264  
 856 UA 0 5.0 16.0 30.0 65.0 77.0 84.0 90.0 94.0 97.0  
 857 UA 100  
 \*

\* \*\*\* STORAGE ROUTING LB01 \*\*\*\*\*

\* DDM \*\*\*\*\* Preserved \*\*\*\*\*

858 KK LB01  
 859 DT LB01D 1.15

860 DI 0 100 1000 10000  
 861 DQ 0 100 1000 10000

\*  
 \*  
 \* DDM \*\*\*\*\* Preserved \*\*\*\*\*

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LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

862 KK CB01  
 863 HC 2

\*  
 \*  
 \* Route is a channel.  
 \* N steps calculation did not work for this route.

864 KK RB01  
 865 KO 21  
 866 RS 1 FLOW 0  
 867 RC 0.035 0.015 0.035 1320 0.0019  
 868 RX 0 1 34 40 50 56 80 81  
 869 RY 12 6 4 0 0 4 6 12  
 \*

870 KK B12 BASIN  
 871 BA .119  
 872 LG 0.25 0.25 4.80 0.38 39  
 873 UC 0.621 0.340  
 874 UA 0 5.0 16.0 30.0 65.0 77.0 84.0 90.0 94.0 97.0  
 875 UA 100  
 \*

876 KK B11 BASIN  
 877 BA .060  
 878 LG 0.24 0.25 4.80 0.39 37  
 879 UC 0.550 0.395  
 880 UA 0 5.0 16.0 30.0 65.0 77.0 84.0 90.0 94.0 97.0  
 881 UA 100  
 \*

\* \*\*\* STORAGE ROUTING LB11 \*\*\*\*\*  
 \* Pond was assumed to have 1 foot of freeboard.  
 \* DDM \*\*\*\*\* Preserved \*\*\*\*\*

882 KK LB11  
 883 DT LB11D 2.87  
 884 DI 0 100 1000 10000  
 885 DQ 0 100 1000 10000

\*  
 \*  
 \* DIVERSION RECALL DC13I  
 \*

886 KK DRDC13  
 887 DR DC13I  
 \*  
 \*  
 \* Route is overland flow. (SLOPE ESTIMATED.)  
 \*

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LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

888 KK RC13S  
 889 RS 62 FLOW 0  
 890 RC 0.035 0.035 0.035 5197 .0058  
 891 RX 0 200 300 400 500 600 700 900  
 892 RY 5 4 3 0 0 3 4 5  
 \*

893 KK B13 BASIN  
 894 BA .222  
 895 LG 0.22 0.26 4.55 0.42 44  
 896 UC 0.517 0.331  
 897 UA 0 5.0 16.0 30.0 65.0 77.0 84.0 90.0 94.0 97.0  
 898 UA 100  
 \*

\* \*\*\* STORAGE ROUTING LB13 \*\*\*\*\*

\*  
\* DDM \*\*\*\*\* Preserved \*\*\*\*\*

899 KK LB13  
900 DT LB13D 18.4  
901 DI 0 100 1000 10000  
902 DQ 0 100 1000 10000

\*  
\*  
\* Route is shallow flow. (SLOPE WAS ESTIMATED.)  
\*

903 KK RB13  
904 RS 1 FLOW 0  
905 RC 0.035 0.035 0.035 1269 0.004  
906 RX 0 200 300 400 500 600 700 900  
907 RY 5 4 3 0 0 3 4 5

908 KK B14 BASIN  
909 BA .149  
910 LG 0.10 0.25 4.60 0.48 80  
911 UC 0.792 0.842  
912 UA 0 5.0 16.0 30.0 65.0 77.0 84.0 90.0 94.0 97.0  
913 UA 100

\* \*\*\* STORAGE ROUTING LB14 \*\*\*\*\*

\*  
\* See the detention calculations in the Appendix.  
\* DDM \*\*\*\*\* Preserved \*\*\*\*\*

914 KK LB14  
915 DT LB14D 10.4  
916 DI 0 100 1000 10000  
917 DQ 0 100 1000 10000

\* DDM \*\*\*\*\* Preserved \*\*\*\*\*

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LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

918 KK CB14  
919 HC 3 0.484

\*  
\*  
\* Route is a MAJOR ARTERIAL. (SLOPE WAS ESTIMATED.)  
\*

920 KK RB14  
921 RS 12 FLOW 0  
922 RC 0.035 0.015 0.035 2746 0.004  
923 RX 0 1 30 40 100 110 145 300  
924 RY 8 4 3 0 0 3 4 5

\* DDM \*\*\*\*\* Preserved \*\*\*\*\*

925 KK CB12  
926 HC 4

\*  
\*  
\* Route is a channel. (SLOPE ESTIMATED FROM UPSTREAM CHANNEL'S SLOPE.)  
\*

927 KK RB12  
928 KO 21  
929 RS 8 FLOW 0  
930 RC 0.035 .035 0.035 4395 0.0019  
931 RX 0 200 330 340 370 380 410 600  
932 RY 9 8 6 0 0 6 8 9

933 KK A01 BASIN  
934 BA .507  
935 LG 0.10 0.25 5.40 0.32 80  
936 UC 0.704 0.318  
937 UA 0 5.0 16.0 30.0 65.0 77.0 84.0 90.0 94.0 97.0  
938 UA 100

\*  
 \* \*\*\* STORAGE ROUTING LA01 \*\*\*\*\*

\* See the detention calculations in the Appendix.  
 \* DDM \*\*\*\*\* Preserved \*\*\*\*\*

939 KK LA01  
 940 DT LA01D 35.2  
 941 DI 0 100 1000 10000  
 942 DQ 0 100 1000 10000

\* DDM \*\*\*\*\* Preserved \*\*\*\*\*

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LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

943 KK CA01  
 944 HC 2  
 \*

945 KK B10 BASIN  
 946 BA .110  
 947 LG 0.21 0.25 4.60 0.49 28  
 948 UC 1.075 1.154  
 949 UA 0 5.0 16.0 30.0 65.0 77.0 84.0 90.0 94.0 97.0  
 950 UA 100

\* \*\*\* STORAGE ROUTING LB10 \*\*\*\*\*

\* DDM \*\*\*\*\* Preserved \*\*\*\*\*

951 KK LB10  
 952 DT LB10D 10.5  
 953 DI 0 100 1000 10000  
 954 DQ 0 100 1000 10000

955 ZZ

RUNOFF SUMMARY  
FLOW IN CUBIC FEET PER SECOND  
TIME IN HOURS, AREA IN SQUARE MILES

OPERATION	STATION	PEAK FLOW	TIME OF PEAK	AVERAGE FLOW FOR MAXIMUM PERIOD			BASIN AREA	MAXIMUM STAGE
				6-HOUR	24-HOUR	72-HOUR		
HYDROGRAPH AT	G03	90.	4.17	14.	4.	2.	0.09	
DIVERSION TO	DG03I	45.	4.17	7.	2.	1.	0.09	
HYDROGRAPH AT	DG03	45.	4.17	7.	2.	1.	0.09	
ROUTED TO	RG03S	44.	4.33	7.	2.	1.	0.09	
HYDROGRAPH AT	G01	27.	4.42	7.	2.	1.	0.05	
2 COMBINED AT	CG01	71.	4.33	14.	4.	2.	0.10	
DIVERSION TO	DG01I	51.	4.33	10.	3.	2.	0.10	
HYDROGRAPH AT	DG01	20.	4.33	4.	1.	1.	0.10	
ROUTED TO	RG01S	15.	4.67	4.	1.	1.	0.10	
HYDROGRAPH AT	F04	35.	4.42	10.	3.	1.	0.06	
HYDROGRAPH AT	G04	45.	4.17	7.	2.	1.	0.05	
ROUTED TO	RG04	41.	4.25	7.	2.	1.	0.05	
HYDROGRAPH AT	G05	50.	4.17	7.	2.	1.	0.05	
ROUTED TO	RG05	43.	4.33	7.	2.	1.	0.05	
HYDROGRAPH AT	G06	178.	4.08	22.	6.	3.	0.13	
DIVERSION TO	LG06D	178.	4.08	16.	4.	2.	0.13	
HYDROGRAPH AT	LG06	101.	4.33	6.	2.	1.	0.13	
DIVERSION TO	DG06I	49.	4.33	3.	1.	0.	0.13	
HYDROGRAPH AT	DG06	52.	4.33	3.	1.	0.	0.13	
DIVERSION TO	LG06AD	52.	4.33	3.	1.	0.	0.13	
HYDROGRAPH AT	LG06A	0.	0.08	0.	0.	0.	0.13	
HYDROGRAPH AT	G02	24.	4.83	10.	3.	2.	0.08	
4 COMBINED AT	CG02	96.	4.33	23.	6.	4.	0.23	
ROUTED TO	RG02	92.	4.42	23.	6.	4.	0.23	

	OPERATION	STATION	PEAK FLOW	TIME OF PEAK	AVERAGE FLOW FOR MAXIMUM PERIOD			BASIN AREA	MAXIMUM STAGE
					6-HOUR	24-HOUR	72-HOUR		
+	HYDROGRAPH AT	F06	65.	4.33	14.	4.	2.	0.10	
+	2 COMBINED AT	CF06	157.	4.42	38.	10.	6.	0.33	
+	ROUTED TO	RF06	156.	4.50	38.	10.	6.	0.33	
+	HYDROGRAPH AT	F05	60.	4.33	13.	3.	2.	0.09	
+	2 COMBINED AT	CF05	212.	4.50	50.	13.	8.	0.42	
+	DIVERSION TO	DF05I	163.	4.50	27.	7.	4.	0.42	
+	HYDROGRAPH AT	DF05	49.	4.17	23.	6.	4.	0.42	
+	ROUTED TO	RF05W	49.	4.25	23.	6.	4.	0.42	
+	3 COMBINED AT	CF04	97.	4.50	37.	10.	6.	0.16	
+	DIVERSION TO	DF04I	52.	4.50	20.	5.	3.	0.16	
+	HYDROGRAPH AT	DF04	45.	4.50	17.	5.	3.	0.16	
+	HYDROGRAPH AT	F05R	163.	4.50	27.	7.	4.	0.42	
+	ROUTED TO	RF05S	155.	4.58	27.	7.	4.	0.42	
+	HYDROGRAPH AT	F03	111.	4.42	26.	7.	4.	0.17	
+	2 COMBINED AT	CF03	258.	4.58	52.	14.	8.	0.51	
+	ROUTED TO	RF03	259.	4.58	52.	14.	8.	0.51	
+	HYDROGRAPH AT	G06R	49.	4.33	3.	1.	0.	0.13	
+	ROUTED TO	RG06S	33.	4.92	3.	1.	0.	0.13	
+	HYDROGRAPH AT	F07	192.	4.33	43.	11.	6.	0.25	
+	DIVERSION TO	LF07D	192.	4.33	31.	8.	4.	0.25	
+	HYDROGRAPH AT	LF07	102.	4.92	14.	3.	2.	0.25	
+	2 COMBINED AT	CF07	135.	4.92	17.	4.	2.	0.31	
+	DIVERSION TO	DF07I	122.	4.92	12.	3.	2.	0.31	
+	HYDROGRAPH AT	DF07	13.	4.75	5.	1.	1.	0.31	
+	ROUTED TO	RF07W	13.	5.33	5.	1.	1.	0.31	
	HYDROGRAPH AT								

OPERATION	STATION	PEAK FLOW	TIME OF PEAK	AVERAGE FLOW FOR MAXIMUM PERIOD			BASIN AREA	MAXIMUM STAGE
				6-HOUR	24-HOUR	72-HOUR		
+ 3 COMBINED AT	F02	28.	4.33	7.	2.	1.	0.05	
+ DIVERSION TO	CF02	277.	4.58	63.	16.	9.	0.59	
+ HYDROGRAPH AT	DF02I	237.	4.58	50.	13.	7.	0.59	
+ ROUTED TO	DF02	40.	4.17	14.	3.	2.	0.59	
+ HYDROGRAPH AT	RF02W	40.	5.25	14.	3.	2.	0.59	
+ ROUTED TO	F04R	52.	4.50	20.	5.	3.	0.16	
+ DIVERSION TO	RF04S	54.	4.50	20.	5.	3.	0.16	
+ HYDROGRAPH AT	DRF04I	27.	4.50	10.	3.	2.	0.16	
+ HYDROGRAPH AT	DRF04	27.	4.50	10.	3.	2.	0.16	
+ 3 COMBINED AT	F01	19.	4.33	4.	1.	1.	0.03	
+ DIVERSION TO	CF01	84.	4.50	28.	7.	4.	0.19	
+ HYDROGRAPH AT	DF01I	0.	0.08	0.	0.	0.	0.19	
+ HYDROGRAPH AT	DF01	84.	4.50	28.	7.	4.	0.19	
+ HYDROGRAPH AT	DRF02	237.	4.58	50.	13.	7.	0.59	
+ ROUTED TO	RF02S	232.	4.67	50.	13.	7.	0.59	
+ HYDROGRAPH AT	E07	69.	4.25	12.	3.	2.	0.08	
+ 2 COMBINED AT	CE07	279.	4.58	62.	16.	9.	0.55	
+ ROUTED TO	RE07	274.	4.67	62.	16.	9.	0.55	
+ HYDROGRAPH AT	E05	43.	4.25	7.	2.	1.	0.05	
+ 2 COMBINED AT	CE05	291.	4.67	68.	18.	10.	0.60	
+ DIVERSION TO	DE05I	266.	4.67	53.	13.	8.	0.60	
+ HYDROGRAPH AT	DE05	25.	4.00	15.	4.	2.	0.60	
+ ROUTED TO	RE05W	25.	4.08	15.	4.	2.	0.60	
+ HYDROGRAPH AT	E04	18.	4.42	5.	1.	1.	0.04	
+ DIVERSION TO	LE04D	18.	4.42	3.	1.	0.	0.04	
+ HYDROGRAPH AT	LE04	12.	5.08	2.	1.	0.	0.04	

