

TECHNICAL DATA NOTEBOOK  
**WITTMANN PHASE IV FLOODPLAIN DELINEATION STUDY**

**Iona Tributary 1 West,  
Iona Tributary 1 West Extension, and  
Iona Tributary 2 West**

Contract FCD 2011C003

November 23, 2011



Prepared for:  
**The Flood Control District of Maricopa County**  
2801 West Durango Street  
Phoenix, Arizona 85009  
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Prepared by:  
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Nathan Ford, P.E.  
*Engineer*

Technical Data Notebook

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**WITTMANN PHASE IV FLOODPLAIN DELINEATION STUDY**  
**Iona Tributary 1 West (T5NR3WS17-1W), Iona Tributary 1**  
**West Extension (T5NR3WS17-1WE), and Iona Tributary 2**  
**West (T5NR3WS17-2W)**

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CONTRACT FCD 2011C003

*Prepared for:*

The Flood Control District of Maricopa County  
2801 West Durango Street  
Phoenix, Arizona 85009  
(602) 506-1501

*Project Manager:*

Jonathan Lesperance, E.I.T., CFM

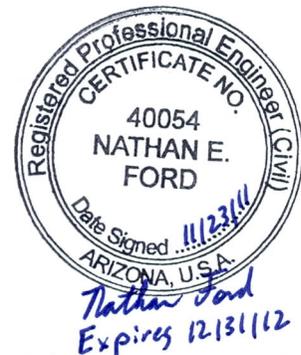
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Nathan Ford, P.E.

November 23, 2011



Wittmann Phase IV  
Floodplain Delineation Study

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Wittmann Phase IV  
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## **Section 1 Introduction**

### **1.1. Purpose of Study**

The purpose of this study is to delineate limited detail Zone AE 100-year floodplains (no floodway) for Iona Tributary 1 West (T5NR3WS17-1W), Iona Tributary 1 West Extension (T5NR3WS17-1WE), and Iona Tributary 2 West (T5NR3WS17-2W) within unincorporated Maricopa County (Refer to Figure 1). The study is located within the City of Surprise Special Planning Area (SPA) 5.

### **1.2. Authority for the Study**

The Flood Control District of Maricopa County (FCDMC) contracted RBF Consulting to perform the study based on existing 2 and 4 foot contour interval topographic mapping. The main contacts, addresses, and other information about the FCDMC and RBF Consulting are:

#### **Flood Control District of Maricopa County**

Address: 2801 West Durango Street  
Phoenix, Arizona 85009  
Phone: (602) 506-1501  
Project Manager: Jonathan Lesperance, E.I.T., CFM

#### **RBF Consulting**

Address: 16605 North 28<sup>th</sup> Avenue, Suite 100  
Phoenix, Arizona 85053  
Phone: (602) 467-2200  
Principal-in-Charge: Scott Larson, P.E.  
Project Manager: Nathan Ford, P.E.

### **1.3. Site Location and Description**

The study watershed has a drainage area of about 16.8 square miles and is located in Maricopa County, Arizona (Refer to Figure 4.1). These washes are tributary to Iona Wash. Iona Wash is a large desert wash with a 100-year peak discharge of approximately 5,650 cfs downstream of the confluence with Iona Tributary 2 West. Iona Wash has a delineated 100-year floodway and floodplain. This report is organized according to Arizona State Standard 1-97 *Instructions for Organizing and Submitting Technical Documentation for Flood Studies* developed by the Arizona Department of Water Resources (ADWR), Flood Mitigation Section, dated November 1997 (ADWR, 1997).

## **1.4. Methodology**

### **1.4.1. Hydrology**

Peak flows were determined for both the 100-year 6-hour and 100-year 24-hour storms using the U.S. Army Corps of Engineers HEC-1 software package, version 4.1, dated June 1998, from the Wittmann Area Drainage Master Study Update (ADMSU) Hydrology and basin prorating as outlined in Section 4 of this report. A more detailed explanation of the hydrologic methodology and results are provided in Section 4.

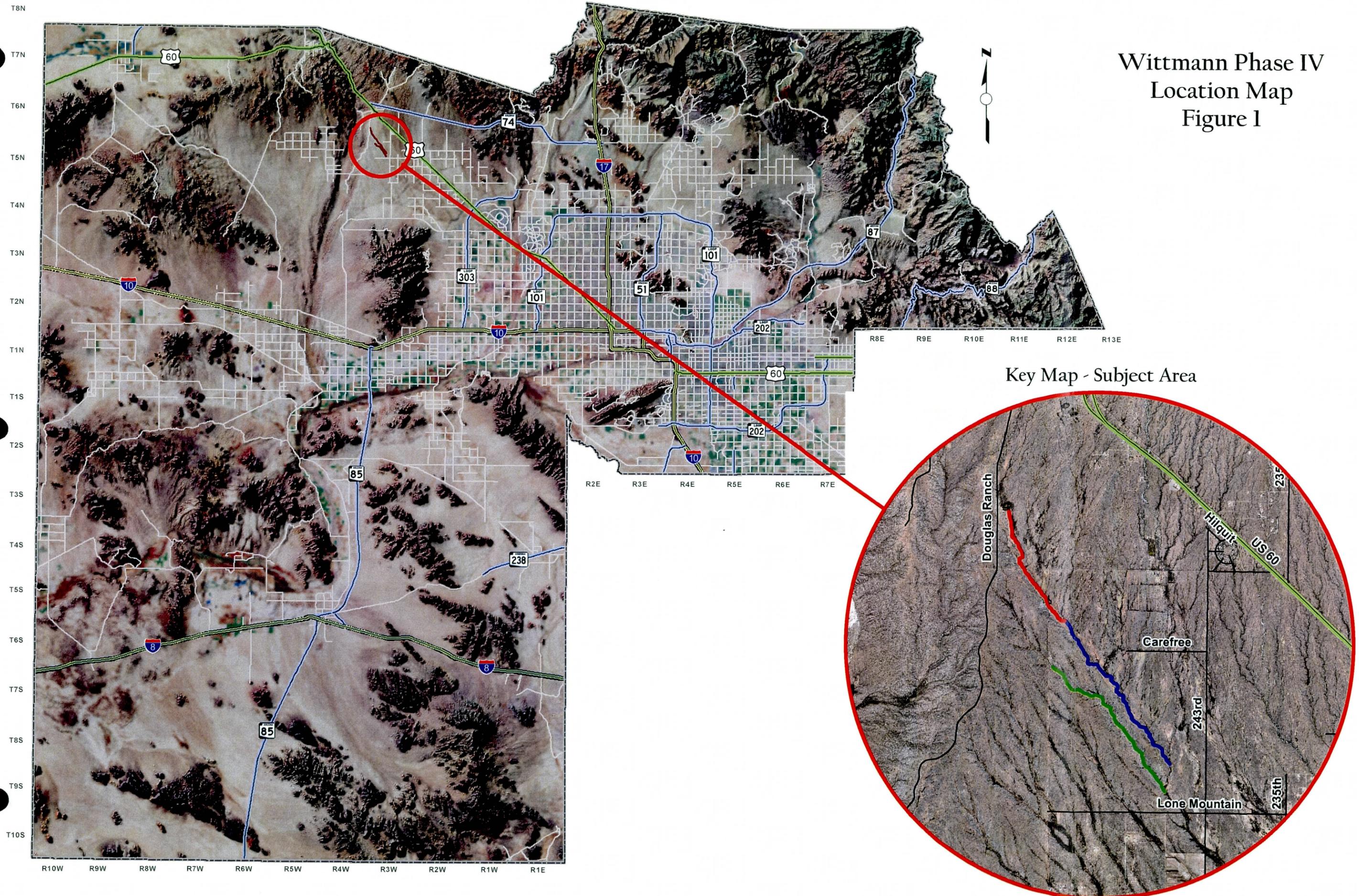
### **1.4.2. Hydraulics and Floodplain Delineation**

The higher peak flow from the 100-year 6-hour and 100-year 24-hour HEC-1 model was used to delineate the floodplain. In all cases the 100-year 6-hour storm was the governing storm that produced the higher water surface elevation. The Zone AE floodplains were delineated using the U.S. Army Corps of Engineers Hydrologic Engineering Center River Analysis System (HEC-RAS) version 4.1.0 dated January 2010.

## **1.5. Summary of Results**

The study resulted in the delineation of over six miles of limited detail Zone AE floodplains (no floodways). The floodplains have been plotted on the Hydraulic Study Maps, located at the end of Appendix E.

# Wittmann Phase IV Location Map Figure 1



Key Map - Subject Area



Wittmann Phase IV  
Floodplain Delineation Study

## Section 2 FEMA Forms

### 2.1. Study Documentation Abstract for FEMA Submittals

Study Documentation Abstract for FEMA Submittals		Initial Study	<input checked="" type="checkbox"/>	Restudy	<input type="checkbox"/>	CLOMR	<input type="checkbox"/>	LOMR	<input checked="" type="checkbox"/>
2.1.1	Date Study Accepted								
2.1.2	Study Contractor Contact Address  Phone Internal Reference #	RBF Consulting Scott Larson, P.E. or Nathan Ford, P.E. 16605 North 28 <sup>th</sup> Avenue, Suite 100 Phoenix, Arizona 85053-7550 602-467-2200 45-104608							
2.1.3	FEMA Technical Review Contractor Contact Address  Phone Internal Reference #								
2.1.4	FEMA Regional Reviewer Phone								
2.1.5	State NFIP Coordinator Phone	Brian Cosson, CFM Arizona Department of Water Resources 602-771-8657							
2.1.6	Local Technical Reviewer Phone	Flood Control District of Maricopa County Jonathan Lesperance, E.I.T., CFM 602-506-4699							
2.1.7	Reach Description	Desert Washes Iona Tributary 1 West, Iona Tributary 1 West Extension, and Iona Tributary 2 West in Maricopa County, Arizona							
2.1.8	USGS Quad Sheet  Original photo date Latest photo revision date	Wittmann, Arizona and Wickenburg SW, Arizona  1965 and 1965 1981 and N/A							
2.1.9	Unique Conditions and Problems	N/A							
2.1.10	Coordination of Q's Discharges (Agency, Date, Comments)	N/A							

**2.2. FEMA Forms**

U.S. DEPARTMENT OF HOMELAND SECURITY  
 FEDERAL EMERGENCY MANAGEMENT AGENCY  
**OVERVIEW & CONCURRENCE FORM**

*O.M.B No. 1660-0016  
 Expires February 28, 2014*

**PAPERWORK BURDEN DISCLOSURE NOTICE**

Public reporting burden for this form is estimated to average 1 hours per response. The burden estimate includes the time for reviewing instructions, searching existing data sources, gathering and maintaining the needed data, and completing, reviewing, and submitting the form. You are not required to respond to this collection of information unless it displays a valid OMB control number. Send comments regarding the accuracy of the burden estimate and any suggestions for reducing this burden to: Information Collections Management, Department of Homeland Security, Federal Emergency Management Agency, 1800 South Bell Street, Arlington, VA 20958-3005, Paperwork Reduction Project (1660-0016). Submission of the form is required to obtain or retain benefits under the National Flood Insurance Program. **Please do not send your completed survey to the above address.**

**PRIVACY ACT STATEMENT**

**AUTHORITY:** The National Flood Insurance Act of 1968, Public Law 90-448, as amended by the Flood Disaster Protection Act of 1973, Public Law 93-234.

**PRINCIPAL PURPOSE(S):** This information is being collected for the purpose of determining an applicant's eligibility to request changes to National Flood Insurance Program (NFIP) Flood Insurance Rate Maps (FIRM).

**ROUTINE USE(S):** The information on this form may be disclosed as generally permitted under 5 U.S.C § 552a(b) of the Privacy Act of 1974, as amended. This includes using this information as necessary and authorized by the routine uses published in DHS/FEMA/NFIP/LOMA-1 National Flood Insurance Program (NFIP); Letter of Map Amendment (LOMA) February 15, 2006, 71 FR 7990.

**DISCLOSURE:** The disclosure of information on this form is voluntary; however, failure to provide the information requested may delay or prevent FEMA from processing a determination regarding a requested change to a (NFIP) Flood Insurance Rate Maps (FIRM).

**A. REQUESTED RESPONSE FROM DHS-FEMA**

This request is for a (check one):

- CLOMR: A letter from DHS-FEMA commenting on whether a proposed project, if built as proposed, would justify a map revision, or proposed hydrology changes (See 44 CFR Ch. 1, Parts 60, 65 & 72).
- LOMR: A letter from DHS-FEMA officially revising the current NFIP map to show the changes to floodplains, regulatory floodway or flood elevations. (See 44 CFR Ch. 1, Parts 60, 65 & 72)

**B. OVERVIEW**

1. The NFIP map panel(s) affected for all impacted communities is (are):

Community No.	Community Name	State	Map No.	Panel No.	Effective Date
Example: 480301	City of Katy	TX	48473C	0005D	02/08/83
480287	Harris County	TX	48201C	0220G	09/28/90
	Refer to attached sheet				

2. a. Flooding Source: Iona Tributary 1 West, Iona Tributary 1 West Extension, and Iona Tributary 2 West

- b. Types of Flooding:  Riverine     Coastal     Shallow Flooding (e.g., Zones AO and AH)
- Alluvial fan     Lakes     Other (Attach Description)

3. Project Name/Identifier: Wittmann Phase IV Floodplain Delineation Study

4. FEMA zone designations affected: AE (choices: A, AH, AO, A1-A30, A99, AE, AR, V, V1-V30, VE, B, C, D, X)

5. Basis for Request and Type of Revision:

a. The basis for this revision request is (check all that apply)

- |   |  |   |   |
|---|--|---|---|
| <input type="checkbox"/> Physical Change      | <input type="checkbox"/> Improved Methodology/Data     | <input type="checkbox"/> Regulatory Floodway Revision | <input type="checkbox"/> Base Map Changes |
| <input type="checkbox"/> Coastal Analysis     | <input checked="" type="checkbox"/> Hydraulic Analysis | <input type="checkbox"/> Hydrologic Analysis          | <input type="checkbox"/> Corrections      |
| <input type="checkbox"/> Weir-Dam Changes     | <input type="checkbox"/> Levee Certification           | <input type="checkbox"/> Alluvial Fan Analysis        | <input type="checkbox"/> Natural Changes  |
| <input type="checkbox"/> New Topographic Data | <input type="checkbox"/> Other (Attach Description)    |   |   |

Note: A photograph and narrative description of the area of concern is not required, but is very helpful during review.

b. The area of revision encompasses the following structures (check all that apply)

Structures:  Channelization  Levee/Floodwall  Bridge/Culvert  
 Dam  Fill  Other (Attach Description)

6.  Documentation of ESA compliance is submitted (required to initiate CLOMR review). Please refer to the instructions for more information.

**C. REVIEW FEE**

Has the review fee for the appropriate request category been included?  Yes Fee amount: \$\_\_\_\_\_  
 No, Attach Explanation

Please see the DHS-FEMA Web site at [http://www.fema.gov/plan/prevent/fhm/frm\\_fees.shtml](http://www.fema.gov/plan/prevent/fhm/frm_fees.shtml) for Fee Amounts and Exemptions.

**D. SIGNATURE**

All documents submitted in support of this request are correct to the best of my knowledge. I understand that any false statement may be punishable by fine or imprisonment under Title 18 of the United States Code, Section 1001.

Name: Jonathan Lesperance, E.I.T., CFM		Company: FCDMC	
Mailing Address: 2801 W. Durango St Phoenix, AZ 85009		Daytime Telephone No.: (602) 506-4699	Fax No.: (602) 506-4601
		E-Mail Address: jonathanlesperance@mail.maricopa.gov	
Signature of Requester (required):		Date:	

As the community official responsible for floodplain management, I hereby acknowledge that we have received and reviewed this Letter of Map Revision (LOMR) or conditional LOMR request. Based upon the community's review, we find the completed or proposed project meets or is designed to meet all of the community floodplain management requirements, including the requirements for when fill is placed in the regulatory floodway, and that all necessary Federal, State, and local permits have been, or in the case of a conditional LOMR, will be obtained. For Conditional LOMR requests, the applicant has documented Endangered Species Act (ESA) compliance to FEMA prior to FEMA's review of the Conditional LOMR application. For LOMR requests, I acknowledge that compliance with Sections 9 and 10 of the ESA has been achieved independently of FEMA's process. For actions authorized, funded, or being carried out by Federal or State agencies, documentation from the agency showing its compliance with Section 7(a)(2) of the ESA will be submitted. In addition, we have determined that the land and any existing or proposed structures to be removed from the SFHA are or will be reasonably safe from flooding as defined in 44CFR 65.2(c), and that we have available upon request by FEMA, all analyses and documentation used to make this determination.

Community Official's Name and Title: Timothy S. Phillips, P.E. Chief Engineer & General Manager		Community Name: FCDMC	
Mailing Address: 2801 W. Durango St Phoenix, AZ 85009		Daytime Telephone No.: (602) 506-4701	Fax No.: (602) 506-4601
		E-Mail Address: tsp@mail.maricopa.gov	
Community Official's Signature (required):		Date:	

**CERTIFICATION BY REGISTERED PROFESSIONAL ENGINEER AND/OR LAND SURVEYOR**

This certification is to be signed and sealed by a licensed land surveyor, registered professional engineer, or architect authorized by law to certify elevation information data, hydrologic and hydraulic analysis, and any other supporting information as per NFIP regulations paragraph 65.2(b) and as described in the MT-2 Forms Instructions. All documents submitted in support of this request are correct to the best of my knowledge. I understand that any false statement may be punishable by fine or imprisonment under Title 18 of the United States Code, Section 1001.

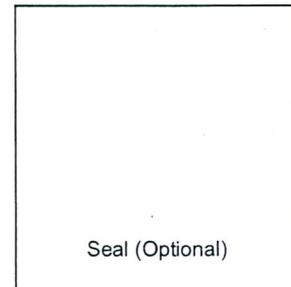
Certifier's Name: Nathan Ford		License No.: 40054	Expiration Date: 12/31/12
Company Name: RBF Consulting		Telephone No.: (602) 467-2200	Fax No.: (602) 467-2201
Signature: <i>Nathan Ford</i>		Date: <i>11/23/11</i>	E-Mail Address: nford@rbf.com

Ensure the forms that are appropriate to your revision request are included in your submittal.

Form Name and (Number)

Required if ...

- |   |   |
|---|---|
| <input checked="" type="checkbox"/> Riverine Hydrology and Hydraulics Form (Form 2) | New or revised discharges or water-surface elevations   |
| <input type="checkbox"/> Riverine Structures Form (Form 3)                          | Channel is modified, addition/revision of bridge/culverts, addition/revision of levee/floodwall, addition/revision of dam |
| <input type="checkbox"/> Coastal Analysis Form (Form 4)                             | New or revised coastal elevations   |
| <input type="checkbox"/> Coastal Structures Form (Form 5)                           | Addition/revision of coastal structure  |
| <input type="checkbox"/> Alluvial Fan Flooding Form (Form 6)                        | Flood control measures on alluvial fans   |



The NFIP map panels affected for all impacted communities are:

Community No.	Community Name	State	Map No.	Panel No.	Effective Date
040037	Maricopa County	AZ	04013C	660G 670G 680G 689H 690H	9/30/05

The Flood Control District of Maricopa County is requesting a fee exemption for the LOMR based on map changes being based on flood hazard information meant to improve upon that shown on the flood map or within the flood study. The new study does not incorporate any manmade modifications within the SFHA.

U.S. DEPARTMENT OF HOMELAND SECURITY  
 FEDERAL EMERGENCY MANAGEMENT AGENCY  
**RIVERINE HYDROLOGY & HYDRAULICS FORM**

*O.M.B No. 1660-0016  
 Expires February 28, 2014*

**PAPERWORK BURDEN DISCLOSURE NOTICE**

Public reporting burden for this form is estimated to average 3.5 hours per response. The burden estimate includes the time for reviewing instructions, searching existing data sources, gathering and maintaining the needed data, and completing, reviewing, and submitting the form. You are not required to respond to this collection of information unless a valid OMB control number appears in the upper right corner of this form. Send comments regarding the accuracy of the burden estimate and any suggestions for reducing this burden to: Information Collections Management, Department of Homeland Security, Federal Emergency Management Agency, 1800 South Bell Street, Arlington VA 20958-3005, Paperwork Reduction Project (1660-0016). Submission of the form is required to obtain or retain benefits under the National Flood Insurance Program. **Please do not send your completed survey to the above address.**

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**DISCLOSURE:** The disclosure of information on this form is voluntary; however, failure to provide the information requested may delay or prevent FEMA from processing a determination regarding a requested change to a NFIP Flood Insurance Rate Maps (FIRM).

Flooding Source: Iona Tributary 1 West and Iona Tributary 1 West Extension

**Note:** Fill out one form for each flooding source studied

**A. HYDROLOGY**

1. Reason for New Hydrologic Analysis (check all that apply)

- |   |  |  |
|---|--|--|
| <input type="checkbox"/> Not revised (skip to section B)    | <input type="checkbox"/> No existing analysis        | <input type="checkbox"/> Improved data                           |
| <input checked="" type="checkbox"/> Alternative methodology | <input type="checkbox"/> Proposed Conditions (CLOMR) | <input type="checkbox"/> Changed physical condition of watershed |

2. Comparison of Representative 1%-Annual-Chance Discharges

Location	Drainage Area (Sq. Mi.)	Effective/FIS (cfs)	Revised (cfs)
----------	-------------------------	---------------------	---------------

3. Methodology for New Hydrologic Analysis (check all that apply)

- |   |   |
|---|---|
| <input type="checkbox"/> Statistical Analysis of Gage Records | <input checked="" type="checkbox"/> Precipitation/Runoff Model → Specify Model: <u>HEC-1 (Wittmann ADMSU)</u> |
| <input type="checkbox"/> Regional Regression Equations        | <input type="checkbox"/> Other (please attach description)  |

Please enclose all relevant models in digital format, maps, computations (including computation of parameters), and documentation to support the new analysis.

4. Review/Approval of Analysis

If your community requires a regional, state, or federal agency to review the hydrologic analysis, please attach evidence of approval/review.

5. Impacts of Sediment Transport on Hydrology

Is the hydrology for the revised flooding source(s) affected by sediment transport?  Yes  No

If yes, then fill out Section F (Sediment Transport) of Form 3. If No, then attach your explanation..

**B. HYDRAULICS**

1. Reach to be Revised

	Description	Cross Section	Water-Surface Elevations (ft.)	
			Effective	Proposed/Revised
Downstream Limit*	<u>Iona Tributary 1 West</u>	<u>0.088</u>	<u>N/A</u>	<u>1722.8</u>
Upstream Limit*	<u>Iona Tributary 1 West Extension</u>	<u>4.161</u>	<u>N/A</u>	<u>1856.9</u>

\*Proposed/Revised elevations must tie-into the Effective elevations within 0.5 foot at the downstream and upstream limits of revision.

2. Hydraulic Method/Model Used: HEC-RAS

3. Pre-Submittal Review of Hydraulic Models\*

DHS-FEMA has developed two review programs, CHECK-2 and CHECK-RAS, to aid in the review of HEC-2 and HEC-RAS hydraulic models, respectively. We recommend that you review your HEC-2 and HEC-RAS models with CHECK-2 and CHECK-RAS.

4.

	<u>Models Submitted</u>	<u>Natural Run</u>	<u>Floodway Run</u>	<u>Datum</u>
Duplicate Effective Model*	File Name: _____	Plan Name: _____	File Name: _____	Plan Name: _____
Corrected Effective Model*	File Name: _____	Plan Name: _____	File Name: _____	Plan Name: _____
Existing or Pre-Project Conditions Model	File Name: _____	Plan Name: _____	File Name: _____	Plan Name: _____
Revised or Post-Project Conditions Model	File Name: _____	Plan Name: _____	File Name: _____	Plan Name: _____
Other - (attach description)	File Name: _____	Plan Name: _____	File Name: _____	Plan Name: _____

\* For details, refer to the corresponding section of the instructions.

Digital Models Submitted? (Required)

**C. MAPPING REQUIREMENTS**

A **certified topographic work map** must be submitted showing the following information (where applicable): the boundaries of the effective, existing, and proposed conditions 1%-annual-chance floodplain (for approximate Zone A revisions) or the boundaries of the 1%- and 0.2%-annual-chance floodplains and regulatory floodway (for detailed Zone AE, AO, and AH revisions); location and alignment of all cross sections with stationing control indicated; stream, road, and other alignments (e.g., dams, levees, etc.); current community easements and boundaries; boundaries of the requester's property; certification of a registered professional engineer registered in the subject State; location and description of reference marks; and the referenced vertical datum (NGVD, NAVD, etc.).

Digital Mapping (GIS/CADD) Data Submitted (preferred)

Topographic Information: 4 ft contour data

Source: Landdata Airborne Systems, Inc. for FCDMC Date: Flown April 2002

Accuracy: \_\_\_\_\_

Note that the boundaries of the existing or proposed conditions floodplains and regulatory floodway to be shown on the revised FIRM and/or FBFM must tie-in with the effective floodplain and regulatory floodway boundaries. Please attach a **copy of the effective FIRM and/or FBFM**, at the same scale as the original, annotated to show the boundaries of the revised 1%-and 0.2%-annual-chance floodplains and regulatory floodway that tie-in with the boundaries of the effective 1%-and 0.2%-annual-chance floodplain and regulatory floodway at the upstream and downstream limits of the area on revision.

Annotated FIRM and/or FBFM (Required)

D. COMMON REGULATORY REQUIREMENTS\*

1. For LOMR/CLOMR requests, do Base Flood Elevations (BFEs) increase?  Yes  No
- a. For CLOMR requests, if either of the following is true, please submit **evidence of compliance with Section 65.12 of the NFIP regulations**:
- The proposed project encroaches upon a regulatory floodway and would result in increases above 0.00 foot compared to pre-project conditions.
  - The proposed project encroaches upon a SFHA with or without BFEs established and would result in increases above 1.00 foot compared to pre-project conditions.
- b. Does this LOMR request cause increase in the BFE and/or SFHA compared with the effective BFEs and/or SFHA?  Yes  No  
If Yes, please attach **proof of property owner notification and acceptance (if available)**. Elements of and examples of property owner notifications can be found in the MT-2 Form 2 Instructions.
2. Does the request involve the placement or proposed placement of fill?  Yes  No
- If Yes, the community must be able to certify that the area to be removed from the special flood hazard area, to include any structures or proposed structures, meets all of the standards of the local floodplain ordinances, and is reasonably safe from flooding in accordance with the NFIP regulations set forth at 44 CFR 60.3(A)(3), 65.5(a)(4), and 65.6(a)(14). Please see the MT-2 instructions for more information.
3. For LOMR requests, is the regulatory floodway being revised?  Yes  No
- If Yes, attach **evidence of regulatory floodway revision notification**. As per Paragraph 65.7(b)(1) of the NFIP Regulations, notification is required for requests involving revisions to the regulatory floodway. (Not required for revisions to approximate 1%-annual-chance floodplains [studied Zone A designation] unless a regulatory floodway is being established. Elements and examples of regulatory floodway revision notification can be found in the MT-2 Form 2 Instructions.)
4. For CLOMR requests, please submit documentation to FEMA and the community to show that you have complied with Sections 9 and 10 of the Endangered Species Act (ESA).

For actions authorized, funded, or being carried out by Federal or State agencies, please submit documentation from the agency showing its compliance with Section 7(a)(2) of the ESA. Please see the MT-2 instructions for more detail.

Not inclusive of all applicable regulatory requirements. For details, see 44 CFR parts 60 and 65.

U.S. DEPARTMENT OF HOMELAND SECURITY  
 FEDERAL EMERGENCY MANAGEMENT AGENCY  
**RIVERINE HYDROLOGY & HYDRAULICS FORM**

*O.M.B No. 1660-0016  
 Expires February 28, 2014*

**PAPERWORK BURDEN DISCLOSURE NOTICE**

Public reporting burden for this form is estimated to average 3.5 hours per response. The burden estimate includes the time for reviewing instructions, searching existing data sources, gathering and maintaining the needed data, and completing, reviewing, and submitting the form. You are not required to respond to this collection of information unless a valid OMB control number appears in the upper right corner of this form. Send comments regarding the accuracy of the burden estimate and any suggestions for reducing this burden to: Information Collections Management, Department of Homeland Security, Federal Emergency Management Agency, 1800 South Bell Street, Arlington VA 20958-3005, Paperwork Reduction Project (1660-0016). Submission of the form is required to obtain or retain benefits under the National Flood Insurance Program. **Please do not send your completed survey to the above address.**

**PRIVACY ACT STATEMENT**

**AUTHORITY:** The National Flood Insurance Act of 1968, Public Law 90-448, as amended by the Flood Disaster Protection Act of 1973, Public Law 93-234.

**PRINCIPAL PURPOSE(S):** This information is being collected for the purpose of determining an applicant's eligibility to request changes to National Flood Insurance Program (NFIP) Flood Insurance Rate Maps (FIRM).

**ROUTINE USE(S):** The information on this form may be disclosed as generally permitted under 5 U.S.C § 552a(b) of the Privacy Act of 1974, as amended. This includes using this information as necessary and authorized by the routine uses published in DHS/FEMA/NFIP/LOMA-1 National Flood Insurance Program (NFIP); Letter of Map Amendment (LOMA) February 15, 2006, 71 FR 7990.

**DISCLOSURE:** The disclosure of information on this form is voluntary; however, failure to provide the information requested may delay or prevent FEMA from processing a determination regarding a requested change to a NFIP Flood Insurance Rate Maps (FIRM).

Flooding Source: Iona Tributary 2 West

**Note:** Fill out one form for each flooding source studied

**A. HYDROLOGY**

1. Reason for New Hydrologic Analysis (check all that apply)

- |   |  |  |
|---|--|--|
| <input type="checkbox"/> Not revised (skip to section B)    | <input type="checkbox"/> No existing analysis        | <input type="checkbox"/> Improved data                           |
| <input checked="" type="checkbox"/> Alternative methodology | <input type="checkbox"/> Proposed Conditions (CLOMR) | <input type="checkbox"/> Changed physical condition of watershed |

2. Comparison of Representative 1%-Annual-Chance Discharges

Location	Drainage Area (Sq. Mi.)	Effective/FIS (cfs)	Revised (cfs)
----------	-------------------------	---------------------	---------------

3. Methodology for New Hydrologic Analysis (check all that apply)

- |   |   |
|---|---|
| <input type="checkbox"/> Statistical Analysis of Gage Records | <input checked="" type="checkbox"/> Precipitation/Runoff Model → Specify Model: <u>Prorate (Wittmann ADMSU)</u> |
| <input type="checkbox"/> Regional Regression Equations        | <input type="checkbox"/> Other (please attach description)  |

Please enclose all relevant models in digital format, maps, computations (including computation of parameters), and documentation to support the new analysis.

4. Review/Approval of Analysis

If your community requires a regional, state, or federal agency to review the hydrologic analysis, please attach evidence of approval/review.

5. Impacts of Sediment Transport on Hydrology

Is the hydrology for the revised flooding source(s) affected by sediment transport?  Yes  No

If yes, then fill out Section F (Sediment Transport) of Form 3. If No, then attach your explanation..

**B. HYDRAULICS**

1. Reach to be Revised

	Description	Cross Section	Water-Surface Elevations (ft.)	
			Effective	Proposed/Revised
Downstream Limit*	<u>Iona Tributary 2 West</u>	<u>0.101</u>	<u>N/A</u>	<u>1710.0</u>
Upstream Limit*	<u>Iona Tributary 2 West</u>	<u>2.272</u>	<u>N/A</u>	<u>1784.8</u>

\*Proposed/Revised elevations must tie-into the Effective elevations within 0.5 foot at the downstream and upstream limits of revision.

2. Hydraulic Method/Model Used: HEC-RAS

3. Pre-Submittal Review of Hydraulic Models\*

DHS-FEMA has developed two review programs, CHECK-2 and CHECK-RAS, to aid in the review of HEC-2 and HEC-RAS hydraulic models, respectively. We recommend that you review your HEC-2 and HEC-RAS models with CHECK-2 and CHECK-RAS.

4.

<u>Models Submitted</u>	<u>Natural Run</u>		<u>Floodway Run</u>		<u>Datum</u>
	File Name:	Plan Name:	File Name:	Plan Name:	
Duplicate Effective Model*	_____	_____	_____	_____	_____
Corrected Effective Model*	File Name: _____	Plan Name: _____	File Name: _____	Plan Name: _____	_____
Existing or Pre-Project Conditions Model	File Name: _____	Plan Name: _____	File Name: _____	Plan Name: _____	_____
Revised or Post-Project Conditions Model	File Name: _____	Plan Name: _____	File Name: _____	Plan Name: _____	_____
Other - (attach description)	File Name: _____	Plan Name: _____	File Name: _____	Plan Name: _____	_____

\* For details, refer to the corresponding section of the instructions.

Digital Models Submitted? (Required)

**C. MAPPING REQUIREMENTS**

A **certified topographic work map** must be submitted showing the following information (where applicable): the boundaries of the effective, existing, and proposed conditions 1%-annual-chance floodplain (for approximate Zone A revisions) or the boundaries of the 1%- and 0.2%-annual-chance floodplains and regulatory floodway (for detailed Zone AE, AO, and AH revisions); location and alignment of all cross sections with stationing control indicated; stream, road, and other alignments (e.g., dams, levees, etc.); current community easements and boundaries; boundaries of the requester's property; certification of a registered professional engineer registered in the subject State; location and description of reference marks; and the referenced vertical datum (NGVD, NAVD, etc.).

Digital Mapping (GIS/CADD) Data Submitted (preferred)

Topographic Information: 4 ft contour data

Source: Landdata Airborne Systems, Inc. for FCDMC

Date: Flown April 2002

Accuracy: \_\_\_\_\_

Note that the boundaries of the existing or proposed conditions floodplains and regulatory floodway to be shown on the revised FIRM and/or FBFM must tie-in with the effective floodplain and regulatory floodway boundaries. Please attach a **copy of the effective FIRM and/or FBFM**, at the same scale as the original, annotated to show the boundaries of the revised 1%-and 0.2%-annual-chance floodplains and regulatory floodway that tie-in with the boundaries of the effective 1%-and 0.2%-annual-chance floodplain and regulatory floodway at the upstream and downstream limits of the area on revision.

Annotated FIRM and/or FBFM (Required)

## D. COMMON REGULATORY REQUIREMENTS\*

1. For LOMR/CLOMR requests, do Base Flood Elevations (BFEs) increase?  Yes  No
- a. For CLOMR requests, if either of the following is true, please submit **evidence of compliance with Section 65.12 of the NFIP regulations**:
- The proposed project encroaches upon a regulatory floodway and would result in increases above 0.00 foot compared to pre-project conditions.
  - The proposed project encroaches upon a SFHA with or without BFEs established and would result in increases above 1.00 foot compared to pre-project conditions.
- b. Does this LOMR request cause increase in the BFE and/or SFHA compared with the effective BFEs and/or SFHA?  Yes  No  
If Yes, please attach **proof of property owner notification and acceptance (if available)**. Elements of and examples of property owner notifications can be found in the MT-2 Form 2 Instructions.
2. Does the request involve the placement or proposed placement of fill?  Yes  No  
If Yes, the community must be able to certify that the area to be removed from the special flood hazard area, to include any structures or proposed structures, meets all of the standards of the local floodplain ordinances, and is reasonably safe from flooding in accordance with the NFIP regulations set forth at 44 CFR 60.3(A)(3), 65.5(a)(4), and 65.6(a)(14). Please see the MT-2 instructions for more information.
3. For LOMR requests, is the regulatory floodway being revised?  Yes  No  
If Yes, attach **evidence of regulatory floodway revision notification**. As per Paragraph 65.7(b)(1) of the NFIP Regulations, notification is required for requests involving revisions to the regulatory floodway. (Not required for revisions to approximate 1%-annual-chance floodplains [studied Zone A designation] unless a regulatory floodway is being established. Elements and examples of regulatory floodway revision notification can be found in the MT-2 Form 2 Instructions.)
4. For CLOMR requests, please submit documentation to FEMA and the community to show that you have complied with Sections 9 and 10 of the Endangered Species Act (ESA).

For actions authorized, funded, or being carried out by Federal or State agencies, please submit documentation from the agency showing its compliance with Section 7(a)(2) of the ESA. Please see the MT-2 instructions for more detail.

Not inclusive of all applicable regulatory requirements. For details, see 44 CFR parts 60 and 65.



## **Section 3 Mapping and Survey Information**

### **3.1. Field Survey Information**

Field survey was collected for the bend on Iona Tributary 1 West between Cross Sections 1.827 and 1.776. Refer to the Ground Elevation Exhibit in Appendix C for additional information.

### **3.2. Mapping**

RBF used existing digital terrain models (DTM) and contour data (2 and 4 ft) provided by the Flood Control District of Maricopa County. Landata Airborne Systems, Inc. created the DTM from digital ortho-photos that were created in 2002 for the Wittmann mapping. The coordinate system is based on NAD 83, Arizona State Plane – Central Zone. The vertical coordinate system is NAVD 88.



## **Section 4 Hydrology**

### **4.1 Method Description**

The Wittmann Area Drainage Master Study Update (ADMSU) Technical Data Notebook, ADMSU Hydrology, Volume HY-Addendum, Contract FCD2002C029, Final Addendum No. 1 July 2005 prepared by Entellus was used as the source of the hydrology analysis. The peak flows for both the 100-year 6-hour and 100-year 24-hour storms existing conditions were compared to determine which storm generated the larger peak flows. The 100-year 6-hour storm generated higher peak flows than the 100-year 24-hour storm for the watershed of Iona Tributary 1 West, Iona Tributary 1 West Extension, and Iona Tributary 2 West and was used for the floodplain delineation.

Additional subdivision of the drainage areas was required for hydraulic modeling where the detailed Wittmann ADMSU sub-basins were too large to accurately model a portion of the wash reach. The Wittmann ADMSU HEC-1 model was modified using two smaller sub-basins to obtain peak flows due to the routing of flows contributing at the concentration point of interest for the Iona Tributary 1 West and Iona Tributary 1 West Extension watershed. The peak flows for the three smaller sub-basins within the Iona Tributary 2 West watershed were determined using the unit peak flows (cfs/sq mi) obtained from the larger Wittmann ADMSU sub-basin and prorating based on the sub-basin area.

### **4.2 Parameter Estimation**

Hydrologic parameters were obtained from the Wittmann ADMSU Hydrology. The following sections briefly discuss the parameter estimation. For more detail refer to the Wittmann ADMSU.

#### **4.2.1 Drainage Area Boundaries**

The Wittmann ADMSU delineates the Iona Tributary 1 West and Iona Tributary 1 West Extension watershed using seventeen sub-basins. The Iona Tributary 1 West and Iona Tributary 1 West Extension watershed extends to the north and northeast crossing the U.S 60, B.N.S.F. Railroad, and State Route 74. The watershed terminates near the Maricopa and Yavapai County line. The drainage area is approximately 15.1 square miles. Additional sub-basin delineation was required within the Wittmann ADMSU sub-basins. Two smaller sub-basins were delineated to more accurately determine the flow rates upstream of the confluence with tributary washes. These two sub-basins are IW353A and IW359A. The Wittmann ADMSU HEC-1 model was modified to account for the routing in lieu of proration for this watershed.

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The Wittmann ADMSU delineates the Iona Tributary 2 West watershed using one sub-basin. This sub-basin is IW350. The Iona Tributary 2 West watershed extends to the northwest and is entirely south of the U.S. 60. The drainage area is approximately 1.66 square miles. Additional sub-basin delineation was required within sub-basin IW350. Three smaller sub-basins were delineated to determine the flow at the confluence with Iona Wash, at Dove Valley Road, and upstream of a potential flow split. Since there is only one ADMSU basin the peak flows were determined using proration based on the sub-basin area and the unit peak flow (cfs/sq mi) obtained from the Wittmann ADMSU sub-basin IW350.

Figures 4.1 and 4.2 show the sub-basin delineation for Iona Tributary 1 West and Iona Tributary 1 West Extension, and Iona Tributary 2 West watershed with aerial photos and topo lines as their respective backgrounds.

### **4.2.2 Watershed Work Maps**

Figure 4.3 shows the sub-basin boundaries overlain on top of the soil map units, according to the Aguila-Carefree, Parts of Maricopa and Pinal Counties, Arizona Soil Survey. Figure 4.4 shows the land use designation obtained from the Maricopa Association of Governments (MAG).

### **4.2.3 Gage Data**

There are no precipitation gages located within the watershed. Seven FCDMC operating precipitation gages are located within a distance of less than six and one-half miles from the watershed boundary. Figure 4.5 shows the location of the FCDMC precipitation gages. One precipitation gage FCDMC No. 5450 (installed on 5/13/1992) is located at an elevation of 1,620 feet and is about one and three-quarters miles south of the watershed boundary. The name of this gage is Patton Road and is located in Section 33, Township 5 North Range 3 West (latitude: 33°44'05.8"; longitude: 112°34'37.9").

Precipitation gage FCDMC No. 5455 (installed on 5/13/1992) is located at an elevation of 1,655 feet and is about three and three-quarters miles east of the watershed boundary. The name of this gage is Wittmann and is located in Section 13, Township 5 North Range 3 West (latitude: 33°46'13.8"; longitude: 112°31'15.7").

Precipitation gage FCDMC No. 5460 (installed on 10/31/1990) is located at an elevation of 1,705 feet and is about five and one-half miles east of the watershed boundary. The name of this gage is Chrysler Proving Ground and is located in Section 7, Township 5 North Range 2 West (latitude: 33°47'03.5"; longitude: 112°30'00.8").

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Precipitation gage FCDMC No. 5475 (installed on 10/1/1982) is located at an elevation of 1,890 feet and is less than one-half mile east of the watershed boundary. The name of this gage is Circle City and is located in Section 32, Township 6 North Range 3 West (latitude: 33°49'20.9"; longitude: 112°35'23.8").

Precipitation gage FCDMC No. 5490 (installed on 10/20/1981) is located at an elevation of 2,685 feet and is about one and one-quarter miles east of the watershed boundary. The name of this gage is Castle Hot Springs and is located in Section 24, Township 7 North Range 3 West (latitude: 33°55'45.4"; longitude: 112°31'47.1").

Precipitation gage FCDMC No. 5220 (installed on 5/13/1992) is located at an elevation of 1,980 feet and is near the western watershed boundary. The name of this gage is Morristown and is located in Section 13, Township 6 North Range 4 West (latitude: 33°51'23.6"; longitude: 112°37'27.3").

Precipitation gage FCDMC No. 5495 (installed on 11/8/2007) is located at an elevation of 1,680 feet and is about six and one-half miles southwest of the watershed boundary. The name of this gage is Daggs Wash and is located in Section 25, Township 5 North Range 5 West (latitude: 33°44'45.2"; longitude: 112°43'30.2").

There are no stream gages located within the Wittmann Phase IV watershed.

### **4.2.4 Statistical Parameters**

Statistical Parameters were not considered as part of this study.

### **4.2.5 Precipitation**

The Wittmann ADSMU used the NOAA Atlas 2 rainfall depths. The point rainfall depths of the 100-year 6-hour storm and 100-year 24-hour storm are 3.40 inches and 4.20 inches, respectively. The point rainfall depths were adjusted by areal reduction factors as outlined in the Wittmann ADSMU.

### **4.2.6 Physical Parameters**

#### *Rainfall Losses*

The Wittmann ADMSU used the Green-Ampt infiltration equations within HEC-1 to estimate rainfall losses. Table 4.1 lists the map unit values that were used in the Wittmann ADMSU to compute the rainfall losses for the eighteen sub-basins within the Wittmann Phase IV watershed.

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**Table 4.1 Soils Characteristics for Green-Ampt Parameters**

Soil ID	Description	XKSAT (in/hr)	% Impervious	% Effective
3	Antho-Carrizo-Maripo complex	0.58	0	100
7	Anthony-Arizo complex, low precipitation	0.62	0	100
13	Carefree-Beardsley complex	0.01	0	100
21	Cipriano very gravelly loam	0.38	0	100
22	Contine clay loam	0.04	0	100
28	Continental-Ohaco complex	0.02	0	100
29	Denure-Momoli-Carrizo complex	0.34	0	100
31	Dixaleta-Rock outcrop complex, 25 to 65 percent slopes	0.33	35	100
33	Eba very gravelly loam, 1 to 8 percent slopes	0.23	0	100
36	Eba-Continental complex, 1 to 8 percent slopes	0.07	0	100
39	Eba-Nickel-Cave association, 3 to 25 percent slopes	0.29	0	100
42	Eba-Pinaleno complex, low precipitation, 3 to 20 percent slopes	0.17	0	100
44	Ebon very gravelly loam, 1 to 8 percent slopes	0.03	0	100
46	Ebon-Contine complex, 1 to 8 percent slopes	0.03	0	100
47	Ebon-Gunsight-Cipriano association, 3 to 25 percent slopes	0.11	0	100
48	Ebon-Pinamt complex, 3 to 20 percent slopes	0.06	0	100
49	Ebon-Pinamt complex, 20 to 40 percent slopes	0.06	0	100
53	Gadsden clay	0.02	0	100
55	Gilman loams	0.27	0	100
58	Gilman-Momoli-Denure complex	0.34	0	100
63	Gran-Wickenburg-Rock outcrop complex, 10 to 65 percent slopes	0.14	25	100
64	Gran-Wickenburg-Rock outcrop complex, low precipitation, 10 to 65 percent slopes	0.14	25	100
66	Greyeagle-Suncity Variant complex, 1 to 7 percent slopes	0.23	0	100
68	Gunsight-Cipriano complex, 1 to 7 percent slopes	0.63	0	100
70	Gunsight-Rillito complex, 1 to 25 percent slopes	0.36	0	100
73	Lehmans-Rock outcrop complex, low precipitation, 8 to 65 percent slopes	0.09	30	100
74	Luke-Cipriano association, 1 to 15 percent slopes	0.08	0	100
75	Mohall loam	0.23	0	100
77	Mohall clay loam	0.05	0	100
94	Nickel-Cave complex, low precipitation, 8 to 30 percent slopes	0.33	0	100
98	Pinamt-Tremant complex, 1 to 10 percent slopes	0.37	0	100
110	Suncity-Cipriano complex, 1 to 7 percent slopes	0.13	0	100
119	Tremant-Suncity complex, 1 to 8 percent slopes	0.14	0	100
121	Tres Hermanos-Anthony complex, 1 to 5 percent slopes	0.12	0	100

The Wittmann ADMSU used the Maricopa Association of Governments (MAG) Land Use. The values used for surface retention loss values, percent

Wittmann Phase IV  
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impervious, and vegetation cover are shown in Table 4.2 for the Wittmann Phase IV watershed.

**Table 4.2 Land Use Characteristics for Green-Ampt Parameters**

Land Use Description	Initial Abstraction (in)	% Impervious	% Vegetation	Soil Condition
Rural Residential (<=1/5du per acre)	0.30	5	30	normal
Estate Residential (<=1 du/acre)	0.30	10	30	normal
Large Lot Residential – Single Family (1 du/acre to 2 du/acre)	0.30	15	50	normal
Educational (Public schools, private schools, universities)	0.25	45	80	normal
Other Employment – low (Proving grounds and landfills)	0.15	80	25	normal
General Transportation (where no detail available)	0.10	20	30	normal
Transportation (Includes railroads, railyards, transit centers, and freeways)	0.10	80	30	normal
General Open Space (where no detail available)	0.35	5	30	normal
Passive Open Space (Includes mountain preserves and washes)	0.35	0	30	normal

The Green-Ampt rainfall loss parameters from the Wittmann ADMSU and the two supplemental sub-basins can be found in Table 4.3.