10-year, 6-hour HEC-1 (Future Conditions)

OPERATION	STATION	PEAK FLOW	TIME OF PEAK	AVERAGE FLOW FOR MAXIMUM PERIOD			BASIN AREA	MAXIMUM STAGE
				6-HOUR	24-HOUR	72-HOUR		
HYDROGRAPH AT	DRF01I	0.	0.08	0.	0.	0.	0.19	
ROUTED TO	RF01S	0.	0.08	0.	0.	0.	0.19	
3 COMBINED AT	CE04	37.	5.08	17.	5.	3.	0.10	
DIVERSION TO	DE04I	0.	0.08	0.	0.	0.	0.10	
HYDROGRAPH AT	DE04	37.	5.08	17.	5.	3.	0.10	
3 COMBINED AT	DUMMY	153.	4.50	62.	16.	9.	0.45	
HYDROGRAPH AT	E08	18.	4.33	4.	1.	1.	0.03	
ROUTED TO	RE08	18.	4.50	4.	1.	1.	0.03	
HYDROGRAPH AT	E06	21.	4.33	5.	1.	1.	0.04	
2 COMBINED AT	CE06	39.	4.42	10.	2.	1.	0.07	
ROUTED TO	RE06	38.	4.92	10.	2.	1.	0.07	
HYDROGRAPH AT	E03	20.	4.83	9.	3.	2.	0.07	
HYDROGRAPH AT	DRF07	122.	4.92	12.	3.	2.	0.31	
ROUTED TO	RF07S	111.	5.42	12.	3.	2.	0.31	
HYDROGRAPH AT	E09	165.	4.33	39.	10.	6.	0.22	
DIVERSION TO	LE09D	165.	4.33	27.	7.	4.	0.22	
HYDROGRAPH AT	LE09	86.	4.92	12.	3.	2.	0.22	
2 COMBINED AT	CE09	158.	5.42	23.	6.	3.	0.50	
DIVERSION TO	DE09I	143.	5.42	19.	5.	3.	0.50	
HYDROGRAPH AT	DE09	15.	4.92	5.	1.	1.	0.50	
ROUTED TO	RE09W	15.	5.25	5.	1.	1.	0.50	
HYDROGRAPH AT	DRE05I	266.	4.67	53.	13.	8.	0.60	
ROUTED TO	RE05S	263.	4.83	53.	13.	8.	0.60	
4 COMBINED AT	CE03	306.	4.83	74.	19.	11.	0.73	
DIVERSION TO	DE03I	275.	4.83	55.	14.	8.	0.73	
HYDROGRAPH AT								

10-year, 6-hour HEC-1 (Future Conditions)

OPERATION	STATION	PEAK FLOW	TIME OF PEAK	AVERAGE FLOW FOR MAXIMUM PERIOD			BASIN AREA	MAXIMUM STAGE
				6-HOUR	24-HOUR	72-HOUR		
+ ROUTED TO	DE03	31.	4.25	19.	5.	3.	0.73	
+ HYDROGRAPH AT	RE03W	31.	4.42	19.	5.	3.	0.73	
+ 2 COMBINED AT	E02	81.	4.42	21.	5.	3.	0.15	
+ DIVERSION TO	CE02	112.	4.42	39.	10.	6.	0.16	
+ HYDROGRAPH AT	DE02I	66.	4.42	12.	3.	2.	0.16	
+ ROUTED TO	DE02	46.	4.08	27.	7.	4.	0.16	
+ HYDROGRAPH AT	RE02W	46.	4.92	27.	7.	4.	0.16	
+ HYDROGRAPH AT	E01	39.	4.33	9.	2.	1.	0.06	
+ ROUTED TO	DRE04I	0.	0.08	0.	0.	0.	0.10	
+ 3 COMBINED AT	RE04S	0.	0.08	0.	0.	0.	0.10	
+ DIVERSION TO	CE01	84.	4.33	36.	10.	6.	0.12	
+ HYDROGRAPH AT	DE01I	23.	4.33	3.	1.	0.	0.12	
+ HYDROGRAPH AT	DE01	61.	4.08	34.	9.	5.	0.12	
+ ROUTED TO	DRE01	23.	4.33	3.	1.	0.	0.12	
+ HYDROGRAPH AT	RE01S	23.	4.58	3.	1.	0.	0.12	
+ 2 COMBINED AT	D05	86.	4.42	22.	6.	3.	0.16	
+ DIVERSION TO	CD05	107.	4.50	25.	6.	4.	0.20	
+ HYDROGRAPH AT	DD05I	55.	4.50	12.	3.	2.	0.20	
+ ROUTED TO	DD05	52.	4.50	13.	3.	2.	0.20	
+ HYDROGRAPH AT	RD05S	52.	4.67	13.	3.	2.	0.20	
+ ROUTED TO	D04	27.	4.50	9.	2.	1.	0.06	
+ HYDROGRAPH AT	RD04	27.	4.58	9.	2.	1.	0.06	
+ 3 COMBINED AT	D01	42.	4.42	12.	3.	2.	0.08	
+ DIVERSION TO	CD01	119.	4.67	33.	8.	5.	0.34	
+ HYDROGRAPH AT	DD01I	53.	4.67	15.	4.	2.	0.34	
+ ROUTED TO	DD01	66.	4.67	18.	5.	3.	0.34	

10-year, 6-hour HEC-1 (Future Conditions)

OPERATION	STATION	PEAK FLOW	TIME OF PEAK	AVERAGE FLOW FOR MAXIMUM PERIOD			BASIN AREA	MAXIMUM STAGE
				6-HOUR	24-HOUR	72-HOUR		
ROUTED TO	RD01E	62.	4.83	18.	5.	3.	0.34	
HYDROGRAPH AT	DRD05	55.	4.50	12.	3.	2.	0.20	
ROUTED TO	RD05E	55.	4.58	12.	3.	2.	0.20	
DIVERSION TO	DD052I	45.	4.58	12.	3.	2.	0.20	
HYDROGRAPH AT	DD052	10.	4.58	1.	0.	0.	0.20	
ROUTED TO	RD05S2	9.	4.83	1.	0.	0.	0.20	
HYDROGRAPH AT	D02	87.	4.33	20.	5.	3.	0.14	
3 COMBINED AT	CD02	136.	4.67	38.	10.	6.	0.30	
ROUTED TO	RD02	136.	4.75	38.	10.	6.	0.30	
HYDROGRAPH AT	DRE02	66.	4.42	12.	3.	2.	0.16	
ROUTED TO	RE02S	59.	4.67	12.	3.	2.	0.16	
HYDROGRAPH AT	DRD052	45.	4.58	12.	3.	2.	0.20	
ROUTED TO	RD05E2	45.	4.67	12.	3.	2.	0.20	
HYDROGRAPH AT	D06	57.	4.42	16.	4.	2.	0.11	
3 COMBINED AT	CD06	157.	4.67	39.	10.	6.	0.28	
ROUTED TO	RD06	157.	4.67	39.	10.	6.	0.28	
HYDROGRAPH AT	D07	27.	4.33	7.	2.	1.	0.05	
2 COMBINED AT	CD07	180.	4.67	46.	12.	7.	0.33	
ROUTED TO	RD07	179.	4.83	46.	12.	7.	0.33	
HYDROGRAPH AT	DRE03	275.	4.83	55.	14.	8.	0.73	
ROUTED TO	RE03S	273.	5.00	55.	14.	8.	0.73	
HYDROGRAPH AT	D08	21.	4.50	8.	2.	1.	0.05	
2 COMBINED AT	CD08	292.	5.00	63.	16.	9.	0.71	
ROUTED TO	RD08	290.	5.17	63.	16.	9.	0.71	
HYDROGRAPH AT	D03	53.	4.42	14.	4.	2.	0.10	
HYDROGRAPH AT								

OPERATION	STATION	PEAK FLOW	TIME OF PEAK	AVERAGE FLOW FOR MAXIMUM PERIOD			BASIN AREA	MAXIMUM STAGE
				6-HOUR	24-HOUR	72-HOUR		
+ ROUNDED TO	DRE09I	143.	5.42	19.	5.	3.	0.50	
+ HYDROGRAPH AT	RE09S	116.	5.67	19.	5.	3.	0.50	
+ DIVERSION TO	D09	179.	4.25	38.	10.	6.	0.22	
+ HYDROGRAPH AT	LD09D	179.	4.25	27.	7.	4.	0.22	
+ 2 COMBINED AT	LD09	92.	4.83	11.	3.	2.	0.22	
+ 5 COMBINED AT	CD09	132.	5.67	29.	7.	4.	0.67	
+ ROUNDED TO	CD03	541.	5.00	164.	42.	24.	2.12	
+ HYDROGRAPH AT	RD03	534.	5.08	164.	42.	24.	2.12	
+ HYDROGRAPH AT	C11	41.	4.25	7.	2.	1.	0.05	
+ HYDROGRAPH AT	C10	38.	4.25	8.	2.	1.	0.05	
+ ROUNDED TO	RC10	38.	4.33	8.	2.	1.	0.05	
+ 3 COMBINED AT	CC11	553.	5.08	175.	45.	26.	2.22	
+ ROUNDED TO	RC11	549.	5.17	175.	45.	26.	2.22	
+ HYDROGRAPH AT	C08	74.	4.33	17.	4.	2.	0.12	
+ 2 COMBINED AT	CC08	572.	5.17	188.	49.	28.	2.34	
+ ROUNDED TO	RC08	565.	5.25	188.	49.	28.	2.34	
+ HYDROGRAPH AT	C06	97.	4.33	22.	5.	3.	0.15	
+ 2 COMBINED AT	CC06	591.	5.25	205.	53.	31.	2.49	
+ ROUNDED TO	RC06	589.	5.33	205.	53.	31.	2.49	
+ HYDROGRAPH AT	DRD01	53.	4.67	15.	4.	2.	0.34	
+ ROUNDED TO	RD01S	53.	4.83	15.	4.	2.	0.34	
+ HYDROGRAPH AT	C07	34.	4.50	10.	3.	2.	0.07	
+ 2 COMBINED AT	CC07	84.	4.67	25.	6.	4.	0.07	
+ ROUNDED TO	RC07	84.	4.83	25.	6.	4.	0.07	
+ HYDROGRAPH AT	C09	28.	4.42	8.	2.	1.	0.06	
+ ROUNDED TO	RC09	28.	4.75	8.	2.	1.	0.06	

10-year, 6-hour HEC-1 (Future Conditions)