**Table 4.3 Green-Ampt Parameters**

BASIN	IA (in)	DTHETA	PSIF	XKSAT (in/hr)	RTIMP (%)
IW350	0.350	0.171	6.789	0.135	5.000
IW353	0.350	0.150	7.596	0.098	5.084
IW353A*	0.350	0.250	5.876	0.191	5.000
IW357	0.330	0.156	8.059	0.083	11.106
IW359	0.345	0.250	5.939	0.187	6.181
IW359A*	0.348	0.210	6.399	0.159	5.491
IW366	0.343	0.250	5.762	0.202	7.207
IW371	0.275	0.250	4.150	0.473	26.250
IW374	0.315	0.161	8.147	0.086	12.602
IW377	0.304	0.382	5.921	0.188	5.357
IW381	0.309	0.190	9.152	0.056	11.882
IW382	0.322	0.275	6.319	0.164	8.175
IW384	0.343	0.150	8.675	0.065	7.206
IW386	0.334	0.192	8.439	0.072	5.278
IW388	0.337	0.156	8.775	0.061	9.239
IW389	0.348	0.130	10.100	0.037	5.526
IW390	0.349	0.250	5.071	0.258	5.221

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BASIN	IA (in)	DTHETA	PSIF	XKSAT (in/hr)	RTIMP (%)
IW390A	0.350	0.250	6.134	0.172	17.033
IW392	0.350	0.150	7.459	0.103	8.347
IW394	0.350	0.327	6.166	0.173	16.632

\* Sub-basins created for the Wittmann Phase IV FDS

*Unit Hydrograph Procedure*

The Wittmann ADMSU used the Clark Unit Hydrograph method to compute the unit hydrographs. The time of concentration ( $T_c$ ) and storage coefficient (R) information from the Wittmann ADMSU is shown in Table 4.4

**Table 4.4 Sub-Basin Time of Concentration Parameters**

Basin	L (mi)	Slope (ft/mi)	100-YR 24-HR $T_c$ (hr)	100-YR 24-HR R (hr)	100-YR 6-HR $T_c$ (hr)	100-YR 6-HR R (hr)
IW350	4.946	34.8	1.363	1.203	1.308	1.150
IW353	4.736	54.9	1.004	0.717	0.996	0.710
IW353A*	1.717	54.7	N/A	N/A	0.629	0.412
IW357	2.981	67.8	0.742	0.752	0.700	0.705
IW359	3.796	46.4	0.979	0.571	0.983	0.574
IW359A*	3.500	46.9	N/A	N/A	0.946	0.659
IW366	2.729	54.2	0.796	0.622	0.763	0.593
IW371	0.541	70.2	0.308	0.327	0.275	0.288
IW374	1.450	67.6	0.475	0.374	0.442	0.345
IW377	1.116	78.9	0.442	0.566	0.408	0.519
IW381	2.406	60.7	0.704	0.873	0.650	0.799
IW382	2.183	56.8	0.688	0.599	0.642	0.555
IW384	0.654	61.1	0.338	0.327	0.308	0.296
IW386	1.805	66.5	0.917	0.945	0.838	0.855
IW388	2.950	61.0	1.200	0.958	1.133	0.900
IW389	1.197	88.6	0.638	0.825	0.588	0.753
IW390	2.038	83.4	1.021	1.302	0.929	1.173
IW390A	2.931	104.1	1.033	0.963	0.971	0.899
IW392	1.972	244.5	0.500	0.278	0.504	0.280
IW394	5.124	218.6	1.367	0.857	1.367	0.857

\* Sub-basins created for the Wittmann Phase IV FDS

*Channel Routing*

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The normal depth storage-outflow channel routing is used to route runoff hydrographs through sub-basins. The routing length, slope, and Manning's n values from the Wittmann ADMSU are shown in Table 4.5.

**Table 4.5 Channel Routing Summary**

Reach	Reach Length (ft)	Slope (ft/ft)	LOB n value	Channel n value	ROB n value
RIW357	16,693	0.0079	0.04	0.04	0.04
RIW359	13,031	0.0059	0.04	0.04	0.04
RIW359**	4,667	0.0051	0.04	0.04	0.04
R359*	9,280	0.0103	0.04	0.04	0.04
RIW366	9,657	0.0070	0.04	0.04	0.04
RIW366**	8,105	0.0070	0.04	0.04	0.04
RIW371	11,583	0.0088	0.04	0.04	0.04
RD371	1,409	0.0014	0.043	0.035	0.043
RIW374	20,437	0.0081	0.04	0.04	0.04
RIW374**	18,876	0.0081	0.04	0.04	0.04
RIW377	6,588	0.0096	0.04	0.04	0.04
RIW381	4,964	0.0096	0.04	0.04	0.04
RIW382	4,964	0.0120	0.04	0.04	0.04
RD382A	1,172	0.0068	0.043	0.035	0.043
RIW384	4,694	0.0124	0.04	0.04	0.04
RIW386	10,311	0.0078	0.043	0.035	0.043
RIW388	7,726	0.0094	0.04	0.04	0.04
RIW389	11,459	0.0124	0.04	0.04	0.04
RIW390	11,626	0.0101	0.043	0.035	0.043
R390A	8,577	0.0134	0.043	0.035	0.043
RIW394	7,697	0.0133	0.043	0.035	0.043

\*\* Modified Routing created for the Wittmann Phase IV FDS

### 4.3 Problems Encountered During the Study

#### 4.3.1 Special Problems and Solutions

None were encountered.

#### 4.3.2 Modeling Warning and Error Messages

The HEC-1 model did not contain any warning or error messages.

### 4.4 Calibration

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Floodplain Delineation Study

No model calibration was performed as part of this study.

## 4.5 Final Results

### 4.5.1 Hydrologic Analysis Results

The results for peak discharges are shown in Table 4.6. The higher peak flow between the 100-year 6-hour and 100-year 24-hour storms should be used for the floodplain delineation. In all cases, the 100-year 6-hour storm was the governing storm that produced the higher peak flows.

Concentration point CIW353 in the Wittmann ADMSU hydrology model combines additional area tributary to Iona Wash. Since the flow in Iona Tributary 1 West at the confluence of Iona Wash is necessary for the floodplain delineation purposes, the Wittmann ADMSU concentration point CIW353 was modified to exclude the flows from Iona Wash. In other words, CIW353 excludes sub-basin IW358, which contributes to Iona Wash instead of Iona Tributary 1 West.

Additional sub-basin delineation was necessary for five reaches. Table 4.7 shows the results of the prorated basins. Iona Tributary 1 West and Iona Tributary 1 West Extension each have one additional sub-basin added to account for flow upstream of the confluence with tributary washes. Iona Tributary 2 West has three sub-basins where flows were obtained from prorating basin IW350.

**Table 4.6 Sub-basin Hydrologic Analysis Results**

Drainage ID	Area (sq mi)	100-YR Peak Discharge (cfs)		Unit Peak (cfs/sq mi)	
		24-HR	6-HR	24-HR	6-HR
IW350	2.18	1,154	1,410	531	648
CIW359	5.97	3,105	3,423	520	573
CIW353*	15.12	4,986	5,467	330	362

\*Modified from the Wittmann ADMSU

**Table 4.7 Prorated Basin Results**

Drainage ID	Area (sq mi)	Calculated Peak Flow (cfs)
CIW353A	6.69	3,561
CIW359A	3.50	2,334
IW350A	1.66	1,074
IW350B	1.25	811
IW350C	1.01	652

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**Table 4.8 Discharges used for Floodplain Delineation**

Wash	Upstream Cross Section	Downstream Cross Section	100-Year Flow (cfs)
Iona Tributary 1 West	1.538	0.088	5,467
Iona Tributary 1 West	2.362	1.604	3,561
Iona Tributary 1 West Extension	2.646	2.456	3,423
Iona Tributary 1 West Extension	4.161	2.741	2,334
Iona Tributary 2 West	0.852	0.101	1,074
Iona Tributary 2 West	1.515	0.946	811
Iona Tributary 2 West	2.272	1.609	652

#### 4.5.2 Verification of Results

The verification of results was performed using three indirect methods. Indirect Method No. 1 makes a comparison between seven envelope curves of maximum observed flood discharges. The seven point values (CIW353, CIW353A, CIW359, CIW359A, IW350A, IW350B, and IW350C) from the Wittmann ADMSU were lower than all seven envelope curves (Refer to the graphs located in Appendix D.6). Indirect Method No. 2 compares USGS data for Arizona. The seven point values from the Wittmann ADMSU are located near the Log Pierson Type 3 Curve and well within the 75% Tolerance Limits. Indirect Method No. 3 compares USGS Regional Regression Equations. The watershed for Iona Tributary 1 West, Iona Tributary 1 West Extension, and Iona Tributary 2 West is located within the Central Arizona Region (R12). This regional equation computes the 100-year discharge by relating the 100-year peak discharge with drainage area where AREA is the drainage area in square miles and ELEV is the mean basin elevation in feet divided by 1,000.

#### Regional Regression Equation for Central Arizona Region (R12)

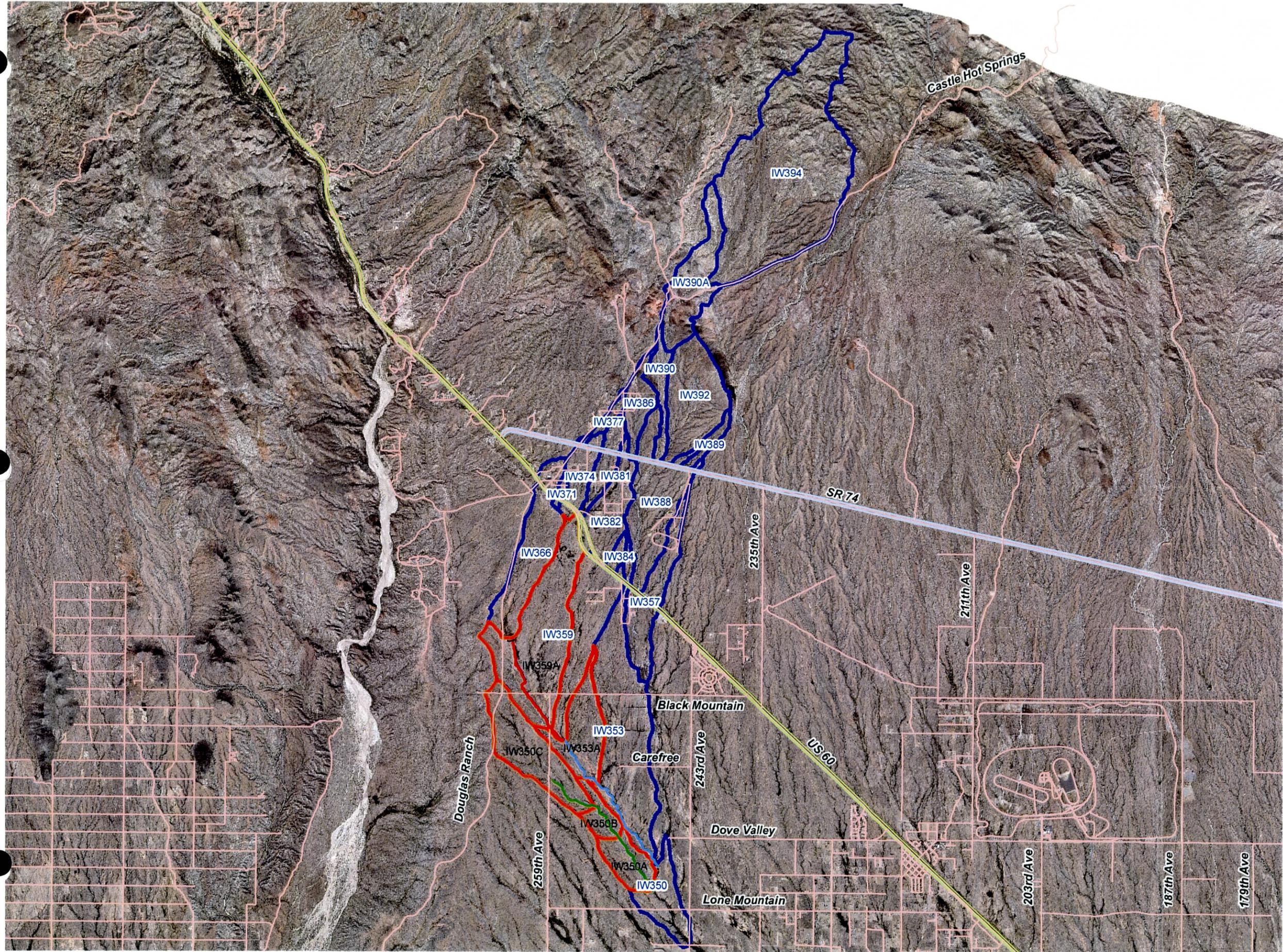
$$Q_{100} = 10^{(6.55 - 3.17 \text{ AREA}^{-0.11} - 0.454 \text{ LOGELEV})}$$

As can be seen in Table 4.9, the comparison shows the results from this study are lower than the USGS regional regression equation. It should be mentioned that the results of the Wittmann ADMSU should be more accurate than those using the regression equation because the study was more detailed.

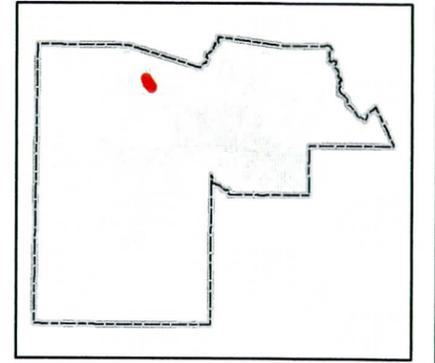
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**Table 4.9 USGS Regional Regression Peak Discharge Comparison**

Drainage ID	Area (sq mi)	Mean Elev (ft)	100-YR Peak Discharge (cfs)		USGS Regional Regression Equation (R12) 100-YR
			24-HR	6-HR	
CIW353	15.12	2548	4,986	5,467	10,334
CIW353A	6.69	2572	N/A	3,561	6,191
CIW359	5.97	2586	3,105	3,423	5,732
CIW359A	3.50	2590	N/A	2,334	3,985
IW350A	1.66	1780	881	1,074	2,743
IW350B	1.25	1790	663	811	2,198
IW350C	1.01	1803	536	652	1,850



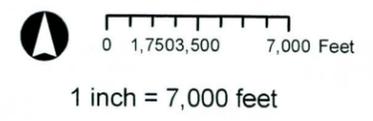
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Floodplain Delineation Study**

**Legend**

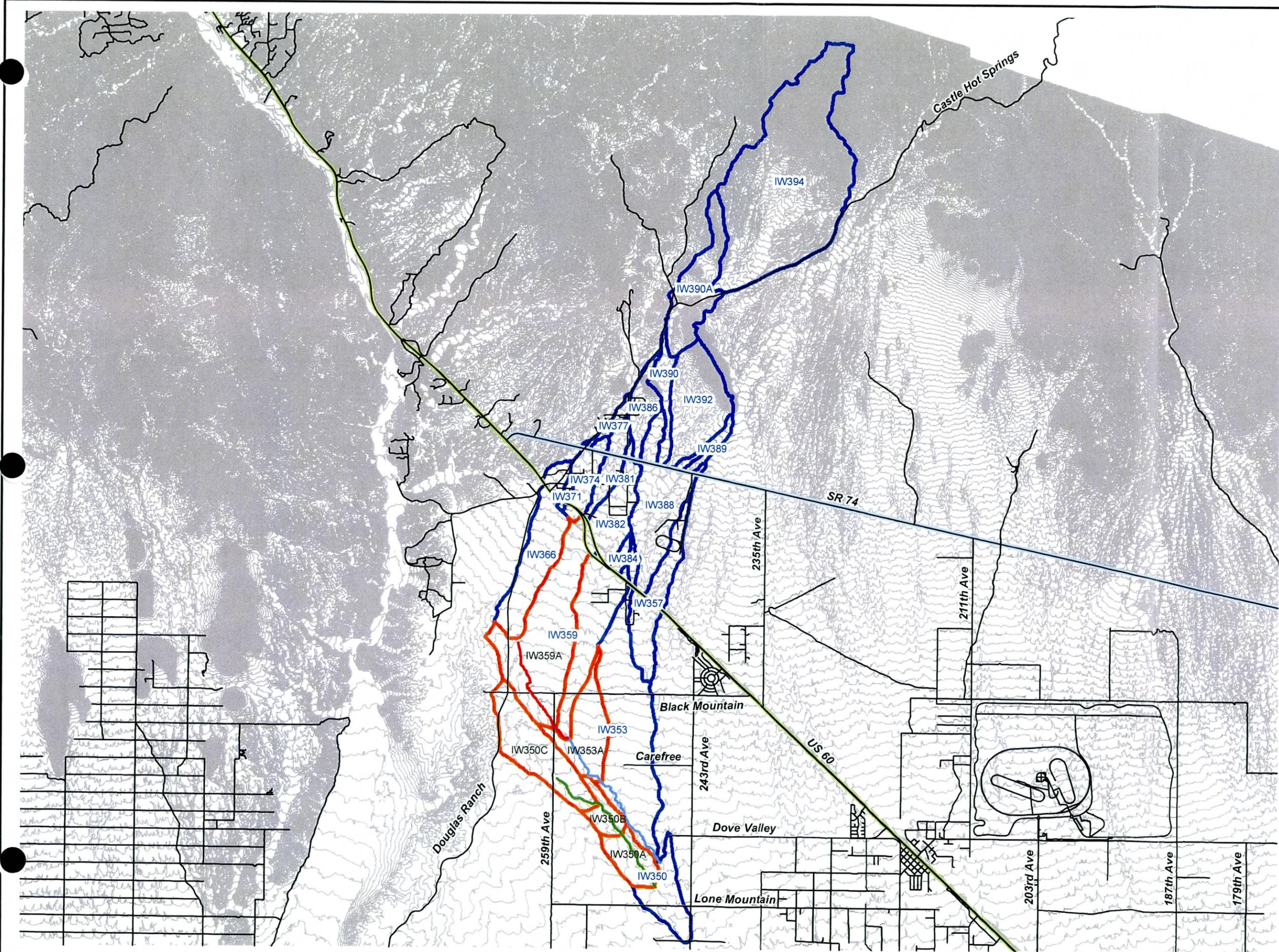
-  Iona Tributary 1 West
-  Iona Tributary 1 West Extension
-  Iona Tributary 2 West
-  Streets
-  Prorated Basins
-  WADMSU Subbasins



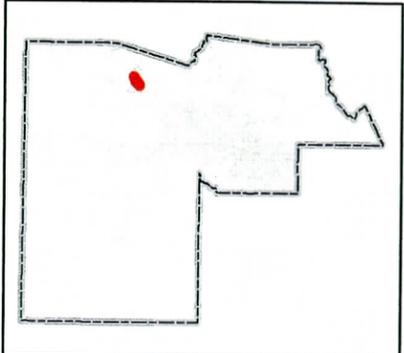
**Sub-basin Boundaries W/ Aerial**

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	DLP	NEF
	PROJECT NUMBER	45104608
DATE	8.8.11	FIGURE
		4.1



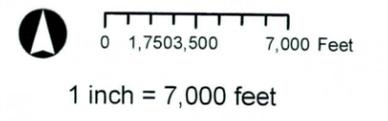
**Flood Control District of Maricopa County**



**Wittmann Phase IV  
Floodplain Delineation Study**

**Legend**

- Iona Tributary 1 West
- Iona Tributary 1 West Extension
- Iona Tributary 2 West
- Prorated Basins
- WADMSU Subbasins
- Contours
- Streets

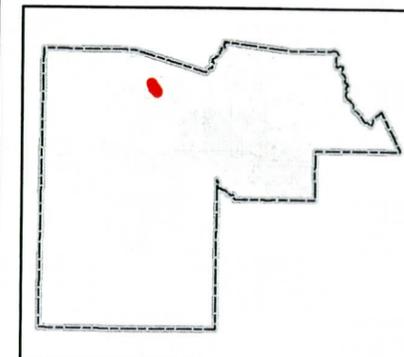


**Sub-basin Boundaries W/ Topo**

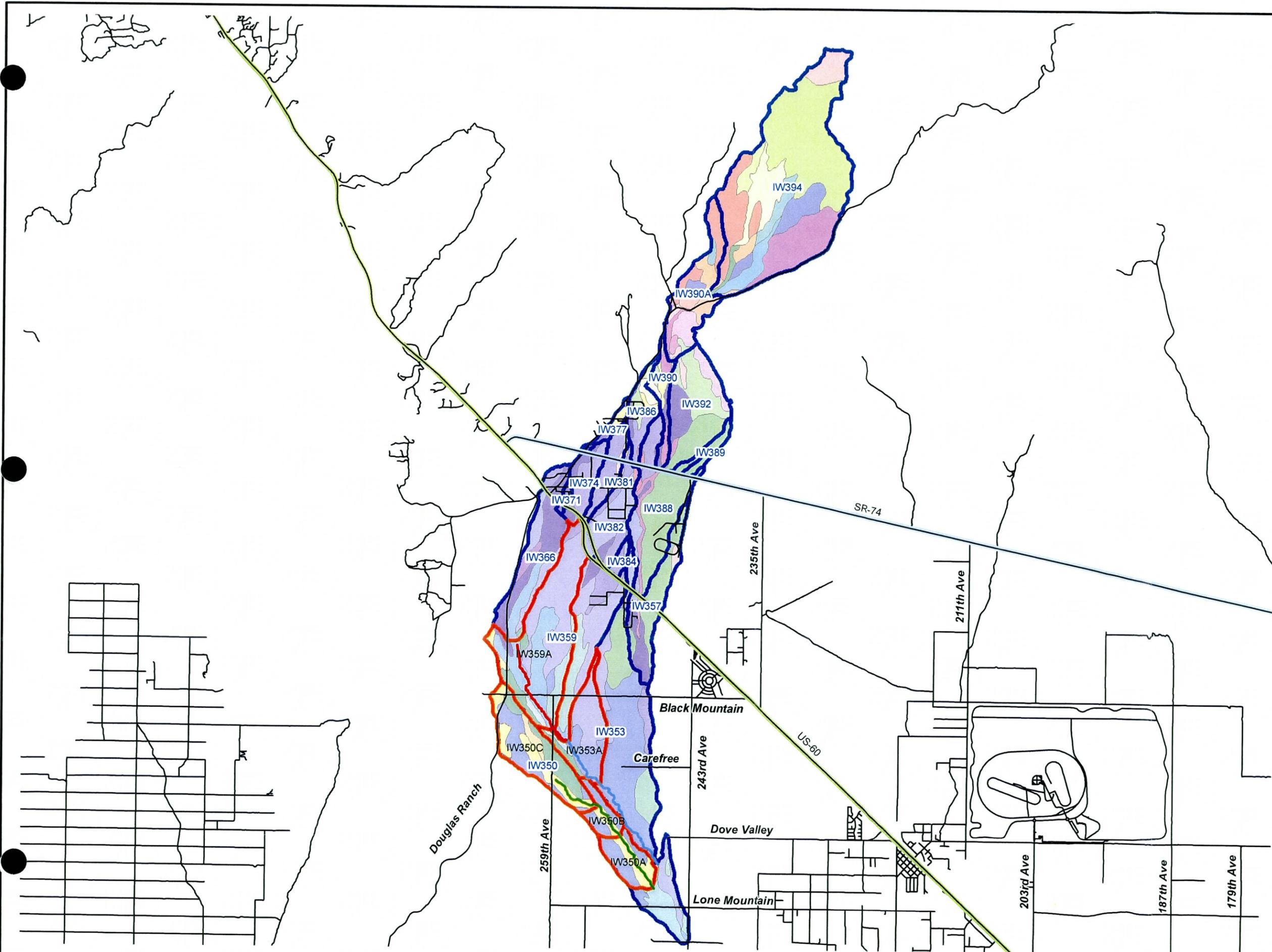
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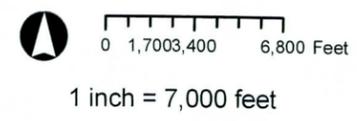
# Flood Control District of Maricopa County



## Wittmann Phase IV Floodplain Delineation Study



- Legend**
- Prorated Basins
  - WADMSU Subbasins
  - Streets
- Soils**
- DESCRIPTION**
- Antho-Carrizo-Maripo complex
  - Anthony-Arizo complex, low precipitation
  - Carefree-Beardsley complex
  - Cipriano very gravelly loam
  - Contine clay loam
  - Continental-Ohaco complex
  - Denure-Momoli-Carrizo complex
  - Dixaleta-Rock outcrop complex, 25 to 65 percent slopes
  - Eba very gravelly loam, 1 to 8 percent slopes
  - Eba-Continental complex, 1 to 8 percent slopes
  - Eba-Nickel-Cave association, 3 to 25 percent slopes
  - Eba-Pinaleno complex, low precipitation, 3 to 20 percent slopes
  - Ebon very gravelly loam, 1 to 8 percent slopes
  - Ebon-Contine complex, 1 to 8 percent
  - Ebon-Gunsight-Cipriano association, 3 to 25 percent slopes
  - Ebon-Pinamnt complex, 20 to 40 percent slopes
  - Ebon-Pinamnt complex, 3 to 20 percent slopes
  - Gadsden clay
  - Gilman loams
  - Gilman-Momoli-Denure complex
  - Gran-Wickenburg-Rock outcrop complex, 1 to 7 percent slopes
  - Gran-Wickenburg-Rock outcrop complex, low precipitation, 10 to 65 percent slopes
  - Greyeagle-Suncity Variant complex, 1 to 7 percent slopes
  - Gunsight-Cipriano complex, 1 to 7 percent slopes
  - Gunsight-Rillito complex, 1 to 25 percent slopes
  - Lehman's-Rock outcrop complex, low precipitation, 8 to 65 percent slopes
  - Luke-Cipriano association, 1 to 15 percent slopes
  - Mohall clay loam
  - Mohall loam
  - Nickel-Cave complex, low precipitation, 8 to 30 percent slopes
  - Pinamnt-Tremant complex, 1 to 10 percent slopes
  - Suncity-Cipriano complex, 1 to 7 percent slopes
  - Tremant-Suncity complex, 1 to 8 percent slopes
  - Tres Hermanos-Anthony complex, 1 to 5 percent slopes

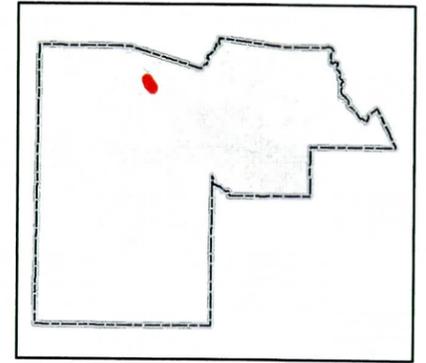


### Sub-basin Boundaries W/ Soils

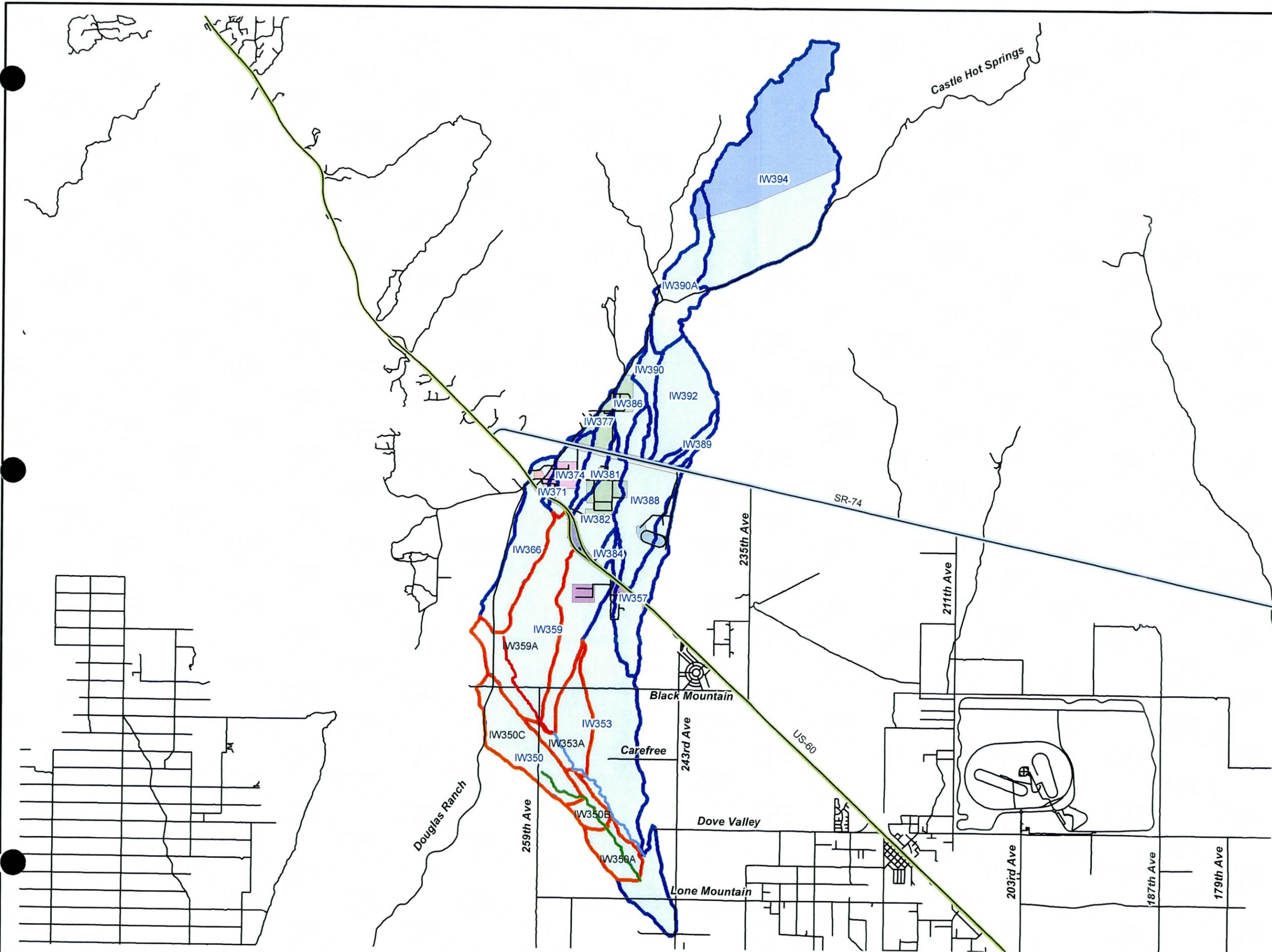
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FIGURE		4.3	

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**Wittmann Phase IV  
Floodplain Delineation Study**



- Legend**
- Iona Tributary 1 West
  - Iona Tributary 1 West Extension
  - Iona Tributary 2 West
  - Streets
  - Prorated Basins
  - WADMSU Subbasins
- Landuse**
- DESCRIPTION**
- Educational (Public schools, private schools and universities)
  - Estate Residential (1/5 du per acre to 1 du per acre)
  - General Open Space (Open space where no detail available)
  - General Transportation (Transportation where no detail available)
  - Large Lot Residential - Single Family (1 du per acre to 2 du per acre)
  - Other Employment - low (Proving grounds and land fills)
  - Passive Open Space (Includes mountain preserves and washes)
  - Rural Residential (<= 1/5 du per acre)
  - Transportation (Includes railroads, railyards, transit centers and freeways)



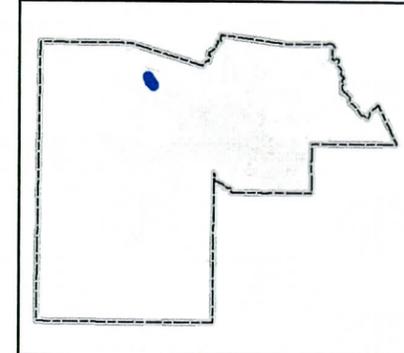
1 inch = 7,000 feet

**Sub-basin Boundaries W/ Land Use**

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FIGURE	4.4

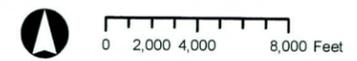
# Flood Control District of Maricopa County



## Wittmann Phase IV Floodplain Delineation Study

### Legend

- Gages
- Iona Tributary 1 West
- Iona Tributary 1 West Extension
- Iona Tributary 2 West
- Streets
- Watershed

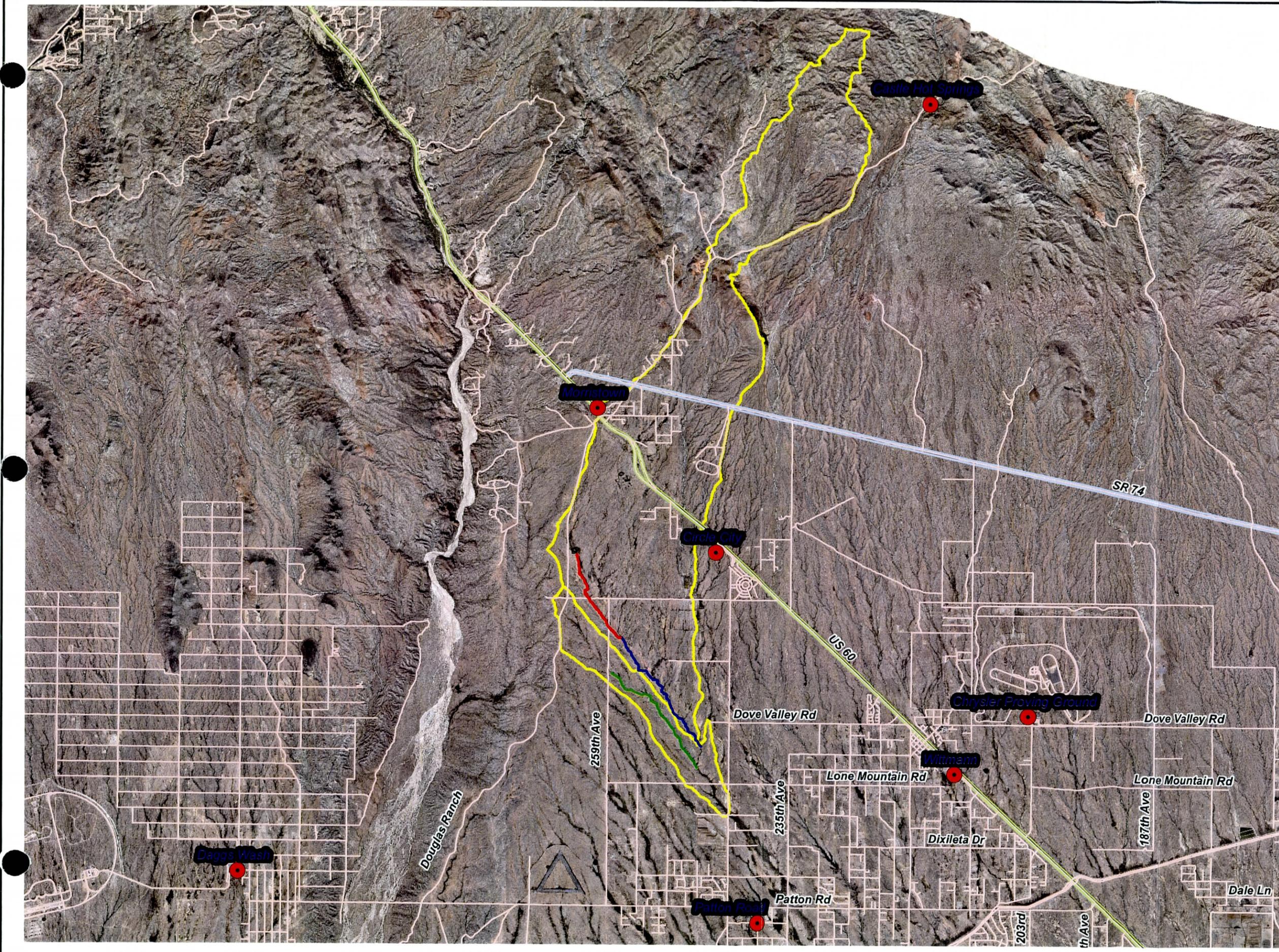


1 inch = 8,000 feet

### Gage Locations

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FIGURE		4.5	





## **Section 5 Hydraulics**

### **5.1. Method Description**

The U.S. Army Corps of Engineers Hydrologic Engineering Center River Analysis System (HEC-RAS) version 4.1.0 dated January 2010 was used to create the hydraulic model and floodplain delineation for this study. Cross sections were created from the existing elevation data provided by the FCDMC.

The name for each wash was obtained from the Wittmann ADMSU Floodplain Delineation Study. Iona Tributary 1 West and Iona Tributary 1 West Extension are used to differentiate where the previous Zone A floodplain delineation terminated. In addition, a name based on the location (Township, Range, and Section) of the wash discharge point was also included.

The floodplain delineation used the larger flow between the 100-year 6-hour and 100-year 24-hour storm. In all cases, the 100-year 6-hour storm event produced the higher peak flows.

According to Appendix C of FEMA's Guidelines and Specifications for Flood Hazard Mapping Partners, starting water-surface elevations for tributaries should be based on normal depth (i.e., slope-area method) unless a coincident peak situation is assumed, or the tributary flow depths are higher than the corresponding main stream events. The downstream boundary condition for both Iona Tributary 1 West and Iona Tributary 2 West used normal depth instead of the known water surface elevation for Iona Wash.

### **5.2. Work Study Maps**

Work Study Maps that show the floodplain delineations have been prepared at a scale of 1 inch = 200 feet, according to FEMA standards using the 2 and 4 foot contour interval topographic mapping. A cover sheet shows the location of the study area. Each Work Study Map shows the floodplain baseline, zone boundaries, and cross sections used in the delineation.

### **5.3. Parameter Estimation**

#### **5.3.1. Roughness Coefficients**

Iona Tributary 1 West, Iona Tributary 1 West Extension, and Iona Tributary 2 West are desert washes. The procedure used to determine the Manning's "n" value roughness coefficients is outlined in the USGS publication "Selection of Manning's Roughness Coefficients for Natural and Constructed Vegetated and Non-Vegetated Channels and Vegetation Maintenance Plan Guidelines for

Wittmann Phase IV  
Floodplain Delineation Study

Vegetated Channels in Central Arizona" (2006). Based on field observations, the Manning's roughness coefficients were fairly similar for each wash channel and overbanks. A list of the roughness coefficients for each wash, photos of each wash, and a description of how the roughness coefficients were obtained is provided in Appendix E.1

### **5.3.2. Expansion and Contraction Coefficients**

The expansion and contraction coefficients used in the HEC-RAS model are 0.1 and 0.3, respectively. There are not any bridges or culverts within the study limits.

### **5.4. Cross Section Description**

Cross sections were located at approximately 500 ft intervals along the washes. Additionally, cross sections were located near confluences and at particular areas of interest including future road crossings. The cross sections are oriented left to right looking downstream. A plot of each cross section is provided in Appendix E.2.

### **5.5. Modeling Considerations**

There are many locations within Iona Tributary 1 West and Tributary 1 West Extension and a few locations within Iona Tributary 2 West that show divided flow conditions creating islands of higher ground. The elevation difference between the island and the adjacent water surface elevation at the majority of these locations was less than one foot and consists of a small area. Therefore, these island areas were shown as floodplain.

### **5.6. Floodway Modeling**

Because this study is only producing limited detail Zone AE, floodways have not been modeled.

### **5.7. Problems Encountered During the Study**

No major problems were encountered during the study. However, the two washes appear to have areas of significant erosion and there is evidence of head cutting occurring since the topography was completed.

There is also a potential break out area at a bend between Cross Section 1.776 and Cross Section 1.827. This section has widened since the topography was completed. The floodplain was drawn based on the 2011 aerial photographs for

Wittmann Phase IV  
Floodplain Delineation Study

the right overbank. Additional survey points were collected to determine if the banks contained the 100-year flows. The water surface elevation exceeds the bank at this location. A limit of study was placed at this location and the potential flow split was not analyzed.

**5.8. Calibration**

Calibration was not performed as part of this study due to a lack of gages on the delineated washes.

**5.9. Final Results**

Refer to the floodplain maps located in the Appendix.



## **Section 6 Erosion and Sediment Transport**

Erosion and sediment transport were not considered in this study.



## **Section 7 Draft FIS Report Data**

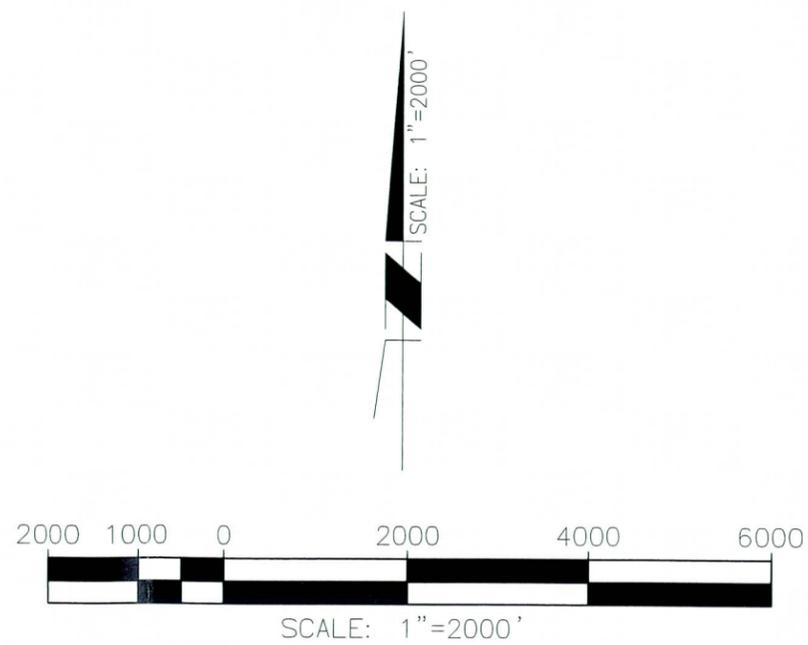
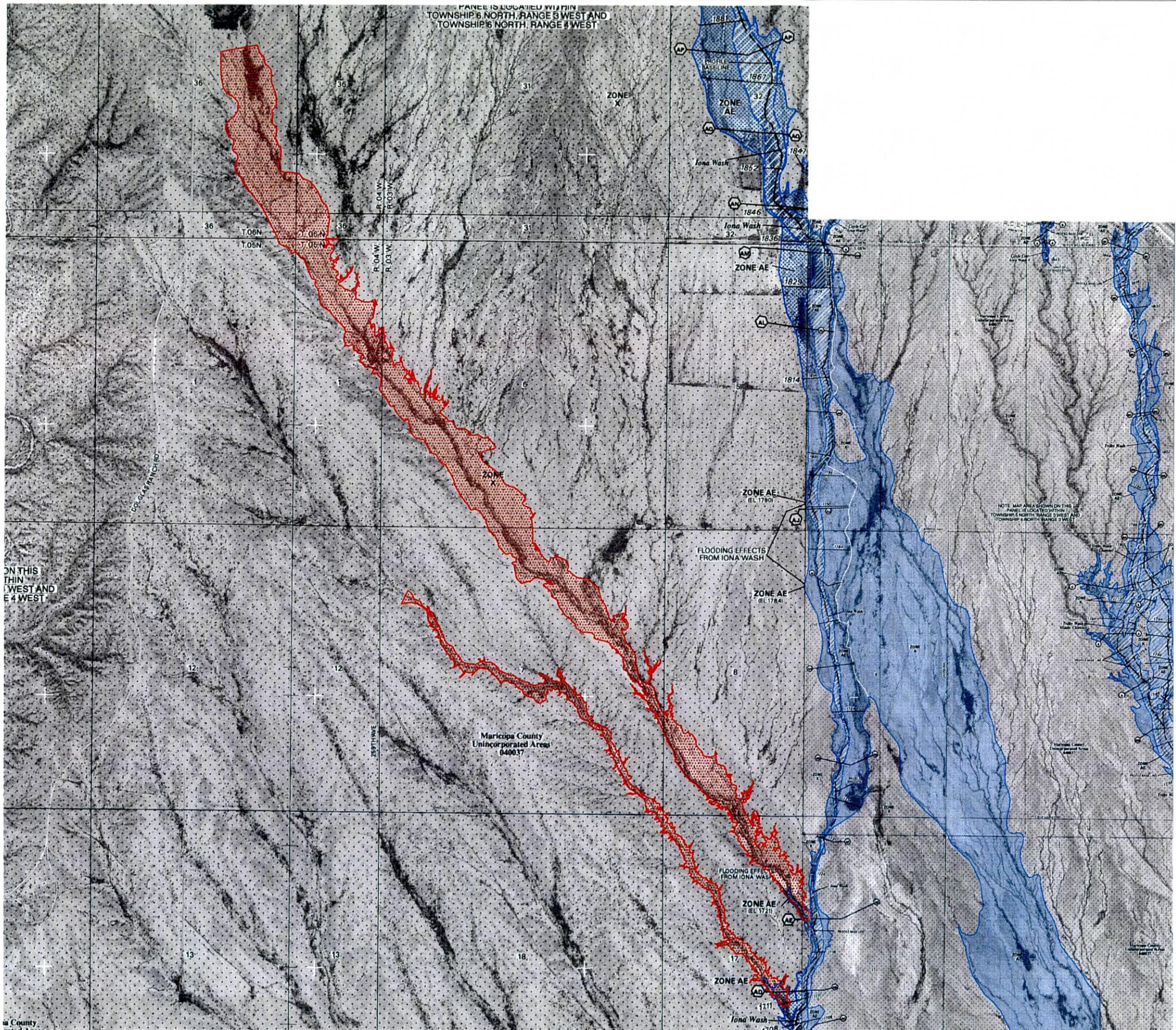
### **7.1. Summary of Discharges**

Refer to Tables 4.6 through 4.8 and the HEC-1 model for the discharges of each subbasin.

### **7.2. Floodway Data and Flood Profiles**

There are no floodway data because this is a limited detail Zone AE Floodplain. Refer to the floodplain profiles located in Appendix E.5.