	OPERATION	STATION	PEAK FLOW	TIME OF PEAK	AVERAGE FLOW FOR MAXIMUM PERIOD			BASIN AREA	MAXIMUM STAGE
					6-HOUR	24-HOUR	72-HOUR		
+	HYDROGRAPH AT	C05	66.	4.50	20.	5.	3.	0.14	
+	HYDROGRAPH AT	C01	16.	4.42	5.	1.	1.	0.03	
+	ROUTED TO	RC01	16.	4.58	5.	1.	1.	0.03	
+	HYDROGRAPH AT	C02	19.	4.42	6.	2.	1.	0.04	
+	5 COMBINED AT	CC02	200.	4.83	63.	16.	9.	0.43	
+	ROUTED TO	RC02	200.	4.83	63.	16.	9.	0.43	
+	HYDROGRAPH AT	C03	32.	4.17	6.	2.	1.	0.04	
+	HYDROGRAPH AT	C12	2.	5.25	1.	0.	0.	0.01	
+	ROUTED TO	RC12	2.	5.75	1.	0.	0.	0.01	
+	HYDROGRAPH AT	C13	121.	4.33	30.	8.	4.	0.18	
+	DIVERSION TO	LC13D	121.	4.33	23.	6.	3.	0.18	
+	HYDROGRAPH AT	LC13	55.	5.17	9.	2.	1.	0.18	
+	2 COMBINED AT	CC13	56.	5.17	10.	3.	1.	0.19	
+	DIVERSION TO	DC13I	15.	5.17	1.	0.	0.	0.19	
+	HYDROGRAPH AT	DC13	41.	5.08	9.	2.	1.	0.19	
+	ROUTED TO	RC13W	41.	5.17	9.	2.	1.	0.19	
+	HYDROGRAPH AT	C04	17.	4.50	5.	1.	1.	0.04	
+	ROUTED TO	RC04	17.	4.58	5.	1.	1.	0.04	
+	5 COMBINED AT	CC03	742.	5.33	272.	71.	41.	3.09	
+	ROUTED TO	RC03	738.	5.42	272.	71.	41.	3.09	
+	HYDROGRAPH AT	B07	18.	4.08	2.	1.	0.	0.02	
+	2 COMBINED AT	CB07	738.	5.42	273.	71.	41.	3.10	
+	ROUTED TO	RB07	732.	5.50	273.	71.	41.	3.10	
+	HYDROGRAPH AT	B05	38.	4.42	11.	3.	2.	0.09	
+	DIVERSION TO	LB05D	1.	2.25	0.	0.	0.	0.09	
	HYDROGRAPH AT								

	OPERATION	STATION	PEAK FLOW	TIME OF PEAK	AVERAGE FLOW FOR MAXIMUM PERIOD			BASIN AREA	MAXIMUM STAGE
					6-HOUR	24-HOUR	72-HOUR		
+		LB05	38.	4.42	11.	3.	2.	0.09	
	HYDROGRAPH AT								
+		B06	9.	4.25	2.	1.	0.	0.01	
	3 COMBINED AT								
+		CB05	744.	5.50	283.	74.	42.	3.20	
	ROUTED TO								
+		RB05	744.	5.50	283.	74.	42.	3.20	
	HYDROGRAPH AT								
+		B04	91.	4.17	13.	3.	2.	0.08	
	DIVERSION TO								
+		LB04D	91.	4.17	13.	3.	2.	0.08	
	HYDROGRAPH AT								
+		LB04	0.	0.08	0.	0.	0.	0.08	
	HYDROGRAPH AT								
+		B02	93.	4.25	17.	4.	2.	0.10	
	DIVERSION TO								
+		LB02D	74.	4.08	5.	1.	1.	0.10	
	HYDROGRAPH AT								
+		LB02	93.	4.25	12.	3.	2.	0.10	
	ROUTED TO								
+		RB02	93.	4.25	12.	3.	2.	0.10	
	HYDROGRAPH AT								
+		B03	5.	4.17	1.	0.	0.	0.01	
	4 COMBINED AT								
+		CB03	750.	5.50	292.	76.	44.	3.39	
	ROUTED TO								
+		RB03	750.	5.50	292.	76.	44.	3.39	
	HYDROGRAPH AT								
+		B01	74.	4.17	9.	2.	1.	0.07	
	DIVERSION TO								
+		LB01D	35.	3.92	2.	1.	0.	0.07	
	HYDROGRAPH AT								
+		LB01	74.	4.17	7.	2.	1.	0.07	
	2 COMBINED AT								
+		CB01	750.	5.50	296.	77.	44.	3.46	
	ROUTED TO								
+		RB01	746.	5.58	296.	77.	44.	3.46	
	HYDROGRAPH AT								
+		B12	101.	4.25	15.	4.	2.	0.12	
	HYDROGRAPH AT								
+		B11	48.	4.25	7.	2.	1.	0.06	
	DIVERSION TO								
+		LB11D	48.	4.25	6.	1.	1.	0.06	
	HYDROGRAPH AT								
+		LB11	17.	4.83	2.	0.	0.	0.06	
	HYDROGRAPH AT								
+		DRDC13	15.	5.17	1.	0.	0.	0.19	
	ROUTED TO								
+		RC13S	8.	6.42	1.	0.	0.	0.19	
	HYDROGRAPH AT								
+		B13	201.	4.25	29.	7.	4.	0.22	

10-year, 6-hour HEC-1 (Future Conditions)

OPERATION	STATION	PEAK FLOW	TIME OF PEAK	AVERAGE FLOW FOR MAXIMUM PERIOD			BASIN AREA	MAXIMUM STAGE
				6-HOUR	24-HOUR	72-HOUR		
DIVERSION TO	LB13D	201.	4.25	29.	7.	4.	0.22	
HYDROGRAPH AT	LB13	0.	0.08	0.	0.	0.	0.22	
ROUTED TO	RB13	0.	0.08	0.	0.	0.	0.22	
HYDROGRAPH AT	B14	96.	4.42	27.	7.	4.	0.15	
DIVERSION TO	LB14D	96.	4.42	21.	5.	3.	0.15	
HYDROGRAPH AT	LB14	39.	5.50	7.	2.	1.	0.15	
3 COMBINED AT	CB14	39.	5.50	7.	2.	1.	0.48	
ROUTED TO	RB14	37.	5.83	7.	2.	1.	0.48	
4 COMBINED AT	CB12	746.	5.58	311.	81.	47.	4.12	
ROUTED TO	RB12	725.	5.75	307.	80.	46.	4.12	
HYDROGRAPH AT	A01	556.	4.25	95.	24.	14.	0.51	
DIVERSION TO	LA01D	556.	4.25	71.	18.	10.	0.51	
HYDROGRAPH AT	LA01	249.	4.75	24.	6.	4.	0.51	
2 COMBINED AT	CA01	753.	5.67	321.	84.	48.	4.63	
HYDROGRAPH AT	B10	35.	4.58	11.	3.	2.	0.11	
DIVERSION TO	LB10D	35.	4.58	11.	3.	2.	0.11	
HYDROGRAPH AT	LB10	0.	0.08	0.	0.	0.	0.11	

\*\*\* NORMAL END OF HEC-1 \*\*\*

**APPENDIX H 100-year, 24-hour future conditions HEC-1 SUMMARY**



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*****
*
* FLOOD HYDROGRAPH PACKAGE (HEC-1)
*   MAY 1991
*   VERSION 4.0.1E
*   Lahey F77L-EM/32 version 5.01
*   Dodson & Associates, Inc.
*   RUN DATE 04/30/02 TIME 10:38:11
*****

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*****
*
* U.S. ARMY CORPS OF ENGINEERS
* HYDROLOGIC ENGINEERING CENTER
* 609 SECOND STREET
* DAVIS, CALIFORNIA 95616
* (916) 551-1748
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THIS PROGRAM REPLACES ALL PREVIOUS VERSIONS OF HEC-1 KNOWN AS HEC1 (JAN 73), HEC1GS, HEC1DB, AND HEC1KW.

THE DEFINITIONS OF VARIABLES -RTIMP- AND -RTIOR- HAVE CHANGED FROM THOSE USED WITH THE 1973-STYLE INPUT STRUCTURE. THE DEFINITION OF -AMSKK- ON RM-CARD WAS CHANGED WITH REVISIONS DATED 28 SEP 81. THIS IS THE FORTRAN77 VERSION

NEW OPTIONS: DAMBREAK OUTFLOW SUBMERGENCE , SINGLE EVENT DAMAGE CALCULATION, DSS:WRITE STAGE FREQUENCY, DSS:READ TIME SERIES AT DESIRED CALCULATION INTERVAL LOSS RATE:GREEN AND AMPT INFILTRATION KINEMATIC WAVE: NEW FINITE DIFFERENCE ALGORITHM

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1
                                     HEC-1 INPUT
                                     PAGE 1

LINE      ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10
1         ID      Project ID: SCOTTSDALE - Major Basin: 01 - Return Period: 100
2         ID      *****
3         ID      **
4         ID      **
5         ID      **
6         ID      *****
7         ID      *****
8         ID      PROJECT: Granite Reef ADMP
9         ID      CLIENT: Flood Control District of Maricopa County
10        ID      PREPARED BY: Entellus, Inc.
11        ID      PROJECT No: FCD Entellus 310.021
12        ID      FILE NAME: FUT-24.DAT CREATED DATE: Mar 28, 2002
13        ID      MODIFIED:
14        ID      STORM: 100-year 24-hour Storm
15        ID      DEVELOPMENT CONDITIONS: Existing Conditions
16        ID
17        ID

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RUNOFF SUMMARY  
FLOW IN CUBIC FEET PER SECOND  
TIME IN HOURS, AREA IN SQUARE MILES

OPERATION	STATION	PEAK FLOW	TIME OF PEAK	AVERAGE FLOW FOR MAXIMUM PERIOD			BASIN AREA	MAXIMUM STAGE
				6-HOUR	24-HOUR	72-HOUR		
HYDROGRAPH AT	G03	133.	12.17	21.	7.	5.	0.09	
DIVERSION TO	DG03I	68.	12.17	10.	3.	2.	0.09	
HYDROGRAPH AT	DG03	65.	12.17	10.	3.	2.	0.09	
ROUTED TO	RG03S	59.	12.33	10.	3.	2.	0.09	
HYDROGRAPH AT	G01	42.	12.25	11.	4.	3.	0.05	
2 COMBINED AT	CG01	101.	12.33	21.	7.	5.	0.10	
DIVERSION TO	DG01I	101.	12.33	21.	7.	5.	0.10	
HYDROGRAPH AT	DG01	0.	0.08	0.	0.	0.	0.10	

	OPERATION	STATION	PEAK FLOW	TIME OF PEAK	AVERAGE FLOW FOR MAXIMUM PERIOD			BASIN AREA	MAXIMUM STAGE
					6-HOUR	24-HOUR	72-HOUR		
+	ROUTED TO	RG01S	0.	0.08	0.	0.	0.	0.10	
+	HYDROGRAPH AT	F04	52.	12.33	14.	5.	4.	0.06	
+	HYDROGRAPH AT	G04	65.	12.08	10.	3.	2.	0.05	
+	ROUTED TO	RG04	64.	12.17	10.	3.	2.	0.05	
+	HYDROGRAPH AT	G05	68.	12.17	11.	3.	2.	0.05	
+	ROUTED TO	RG05	66.	12.25	11.	3.	2.	0.05	
+	HYDROGRAPH AT	G06	236.	12.08	31.	11.	8.	0.13	
+	DIVERSION TO	LG06D	112.	11.92	13.	4.	3.	0.13	
+	HYDROGRAPH AT	LG06	236.	12.08	24.	7.	5.	0.13	
+	DIVERSION TO	DG06I	96.	12.08	11.	3.	2.	0.13	
+	HYDROGRAPH AT	DG06	141.	12.08	13.	4.	3.	0.13	
+	DIVERSION TO	LG06AD	141.	12.08	9.	2.	2.	0.13	
+	HYDROGRAPH AT	LG06A	18.	12.50	5.	2.	1.	0.13	
+	HYDROGRAPH AT	G02	69.	12.25	16.	5.	4.	0.08	
+	4 COMBINED AT	CG02	194.	12.25	39.	13.	9.	0.23	
+	ROUTED TO	RG02	189.	12.33	39.	13.	9.	0.23	
+	HYDROGRAPH AT	F06	104.	12.25	21.	7.	5.	0.10	
+	2 COMBINED AT	CF06	288.	12.33	60.	20.	14.	0.33	
+	ROUTED TO	RF06	278.	12.42	60.	20.	14.	0.33	
+	HYDROGRAPH AT	F05	95.	12.25	19.	6.	4.	0.09	
+	2 COMBINED AT	CF05	360.	12.33	78.	26.	19.	0.42	
+	DIVERSION TO	DF05I	310.	12.33	45.	11.	8.	0.42	
+	HYDROGRAPH AT	DF05	50.	11.83	33.	14.	11.	0.42	
+	ROUTED TO	RF05W	50.	12.00	33.	14.	11.	0.42	
+	3 COMBINED AT	CF04	101.	12.33	48.	19.	14.	0.17	
+	DIVERSION TO	DF04I	54.	12.33	25.	10.	8.	0.17	
	HYDROGRAPH AT								