**7.3. Annotated FIRM**



**LEGEND**

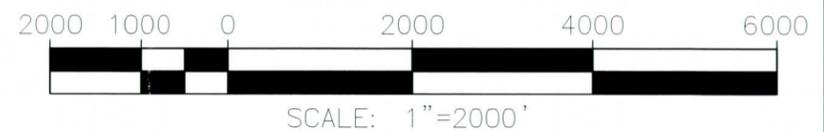
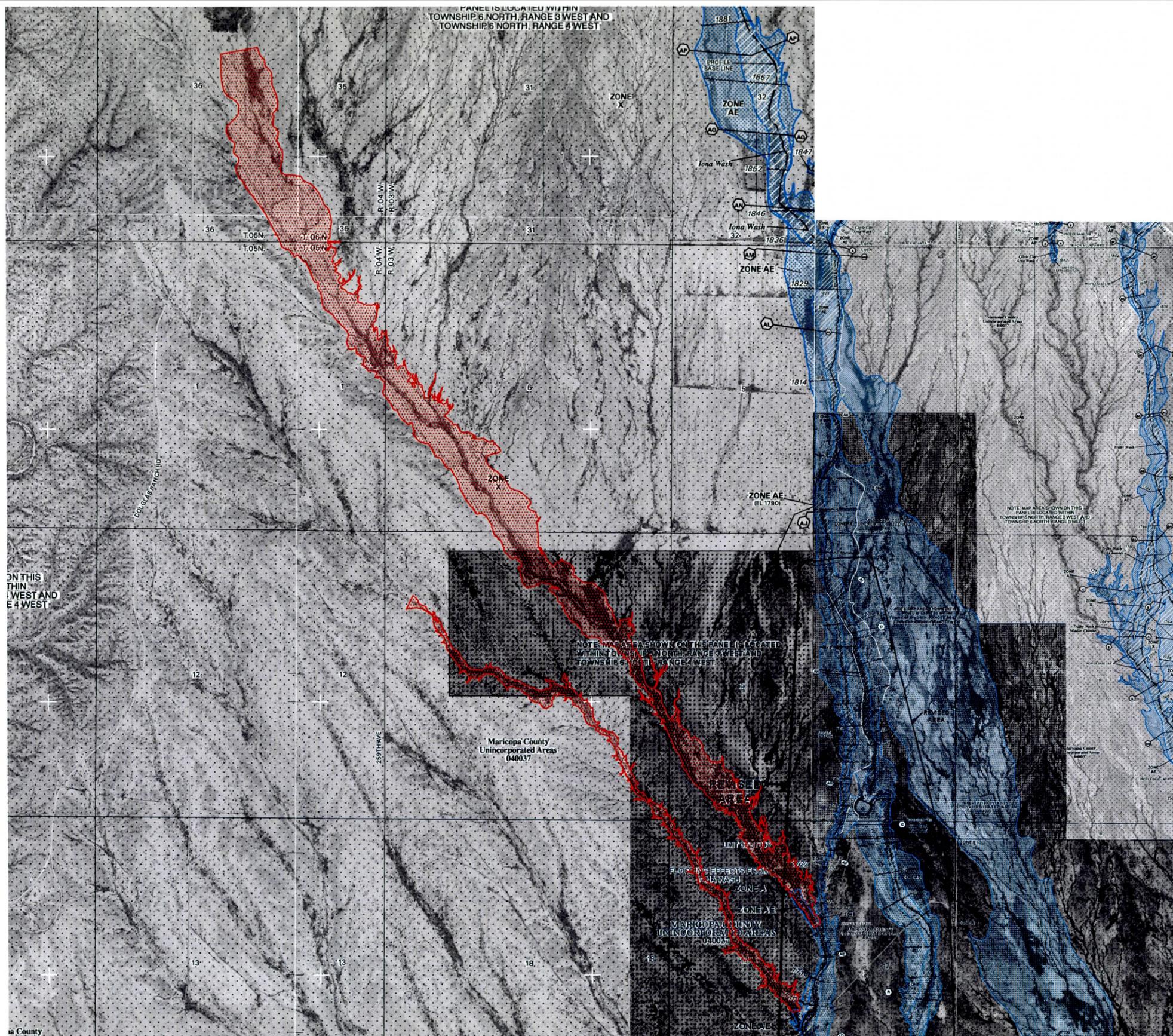
 PROPOSED ZONE AE

**ANNOTATED FIRM**

SOURCE:  
 FIRM PANELS 04013C0660G, 04013C0670G,  
 04013C0680G, 04013C0687H, 04013C0689H,  
 04013C0690H  
 DATE: SEPTEMBER 30, 2005

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### LEGEND

 PROPOSED ZONE AE

## ANNOTATED FIRM W/ LOMR DATED FEB. 4, 2011

SOURCE:  
 FIRM PANELS 04013C0660G, 04013C0670G,  
 04013C0680G, 04013C0687H, 04013C0689H,  
 04013C0690H  
 DATE: SEPTEMBER 30, 2005



**Appendix A    References**

Wittmann Phase IV  
Floodplain Delineation Study

## **A.1 Data Collection Summary**

The following reports and studies were used in the preparation of this study.

Arizona Department of Water Resources (1997). "Instructions for Organizing and Submitting Technical Documentation for Flood Studies," Flood Mitigation Section, Arizona Department of Water Resources, Phoenix, Arizona 85004.

Mason, Robert R. Jr., King, Jeffrey N, and Thomas, Wilbert O. Jr. (1999). "National Flood-Frequency Program – Methods for Estimating Flood Magnitude and Frequency in Rural Areas In Arizona" U.S. Geological Survey Fact Sheet 111-98.

Maricopa Association of Governments (MAG). The digital GIS land use maps for Maricopa County.

Wittmann Area Drainage Master Study Update Hydrology Report Final Addendum (July 2005) prepared by Entellus for the Flood Control District of Maricopa County.

Wittmann Area Drainage Master Study Update Technical Data Notebook (September 2005) prepared by Entellus for the Flood Control District of Maricopa County.

## **A.2 Referenced Documents**

Hydrologic Engineering Center (1991). "HEC-1 Flood Hydrograph Package, User's Manual," US Army Corps of Engineers.

Sabol, G. V., Rumann, J. M., Khalili, D., Waters, S. D., and T. Lehman (January, 1995). "Drainage Design Manual for Maricopa County, Arizona, Volume I, Hydrology," prepared for Flood Control District of Maricopa County.

United States Geological Survey (2006). "Selection of Manning's Roughness Coefficients for Natural and Constructed Vegetated and Non-Vegetated Channels, and Vegetation Maintenance Plan Guidelines for Vegetated Channels in Central Arizona"

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*Wittmann Phase IV Floodplain Delineation Study*  
**DATA COLLECTION REPORT**

Prepared for



Flood Control District of Maricopa County  
2801 West Durango Street  
Phoenix, AZ 85009

Prepared by



RBF Consulting  
16605 North 28th Avenue, Suite 100  
Phoenix, Arizona 85053

August 2011

FCD 2011C003  
RBF 45104608

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## 1. DATA COLLECTION AND FINDINGS

This section is intended to describe the data collection efforts for the Wittmann Phase IV Floodplain Delineation Study of Iona Tributary 1 West (T5NR3WS17-1W), Iona Tributary 1 West Extension (T5NR3WS17-1WE), and Iona Tributary 2 West (T5NR3WS17-2W) and provide a list of the data materials received from agencies and stakeholders. A considerable amount of background information was gathered, including reports and data from the FCDMC relative to the Wittmann ADMSU, MCDOT, and the City of Surprise. This report also outlines observations gathered during the project site visit.

## 2. DATA SOURCES

An extensive amount of information was sought and received from a variety of government agencies and stakeholders. Refer to Table 1 *List of Project Contacts* for those contacted during the data collection portion of the project. Some information was provided first hand from project stakeholders, supplemental information was also derived via email or through websites to complement the primary data sources. These stakeholders include:

- Flood Control District of Maricopa County
- City of Surprise
- FEMA
- Maricopa County Assessor
- Maricopa County Planning & Development
- Maricopa Association of Governments
- Maricopa County Department of Transportation
- State Land Department

## 3. DATA TYPES OBTAINED

The data received was provided in a variety of different formats. Table 2 *List of Collected Reports and Electronic Files* identifies and summarizes the data obtained and data types associated with each report, map, or piece of data received. A few examples of the various data types include:

- GIS Database layers
- Aerial Photography
- Area Topography
- Site Visit Photos
- Transportation Corridor Alignment Studies
- Flood Insurance Rate Maps
- Land Ownership Data
- Hydrology and Hydraulic Models and Reports
- Land Use Plans

## 4. PRELIMINARY FINDINGS OF FACT

### 4.1 *PROPERTY OWNERSHIP*

The Study Area includes mostly Arizona State Land with a few private landowners near the confluence of Iona Wash. Refer to Figure 1 and Table 3 for further illustration. Conversations with the City

of Surprise and Maricopa County Planning and Development indicate that no development plans are currently being processed in the study area.

#### 4.2 EXISTING FLOODPLAIN

The existing FEMA floodplain per the Flood Insurance Rate Map (FIRM) panels 04013C0660G, 04013C0670G, and 04013C0690H, effective September 30, 2005 classifies Iona Tributary 1 West, Iona Tributary 1 West Extension, and Iona Tributary 2 West as Zone X. Zone X is defined as “Areas of the 0.2% annual chance flood; areas of 1% annual chance flood with average depths less than 1 foot or with drainage areas less than 1 square mile; and areas protected by levees from 1% annual chance flood”. These washes are tributary to Iona Wash. Iona Wash is classified as Zone AE floodplain and floodway. Zone AE is defined as “base flood elevation determined”. Refer to Figure 2 for additional information.

The Wittmann Area Drainage Master Study Update (ADMSU) delineated Iona Tributary 1 West (referred to as Iona Tributary in ADMSU) as Zone A floodplain. Iona Tributary 1 West Extension and Iona Tributary 2 West were not delineated as part of the Wittmann ADMSU. The Wittmann ADMSU Floodplain Delineation Study was approved by FEMA as the best available data to use for floodplain management purposes in a letter dated September 26, 2007 FEMA case number 07-09-1634P. These floodplains will be published on the next effective FIRM publication. Refer to Figure 3 for additional information.

#### 4.3 PROPOSED FLOODPLAIN

DEA proposed Zone A floodplain on portions of Iona Tributary 1 West Extension (referred to as Iona Tributary in DEA study) and Iona Tributary 2 West (referred to as T5N-R3W-S17 in DEA Study) as part of the City of Surprise Special Planning Areas 4 and 5 Zone A Floodplain Delineation Study. These floodplains have not been submitted to FEMA. Refer to Figure 4 for the results of this study.

#### 4.4 GIS DATABASE, TOPOGRAPHY, AND AERIAL PHOTOGRAPHY

The FCDMC provided their most recent GIS database, topography, and aerial photography for the Study Area. This information was very useful for a variety of applications relative to aerial surveys, assessing development, and evaluating potential changes to hydrologic modeling.

#### 4.5 REPORTS AND MODELS

RBF received existing hydrologic studies prepared by Entellus for the Wittmann ADMSU in July 2005 and the floodplain delineation report prepared in September 2005. The Wittmann ADMSU shows the Iona Tributary 1 West watershed is 8.78 square miles and is divided into ten subbasins. The Iona Tributary 2 West watershed is 1.66 square miles and is located within one subbasin. Refer to Figure 3 to see a portion of the subbasin delineation and the Zone AE floodplains delineated as part of the Wittmann ADMSU.

DEA prepared a Technical Data Notebook for the City of Surprise Special Planning Areas 4 and 5 Zone A Floodplain Delineation Study in July 2008. DEA delineated intermediate basins and used a unit flowrate method to determine the flows based on the cfs/acre from the Wittmann ADMSU HEC-1 model. DEA used HEC-RAS to model the proposed Zone A floodplains. Refer to Figure 4 to see the proposed Zone A floodplains for Iona Tributary 1 West Extension and Iona Tributary 2 West.

#### 4.6 GAGES

There are not any stream gages located on Iona Tributary 1 West, Iona Tributary 1 West Extension, or Iona Tributary 2 West. In addition, there are not any rain gages within the Iona Tributary 1 West, Iona Tributary 1 West Extension, and Iona Tributary 2 West watershed but there are several rain gages located near the watershed boundaries. Refer to Figure 5 which shows the gage name and location in relationship to the watershed.

#### 4.7 STRUCTURES

Two stock tanks inline with Iona Tributary 1 West Extension were observed on the aerial photographs and on the site visit. No culverts were observed during the site visit. Conversations with the City of Surprise indicated no roadway improvement projects or development plans are currently planned for this area. MCDOT indicates that several corridor feasibility studies have been or will be completed in the near future. Turner Parkway is planned directly west of the proposed floodplain delineation study area limits. Wild Rose Parkway and Dove Valley Parkway are studies that should commence within the next year.

#### 4.8 SITE VISIT

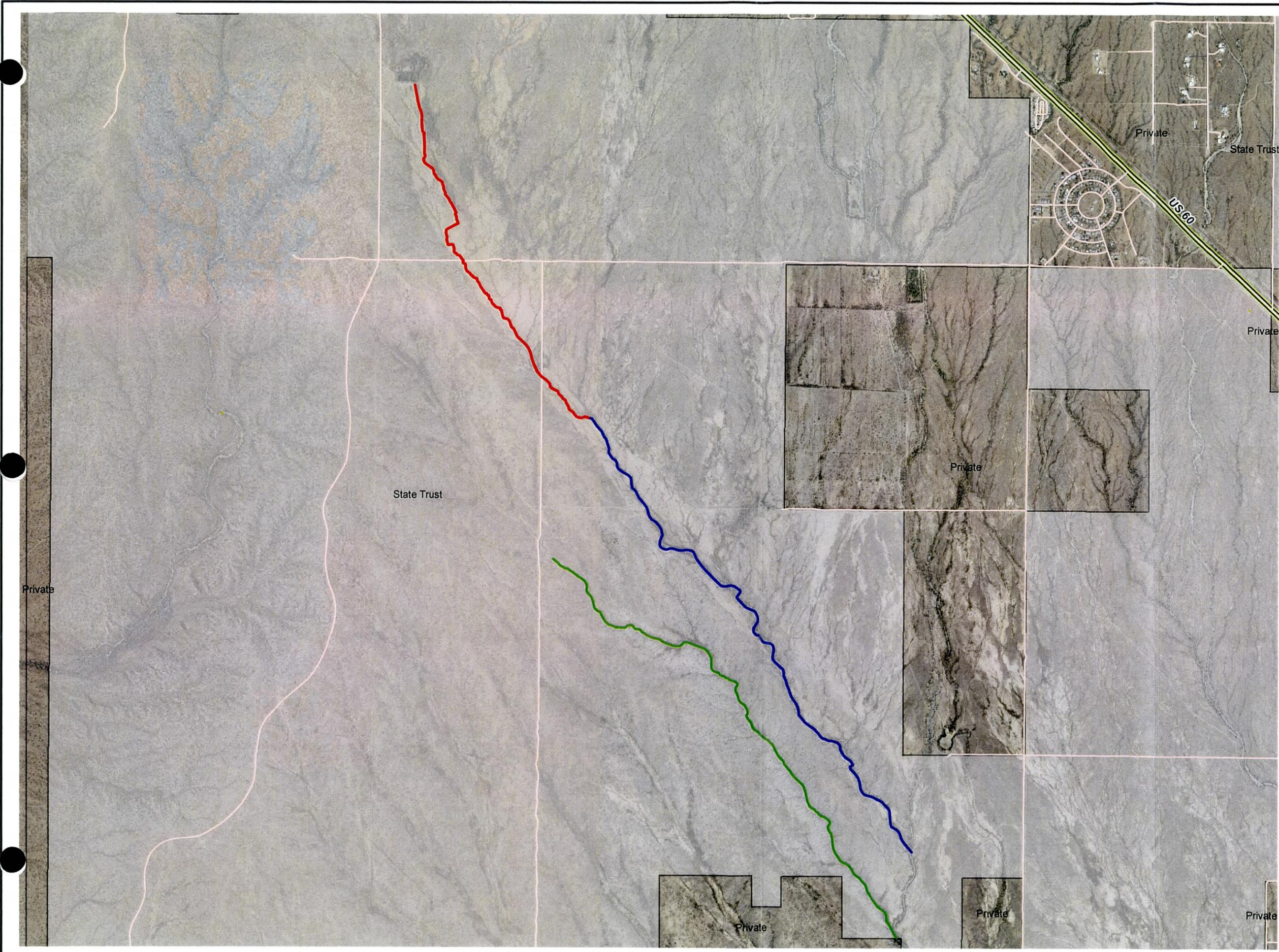
On July 27, 2011, RBF and District Staff conducted an initial site visit of the project study area. The field reconnaissance included a number of photographs and notations to obtain a preliminary understanding of the wash characteristics and other variables influencing the study area. Many areas within the entire study area were observed, many of the notable areas include:

- State Land is fenced and has been used to graze cattle – no development has occurred
- Two stock tanks have been built inline with Iona Tributary 1 West Extension
- Limited access to the study area – few dirt roads
- No culverts were observed during the site visit – washes flow over the dirt roads
- The washes are well defined by incised channels and by the amount of vegetation adjacent to the channels

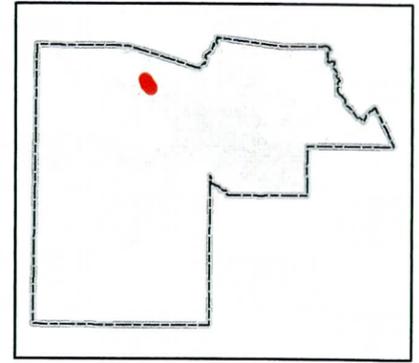
Please refer to the photographs illustrating the channel character and areas of interest.

*Figure 1 - Property Ownership*





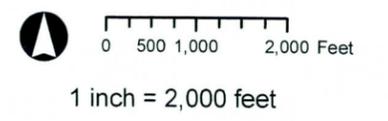
**Flood Control District  
of Maricopa County**



**Wittmann Phase IV  
Floodplain Delineation Study**

**Legend**

- Iona Tributary 1 West
- Iona Tributary 1 West Extension
- Iona Tributary 2 West
- Parcels
- Streets
- Bureau of Land Mgmt.
- Private Land
- State Trust Land



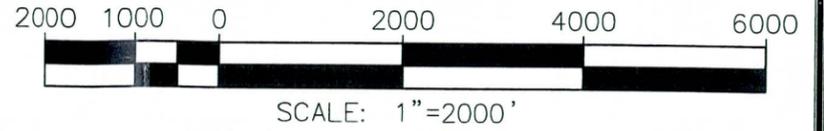
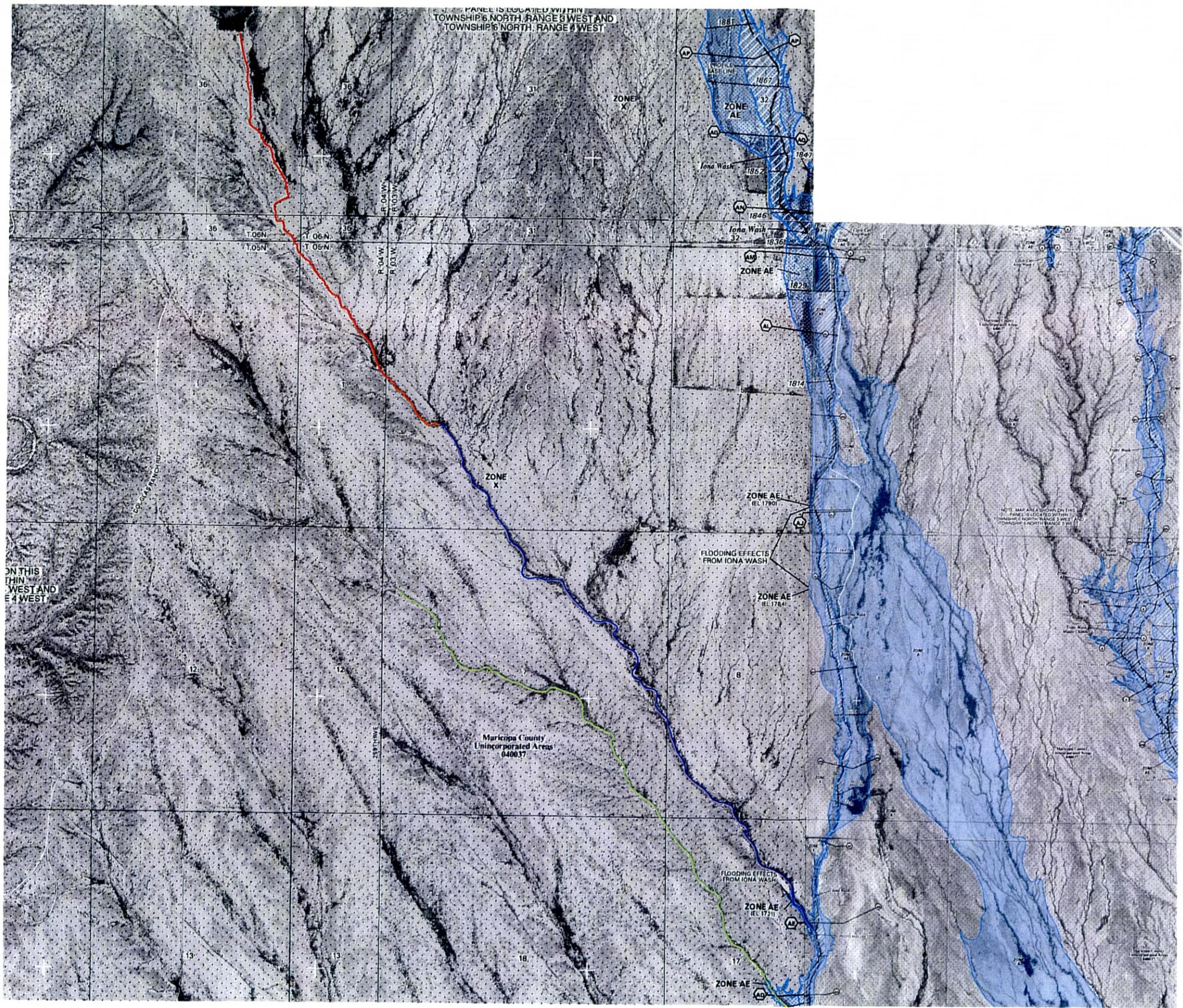
**Property Ownership**

**RBF** CONSULTING  
 PLANNING ■ DESIGN ■ CONSTRUCTION  
 16605 NORTH 28th AVENUE, SUITE 100  
 PHOENIX, ARIZONA 85053-7550  
 602-467-2200 FAX 602-467-2201 www.RBF.com