OPERATION	STATION	PEAK FLOW	TIME OF PEAK	AVERAGE FLOW FOR MAXIMUM PERIOD			BASIN AREA	MAXIMUM STAGE
				6-HOUR	24-HOUR	72-HOUR		
+ HYDROGRAPH AT	DF04	47.	12.33	22.	9.	6.	0.17	
+ Routed to	F05R	310.	12.33	45.	11.	8.	0.42	
+ HYDROGRAPH AT	RF05S	304.	12.50	45.	11.	8.	0.42	
+ 2 COMBINED AT	F03	167.	12.33	37.	12.	9.	0.17	
+ Routed to	CF03	453.	12.42	81.	23.	17.	0.56	
+ HYDROGRAPH AT	RF03	452.	12.50	81.	23.	17.	0.56	
+ Routed to	G06R	96.	12.08	11.	3.	2.	0.13	
+ HYDROGRAPH AT	RG06S	71.	12.42	11.	3.	2.	0.13	
+ DIVERSION TO	F07	268.	12.25	61.	21.	15.	0.25	
+ HYDROGRAPH AT	LF07D	214.	12.08	25.	8.	6.	0.25	
+ 2 COMBINED AT	LF07	268.	12.25	48.	14.	10.	0.25	
+ DIVERSION TO	CF07	308.	12.42	58.	17.	12.	0.31	
+ HYDROGRAPH AT	DF07I	295.	12.42	45.	11.	8.	0.31	
+ Routed to	DF07	13.	12.17	13.	6.	4.	0.31	
+ HYDROGRAPH AT	RF07W	13.	12.92	13.	6.	4.	0.31	
+ 3 COMBINED AT	F02	44.	12.25	10.	3.	2.	0.05	
+ DIVERSION TO	CF02	502.	12.42	101.	32.	23.	0.63	
+ HYDROGRAPH AT	DF02I	472.	12.42	81.	23.	17.	0.63	
+ Routed to	DF02	30.	11.92	20.	9.	6.	0.63	
+ HYDROGRAPH AT	RF02W	30.	12.00	20.	9.	6.	0.63	
+ Routed to	F04R	54.	12.33	25.	10.	8.	0.17	
+ DIVERSION TO	RF04S	52.	12.50	25.	10.	8.	0.17	
+ HYDROGRAPH AT	DRF04I	26.	12.50	13.	5.	4.	0.17	
+ Routed to	DRF04	26.	12.50	13.	5.	4.	0.17	
+ 3 COMBINED AT	F01	30.	12.25	6.	2.	2.	0.03	
+ Routed to	CF01	82.	12.33	39.	16.	12.	0.10	

100-year, 24-hour HEC-1 (Future Conditions)

OPERATION	STATION	PEAK FLOW	TIME OF PEAK	AVERAGE FLOW FOR MAXIMUM PERIOD			BASIN AREA	MAXIMUM STAGE
				6-HOUR	24-HOUR	72-HOUR		
DIVERSION TO	DF01I	0.	0.08	0.	0.	0.	0.10	
HYDROGRAPH AT	DF01	82.	12.33	39.	16.	12.	0.10	
HYDROGRAPH AT	DRF02	472.	12.42	81.	23.	17.	0.63	
ROUTED TO	RF02S	466.	12.50	81.	23.	17.	0.63	
HYDROGRAPH AT	E07	107.	12.17	17.	6.	4.	0.08	
2 COMBINED AT	CE07	528.	12.50	97.	29.	21.	0.66	
ROUTED TO	RE07	523.	12.58	97.	29.	21.	0.66	
HYDROGRAPH AT	E05	65.	12.17	10.	3.	2.	0.05	
2 COMBINED AT	CE05	554.	12.50	107.	32.	23.	0.71	
DIVERSION TO	DE05I	529.	12.50	92.	25.	18.	0.71	
HYDROGRAPH AT	DE05	25.	11.92	15.	7.	5.	0.71	
ROUTED TO	RE05W	25.	12.08	15.	7.	5.	0.71	
HYDROGRAPH AT	E04	30.	12.33	8.	2.	2.	0.04	
DIVERSION TO	LE04D	26.	12.17	2.	1.	1.	0.04	
HYDROGRAPH AT	LE04	30.	12.33	6.	2.	1.	0.04	
HYDROGRAPH AT	DRF01I	0.	0.08	0.	0.	0.	0.10	
ROUTED TO	RF01S	0.	0.08	0.	0.	0.	0.10	
3 COMBINED AT	CE04	54.	12.33	21.	9.	6.	0.07	
DIVERSION TO	DE04I	0.	0.08	0.	0.	0.	0.07	
HYDROGRAPH AT	DE04	54.	12.33	21.	9.	6.	0.07	
3 COMBINED AT	DUMMY	182.	12.33	82.	33.	24.	0.34	
HYDROGRAPH AT	E08	30.	12.25	7.	2.	2.	0.03	
ROUTED TO	RE08	28.	12.42	7.	2.	2.	0.03	
HYDROGRAPH AT	E06	34.	12.25	8.	3.	2.	0.04	
2 COMBINED AT	CE06	61.	12.33	14.	5.	3.	0.07	
ROUTED TO								

	OPERATION	STATION	PEAK FLOW	TIME OF PEAK	AVERAGE FLOW FOR MAXIMUM PERIOD			BASIN AREA	MAXIMUM STAGE
					6-HOUR	24-HOUR	72-HOUR		
+		RE06	57.	12.67	14.	5.	3.	0.07	
	HYDROGRAPH AT								
+		E03	32.	12.67	14.	5.	4.	0.07	
	HYDROGRAPH AT								
+		DRF07	295.	12.42	45.	11.	8.	0.31	
	ROUTED TO								
+		RF07S	276.	12.67	45.	11.	8.	0.31	
	HYDROGRAPH AT								
+		E09	230.	12.25	55.	19.	14.	0.22	
	DIVERSION TO								
+		LE09D	181.	12.08	22.	7.	5.	0.22	
	HYDROGRAPH AT								
+		LE09	230.	12.25	42.	12.	9.	0.22	
	2 COMBINED AT								
+		CE09	427.	12.67	87.	23.	17.	0.51	
	DIVERSION TO								
+		DE09I	412.	12.67	77.	20.	15.	0.51	
	HYDROGRAPH AT								
+		DE09	15.	12.25	10.	3.	2.	0.51	
	ROUTED TO								
+		RE09W	15.	12.58	10.	3.	2.	0.51	
	HYDROGRAPH AT								
+		DRE05I	529.	12.50	92.	25.	18.	0.71	
	ROUTED TO								
+		RE05S	516.	12.67	92.	25.	18.	0.71	
	4 COMBINED AT								
+		CE03	617.	12.67	128.	37.	27.	0.86	
	DIVERSION TO								
+		DE03I	586.	12.67	106.	28.	20.	0.86	
	HYDROGRAPH AT								
+		DE03	31.	12.08	22.	9.	7.	0.86	
	ROUTED TO								
+		RE03W	31.	12.25	22.	9.	7.	0.86	
	HYDROGRAPH AT								
+		E02	129.	12.33	31.	10.	7.	0.15	
	2 COMBINED AT								
+		CE02	160.	12.33	52.	19.	14.	0.18	
	DIVERSION TO								
+		DE02I	114.	12.33	20.	5.	4.	0.18	
	HYDROGRAPH AT								
+		DE02	46.	11.92	33.	14.	10.	0.18	
	ROUTED TO								
+		RE02W	46.	12.83	33.	14.	10.	0.18	
	HYDROGRAPH AT								
+		E01	59.	12.25	13.	4.	3.	0.06	
	HYDROGRAPH AT								
+		DRE04I	0.	0.08	0.	0.	0.	0.07	
	ROUTED TO								
+		RE04S	0.	0.08	0.	0.	0.	0.07	
	3 COMBINED AT								
+		CE01	105.	12.25	45.	18.	13.	0.17	

100-year, 24-hour HEC-1 (Future Conditions)

OPERATION	STATION	PEAK FLOW	TIME OF PEAK	AVERAGE FLOW FOR MAXIMUM PERIOD			BASIN AREA	MAXIMUM STAGE
				6-HOUR	24-HOUR	72-HOUR		
+ DIVERSION TO	DE01I	44.	12.25	5.	1.	1.	0.17	
+ HYDROGRAPH AT	DE01	61.	11.92	40.	17.	12.	0.17	
+ HYDROGRAPH AT	DRE01	44.	12.25	5.	1.	1.	0.17	
+ ROUTED TO	RE01S	42.	12.42	5.	1.	1.	0.17	
+ HYDROGRAPH AT	D05	136.	12.33	33.	11.	8.	0.16	
+ 2 COMBINED AT	CD05	173.	12.42	38.	12.	9.	0.28	
+ DIVERSION TO	DD05I	94.	12.42	19.	6.	4.	0.28	
+ HYDROGRAPH AT	DD05	79.	12.42	18.	6.	4.	0.28	
+ ROUTED TO	RD05S	76.	12.58	18.	6.	4.	0.28	
+ HYDROGRAPH AT	D04	44.	12.33	13.	4.	3.	0.06	
+ ROUTED TO	RD04	43.	12.50	13.	4.	3.	0.06	
+ HYDROGRAPH AT	D01	67.	12.33	17.	6.	4.	0.08	
+ 3 COMBINED AT	CD01	180.	12.50	48.	16.	11.	0.26	
+ DIVERSION TO	DD01I	77.	12.50	22.	7.	5.	0.26	
+ HYDROGRAPH AT	DD01	103.	12.50	27.	9.	6.	0.26	
+ ROUTED TO	RD01E	102.	12.58	27.	9.	6.	0.26	
+ HYDROGRAPH AT	DRD05	94.	12.42	19.	6.	4.	0.28	
+ ROUTED TO	RD05E	93.	12.42	19.	6.	4.	0.28	
+ DIVERSION TO	DD052I	65.	12.42	16.	5.	4.	0.28	
+ HYDROGRAPH AT	DD052	28.	12.42	3.	1.	1.	0.28	
+ ROUTED TO	RD05S2	24.	12.75	3.	1.	1.	0.28	
+ HYDROGRAPH AT	D02	137.	12.25	29.	9.	7.	0.14	
+ 3 COMBINED AT	CD02	230.	12.50	58.	19.	14.	0.36	
+ ROUTED TO	RD02	228.	12.50	58.	19.	14.	0.36	
+ HYDROGRAPH AT	DRE02	114.	12.33	20.	5.	4.	0.18	
+ ROUTED TO								

100-year, 24-hour HEC-1 (Future Conditions)

OPERATION	STATION	PEAK FLOW	TIME OF PEAK	AVERAGE FLOW FOR MAXIMUM PERIOD			BASIN AREA	MAXIMUM STAGE
				6-HOUR	24-HOUR	72-HOUR		
+ HYDROGRAPH AT	RE02S	110.	12.50	20.	5.	4.	0.18	
+ ROUTED TO	DRD052	65.	12.42	16.	5.	4.	0.28	
+ HYDROGRAPH AT	RD05E2	65.	12.50	16.	5.	4.	0.28	
+ 3 COMBINED AT	D06	91.	12.33	23.	7.	5.	0.11	
+ ROUTED TO	CD06	257.	12.50	59.	18.	13.	0.36	
+ HYDROGRAPH AT	RD06	257.	12.50	59.	18.	13.	0.36	
+ 2 COMBINED AT	D07	44.	12.25	10.	3.	2.	0.05	
+ ROUTED TO	CD07	293.	12.50	69.	21.	15.	0.41	
+ HYDROGRAPH AT	RD07	283.	12.67	69.	21.	15.	0.41	
+ ROUTED TO	DRE03	586.	12.67	106.	28.	20.	0.86	
+ HYDROGRAPH AT	RE03S	569.	12.83	106.	28.	20.	0.86	
+ 2 COMBINED AT	D08	34.	12.42	11.	4.	3.	0.05	
+ ROUTED TO	CD08	596.	12.83	117.	32.	23.	0.88	
+ HYDROGRAPH AT	RD08	585.	12.92	117.	32.	23.	0.88	
+ HYDROGRAPH AT	D03	84.	12.33	21.	7.	5.	0.10	
+ ROUTED TO	DRE09I	412.	12.67	77.	20.	15.	0.51	
+ HYDROGRAPH AT	RE09S	362.	12.92	77.	20.	15.	0.51	
+ DIVERSION TO	D09	246.	12.25	53.	18.	13.	0.22	
+ HYDROGRAPH AT	LD09D	207.	12.08	22.	7.	5.	0.22	
+ 2 COMBINED AT	LD09	246.	12.25	40.	12.	8.	0.22	
+ 5 COMBINED AT	CD09	459.	12.83	116.	32.	23.	0.69	
+ ROUTED TO	CD03	1484.	12.83	369.	108.	78.	2.43	
+ HYDROGRAPH AT	RD03	1482.	12.92	369.	108.	78.	2.43	
+ HYDROGRAPH AT	C11	63.	12.17	10.	3.	2.	0.05	
+ ROUTED TO	C10	59.	12.17	11.	4.	3.	0.05	
+ HYDROGRAPH AT	RC10	59.	12.25	11.	4.	3.	0.05	