		DRAFTED	CHECKED
		DLP	NEF
PROJECT NUMBER		45104608	
DATE		8.25.11	
FIGURE		1	

*Figure 2 - Existing FEMA Floodplain*





**LEGEND**

- IONA TRIBUTARY 1 WEST
- IONA TRIBUTARY 1 WEST EXTENSION
- IONA TRIBUTARY 2 WEST

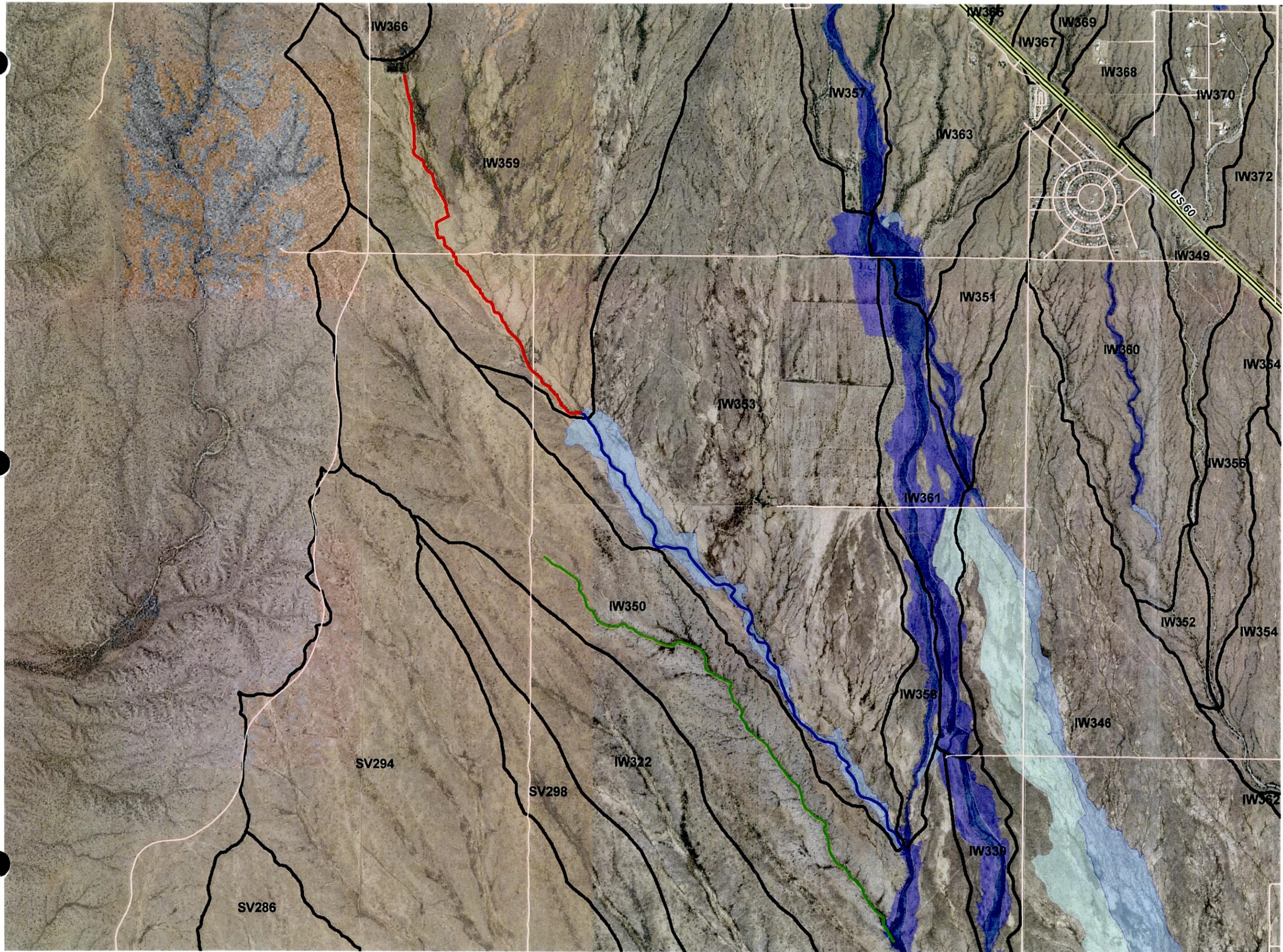
**FIGURE 2  
EXISTING FEMA FLOODPLAIN**

SOURCE:  
 FIRM PANELS 04013C0660G, 04013C0670G,  
 04013C0680G, 04013C0687H, 04013C0689H,  
 04013C0690H  
 DATE: SEPTEMBER 30, 2005

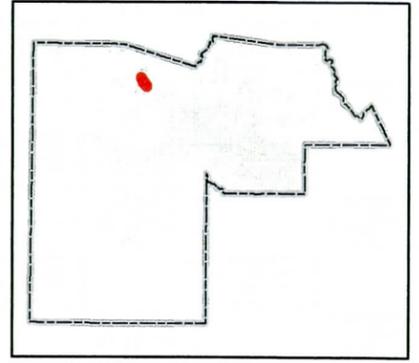
H:\V\DATA\45104608\CADD\STR\WATER\EXHIBITS\WITTMANN\PHASE IV\4608-FIRM.DWG DPEARCE 8/25/11 11:07 am

*Figure 3 - Wittmann Area Drainage Master Study Update*





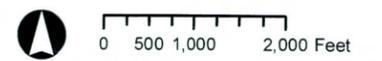
**Flood Control District  
of Maricopa County**



**Wittmann Phase IV  
Floodplain Delineation Study**

**Legend**

- Iona Tributary 1 West
- Iona Tributary 1 West Extension
- Iona Tributary 2 West
- Streets
- Wittmann ADMSU Subbasins
- A
- AE
- AO
- FW



1 inch = 2,000 feet

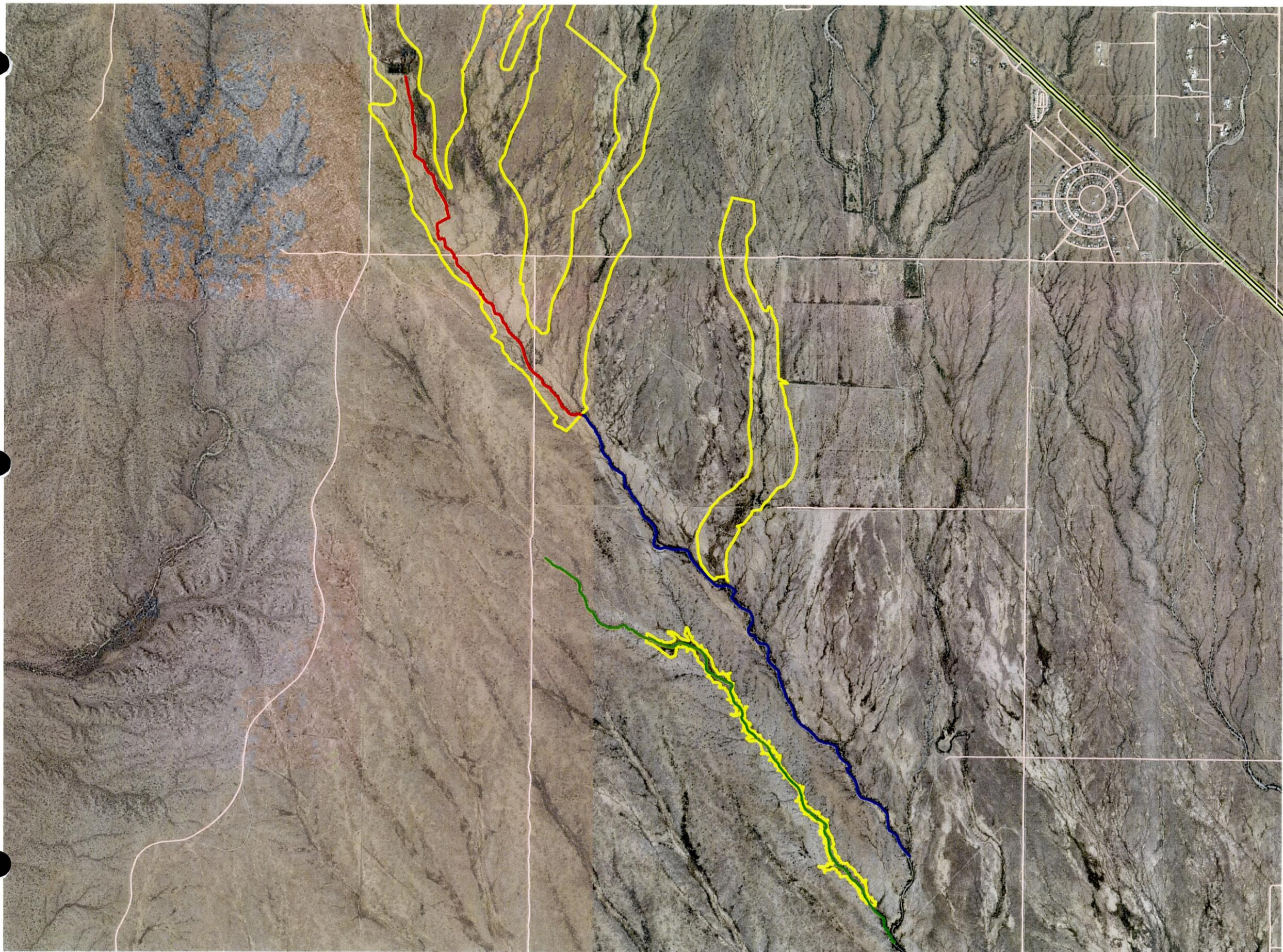
**Wittmann ADMSU**

**RBF** PLANNING ■ DESIGN ■ CONSTRUCTION  
 16605 NORTH 28th AVENUE, SUITE 100  
 PHOENIX, ARIZONA 85053-7550  
 602-467-2200 FAX 602-467-2201 www.RBF.com

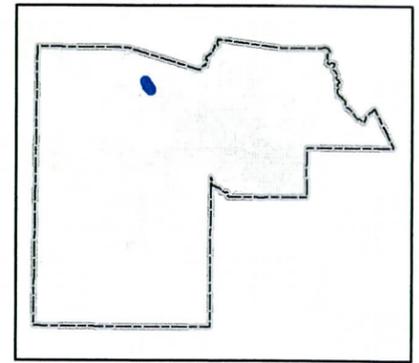
	DRAFTED	CHECKED
	DLP	NEF
PROJECT NUMBER	45104608	
DATE	8.25.11	
	FIGURE	3

*Figure 4 – Special Planning Areas 4 and 5 Zone A FDS*





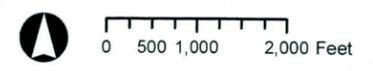
**Flood Control District  
of Maricopa County**



**Wittmann Phase IV  
Floodplain Delineation Study**

**Legend**

-  Iona Tributary 1 West
-  Iona Tributary 1 West Extension
-  Iona Tributary 2 West
-  Zone A Floodplain



1 inch = 2,000 feet

**Surprise Special Planning Areas  
4 & 5 Zone A FDS**

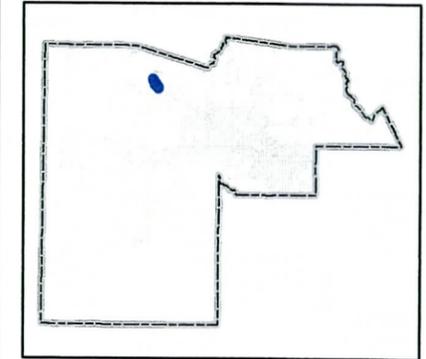
**RBF** CONSULTING PLANNING ■ DESIGN ■ CONSTRUCTION  
16605 NORTH 28th AVENUE, SUITE 100  
PHOENIX, ARIZONA 85053-7550  
602-467-2200 FAX 602-467-2201 www.RBF.com

 	DRAFTED <b>DLP</b>	CHECKED <b>NEF</b>
	PROJECT NUMBER <b>45104608</b>	
	DATE <b>7.20.11</b>	
	FIGURE <b>4</b>	

*Figure 5 – Gage Locations*



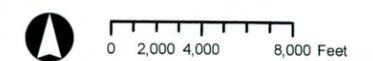
# Flood Control District of Maricopa County



## Wittmann Phase IV Floodplain Delineation Study

### Legend

- Gages
- ~ Iona Tributary 1 West
- ~ Iona Tributary 1 West Extension
- ~ Iona Tributary 2 West
- Streets
- Watershed

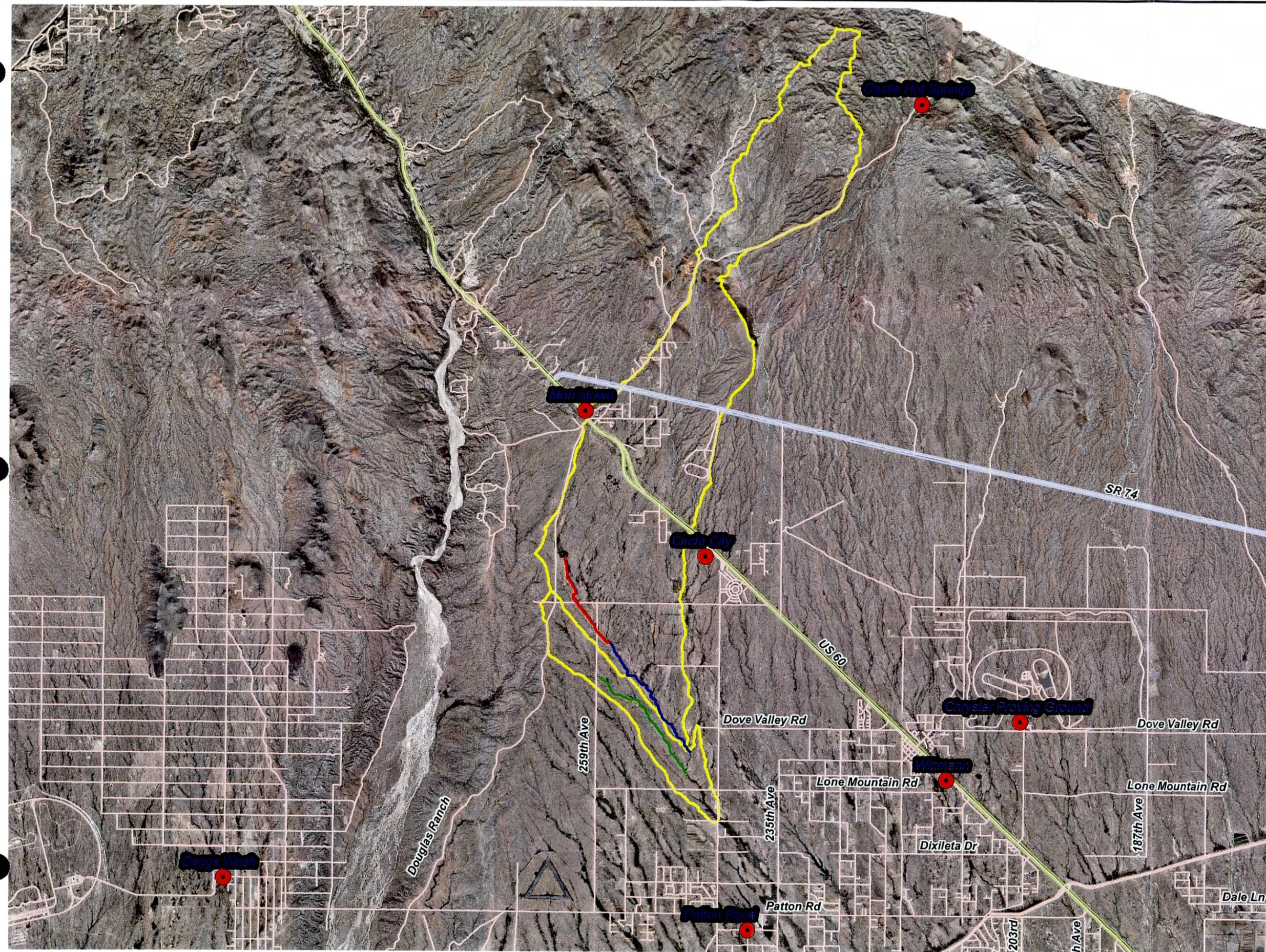


1 inch = 8,000 feet

### Gage Locations

**RBF CONSULTING** PLANNING ■ DESIGN ■ CONSTRUCTION  
 16605 NORTH 28th AVENUE, SUITE 100  
 PHOENIX, ARIZONA 85053-7550  
 602-467-2200 FAX 602-467-2201 www.RBF.com

	DRAFTED	CHECKED
	DLP	NEF
PROJECT NUMBER	45104608	
DATE	8.25.11	
FIGURE	5	



*Site Visit Photos*





**Stock Tank Upstream of Iona Tributary 1 West Extension Limit of Study**



**Stock Tank on Iona Tributary 1 West Extension**



Looking Downstream at Iona Tributary 1 West Extension from Downstream of Stock Tank



Looking Downstream at Iona Tributary 1 West from the Carefree Highway Alignment



**Looking Upstream at Confluence of Iona Tributary 2 West with Iona Wash**



**Looking downstream at Iona Wash from Confluence with Iona Tributary 2 West**

*Table 1 – List of Project Contacts*

**Table 1 Data Collection Contacts**

Name	Organization	email	Phone	Address	City	State	Zip
Jonathan Lesperance	FCDMC	<a href="mailto:jonathanlesperance@mail.maricopa.gov">jonathanlesperance@mail.maricopa.gov</a>	602-506-4699	2801 W Durango St	Phoenix	AZ	85009
Richard Harris	FCDMC	<a href="mailto:rph@mail.maricopa.gov">rph@mail.maricopa.gov</a>	602-506-4528	2801 W Durango St	Phoenix	AZ	85009
Robert Eroh	City of Surprise	<a href="mailto:robert.eroh@surpriseaz.com">robert.eroh@surpriseaz.com</a>	623-222-6000	16000 N Civic Center Plaza	Surprise	AZ	85374
Jason Mahkovtz	City of Surprise	<a href="mailto:jason.mahkovtz@surpriseaz.com">jason.mahkovtz@surpriseaz.com</a>	623-222-6147	16000 N Civic Center Plaza	Surprise	AZ	85374
Denise Lacey	MCDOT	<a href="mailto:deniselacey@mail.maricopa.gov">deniselacey@mail.maricopa.gov</a>	602-506-6172	2901 W Durango St	Phoenix	AZ	85009
Eric Mayer	MCDOT	<a href="mailto:ericmayer@mail.maricopa.gov">ericmayer@mail.maricopa.gov</a>	602-506-8367	2901 W Durango St	Phoenix	AZ	85009
Michael Norris	Maricopa County P&D	<a href="mailto:michaelnorris@mail.maricopa.gov">michaelnorris@mail.maricopa.gov</a>	602-372-4114	501 N 44th Street, Ste 200	Phoenix	AZ	85008
Manny Patel	Arizona State Land Department	<a href="mailto:mpatel@land.az.gov">mpatel@land.az.gov</a>		1616 W Adams St	Phoenix	AZ	85007

*Table 2 – List of Collected Reports and Electronic Files*



**Table 2 List of Collected Reports and Electronic Files**

<b>Document</b>	<b>Data Description</b>	<b>Media Type</b>	<b>Prepared By</b>
Wittmann ADMSU Hydrology Report Final Addendum July 2005	Updated Hydrology for the Wittmann ADMSU	PDF	Entellus
Wittmann ADMSU Technical Data Notebook September 2005	Floodplain Delineation	PDF	Entellus
Surprise Special Planning Areas 4 and 5 Zone A Floodplain Delineation Study Technical Data Notebook Volumes 1-2 July 2006	Zone A Floodplain Delineation	PDF	David Evans and Associates, Inc.
Turner Parkway Corridor Feasibility Study I-10 to State Route 74	Corridor Feasibility Study	PDF	CK Group, Inc.

*Table 3 – Property Ownership List*



Table 3 Property Ownership List

Iona Tributary 1 West, Iona Tributary 1 West Extension, and Iona Tributary 2 West Floodplain Delineation Study, July 2011, FCD 2011C003

APN	Owner Name	Owner Address	City	State	Zip	USE Code	USE Code	Tax Owner	Tax Address	City	State	Zip	LotArea_Ac
50329002D	LUCKY COYOTE L L C	29811 N 255TH DR	WITTMANN	AZ	85361	SINGLE FAMILY RESIDENTIAL	RANCH PROPERTY	LUCKY COYOTE L L C	29811 N 255TH DR	WITTMANN	AZ	853615507	10.029
50329001	M&I MARSHALL & ILSLEY BANK	2525 E CAMELBACK RD 3RD FL	PHOENIX	AZ	85016	VACANT_LAND	RANCH PROPERTY	M&I MARSHALL & ILSLEY BANK	2525 E CAMELBACK RD 3RD FL	PHOENIX	AZ	850169239	39.997
50329002N	JHAA DEVELOPMENTLLC	29426 N 145TH PL	SCOTTSDALE	AZ	85262	VACANT_LAND	VACANT RESIDENTIAL RURAL NON-SUBDIVIDED	JHAA DEVELOPMENTLLC	29426 N 145TH PL	SCOTTSDALE	AZ	852627072	5.009
50329002C	ALOHA REVOCABLE LIVING TRUST	412 E CAREFREE HWY	PHOENIX	AZ	85085	VACANT_LAND	RANCH PROPERTY	ALOHA REVOCABLE LIVING TRUST	412 E CAREFREE HWY	PHOENIX	AZ	850857476	9.998
50329002L	ARRASMITH STEPHEN L/CAROLE MILLER	4903 W BANFF LN	GLENDALE	AZ	85306	VACANT_LAND	VACANT RESIDENTIAL RURAL NON-SUBDIVIDED	ARRASMITH STEPHEN L/CAROLE MILLER	4903 W BANFF	GLENDALE	AZ	853063505	1.005
50329002K	ARRASMITH STEPHEN L/CAROLE MILLER	4903 W BANFF LN	GLENDALE	AZ	85306	STATE OWNERSHIP	VACANT RESIDENTIAL RURAL NON-SUBDIVIDED	ARRASMITH STEPHEN L/CAROLE MILLER	4903 W BANFF	GLENDALE	AZ	853063505	1.019
50329002G	ARRASMITH STEPHEN L/CAROLE MILLER	4903 W BANFF LN	GLENDALE	AZ	85306		VACANT RESIDENTIAL RURAL NON-SUBDIVIDED	ARRASMITH STEPHEN L/CAROLE MILLER	4903 W BANFF	GLENDALE	AZ	853063505	1.002
50329002H	ARRASMITH STEPHEN L/CAROLE MILLER	4903 W BANFF LN	GLENDALE	AZ	85306		VACANT RESIDENTIAL RURAL NON-SUBDIVIDED	ARRASMITH STEPHEN L/CAROLE MILLER	4903 W BANFF LN	GLENDALE	AZ	853063505	1.001
50329002J	ARRASMITH STEPHEN L/CAROLE MILLER	4903 W BANFF LN	GLENDALE	AZ	85306		VACANT RESIDENTIAL RURAL NON-SUBDIVIDED	ARRASMITH STEPHEN L/CAROLE MILLER	4903 W BANFF	GLENDALE	AZ	853063505	1.01

State Land Department

Notes: List generated from Maricopa County Assessor Parcels Information  
 Parcels shown are within 1000 feet of floodplain baseline  
 The majority of the study parcels are State Land Parcels



**Appendix B General Documentation and  
Correspondence**



**B.1 Special Problem Reports**



**B.2 Contact (Telephone) Reports**



**B.3 Meeting Minutes or Reports**

**FCD 2011C003**  
**Wittmann Phase IV Floodplain Delineation Study**  
**Scoping Meeting**

**July 6, 2011**

**Meeting Minutes**

**1. Scope comments/revisions**

The wash names were discussed and how the names should be shown on the contract. The preference is to refer to the washes as Iona Tributary 1 West and 2 West with the Township, Range, and Section in parentheses. Iona Tributary 1 West is delineated as Zone A and Iona Tributary 1 West Extension would have the same name with the exception of the extension. The extension portion is the wash upstream of the previous Zone A floodplain delineation.

RBF is currently updating State Standard 1-97. Scope is to be updated to reference State Standard 1-97 or newer version. If State Standard is accepted before submittal to FEMA, RBF may be able to use updated version.

RBF asked the status of the USGS update of the Regional Regression Equations. RBF directed to plan on using the existing equations for the comparative analysis unless USGS completes project sooner. It is anticipated to be a year or more before completion.

RBF to update flow lines in locations that do not follow the wash or contours well.

New Public Information Officer at the FCDMC is Gant Wegner.

Final Deliverables may be sealed PDFs instead of sealed mylars.

Nathan to provide Jonathan with the track changes copy of the scope later today.

**2. Project schedule**

Minor updates were discussed. Goal is to submit to FEMA in November.

**3. Special considerations**

Terminate the floodplain delineation at the stock tank along Iona Tributary 1 West Extension.

Discuss the US60, railroad, and storage capacity in the Data Collection Report.

Evaluate the impact of the non-certified embankment of the railroad and US60 and the potential for with and without levee flows. Further information is needed. After the meeting Jonathan looked at the storage upstream of the railroad and does not believe the storage is sufficient to warrant both analyses.

Richard requested the GIS submittal not wait until FEMA approval so there is a pending layer is the FCDMC database.

RBF to complete information release form for the GIS data request after the meeting with the receptionist. RBF to provide shapefile of the area requested for the GIS data. Jonathan to submit GIS request to Eric Feldman after receiving the shapefile information from RBF.

**4. Fee estimate**

A draft fee estimate was discussed. RBF to provide a draft copy to Jonathan incorporating the requested modifications later today. After Jonathan's review a signed copy will be provided.

FCD 2011C003  
Wittmann Phase IV Floodplain Delineation Study  
Scoping Meeting

July 6, 2011

Sign In Sheet

	<b>Name</b>	<b>Organization</b>	<b>Phone Number</b>	<b>E-Mail</b>
1.	Nathan Ford	RBF Consulting	602-467-2200	nford@rbf.com
2.	Scott Larson	RBF Consulting	602-467-2200	slarson@rbf.com
3.	Jonathan Lesperance	FCDMC	602-506-4699	jonathanlesperance@mail.maricopa.gov
4.	Richard Harris	FCDMC	602-506-4582	rph@mail.maricopa.gov

FCD 2011C003  
Wittmann Phase IV Floodplain Delineation Study  
Progress Meeting

August 25, 2011

Agenda and Minutes

1. Data Collection Report Comments

RBF provided a revised Data Collection Report that addressed the provided comments. Some of Richard's comments will be added to the TDN at a later date such as the mailing list and site photos.

2. Hydrology Section 4 Comments

RBF provided a revised Section 4 that addressed the comments. Discussed why proration was not used in IW353 and IW359. The HEC-1 model was adjusted to account for the routing of multiple drainage paths. Proration was used for IW350 since there is only one basin.

3. Cross Section Submittal

RBF provided cross section submittal for review. Jonathan requested the cross sections be provided in shapefile format. Richard requested that the key map on each sheet have the sheet labeled and the sections not as dark.

4. Upcoming Submittals

Discussed the upcoming submittal of the n-value report to be submitted by September 7.

5. Invoice

RBF provided invoice and earned value analysis graph. District requested a summary of the work completed for the July invoice.

FCD 2011C003  
Wittmann Phase IV Floodplain Delineation Study  
Progress Meeting

August 25, 2011

Sign In Sheet

	<b>Name</b>	<b>Organization</b>	<b>Phone Number</b>	<b>E-Mail</b>
1.	Nathan Ford	RBF Consulting	602-467-2200	nford@rbf.com
2.	Jonathan Lesperance	FCDMC	602-506-501	jonathanlesperance@mail.maricopa.gov
3.	Richard Harris	FCDMC	602-506-4528	rph@mail.maricopa.gov
4.				

**FCD 2011C003  
Wittmann Phase IV Floodplain Delineation Study  
Progress Meeting**

**October 27, 2011**

**Agenda and Meeting Minutes**

**1. Review Comments of Floodplain Work Maps**

**RBF provided response to review comments.**

**2. Survey of Bend**

**RBF to include ground elevation exhibit in Appendix C. Add limit of study to the floodplain work maps at this location.**

**3. Section 5 Submittal**

**Jonathan to provide review comments tomorrow for Section 5.**

**4. Upcoming Submittals**

**RBF to submit the draft TDN and GIS shapefiles on November 14.**

**5. Invoice**

**RBF submitted the monthly invoice.**

FCD 2011C003  
Wittmann Phase IV Floodplain Delineation Study  
Progress Meeting

October 27, 2011

Sign In Sheet

	<b>Name</b>	<b>Organization</b>	<b>Phone Number</b>	<b>E-Mail</b>
1.	Nathan Ford	RBF Consulting	602-467-2200	nford@rbf.com
2.	Jonathan Lesperance	FCDMC	602-506-501	jonathanlesperance@mail.maricopa.gov
3.				
4.				



**B.4 General Correspondence**



**B.5 Contract Documents**

EXHIBIT A



**SCOPE OF WORK**

CONTRACT FCD 2011C003  
Work Assignment No. 1

Floodplain Delineation Study of Washes Iona Tributary 1 West  
(T5NR3WS17-1W), Iona Tributary 1 West Extension  
(T5NR3WS17-1WE), & Iona Tributary 2 West (T5NR3WS17-2W)

## EXHIBIT A

### SCOPE OF WORK

#### CONTRACT FCD 2011C003

#### Work Assignment No. 1

### Floodplain Delineation Study of Washes Iona Tributary 1 West (T5NR3WS17-1W), Iona Tributary 1 West Extension (T5NR3WS17-1WE), & Iona Tributary 2 West (T5NR3WS17-2W)

#### GENERAL

This study will re-delineate limited detail (without floodway) Zone AE 100-year floodplains for Iona Tributary 1 West (T5NR3WS17-1W) and delineate limited detail Zone AE 100-year floodplains for Iona Tributary 1 West Extension (T5NR3WS17-1WE), and Iona Tributary 2 West (T5NR3WS17-2W) Refer to Exhibit 1) using existing 4-foot contour interval mapping provided by the Flood Control District of Maricopa County. Approximately six (6) linear miles of limited detail 100-year floodplains will be delineated.

The CONSULTANT will use existing hydrology both directly and to develop intermediate discharges, and will develop new HEC-RAS modeling to determine the revised floodplain boundaries for Washes Iona Tributary 1 West (T5NR3WS17-1W), Iona Tributary 1 West Extension (T5NR3WS17-1WE), and Iona Tributary 2 West (T5NR3WS17-2W).

All work must meet the requirements of the DISTRICT's Consultant Guidelines, Third Edition – December 1, 2003 - Revision 1. All work must also meet Arizona Department of Water Resources (ADWR) and Federal Emergency Management Agency (FEMA) requirements for limited detail Zone AE floodplain delineation studies. Prior to the finalization of this contract, the DISTRICT must review and accept the results of this study, and all items called for in this Scope of Work must be delivered to the DISTRICT.

All work must be completed within six hundred sixty five (665) days from the Notice to Proceed (NTP) which includes at least eighty four (84) days for DISTRICT reviews and the FEMA review period. All project activities including study data submittals and Milestones shall occur as defined in the attached Work Schedule dated 7/6/11. The Work Schedule may be updated during the course of the project given prior approval by the DISTRICT'S Project Manager.

#### TASK 1 - COORDINATION

- 1.1 Within seven (7) days of the NTP the CONSULTANT will submit a project schedule to the DISTRICT's Project Manager showing coordination meetings and completion dates for each task identified in the Scope of Work (SOW). The schedule shall also show product submittal dates and DISTRICT product review periods. The CONSULTANT will update the project schedule as necessary and when otherwise directed by the DISTRICT.
- 1.2 The CONSULTANT will participate in regular coordination meetings (at least on a monthly basis) with the DISTRICT's Project Manager and in milestone coordination meetings in the development of the hydraulic analyses. The CONSULTANT is responsible for the minutes of any meetings.

Draft meeting minutes must be delivered to the DISTRICT within seven (7) working days of any monthly meeting. Coordination and milestone meetings should be combined whenever possible.

1.3 The CONSULTANT will submit an estimate of the total monthly billing projections within seven (7) days of the NTP. Thereafter, this estimate will be updated and submitted to the DISTRICT's Project Manager at least ten (10) days before the end of each quarter.

1.4 The CONSULTANT will submit monthly progress reports at submittal of monthly invoices. The report shall be brief and should be no longer than two (2) typed pages. At a minimum, the monthly report shall contain the following:

- a. A description of the work accomplished by task during the reporting month.
- b. Percent (%) completed for the month and percent (%) cumulative completed for each task.
- c. A brief description of the work to be accomplished in the following month.
- d. A description of any problems encountered.
- e. An Earned Value Table and Graph to illustrate the actual work status compared to initial approved projection.

1.5 The DISTRICT will be responsible for placing the legal advertising at the beginning of the study. The advertisement will run once in the Arizona Business Gazette. After the newspaper runs the ad, the DISTRICT will supply the CONSULTANT with the original affidavit of publication from the newspaper for each day that the ad ran.

1.6 The DISTRICT has prepared a separate set of guidelines for the CONSULTANT on conducting public involvement and public information activities for the DISTRICT. A copy of these guidelines will be provided to the CONSULTANT by the Public Information Office and shall be used by the CONSULTANT if or when preparing public information related materials.

1.7 The DISTRICT will provide any public notice beyond that described in task 1.5

1.8 (Optional) The CONSULTANT will participate in a public information Open-house meeting prior to sending the study Technical Data Notebook (TDN) to FEMA. The CONSULTANT will prepare any displays for the meeting following DISTRICT recommendations. The DISTRICT will reserve the meeting venue and secure a certificate of insurance. The CONSULTANT will provide refreshments for the open house.

1.9 CONSULTANT/DISTRICT Performance Evaluations will be performed. A formal evaluation will be performed at the completion of the project upon receipt of all deliverables.

## **TASK 2 - DATA COLLECTION**

2.1 The CONSULTANT will collect and review pertinent data from the DISTRICT and other outside sources. Data to be collected will include previous flood hazard reports and hydrology for the study area; existing readily available topographic mapping; proposed development plans, historical flooding information; as-built plans for existing structures; FEMA Flood Hazard Boundary Maps and any Letters of Map Amendment and/or Revisions, and other pertinent information.

2.2 The CONSULTANT will consult with officials from the DISTRICT and the City of Surprise and the County and State transportation departments, in order to identify local flooding problems and to

obtain information on current and planned public works projects, channel modifications, storm-drainage systems, development, and corporate limits.

- 2.3 A written summary of the data collected will be included as a section in the Technical Data Notebook (TDN). A preliminary draft of this section is due within twenty-one (21) days of Notice to Proceed. The section will be updated to include information developed through continued research that may be necessary as the project proceeds.

### **TASK 3 - TOPOGRAPHIC MAPPING**

- 3.1 Topographic mapping with 4-foot contour intervals (including digital terrain model data) will be provided for the study area by the DISTRICT. This topographic mapping will be referenced to the Arizona Central State Plane Coordinate System, which is tied to the North American Datum 1983 High-Accuracy Reference Network (NAD 83 HARN), horizontally; and the North American Vertical Datum 1988 (NAVD 88), vertically. The DISTRICT will provide the CONSULTANT with aerial photography imagery that is recent enough to be appropriate to the level of study and contiguous within the study area.

### **TASK 4 - FIELD SURVEY**

- 4.1 As needed, field surveys and measurements of bridges, culverts, and hydraulic structures are to be obtained by either the CONSULTANT or the DISTRICT when as-built plans are not available, or when conditions have changed that may impact the delineation. Geodetic Densification and Cadastral Survey (GDACS) control will be the basis of field survey. This information should be reduced and compiled into an 11" x 17" (maximum size) drawing format approved by the DISTRICT, for inclusion in the TDN. The information presented in the drawing should be in a format appropriate for use in future HEC-RAS models. It may be necessary to field survey some structures since the as-built plans may not be on the same vertical datum as the study.
- 4.2 Copies of the survey field books and office calculations must be supplied by the CONSULTANT to the DISTRICT for inclusion in the TDN. An Arizona Registered Land Surveyor (RLS) must seal and sign the survey notes. This information can be submitted separately if approval is obtained from the DISTRICT's Project Manager.

### **TASK 5 - HYDROLOGY**

- 5.1 Hydrologic Modeling

The hydrology from the previous study for this area (Wittmann Area Drainage Master Study Update (ADMSU) contract FCD 2002-29) will be used for this study in conjunction with more recent study results that may also be found to be appropriate. Therefore, the CONSULTANT will only need to complete minimal additional hydrologic calculations. A Hydrologic summary will be prepared by the CONSULTANT as a section in the TDN and it will be submitted for review and approval by the DISTRICT prior to commencing with floodplain modeling. The report will be prepared as a draft of Section 4 of the Technical Data Notebook and will be prepared in accordance with ADWR State Standards Attachment 1-97 (SS1-97) or most recent version. The report will be organized as specified by the DISTRICT, following SS 1-97 format or most recent version. Specific deviations from this scope shall not be undertaken without the specific written authorization from the DISTRICT'S Project Manager. The report will include backup documentation from the existing hydrology as well as comprehensive drainage displays to show all sub-basins and concentration

points with discharge tables. A comparative analysis against regional regression equations will be included in the report.

## 5.2 Meeting and Field Visits

One (1) meeting associated with the development of the hydrologic data shall be held with DISTRICT staff at the following milestone:

- A. Meeting number 1: This meeting will occur after the preliminary peak discharges have been estimated. At this time, the CONSULTANT will have an opportunity to discuss any preliminary comments by the DISTRICT.

## 5.3 Review and Approval

The CONSULTANT shall obtain approval from the DISTRICT at each of the following steps:

- a. Preliminary discharge results (Meeting 1)
- b. Cross section locations with respect to changes in discharge

## 5.4 The Hydrologic Report

5.4.1 The findings of the hydrologic study will be presented in Section 4 of the Technical Data Notebook and will be prepared in accordance with ADWR State Standards Attachment 1-97 (SSA 1-97) or most recent version. The report will be organized as specified by the DISTRICT, following SS 1-97 format. The report will include backup documentation from the existing hydrology as well as comprehensive drainage displays to show all sub-basins and concentration points with discharge tables. A comparative analysis against regression equations will be included in the report. Specific deviations from this hydrologic scope shall not be undertaken without the specific written authorization from the DISTRICT's Project Manager.

## 5.5 Tables and Figures

5.5.1 Drainage Maps(s) showing the sub-basins, major man-made structures such as highways, levees, railroads, or culverts, and references (i.e., street names, Township, Range, Section, etc.) at a scale to be agreed upon with DISTRICT Staff shall be included in the TDN as support for the related hydraulic analysis. A table will be prepared to show the results of the study (i.e. 6-hour and 24-hour peak flows, etc.) at major concentration points. The level of detail of this map is to be developed during the study period.

## 5.6 Digital Hydrologic Data Deliverables

Digital data in a GIS shape file format for any newly developed hydrologic information will be prepared in conformance with the DISTRICT's Hydrologic Information System Data Delivery Specifications, Revision 3.1. The following themes are the ones generally used for Hydrologic data. However, for this study there may not be data for every theme identified, or the CONSULTANT might develop data for themes not listed. Therefore, only those themes, for which there are data, need to be completed. If the CONSULTANT has data that doesn't fit one of the themes listed here, the DISTRICT's Project Manager shall be contacted to determine the appropriate theme for that data.

- a. drnbsn (Drainage Basins)

- b. drmpthln (Drainage Flowpaths and routes)
- c. drnpthpt (Concentration Points)

## TASK 6 - FLOODPLAIN DELINEATION

- 6.1 Limited detail Zone AE Floodplain delineations will be performed for approximately six (6) linear miles of washes Iona Tributary 1 West (T5NR3WS17-1W), Iona Tributary 1 West Extension (T5NR3WS17-1WE) and Iona Tributary 2 West (T5NR3WS17-2W). Hydraulic information upon which the re-delineations will be based will be obtained by modeling using the U.S. Army Corps of Engineers latest release of HEC-RAS (version 4.1.0 or later) and methodology acceptable to FEMA as defined in their Guidelines and Specification for Flood Hazard Mapping Partners, April 2003 and FIA Document 12, Appeals, Revisions, and Amendments to Flood Insurance Maps, December 1993. The completed delineation work will meet the requirements for floodplain delineations as prescribed by FEMA and the ADWR, including the Arizona State Standard for Hydraulic Modeling (SS9-02). The modeling and attendant re-delineations will tie-in vertically and horizontally at the downstream pending floodplain confluence of the Iona Wash floodplain boundary that was generated through the Wittmann ADMSU contract FCD 2002C029.
- 6.2 The floodplain delineation maps shall have a scale of 1 inch = 200 feet. The hydraulic modeling and delineation work maps shall be referenced to the North American Vertical Datum of 1988 (NAVD 88). This work shall include inserting the cross section and BFE boundary line work, river mile ticks and water surface elevation contour line work. If ponding-area floodplains occur in the study area the CONSULTANT shall provide the DISTRICT with the water surface elevations for these floodplain Zone AH ponding areas. If appropriate, the CONSULTANT will delineate conveyance areas, as floodplain Zone AE, between ponding areas.

The CONSULTANT shall provide annotated FIRM panels to illustrate the proposed floodplain boundaries in comparison with the effective information. The annotated FIRM panels will be part of a draft addition to the FIS, which shall also include water surface profiles data tables. These materials are normally included in Section Number 7 of the TDN. Excerpts from the effective FIS, as they relate to the study, shall be included within the TDN appendices.

The CONSULTANT will obtain DISTRICT approval at each of the following steps:

- a. Estimation of Manning's "n" values.
  - b. Cross-section location and orientation (with respect to channel bed slope changes and mass flow paths)
  - c. Hydraulic model feature names in context with CAD/GIS standards
  - d. Hydraulic model documentation
  - e. Floodplain (natural) delineation.
  - f. Draft Hydraulics TDN section documentation (Section 5 of the TDN).
- 6.3 Field Reconnaissance
- 6.3.1 The CONSULTANT will conduct a field reconnaissance of the study area. This will include observation of channel and floodplain conditions for estimating Manning's "n" values; photographic documentation of floodplain characteristics; determination of channel bank characteristics; determination of channel bed particle size; observation of possible overflow area; observation and classification of levees or other flood control structures and nominal

measurement of bridge and culvert dimensions not included in the survey task. Such measurement will be made to classify smaller cross-drainage structures as non-effective.

6.3.2 Manning's "n" values are to be determined using the methodology in the USGS report, *Selection of Manning's Roughness Coefficient for Natural and Constructed Vegetated and Non-Vegetated Channel, and Vegetation Maintenance Plan Guidelines for Vegetated Channels in Central Arizona, 2006*. If approved by the DISTRICT, another report entitled, *Estimated Manning's Roughness Coefficients for Stream Channels and Floodplains in Maricopa County, 1991*, may be substituted. Copies of these reports are available through the DISTRICT. Manning's "n" values will be presented for typical reach types observed in the project area, rather than specific reaches of specifically named watercourses. It is anticipated that several different reach types will be identified during the field reconnaissance. Supplemental field photos will be used to substantiate slight roughness value variations from otherwise typical reach types, for locations where slight roughness value variations can be applied without providing complete calculations. These variations will be fully documented in Section 5 of the TDN and in the hydraulic model's general Description.

6.3.3 Representative Manning's "n" values will be selected and documented in a reconnaissance, "n value" report. The report will present the calculated determination of channel and overbank "n" values using captioned color photographs for each type in the project area, and the extents of each type will be displayed on an exhibit. The "n value" report will also discuss floodplain conditions affecting the delineation, describe all structures and obstructions, and provide color photos or photocopies of major hydraulic structures. Photo location for channels, structures, and "n" value determinations will then be displayed on reduced scale mapping and included in the draft TDN Hydraulic documentation appendix and in all subsequent TDN versions.

#### 6.4 Cross Sections

6.4.1 The location and alignment of cross sections and channel centerlines will be submitted for the DISTRICT's review and approval. Cross sections will be spaced approximately every five hundred feet, unless geographic or structural constraints dictate otherwise, and will extend the full width of the area inundated by 100-year floodwaters. Cross section placement locations shall be selected with respect to grade-breaks in the channel profile and existing structures such as roads, railways, drainage features and confluences, as well as at future roadway alignments and cadastral section lines. Cross section spacing should be done in such a way as to minimize computational head loss and where natural channel bed grade breaks evidently occur in thalweg profile plots. Cross section stationing will be from left to right looking downstream with the hydraulic baseline at station 10,000. Identification of cross section will be in river miles, increasing upstream. The station will tie into the specified river mile of any existing FEMA studies. The cross sections may need to be reoriented or altered after running the HEC-RAS model to ensure that they are perpendicular to the center of mass of flow per FEMA criteria. Cross sections developed by the HEC-RAS interpolation feature are not to be used. The CONSULTANT must coordinate and then document the methodology for generating the cross section geometric data. Acceptable methods might include collecting the data directly off paper copies of the DISTRICT's 4-foot contour interval mapping, use of a computer program to develop the data from digital information, or from field surveys.

- 6.4.2 The HEC-RAS output in the TDN, in addition to summary tables and a full output report, shall include fully labeled profile and cross section plots.
- 6.4.3 Check-RAS reports and RAS-PLOT profiles of the 100-year event will be created for inclusion into the TDN. Digital export file (dxf) format of the profiles will also be provided.
- 6.5 Bridges and culverts must be modeled according to HEC-RAS modeling requirements for the selected routine. Where multiple bridges occur, each bridge will be modeled separately. The HEC-RAS modeling results for bridges, culverts, and other hydraulic structures must be checked by using independent methods approved by the DISTRICT to analyze these structures. The hydraulic effects of bridges and culverts shall be incorporated into assessing the floodplain around such structures, especially in areas where ponding will occur. Minor conveyance structures such as small culverts (i.e. less than 30" in diameter), or, structures considered likely to become clogged during the 100-year peak discharge, shall not be included in the hydraulic analyses.
- 6.6 It is possible that during the course of the hydraulic modeling, it will be discovered that the 100-year event flows break out of one or more of the reaches being studied. If the modeling suggests that any flow breakouts occur, the CONSULTANT will promptly contact the DISTRICT to coordinate an appropriate course of action, which may include modification of the SOW and fee for the project.
- 6.7 Flood Zones will be determined according to FEMA criteria and will be clearly labeled on the final drawings. The drawing cover sheets shall show the study title and District contract number, and should show both location and vicinity maps, and a sheet index in a cadastral section background. A list of survey control monuments (or ERM's) with elevations and coordinates shall be shown along with topographic mapping contour interval, vertical and horizontal datums, coordinates, and the conversion factor between NAVD88 and NGVD29 vertical datums. A field survey certification note (if necessary) will be provided and will include the signature and seal of the RLS responsible for all ground survey. The dates of both the topographic flights and the aerial photography flights shall be listed.
- 6.8 The main project Description box of the HEC-RAS models should include the following:
- a. Project Name, FCD Contract Number, work assignment number, and project control number
  - b. Consultant Name, phone number, address, website address, and company Job Number
  - c. Study Purpose
  - d. File Name and latest run date/final date if completed
  - e. Vertical Datum of the model, base map date, and base map contractor information
  - f. Any notable features that are considered unique or unusual to the hydraulic modeling
  - g. HEC-RAS program version
  - h. Source of Hydrology and discharges used
  - i. Wash names including River and Reach Names used in the HEC-RAS modeling
  - j. Subsequent update information, if any

In addition, minor descriptions should be added to the model for hydraulic cross sections located above or below drainage structures, at section lines, at highways and railway crossings, at canals,

and at confluences. Model descriptions should be added to culverts and lateral structures, and at any other features considered pertinent to the modeling.

6.9 The CONSULTANT will provide work maps using the DISTRICT's most recent contour mapping. The work map drawings will be 24" X 36" in size. The work map scale will be 1" = 200'. The cover sheet of the work map drawings shall be prepared in a manner to be approved by the DISTRICT. Each study sheet work map drawings shall include the project title, contract number, source of topographic mapping, and a location map showing the geographic range covered by each specific mapping sheet. Each study sheet will include watercourse names and existing floodplain boundaries, proposed floodplain boundaries, a north arrow, map scale section lines and section corners, current streets and highway names, subdivision boundary names, Horizontal and Vertical Datum references (State Plane Coordinate System, NAD 83, and NAVD 88) any of MCDOT's GDACS monuments and NGVD monuments located within individual sheet boundaries, major drainage features, corporate boundaries, hydraulic cross section lines, sheet index map, peak discharges, and relative Township and Range labels. A conversion factor from NAVD 88 to NGVD 29 should also be shown.

6.10 Technical Data Notebook (TDN)

6.10.1 The findings of the floodplain delineation study will be presented in Section 5 of the TDN and will be prepared in accordance with ADWR State Standards Attachment 1-97 (SS1-97) or most recent version. The report will be organized as specified by the DISTRICT standards, following SS1-97 format.

6.10.2 The hydraulic analysis/HEC-RAS output in the TDN, in addition to summary tables and full output report, should include fully labeled cross section plots. Section plots should be grouped per study reach, and reach groups should be separated and then indexed within the main report to expedite data recovery.