100-year, 24-hour HEC-1 (Future Conditions)

OPERATION	STATION	PEAK FLOW	TIME OF PEAK	AVERAGE FLOW FOR MAXIMUM PERIOD			BASIN AREA	MAXIMUM STAGE
				6-HOUR	24-HOUR	72-HOUR		
3 COMBINED AT	CC11	1517.	12.92	389.	114.	83.	2.54	
ROUTED TO	RC11	1506.	12.92	389.	114.	83.	2.54	
HYDROGRAPH AT	C08	117.	12.25	24.	8.	6.	0.12	
2 COMBINED AT	CC08	1555.	12.92	411.	122.	88.	2.66	
ROUTED TO	RC08	1538.	13.00	411.	122.	88.	2.66	
HYDROGRAPH AT	C06	153.	12.25	31.	10.	7.	0.15	
2 COMBINED AT	CC06	1592.	13.00	441.	132.	95.	2.81	
ROUTED TO	RC06	1589.	13.00	441.	132.	95.	2.81	
HYDROGRAPH AT	DRD01	77.	12.50	22.	7.	5.	0.26	
ROUTED TO	RD01S	76.	12.67	22.	7.	5.	0.26	
HYDROGRAPH AT	C07	54.	12.33	15.	5.	4.	0.07	
2 COMBINED AT	CC07	124.	12.58	36.	12.	9.	0.17	
ROUTED TO	RC07	122.	12.75	36.	12.	9.	0.17	
HYDROGRAPH AT	C09	46.	12.33	12.	4.	3.	0.06	
ROUTED TO	RC09	41.	12.67	12.	4.	3.	0.06	
HYDROGRAPH AT	C05	103.	12.42	29.	10.	7.	0.14	
HYDROGRAPH AT	C01	26.	12.33	7.	2.	2.	0.03	
ROUTED TO	RC01	25.	12.50	7.	2.	2.	0.03	
HYDROGRAPH AT	C02	29.	12.33	9.	3.	2.	0.04	
5 COMBINED AT	CC02	297.	12.67	92.	30.	22.	0.44	
ROUTED TO	RC02	297.	12.67	92.	30.	22.	0.44	
HYDROGRAPH AT	C03	47.	12.17	9.	3.	2.	0.04	
HYDROGRAPH AT	C12	3.	12.83	2.	1.	1.	0.01	
ROUTED TO	RC12	3.	13.33	2.	1.	1.	0.01	
HYDROGRAPH AT	C13	170.	12.25	43.	15.	11.	0.18	
DIVERSION TO								

100-year, 24-hour HEC-1 (Future Conditions)

OPERATION	STATION	PEAK FLOW	TIME OF PEAK	AVERAGE FLOW FOR MAXIMUM PERIOD			BASIN AREA	MAXIMUM STAGE
				6-HOUR	24-HOUR	72-HOUR		
+ HYDROGRAPH AT	LC13D	159.	12.17	18.	6.	4.	0.18	
+ 2 COMBINED AT	LC13	167.	12.33	33.	9.	7.	0.18	
+ DIVERSION TO	CC13	168.	12.33	35.	10.	7.	0.19	
+ HYDROGRAPH AT	DC13I	127.	12.33	13.	3.	2.	0.19	
+ ROUTED TO	DC13	41.	12.33	21.	7.	5.	0.19	
+ HYDROGRAPH AT	RC13W	41.	12.42	21.	7.	5.	0.19	
+ ROUTED TO	C04	28.	12.33	8.	3.	2.	0.04	
+ ROUTED TO	RC04	28.	12.50	8.	3.	2.	0.04	
+ 5 COMBINED AT	CC03	1899.	13.00	565.	173.	125.	3.37	
+ ROUTED TO	RC03	1884.	13.00	565.	173.	125.	3.37	
+ HYDROGRAPH AT	B07	25.	12.08	3.	1.	1.	0.02	
+ 2 COMBINED AT	CB07	1886.	13.00	568.	174.	126.	3.39	
+ ROUTED TO	RB07	1881.	13.08	568.	174.	126.	3.39	
+ HYDROGRAPH AT	B05	62.	12.33	16.	5.	4.	0.09	
+ DIVERSION TO	LB05D	1.	2.58	0.	0.	0.	0.09	
+ HYDROGRAPH AT	LB05	62.	12.33	16.	5.	4.	0.09	
+ HYDROGRAPH AT	B06	14.	12.17	3.	1.	1.	0.01	
+ 3 COMBINED AT	CB05	1916.	13.08	586.	180.	131.	3.49	
+ ROUTED TO	RB05	1915.	13.08	586.	180.	131.	3.49	
+ HYDROGRAPH AT	B04	125.	12.08	19.	6.	5.	0.08	
+ DIVERSION TO	LB04D	125.	12.08	19.	6.	5.	0.08	
+ HYDROGRAPH AT	LB04	0.	0.08	0.	0.	0.	0.08	
+ HYDROGRAPH AT	B02	134.	12.17	24.	8.	6.	0.10	
+ DIVERSION TO	LB02D	7.	10.42	4.	1.	1.	0.10	
+ HYDROGRAPH AT	LB02	134.	12.17	24.	7.	5.	0.10	
+ ROUTED TO	RB02	133.	12.17	24.	7.	5.	0.10	

100-year, 24-hour HEC-1 (Future Conditions)

OPERATION	STATION	PEAK FLOW	TIME OF PEAK	AVERAGE FLOW FOR MAXIMUM PERIOD			BASIN AREA	MAXIMUM STAGE
				6-HOUR	24-HOUR	72-HOUR		
HYDROGRAPH AT	B03	8.	12.17	1.	0.	0.	0.01	
+ 4 COMBINED AT	CB03	1944.	13.08	609.	187.	135.	3.66	
+ ROUTED TO	RB03	1933.	13.08	609.	187.	135.	3.66	
+ HYDROGRAPH AT	B01	113.	12.08	13.	4.	3.	0.07	
+ DIVERSION TO	LB01D	3.	10.00	2.	1.	0.	0.07	
+ HYDROGRAPH AT	LB01	113.	12.08	13.	4.	3.	0.07	
+ 2 COMBINED AT	CB01	1940.	13.08	621.	190.	138.	3.73	
+ ROUTED TO	RB01	1938.	13.17	621.	190.	138.	3.73	
+ HYDROGRAPH AT	B12	164.	12.17	22.	7.	5.	0.12	
+ HYDROGRAPH AT	B11	78.	12.17	11.	3.	2.	0.06	
+ DIVERSION TO	LB11D	74.	12.08	5.	1.	1.	0.06	
+ HYDROGRAPH AT	LB11	71.	12.25	7.	2.	1.	0.06	
+ HYDROGRAPH AT	DRDC13	127.	12.33	13.	3.	2.	0.19	
+ ROUTED TO	RC13S	81.	13.17	13.	3.	2.	0.19	
+ HYDROGRAPH AT	B13	310.	12.17	43.	14.	10.	0.22	
+ DIVERSION TO	LB13D	310.	12.17	34.	9.	7.	0.22	
+ HYDROGRAPH AT	LB13	95.	12.58	14.	4.	3.	0.22	
+ ROUTED TO	RB13	72.	12.83	14.	4.	3.	0.22	
+ HYDROGRAPH AT	B14	135.	12.33	38.	13.	10.	0.15	
+ DIVERSION TO	LB14D	132.	12.25	17.	5.	4.	0.15	
+ HYDROGRAPH AT	LB14	126.	12.50	28.	8.	6.	0.15	
+ 3 COMBINED AT	CB14	185.	13.08	53.	15.	11.	0.52	
+ ROUTED TO	RB14	181.	13.17	53.	15.	11.	0.52	
+ 4 COMBINED AT	CB12	2122.	13.17	695.	213.	154.	4.43	
+ ROUTED TO	RB12	2019.	13.42	695.	213.	154.	4.43	
+ HYDROGRAPH AT								

100-year, 24-hour HEC-1 (Future Conditions)

	OPERATION	STATION	PEAK FLOW	TIME OF PEAK	AVERAGE FLOW FOR MAXIMUM PERIOD			BASIN AREA	MAXIMUM STAGE
					6-HOUR	24-HOUR	72-HOUR		
+		A01	745.	12.17	131.	45.	33.	0.51	
	DIVERSION TO								
+		LA01D	619.	12.08	58.	18.	13.	0.51	
	HYDROGRAPH AT								
+		LA01	741.	12.25	95.	27.	20.	0.51	
	2 COMBINED AT								
+		CA01	2096.	13.42	784.	239.	173.	4.94	
	HYDROGRAPH AT								
+		B10	65.	12.42	17.	5.	4.	0.11	
	DIVERSION TO								
+		LB10D	65.	12.42	17.	5.	4.	0.11	
	HYDROGRAPH AT								
+		LB10	1.	20.50	0.	0.	0.	0.11	

\*\*\* NORMAL END OF HEC-1 \*\*\*

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* FLOOD HYDROGRAPH PACKAGE (HEC-1)
* MAY 1991
* VERSION 4.0.1E
* Lahey F77L-EM/32 version 5.01
* Dodson & Associates, Inc.
* RUN DATE 04/30/02 TIME 10:38:11
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*
* U.S. ARMY CORPS OF ENGINEERS
* HYDROLOGIC ENGINEERING CENTER
* 609 SECOND STREET
* DAVIS, CALIFORNIA 95616
* (916) 551-1748
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THIS PROGRAM REPLACES ALL PREVIOUS VERSIONS OF HEC-1 KNOWN AS HEC1 (JAN 73), HEC1GS, HEC1DB, AND HEC1KW.

THE DEFINITIONS OF VARIABLES -RTIMP- AND -RTIOR- HAVE CHANGED FROM THOSE USED WITH THE 1973-STYLE INPUT STRUCTURE. THE DEFINITION OF -AMSK- ON RM-CARD WAS CHANGED WITH REVISIONS DATED 28 SEP 81. THIS IS THE FORTRAN77 VERSION

NEW OPTIONS: DAMBREAK OUTFLOW SUBMERGENCE , SINGLE EVENT DAMAGE CALCULATION, DSS:WRITE STAGE FREQUENCY, DSS:READ TIME SERIES AT DESIRED CALCULATION INTERVAL LOSS RATE:GREEN AND AMPT INFILTRATION KINEMATIC WAVE: NEW FINITE DIFFERENCE ALGORITHM

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1
HEC-1 INPUT
PAGE 1

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10
1 ID Project ID: SCOTTSDALE - Major Basin: 01 - Return Period: 100
2 ID *****
3 ID **
4 ID ** Granite Reef ADMP **
5 ID **
6 ID *****
7 ID *****
8 ID PROJECT: Granite Reef ADMP
9 ID CLIENT: Flood Control District of Maricopa County
10 ID PREPARED BY: Entellus, Inc.
11 ID PROJECT No: FCD Entellus 310.021
12 ID FILE NAME: FUT-24.DAT CREATED DATE: Mar 28, 2002
13 ID MODIFIED:
14 ID STORM: 100-year 24-hour Storm
15 ID DEVELOPMENT CONDITIONS: Existing Conditions
16 ID
17 ID

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1

RUNOFF SUMMARY  
FLOW IN CUBIC FEET PER SECOND  
TIME IN HOURS, AREA IN SQUARE MILES

OPERATION	STATION	PEAK FLOW	TIME OF PEAK	AVERAGE FLOW FOR MAXIMUM PERIOD			BASIN AREA	MAXIMUM STAGE
				6-HOUR	24-HOUR	72-HOUR		
+	HYDROGRAPH AT							
+	G03	133.	12.17	21.	7.	5.	0.09	
+	DIVERSION TO							
+	DG03I	68.	12.17	10.	3.	2.	0.09	
+	HYDROGRAPH AT							
+	DG03	65.	12.17	10.	3.	2.	0.09	
+	ROUTED TO							
+	RG03S	59.	12.33	10.	3.	2.	0.09	
+	HYDROGRAPH AT							
+	G01	42.	12.25	11.	4.	3.	0.05	
+	2 COMBINED AT							
+	CG01	101.	12.33	21.	7.	5.	0.10	
+	DIVERSION TO							
+	DG01I	101.	12.33	21.	7.	5.	0.10	
+	HYDROGRAPH AT							
+	DG01	0.	0.08	0.	0.	0.	0.10	

OPERATION	STATION	PEAK FLOW	TIME OF PEAK	AVERAGE FLOW FOR MAXIMUM PERIOD			BASIN AREA	MAXIMUM STAGE
				6-HOUR	24-HOUR	72-HOUR		
ROUTED TO	RG01S	0.	0.08	0.	0.	0.	0.10	
+ HYDROGRAPH AT	F04	52.	12.33	14.	5.	4.	0.06	
+ HYDROGRAPH AT	G04	65.	12.08	10.	3.	2.	0.05	
+ ROUTED TO	RG04	64.	12.17	10.	3.	2.	0.05	
+ HYDROGRAPH AT	G05	68.	12.17	11.	3.	2.	0.05	
+ ROUTED TO	RG05	66.	12.25	11.	3.	2.	0.05	
+ HYDROGRAPH AT	G06	236.	12.08	31.	11.	8.	0.13	
+ DIVERSION TO	LG06D	112.	11.92	13.	4.	3.	0.13	
+ HYDROGRAPH AT	LG06	236.	12.08	24.	7.	5.	0.13	
+ DIVERSION TO	DG06I	96.	12.08	11.	3.	2.	0.13	
+ HYDROGRAPH AT	DG06	141.	12.08	13.	4.	3.	0.13	
+ DIVERSION TO	LG06AD	141.	12.08	9.	2.	2.	0.13	
+ HYDROGRAPH AT	LG06A	18.	12.50	5.	2.	1.	0.13	
+ HYDROGRAPH AT	G02	69.	12.25	16.	5.	4.	0.08	
+ 4 COMBINED AT	CG02	194.	12.25	39.	13.	9.	0.23	
+ ROUTED TO	RG02	189.	12.33	39.	13.	9.	0.23	
+ HYDROGRAPH AT	F06	104.	12.25	21.	7.	5.	0.10	
+ 2 COMBINED AT	CF06	288.	12.33	60.	20.	14.	0.33	
+ ROUTED TO	RF06	278.	12.42	60.	20.	14.	0.33	
+ HYDROGRAPH AT	F05	95.	12.25	19.	6.	4.	0.09	
+ 2 COMBINED AT	CF05	360.	12.33	78.	26.	19.	0.42	
+ DIVERSION TO	DF05I	310.	12.33	45.	11.	8.	0.42	
+ HYDROGRAPH AT	DF05	50.	11.83	33.	14.	11.	0.42	
+ ROUTED TO	RF05W	50.	12.00	33.	14.	11.	0.42	
+ 3 COMBINED AT	CF04	101.	12.33	48.	19.	14.	0.17	
+ DIVERSION TO	DF04I	54.	12.33	25.	10.	8.	0.17	
+ HYDROGRAPH AT								

OPERATION	STATION	PEAK FLOW	TIME OF PEAK	AVERAGE FLOW FOR MAXIMUM PERIOD			BASIN AREA	MAXIMUM STAGE
				6-HOUR	24-HOUR	72-HOUR		
+ HYDROGRAPH AT	DF04	47.	12.33	22.	9.	6.	0.17	
+ ROUTED TO	F05R	310.	12.33	45.	11.	8.	0.42	
+ HYDROGRAPH AT	RF05S	304.	12.50	45.	11.	8.	0.42	
+ 2 COMBINED AT	F03	167.	12.33	37.	12.	9.	0.17	
+ ROUTED TO	CF03	453.	12.42	81.	23.	17.	0.56	
+ HYDROGRAPH AT	RF03	452.	12.50	81.	23.	17.	0.56	
+ ROUTED TO	G06R	96.	12.08	11.	3.	2.	0.13	
+ HYDROGRAPH AT	RG06S	71.	12.42	11.	3.	2.	0.13	
+ DIVERSION TO	F07	268.	12.25	61.	21.	15.	0.25	
+ HYDROGRAPH AT	LF07D	214.	12.08	25.	8.	6.	0.25	
+ 2 COMBINED AT	LF07	268.	12.25	48.	14.	10.	0.25	
+ DIVERSION TO	CF07	308.	12.42	58.	17.	12.	0.31	
+ HYDROGRAPH AT	DF07I	295.	12.42	45.	11.	8.	0.31	
+ ROUTED TO	DF07	13.	12.17	13.	6.	4.	0.31	
+ HYDROGRAPH AT	RF07W	13.	12.92	13.	6.	4.	0.31	
+ 3 COMBINED AT	F02	44.	12.25	10.	3.	2.	0.05	
+ DIVERSION TO	CF02	502.	12.42	101.	32.	23.	0.63	
+ HYDROGRAPH AT	DF02I	472.	12.42	81.	23.	17.	0.63	
+ ROUTED TO	DF02	30.	11.92	20.	9.	6.	0.63	
+ HYDROGRAPH AT	RF02W	30.	12.00	20.	9.	6.	0.63	
+ ROUTED TO	F04R	54.	12.33	25.	10.	8.	0.17	
+ DIVERSION TO	RF04S	52.	12.50	25.	10.	8.	0.17	
+ HYDROGRAPH AT	DRF04I	26.	12.50	13.	5.	4.	0.17	
+ HYDROGRAPH AT	DRF04	26.	12.50	13.	5.	4.	0.17	
+ 3 COMBINED AT	F01	30.	12.25	6.	2.	2.	0.03	
+ HYDROGRAPH AT	CF01	82.	12.33	39.	16.	12.	0.10	

100-year, 24-hour HEC-1 (Future Conditions)

OPERATION	STATION	PEAK FLOW	TIME OF PEAK	AVERAGE FLOW FOR MAXIMUM PERIOD			BASIN AREA	MAXIMUM STAGE
				6-HOUR	24-HOUR	72-HOUR		
DIVERSION TO	DF01I	0.	0.08	0.	0.	0.	0.10	
HYDROGRAPH AT	DF01	82.	12.33	39.	16.	12.	0.10	
HYDROGRAPH AT	DRF02	472.	12.42	81.	23.	17.	0.63	
ROUTED TO	RF02S	466.	12.50	81.	23.	17.	0.63	
HYDROGRAPH AT	E07	107.	12.17	17.	6.	4.	0.08	
2 COMBINED AT	CE07	528.	12.50	97.	29.	21.	0.66	
ROUTED TO	RE07	523.	12.58	97.	29.	21.	0.66	
HYDROGRAPH AT	E05	65.	12.17	10.	3.	2.	0.05	
2 COMBINED AT	CE05	554.	12.50	107.	32.	23.	0.71	
DIVERSION TO	DE05I	529.	12.50	92.	25.	18.	0.71	
HYDROGRAPH AT	DE05	25.	11.92	15.	7.	5.	0.71	
ROUTED TO	RE05W	25.	12.08	15.	7.	5.	0.71	
HYDROGRAPH AT	E04	30.	12.33	8.	2.	2.	0.04	
DIVERSION TO	LE04D	26.	12.17	2.	1.	1.	0.04	
HYDROGRAPH AT	LE04	30.	12.33	6.	2.	1.	0.04	
HYDROGRAPH AT	DRF01I	0.	0.08	0.	0.	0.	0.10	
ROUTED TO	RF01S	0.	0.08	0.	0.	0.	0.10	
3 COMBINED AT	CE04	54.	12.33	21.	9.	6.	0.07	
DIVERSION TO	DE04I	0.	0.08	0.	0.	0.	0.07	
HYDROGRAPH AT	DE04	54.	12.33	21.	9.	6.	0.07	
3 COMBINED AT	DUMMY	182.	12.33	82.	33.	24.	0.34	
HYDROGRAPH AT	E08	30.	12.25	7.	2.	2.	0.03	
ROUTED TO	RE08	28.	12.42	7.	2.	2.	0.03	
HYDROGRAPH AT	E06	34.	12.25	8.	3.	2.	0.04	
2 COMBINED AT	CE06	61.	12.33	14.	5.	3.	0.07	
ROUTED TO								

100-year, 24-hour HEC-1 (Future Conditions)

	OPERATION	STATION	PEAK FLOW	TIME OF PEAK	AVERAGE FLOW FOR MAXIMUM PERIOD			BASIN AREA	MAXIMUM STAGE
					6-HOUR	24-HOUR	72-HOUR		
+		RE06	57.	12.67	14.	5.	3.	0.07	
	HYDROGRAPH AT								
+		E03	32.	12.67	14.	5.	4.	0.07	
	HYDROGRAPH AT								
+		DRF07	295.	12.42	45.	11.	8.	0.31	
	ROUTED TO								
+		RF07S	276.	12.67	45.	11.	8.	0.31	
	HYDROGRAPH AT								
+		E09	230.	12.25	55.	19.	14.	0.22	
	DIVERSION TO								
+		LE09D	181.	12.08	22.	7.	5.	0.22	
	HYDROGRAPH AT								
+		LE09	230.	12.25	42.	12.	9.	0.22	
	2 COMBINED AT								
+		CE09	427.	12.67	87.	23.	17.	0.51	
	DIVERSION TO								
+		DE09I	412.	12.67	77.	20.	15.	0.51	
	HYDROGRAPH AT								
+		DE09	15.	12.25	10.	3.	2.	0.51	
	ROUTED TO								
+		RE09W	15.	12.58	10.	3.	2.	0.51	
	HYDROGRAPH AT								
+		DRE05I	529.	12.50	92.	25.	18.	0.71	
	ROUTED TO								
+		RE05S	516.	12.67	92.	25.	18.	0.71	
	4 COMBINED AT								
+		CE03	617.	12.67	128.	37.	27.	0.86	
	DIVERSION TO								
+		DE03I	586.	12.67	106.	28.	20.	0.86	
	HYDROGRAPH AT								
+		DE03	31.	12.08	22.	9.	7.	0.86	
	ROUTED TO								
+		RE03W	31.	12.25	22.	9.	7.	0.86	
	HYDROGRAPH AT								
+		E02	129.	12.33	31.	10.	7.	0.15	
	2 COMBINED AT								
+		CE02	160.	12.33	52.	19.	14.	0.18	
	DIVERSION TO								
+		DE02I	114.	12.33	20.	5.	4.	0.18	
	HYDROGRAPH AT								
+		DE02	46.	11.92	33.	14.	10.	0.18	
	ROUTED TO								
+		RE02W	46.	12.83	33.	14.	10.	0.18	
	HYDROGRAPH AT								
+		E01	59.	12.25	13.	4.	3.	0.06	
	HYDROGRAPH AT								
+		DRE04I	0.	0.08	0.	0.	0.	0.07	
	ROUTED TO								
+		RE04S	0.	0.08	0.	0.	0.	0.07	
	3 COMBINED AT								
+		CE01	105.	12.25	45.	18.	13.	0.17	

100-year, 24-hour HEC-1 (Future Conditions)

OPERATION	STATION	PEAK FLOW	TIME OF PEAK	AVERAGE FLOW FOR MAXIMUM PERIOD			BASIN AREA	MAXIMUM STAGE
				6-HOUR	24-HOUR	72-HOUR		
+ DIVERSION TO	DE01I	44.	12.25	5.	1.	1.	0.17	
+ HYDROGRAPH AT	DE01	61.	11.92	40.	17.	12.	0.17	
+ HYDROGRAPH AT	DRE01	44.	12.25	5.	1.	1.	0.17	
+ ROUTED TO	RE01S	42.	12.42	5.	1.	1.	0.17	
+ HYDROGRAPH AT	D05	136.	12.33	33.	11.	8.	0.16	
+ 2 COMBINED AT	CD05	173.	12.42	38.	12.	9.	0.28	
+ DIVERSION TO	DD05I	94.	12.42	19.	6.	4.	0.28	
+ HYDROGRAPH AT	DD05	79.	12.42	18.	6.	4.	0.28	
+ ROUTED TO	RD05S	76.	12.58	18.	6.	4.	0.28	
+ HYDROGRAPH AT	D04	44.	12.33	13.	4.	3.	0.06	
+ ROUTED TO	RD04	43.	12.50	13.	4.	3.	0.06	
+ HYDROGRAPH AT	D01	67.	12.33	17.	6.	4.	0.08	
+ 3 COMBINED AT	CD01	180.	12.50	48.	16.	11.	0.26	
+ DIVERSION TO	DD01I	77.	12.50	22.	7.	5.	0.26	
+ HYDROGRAPH AT	DD01	103.	12.50	27.	9.	6.	0.26	
+ ROUTED TO	RD01E	102.	12.58	27.	9.	6.	0.26	
+ HYDROGRAPH AT	DRD05	94.	12.42	19.	6.	4.	0.28	
+ ROUTED TO	RD05E	93.	12.42	19.	6.	4.	0.28	
+ DIVERSION TO	DD052I	65.	12.42	16.	5.	4.	0.28	
+ HYDROGRAPH AT	DD052	28.	12.42	3.	1.	1.	0.28	
+ ROUTED TO	RD05S2	24.	12.75	3.	1.	1.	0.28	
+ HYDROGRAPH AT	D02	137.	12.25	29.	9.	7.	0.14	
+ 3 COMBINED AT	CD02	230.	12.50	58.	19.	14.	0.36	
+ ROUTED TO	RD02	228.	12.50	58.	19.	14.	0.36	
+ HYDROGRAPH AT	DRE02	114.	12.33	20.	5.	4.	0.18	
+ ROUTED TO								

100-year, 24-hour HEC-1 (Future Conditions)

OPERATION	STATION	PEAK FLOW	TIME OF PEAK	AVERAGE FLOW FOR MAXIMUM PERIOD			BASIN AREA	MAXIMUM STAGE
				6-HOUR	24-HOUR	72-HOUR		
+ HYDROGRAPH AT	RE02S	110.	12.50	20.	5.	4.	0.18	
+ ROUTED TO	DRD052	65.	12.42	16.	5.	4.	0.28	
+ HYDROGRAPH AT	RD05E2	65.	12.50	16.	5.	4.	0.28	
+ 3 COMBINED AT	D06	91.	12.33	23.	7.	5.	0.11	
+ ROUTED TO	CD06	257.	12.50	59.	18.	13.	0.36	
+ HYDROGRAPH AT	RD06	257.	12.50	59.	18.	13.	0.36	
+ 2 COMBINED AT	D07	44.	12.25	10.	3.	2.	0.05	
+ ROUTED TO	CD07	293.	12.50	69.	21.	15.	0.41	
+ HYDROGRAPH AT	RD07	283.	12.67	69.	21.	15.	0.41	
+ ROUTED TO	DRE03	586.	12.67	106.	28.	20.	0.86	
+ HYDROGRAPH AT	RE03S	569.	12.83	106.	28.	20.	0.86	
+ 2 COMBINED AT	D08	34.	12.42	11.	4.	3.	0.05	
+ ROUTED TO	CD08	596.	12.83	117.	32.	23.	0.88	
+ HYDROGRAPH AT	RD08	585.	12.92	117.	32.	23.	0.88	
+ HYDROGRAPH AT	D03	84.	12.33	21.	7.	5.	0.10	
+ ROUTED TO	DRE09I	412.	12.67	77.	20.	15.	0.51	
+ HYDROGRAPH AT	RE09S	362.	12.92	77.	20.	15.	0.51	
+ DIVERSION TO	D09	246.	12.25	53.	18.	13.	0.22	
+ HYDROGRAPH AT	LD09D	207.	12.08	22.	7.	5.	0.22	
+ 2 COMBINED AT	LD09	246.	12.25	40.	12.	8.	0.22	
+ 5 COMBINED AT	CD09	459.	12.83	116.	32.	23.	0.69	
+ ROUTED TO	CD03	1484.	12.83	369.	108.	78.	2.43	
+ HYDROGRAPH AT	RD03	1482.	12.92	369.	108.	78.	2.43	
+ HYDROGRAPH AT	C11	63.	12.17	10.	3.	2.	0.05	
+ ROUTED TO	C10	59.	12.17	11.	4.	3.	0.05	
+ HYDROGRAPH AT	RC10	59.	12.25	11.	4.	3.	0.05	

100-year, 24-hour HEC-1 (Future Conditions)

	OPERATION	STATION	PEAK FLOW	TIME OF PEAK	AVERAGE FLOW FOR MAXIMUM PERIOD			BASIN AREA	MAXIMUM STAGE
					6-HOUR	24-HOUR	72-HOUR		
+	3 COMBINED AT	CC11	1517.	12.92	389.	114.	83.	2.54	
	ROUTED TO	RC11	1506.	12.92	389.	114.	83.	2.54	
+	HYDROGRAPH AT	C08	117.	12.25	24.	8.	6.	0.12	
+	2 COMBINED AT	CC08	1555.	12.92	411.	122.	88.	2.66	
+	ROUTED TO	RC08	1538.	13.00	411.	122.	88.	2.66	
+	HYDROGRAPH AT	C06	153.	12.25	31.	10.	7.	0.15	
+	2 COMBINED AT	CC06	1592.	13.00	441.	132.	95.	2.81	
+	ROUTED TO	RC06	1589.	13.00	441.	132.	95.	2.81	
+	HYDROGRAPH AT	DRDO1	77.	12.50	22.	7.	5.	0.26	
+	ROUTED TO	RD01S	76.	12.67	22.	7.	5.	0.26	
+	HYDROGRAPH AT	C07	54.	12.33	15.	5.	4.	0.07	
+	2 COMBINED AT	CC07	124.	12.58	36.	12.	9.	0.17	
+	ROUTED TO	RC07	122.	12.75	36.	12.	9.	0.17	
+	HYDROGRAPH AT	C09	46.	12.33	12.	4.	3.	0.06	
+	ROUTED TO	RC09	41.	12.67	12.	4.	3.	0.06	
+	HYDROGRAPH AT	C05	103.	12.42	29.	10.	7.	0.14	
+	HYDROGRAPH AT	C01	26.	12.33	7.	2.	2.	0.03	
+	ROUTED TO	RC01	25.	12.50	7.	2.	2.	0.03	
+	HYDROGRAPH AT	C02	29.	12.33	9.	3.	2.	0.04	
+	5 COMBINED AT	CC02	297.	12.67	92.	30.	22.	0.44	
+	ROUTED TO	RC02	297.	12.67	92.	30.	22.	0.44	
+	HYDROGRAPH AT	C03	47.	12.17	9.	3.	2.	0.04	
+	HYDROGRAPH AT	C12	3.	12.83	2.	1.	1.	0.01	
+	ROUTED TO	RC12	3.	13.33	2.	1.	1.	0.01	
+	HYDROGRAPH AT	C13	170.	12.25	43.	15.	11.	0.18	
	DIVERSION TO								

100-year, 24-hour HEC-1 (Future Conditions)

OPERATION	STATION	PEAK FLOW	TIME OF PEAK	AVERAGE FLOW FOR MAXIMUM PERIOD			BASIN AREA	MAXIMUM STAGE
				6-HOUR	24-HOUR	72-HOUR		
+ HYDROGRAPH AT	LC13D	159.	12.17	18.	6.	4.	0.18	
+ 2 COMBINED AT	LC13	167.	12.33	33.	9.	7.	0.18	
+ DIVERSION TO	CC13	168.	12.33	35.	10.	7.	0.19	
+ HYDROGRAPH AT	DC13I	127.	12.33	13.	3.	2.	0.19	
+ ROUTED TO	DC13	41.	12.33	21.	7.	5.	0.19	
+ HYDROGRAPH AT	RC13W	41.	12.42	21.	7.	5.	0.19	
+ ROUTED TO	C04	28.	12.33	8.	3.	2.	0.04	
+ ROUTED TO	RC04	28.	12.50	8.	3.	2.	0.04	
+ 5 COMBINED AT	CC03	1899.	13.00	565.	173.	125.	3.37	
+ ROUTED TO	RC03	1884.	13.00	565.	173.	125.	3.37	
+ HYDROGRAPH AT	B07	25.	12.08	3.	1.	1.	0.02	
+ 2 COMBINED AT	CB07	1886.	13.00	568.	174.	126.	3.39	
+ ROUTED TO	RB07	1881.	13.08	568.	174.	126.	3.39	
+ HYDROGRAPH AT	B05	62.	12.33	16.	5.	4.	0.09	
+ DIVERSION TO	LB05D	1.	2.58	0.	0.	0.	0.09	
+ HYDROGRAPH AT	LB05	62.	12.33	16.	5.	4.	0.09	
+ HYDROGRAPH AT	B06	14.	12.17	3.	1.	1.	0.01	
+ 3 COMBINED AT	CB05	1916.	13.08	586.	180.	131.	3.49	
+ ROUTED TO	RB05	1915.	13.08	586.	180.	131.	3.49	
+ HYDROGRAPH AT	B04	125.	12.08	19.	6.	5.	0.08	
+ DIVERSION TO	LB04D	125.	12.08	19.	6.	5.	0.08	
+ HYDROGRAPH AT	LB04	0.	0.08	0.	0.	0.	0.08	
+ HYDROGRAPH AT	B02	134.	12.17	24.	8.	6.	0.10	
+ DIVERSION TO	LB02D	7.	10.42	4.	1.	1.	0.10	
+ HYDROGRAPH AT	LB02	134.	12.17	24.	7.	5.	0.10	
+ ROUTED TO	RB02	133.	12.17	24.	7.	5.	0.10	

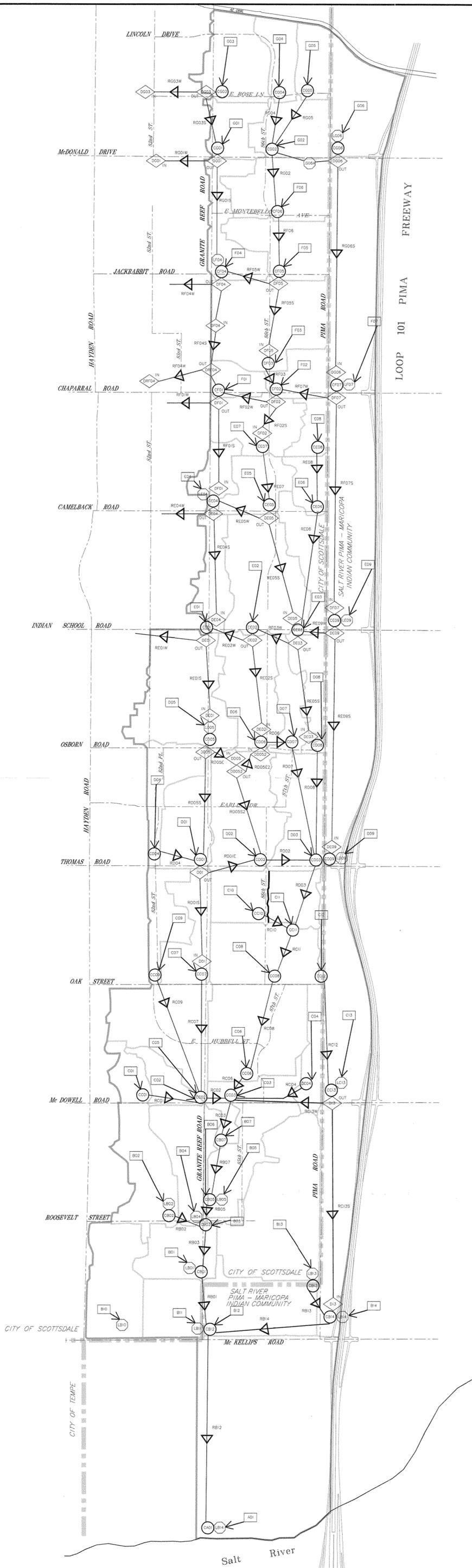
100-year, 24-hour HEC-1 (Future Conditions)

OPERATION	STATION	PEAK FLOW	TIME OF PEAK	AVERAGE FLOW FOR MAXIMUM PERIOD			BASIN AREA	MAXIMUM STAGE
				6-HOUR	24-HOUR	72-HOUR		
HYDROGRAPH AT	B03	8.	12.17	1.	0.	0.	0.01	
+ 4 COMBINED AT	CB03	1944.	13.08	609.	187.	135.	3.66	
+ ROUTED TO	RB03	1933.	13.08	609.	187.	135.	3.66	
+ HYDROGRAPH AT	B01	113.	12.08	13.	4.	3.	0.07	
+ DIVERSION TO	LB01D	3.	10.00	2.	1.	0.	0.07	
+ HYDROGRAPH AT	LB01	113.	12.08	13.	4.	3.	0.07	
+ 2 COMBINED AT	CB01	1940.	13.08	621.	190.	138.	3.73	
+ ROUTED TO	RB01	1938.	13.17	621.	190.	138.	3.73	
+ HYDROGRAPH AT	B12	164.	12.17	22.	7.	5.	0.12	
+ HYDROGRAPH AT	B11	78.	12.17	11.	3.	2.	0.06	
+ DIVERSION TO	LB11D	74.	12.08	5.	1.	1.	0.06	
+ HYDROGRAPH AT	LB11	71.	12.25	7.	2.	1.	0.06	
+ HYDROGRAPH AT	DRDC13	127.	12.33	13.	3.	2.	0.19	
+ ROUTED TO	RC13S	81.	13.17	13.	3.	2.	0.19	
+ HYDROGRAPH AT	B13	310.	12.17	43.	14.	10.	0.22	
+ DIVERSION TO	LB13D	310.	12.17	34.	9.	7.	0.22	
+ HYDROGRAPH AT	LB13	95.	12.58	14.	4.	3.	0.22	
+ ROUTED TO	RB13	72.	12.83	14.	4.	3.	0.22	
+ HYDROGRAPH AT	B14	135.	12.33	38.	13.	10.	0.15	
+ DIVERSION TO	LB14D	132.	12.25	17.	5.	4.	0.15	
+ HYDROGRAPH AT	LB14	126.	12.50	28.	8.	6.	0.15	
+ 3 COMBINED AT	CB14	185.	13.08	53.	15.	11.	0.52	
+ ROUTED TO	RB14	181.	13.17	53.	15.	11.	0.52	
+ 4 COMBINED AT	CB12	2122.	13.17	695.	213.	154.	4.43	
+ ROUTED TO	RB12	2019.	13.42	695.	213.	154.	4.43	
+ HYDROGRAPH AT								

100-year, 24-hour HEC-1 (Future Conditions)

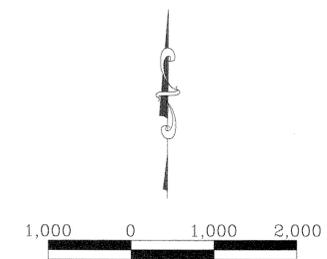
OPERATION	STATION	PEAK FLOW	TIME OF PEAK	AVERAGE FLOW FOR MAXIMUM PERIOD			BASIN AREA	MAXIMUM STAGE
				6-HOUR	24-HOUR	72-HOUR		
+ DIVERSION TO	A01	745.	12.17	131.	45.	33.	0.51	
+ HYDROGRAPH AT	LA01D	619.	12.08	58.	18.	13.	0.51	
+ 2 COMBINED AT	LA01	741.	12.25	95.	27.	20.	0.51	
+ HYDROGRAPH AT	CA01	2096.	13.42	784.	239.	173.	4.94	
+ DIVERSION TO	B10	65.	12.42	17.	5.	4.	0.11	
+ HYDROGRAPH AT	LB10D	65.	12.42	17.	5.	4.	0.11	
+ HYDROGRAPH AT	LB10	1.	20.50	0.	0.	0.	0.11	

\*\*\* NORMAL END OF HEC-1 \*\*\*



**LEGEND**

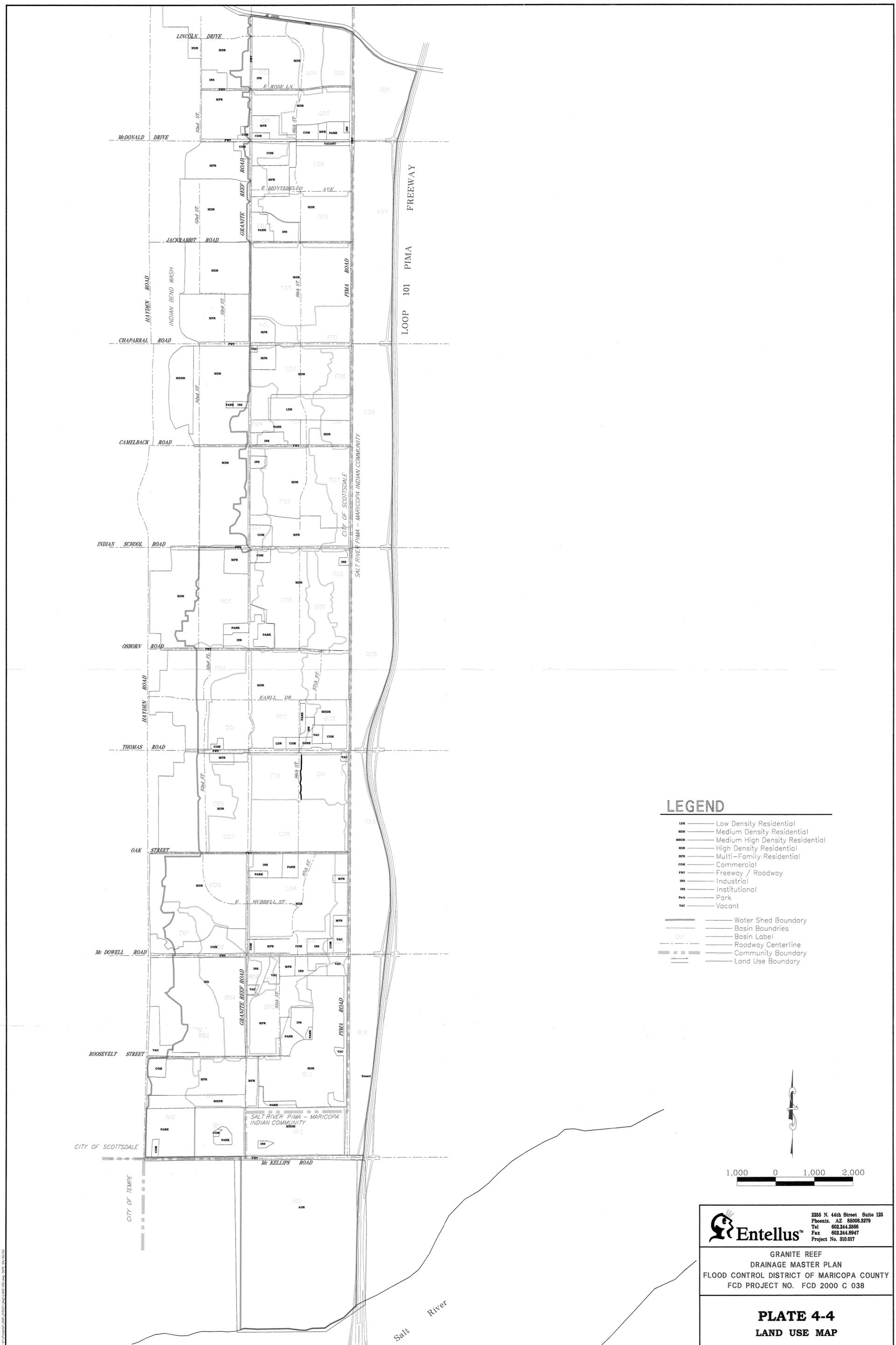
	Basin Boundary
	Basin Label
	Concentration Point
	Diversion Point
	Diversion Recovery Point
	Routing Reach
	Storage Routing
	Roadway Center Line
	Community Boundary
	Watershed Boundary



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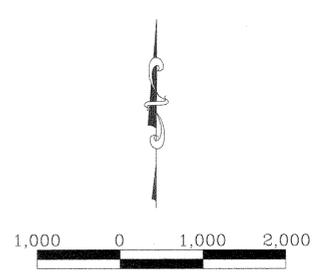
**GRANITE REEF  
DRAINAGE MASTER PLAN  
FLOOD CONTROL DISTRICT OF MARICOPA COUNTY  
FCD PROJECT NO. FCD 2000 C 038**

**PLATE 4-2  
HEC-1 SCHEMATIC DRAWING**



### LEGEND

LDR	Low Density Residential
MDR	Medium Density Residential
MHDR	Medium High Density Residential
HDR	High Density Residential
MFR	Multi-Family Residential
COM	Commercial
FWY	Freeway / Roadway
IND	Industrial
INS	Institutional
PARK	Park
VAC	Vacant
	Water Shed Boundary
	Basin Boundaries
	Basin Label
	Roadway Centerline
	Community Boundary
	Land Use Boundary



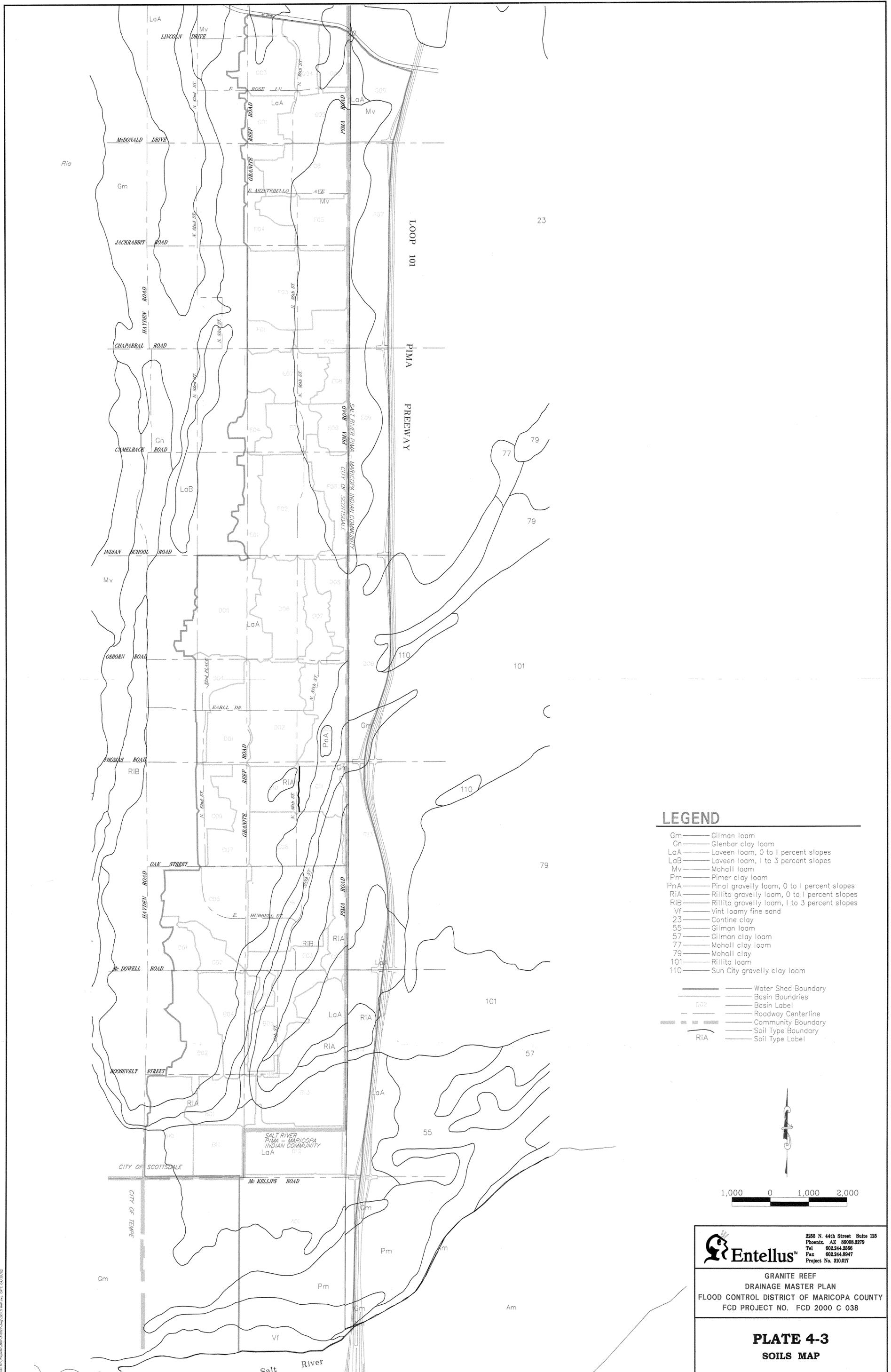
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FCD PROJECT NO. FCD 2000 C 038

PLATE 4-4  
LAND USE MAP

FILE: N:\Projects\810\810\_038\_Land Use Map.dwg DATE: 04/29/03

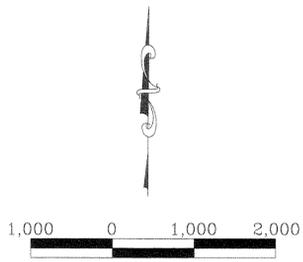


### LEGEND

Gm	—	Gilman loam
Gn	—	Glenbar clay loam
LaA	—	Laveen loam, 0 to 1 percent slopes
LaB	—	Laveen loam, 1 to 3 percent slopes
Mv	—	Mohall loam
Pm	—	Pimer clay loam
PnA	—	Pinal gravelly loam, 0 to 1 percent slopes
RiA	—	Rillito gravelly loam, 0 to 1 percent slopes
RiB	—	Rillito gravelly loam, 1 to 3 percent slopes
Vf	—	Vint loamy fine sand
23	—	Contine clay
55	—	Gilman loam
57	—	Gilman clay loam
77	—	Mohall clay loam
79	—	Mohall clay
101	—	Rillito loam
110	—	Sun City gravelly clay loam

—	—	Water Shed Boundary
—	—	Basin Boundaries
002	—	Basin Label
—	—	Roadway Centerline
—	—	Community Boundary
—	—	Soil Type Boundary
RiA	—	Soil Type Label




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**GRANITE REEF  
 DRAINAGE MASTER PLAN  
 FLOOD CONTROL DISTRICT OF MARICOPA COUNTY  
 FCD PROJECT NO. FCD 2000 C 038**

**PLATE 4-3  
 SOILS MAP**

FILE: N:\Projects\2000\2000\_403\_Soils\_Map.dwg PLOT: 04/29/02