6.11 The CONSULTANT shall review the output of the HEC-RAS computer model to ensure the flow depths and velocities are reasonable and realistic with respect to those expected in this watershed. Adjustments to the model input should be completed as necessary to obtain the most realistic results. The CONSULTANT shall provide proper QA/QC of the models and TDN before delivering to the DISTRICT.

6.12 The CONSULTANT shall fill out all the forms required by FEMA (MT-2 forms) for the submittal of a Floodplain Delineation Study as a Letter of Map Revision (LOMR) request.

**TASK 7 – DIGITAL DATA**

Digital data shall be delivered in a modified shape file format per Shape File Data Delivery Specifications V1.2, date May 1<sup>st</sup>, 2007 and will be prepared in conformance with the above standards. The following themes are generally used for the data developed for Floodplain Delineation Studies. However, for this study there may not be data for every theme identified here, or the CONSULTANT might develop data for themes not listed here. Therefore, only those themes for which there are data need to be completed. If the CONSULTANT has data that does not fit one of the themes listed here, the DISTRICT's Project Manager shall be contacted to determine the appropriate theme for that data.

a. PRJ (Project Boundaries)

b. DQ (Data Quality)

- c. FPSRFFCD (Floodplain FCD Water Surface Elevation)
- d. FPXFCD (Floodplain FCD Cross Section)
- e. FPZNFCD (Floodplain FCD Zone)
- f. FPBLN (Floodplain Baseline Route System)
- g. PRJDAT (project identification)

**TASK 8 – DELIVERABLES**

- 8.1 Both paper and electronic deliverables will be made at the completion of each task.
- 8.2 Prior to FEMA Submittal: The CONSULTANT will deliver the following items to the DISTRICT.
  - 8.2.1 Two (2) copies of the Draft TDN including draft work maps according to ADWR State Standards Attachment 1-97 (SS 1-97) or most recent version. The report shall be organized as specified by the DISTRICT standards, following SS 1-97 format.
  - 8.2.2 Preliminary Digital Data Delivery including all digital files listed under Task 7 applicable to the final deliverable. The DISTRICT will be responsible for transmittal of one set of data to the City of Surprise.
- 8.3 FEMA Submittal: The CONSULTANT will submit the following items to the DISTRICT for review by FEMA and any other appropriate governmental agency. All of the following products are considered deliverables for the FEMA submittal:
  - 8.3.1 Two (2) complete sets of blackline paper topographic base maps with aerial photo background and the floodplain delineations shown. All drawings will be signed and sealed by persons of appropriate professional registration(s). Each registrant will provide a specific statement as to what service they performed. The DISTRICT will be responsible for transmittal of one set of data to the City of Surprise.
  - 8.3.2 Three (3) complete copies of the Technical Data Notebook, including completed FEMA forms, annotated Flood Insurance Rate Maps showing the proposed delineation and HEC-RAS input/output files on diskettes. The Technical Data Notebook will be prepared in accordance with ADWR State Standards Attachment 1-97 (SS 1-97) or most recent version. The notebook will be organized as specified by the DISTRICT, following SS 1-97 format. The DISTRICT will be responsible for transmittal of one set of data to the City of Surprise.
- 8.4 Final Submittal: The following products are considered deliverables for the final submittal to the DISTRICT after FEMA approval is issued.
  - 8.4.1 One (1) complete set of Mylars and two (2) complete sets of sealed blackline paper flood study work topographic map sets with the aerial photo background and the floodplain delineations shown or sealed PDF copies. The DISTRICT will be responsible for transmittal of one set of data to the City of Surprise. All drawings will be signed and sealed by persons of appropriate professional registration(s). Each registrant will provide a specific statement as to what service they performed.
  - 8.4.2 All remaining hydrologic and floodplain delineation data in conformance with the DISTRICT's HIS Specifications.

8.4.3 Three (3) complete copies of the Technical Data Notebook including HEC-RAS input/output files on compact disc (based upon the level of changes during the FEMA review process, this may require only change-outs of modified pages and/or disks). The Technical Data Notebook will be prepared in accordance with ADWR State Standards Attachment 1-97 (SS 1-97) or most recent version. The notebook will be organized as specified by the DISTRICT, following SS 1-97 format. This submittal of the Technical Data Notebook shall include any correspondence and/or meeting minutes with the reviewing agencies and shall reflect any revisions required by those reviewing agencies. Revisions may include, but are not limited to, addressing FEMA's comments, modifications to the delineation maps, the HEC-RAS model, and/or the Final Report. The report shall also be prepared in PDF format. The DISTRICT will be responsible for transmittal of one set of data to the City of Surprise.



**B.6 Public Information**

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# Arizona Business Gazette

The business resource

PO BOX 194  
Phoenix, Arizona 85001-0194  
(602) 444-7315 FAX (602) 444-7364

**ANNOUNCEMENT OF INTENT TO PERFORM A FLOODPLAIN DELINEATION STUDY WITHIN THE TRILBY WASH WATERSHED**

The Flood Control District of Maricopa County (FCDMC) has contracted with RBF Consulting to prepare a study documentation package of limited detail Zone AE floodplain (without floodway) for two washes in the Trilby Wash Watershed. The study area is located in the northwestern part of Maricopa County in the Wittmann area, and is bounded by the U.S. Highway 60 to the north and the Central Arizona Project (CAP) to the south.

The study will examine and evaluate the flood hazard areas in the watershed to determine limited detail floodplain limits. Updated flood hazard boundaries will then be used to determine the flood insurance rates used by the Federal Emergency Management Agency (FEMA) for anticipated future development in the area.

This announcement is intended to inform all interested persons and communities of the commencement of this study so that they may have an opportunity to bring any relevant technical information to the attention of the FCDMC/FEMA, to be considered during the course of this study. Your comments should be addressed to Mr. Jonathan E. Lesperance at the Flood Control District of Maricopa County, 2801 West Durango Street, Phoenix, AZ 85009, (602) 506-4692.

Published: July 21, 2011

STATE OF ARIZONA  
COUNTY OF MARICOPA

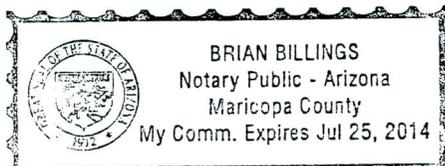
} SS.

Mark Gilmore, being first duly sworn, upon oath deposes and says: That he is the Legal Ad Rep of the Arizona Business Gazette, a newspaper of general circulation in the county of Maricopa, State of Arizona, published weekly at Phoenix, Arizona, and that the copy hereto attached is a true copy of the advertisement published in the said paper on the dates indicated.

7/21/2011

  
\_\_\_\_\_

Sworn to before me this  
21ST day of  
JULY 2011



  
\_\_\_\_\_  
Notary Public



**B.7 FEMA Correspondence**



# Flood Control District of Maricopa County

## INTEROFFICE MEMORANDUM

**Date:** November 28, 2011  
**To:** Timothy S. Phillips, P.E., Chief Engineer and General Manager  
**From:** Jonathan Lesperance, Hydrology and Hydraulics Branch  
**Subject:** Wittmann Phase IV Floodplain Delineation Study TDN, Contract FCD 2011C003

The floodplain study for the Wittmann Phase IV FDS is ready for use as the best available technical information. The study documentation will be sent to FEMA for review and incorporation into the County's FIRM panels. Please find attached an MT-2 form requesting your signature.

The background for the study includes the following:

The study delineates approximately 6.5 linear miles of limited detail Zone AE floodplain (no floodway). The topographic basis for the study is 2 and 4-foot contour interval mapping in NAVD88 vertical datum by Landata Airborne Systems, Inc. The photography for topographic mapping of the study area was flown April 23, 2002. The study Consultant was RBF Consulting. The project manager for the Consultant was Nathan Ford, P.E. The project manager for the District was Jonathan Lesperance.

Please concur and authorize below the use of this new study.

Date: <u>11/28/11</u> <i>Jonathan Lesperance</i> Jonathan Lesperance Project Manager	Date: <u>12/7/11</u> <i>T.S.P.</i> Timothy S. Phillips, P.E. Chief Engineer and General Manager
Date: <u>12/5/11</u> <i>A. Motamedi</i> Amir Motamedi, P.E. Hydrology/Hydraulics Branch Manager	Date: <u>12/5/2011</u> <i>Ed Raleigh</i> Ed Raleigh, P.E. Engineering Division Manager
Date: <u>12/6/2011</u> <i>Kelli Sertich</i> Kelli Sertich, AICP Floodplain Management & Services Division Manager	Date: _____
File Copies: 1. _____ 2. _____	<input checked="" type="checkbox"/> YES <input type="checkbox"/> GIS Posted (Pending Floodplain Only) Date: _____ <input checked="" type="checkbox"/> NO GIS In Progress

	Intl	Date
<b>Chief Engineer/General Manager</b>		
5 Phillips, T.		
Lemmon, J.		
Medina, A.		
<b>PIO</b>		
Munoz, J.		
Dodd, E.		
Smith, D.		
Wegner, G.		
<b>Human Resources</b>		
Trottie, J.		
<b>Facilities Mgt.</b>		
Robinson, C.		
Sanborn, P.		
Leon, J.		
Willard, M.		
<b>Admin. Support</b>		
Mendenhall, L.		
Harrington, C.		
Charles, S.		
Delpier, C.		
Figueroa, L.		
Gonzalez, J.		
Johnson, T.		
Lepe, N.		
Lopez, D.		
Summers, A.		
Wu, S.		
<b>Administration Division</b>		
Wilson, D.		
<b>Contract Services</b>		
Bauer, M.		
Rogers, S.		
Schafer, P.		
<b>Financial Services</b>		
Artega, C.		
Dungan, G.		
Jasinski, C.		
Mageo, H.		
Scott, K.		
Stevens, D.		
Turner, D.		
<b>Engineering Division</b>		
3 Raleigh, E.		12/5/2011
Merkevicius, M.		
<b>Special Projects</b>		
Loomis, T.		
Garcia, A.		
Gerlach, S.		
Riano, A.		

	Intl	Date
<b>Hydrology/Hydraulics</b>		
2 Motamedi, A.	X	12/5
Cox, J.		
Gross, K.		
Harris, R.		
Holmes, J.		
1 Lesperance, J.	JFL	11/28
Rakestraw, K.		
Register, C.		
Serago, J.		
Shelton, J.		
Tucker, S.		
<b>Civil/Structures</b>		
Riddle, J.		
Brown, S.		
Glover, A.		
Guzman, J.		
Hanumaiah, K.		
Hardesty, A.		
Jones, M.		
Kehlenbach, F.		
Moore, G.		
Shapiro, G.		
Wagner, J.		
<b>Mapping &amp; Surveying</b>		
Stock, J.		
Ashley, J.		
Orr, J.		
<b>Flood Warning</b>		
Waters, S.		
Gardner, D.		
Miller, C.		
<b>Hydromets</b>		
Kiefer, T.		
Bushelman, J.		
Irizarry, B.		
Ontiveros, A.		
Randez, R.		
<b>Water Quality</b>		
Oller, M.		
Naud, B.		
<b>Eng App Dev/ River Mechanics</b>		
Zhao, B.		
Card, G.		
Chill, J.		
Li, S.		
Liu, D.		
Pacheco, R.		
Waskowsky, R.		
<b>GIS Division</b>		
Dent, M.		
Smith, J.		

	Intl	Date
<b>System Programmers-GIS</b>		
Wolfson, L.		
Agne, M.		
Akuoko, K.		
Bota, G.		
Brewer, M.		
Conklin, J.		
LaVallee, K.		
Sanchez, J.		
<b>Mapping Services-GIS</b>		
Bruffy, S.		
Feldman, E.		
Furnas, C.		
McGlinicy, B.		
Williams, T.		
Wong, N.		
<b>O &amp; M Division</b>		
Klenner, C.		
Wiley, R.		
<b>O&amp;M Supervisor</b>		
Rubin, S.		
<b>Central Yard</b>		
Long, G.		
Norton, D.		
<b>North Yard</b>		
Shaffer, R.		
Wise, J.		
<b>East Yard</b>		
Loy, E.		
<b>Ecology</b>		
Siegfried, T.		
<b>O&amp;M/WCC</b>		
Rivera, C.		
Armtz, E.		
Buruato, A.		
Michael, D.		
Ramirez, M.		
Wallace, N.		
<b>Shop</b>		
Olson, E.		
Byington, I.		
Pacheco, E.		
Villalobos, I.		
<b>PPM Division</b>		
Rerick, D.		
Cooper, H.		
Holcomb, D.		
Melo-Rodriguez, P.		
Reinbold, L.		
<b>CIP/Policy</b>		
Fazio, C.		

	Intl	Date
<b>Planning</b>		
Williams, D.		
Abouraiyan, A.		
Ganados, B.		
Hathaway, J.		
Jones, G.		
Lokey, B.		
Pinto, T.		
Pokorski, J.		
Samples, M.		
Stuart, D.		
Swick, V.		
Terry, F.		
<b>Structures Management Branch</b>		
Renckly, T.		
Degerness, D.		
Duffy, D.		
Greenslade, M.		
Lawrence, D.		
Leal, B.		
Minero, A.		
Sherman, S.		
<b>Project Mgmt.</b>		
Vogel, S.		
Beuche, A.		
Duncan, M.		
Maiers, G.		
Ohler, B.		
Stevens, B.		
Wesch, G.		
<b>Construction</b>		
Huber, F.		
Nase, T.		
Lewis, M.		
Rodriguez Jr, J.		
Salisbury, D.		
Shivaswamy, S.		
Stanley, M.		
Towers, M.		
<b>Real Estate Division</b>		
Wilson, M.		
Bell, L.		
Lawton, H.		
<b>Prop. Acq.</b>		
Scott, G.		
Berg, S.		
Kottmer, K.		
Leonard, G.		
Mertz, P.		
Pettigrew, K.		
Romero, K.		
Sachs, R.		

	Intl	Date
<b>Property Management</b>		
Cunningham, D.		
Amos, L.		
Anderssohn, K.		
Dickie, A.		
Warburton, S.		
<b>Property Engineering</b>		
Green, K.		
Burns, J.		
Campbell, J.		
Gutierrez, A.		
King, M.		
Sanchez, J.		
Van Schoyck, L.		
Stanczyk, S.		
Velasco, D.		
<b>FMS Management &amp; Services</b>		
4 Sertich, K.	FAS	12/6/11
Gorbenko, A.		
<b>MPTP</b>		
Murphy, T.		
Brown, F.		
Frago, M.		
Tavassoli, F.		
McGuire, S.		
<b>Technical Plan Review</b>		
Thomas, L.		
DeRoulhac, K.		
DeVlieg, C.		
Lapp, S.		
Teneva, M.		
Nangare, M.		
<b>FMS Floodplain</b>		
Schneeman, J.		
Castillo, K.		
Gattuso, C.		
Jimenez, L.		
LeBeau, D.		
Mendoza, K. (P&D)		
<b>Enforcement /Inspections</b>		
Smith, M.		
Carroll, D.		
South J.		
Bancod, F.		
Schwartz, G.		
<b>Central Files</b>		
File#		
<b>Comments</b>		



# Flood Control District of Maricopa County

## Board of Directors

Fulton Brock, District 1  
Don Stapley, District 2  
Andrew Kunasek, District 3  
Max Wilson, District 4  
Mary Rose Wilcox, District 5

[www.fcd.maricopa.gov](http://www.fcd.maricopa.gov)

2801 West Durango Street  
Phoenix, Arizona 85009  
Phone: 602-506-1501  
Fax: 602-506-4601  
TT: 602-505-5897

November 28, 2011

LOMR Manager  
LOMC Clearinghouse  
7390 Coca Cola Drive, Suite 204  
Hanover, MD 21076

Subject: Technical Data Notebook – Wittmann Phase IV Floodplain Delineation Study,  
Maricopa County, Arizona (Contract FCD 2011C003 – Work Assignment No. 1)  
by RBF Consulting

Communities: Maricopa County and Unincorporated Areas, Community No. 040037

Flooding Sources: Iona Wash and Tributaries

FIRM panels affected: 04013C660G, 04013C670G, 04013C680G, 04013C689H,  
04013C690H

LOMR Manager:

Enclosed is the technical supporting data for the Letter of Map Revision request to delineate 6.5 linear miles of limited detail Zone AE floodplain (without floodway) along tributaries to Iona Wash in Unincorporated Maricopa County, Arizona. The study area is located in the northwest portion of Maricopa County. These washes have not been studied before and therefore we request the review fee be waived. However, please let us know if there is a fee required.

If you have any questions, please contact me at (602) 506-4699, or by email at: [jonathanlesperance@mail.maricopa.gov](mailto:jonathanlesperance@mail.maricopa.gov).

Sincerely,

A handwritten signature in cursive script that reads "Jonathan Lesperance".

Jonathan Lesperance, CFM  
Hydrology and Hydraulics Branch

Enclosures: Technical Data Notebook

Copy to:

Brian Cosson, CFM  
NFIP State Coordinator  
Arizona Department of Water Resources  
Office of Dam Safety and Flood Mitigation  
3550 N. Central Ave.  
Phoenix, AZ 85012

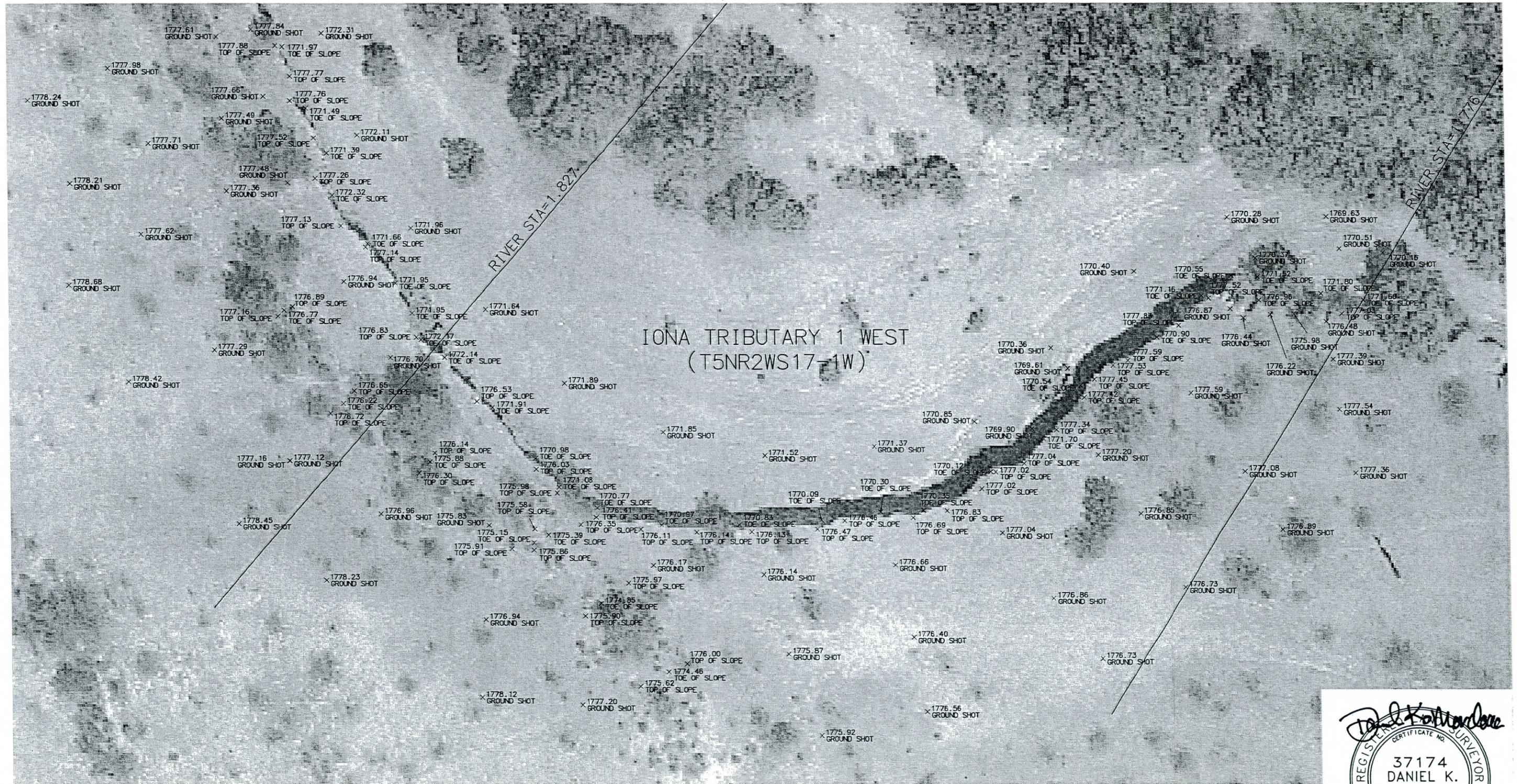
Robert Bezek  
Federal Emergency Management Agency  
Region IX  
1111 Broadway, Suite 1200  
Oakland, CA 94607

Nathan Ford, P.E. CFM  
RBF Consulting  
16605 North 28<sup>th</sup> Ave, Suite 100  
Phoenix, AZ 85053-7550



**Appendix C Survey Field Notes**

# WITTMANN PHASE IV FDS GROUND ELEVATION EXHIBIT IN TOWNSHIP 5 NORTH, RANGE 3 WEST, SECTION 7



*Daniel K. Mardock*

REGISTERED PROFESSIONAL SURVEYOR  
 CERTIFICATE NO. 37174  
 DANIEL K. MARDOCK  
 Date Signed: 11-7-11  
 ARIZONA, U.S.A.  
 Expires: 03/31/13



PLANNING ■ DESIGN ■ CONSTRUCTION

16605 NORTH 28th AVENUE, SUITE 100  
 PHOENIX, ARIZONA 85053-7550  
 602.467.2200 ■ FAX 602.467.2201 ■ www.RBF.com



**Appendix D Hydrologic Analysis Supporting  
Documentation**



Wittmann Phase IV  
Floodplain Delineation Study

**D.1 Precipitation Data**

(Information obtained from July 2005 Wittmann ADMSU Hydrology Final Addendum prepared by Entellus)

Flood Control District of Maricopa County  
 EXAMPLE2 - Single Storm, 24 Hr, Green Ampt, S-Graph, Normal Depth  
 Rainfall Data

Page 1

9/02/2003

Primary Zone Number: 7    Latitude: 0.0    Elevation: 0  
 Short Duration Zone Number: 8    Longitude: 0.0

Duration	Point Values (in)						
	2-Yr	5-Yr	10-Yr	25-Yr	50-Yr	100-Yr	500-yr
5 MIN	0.39	0.48	0.54	0.63	0.70	0.77	0.93
10 MIN	0.58	0.72	0.82	0.96	1.07	1.18	1.44
15 MIN	0.71	0.90	1.03	1.22	1.37	1.52	1.86
30 MIN	0.94	1.20	1.39	1.65	1.86	2.06	2.53
1 HOUR	1.14	1.49	1.72	2.06	2.31	2.57	3.17
2 HOUR	1.23	1.62	1.89	2.27	2.56	2.85	3.53
3 HOUR	1.29	1.72	2.01	2.41	2.73	3.04	3.77
6 HOUR	1.40	1.89	2.22	2.68	3.04	3.40	4.22
12 HOUR	1.50	2.07	2.45	2.98	3.39	3.80	4.74
24 HOUR	1.60	2.25	2.68	3.28	3.74	4.20	5.26

APPENDIX D.1.1

Page 1 of 5





Wittmann Phase IV  
Floodplain Delineation Study

**D.2 Physical Parameter Calculations**

(Information obtained from July 2005 Wittmann ADMSU Hydrology Final Addendum prepared by Entellus)

APPENDIX D.2.1: Soil Type Summary



CLIENT: FCDMC

JOB: Wittmann Area Drainage Master Study Update

SHEET \_\_\_\_\_ OF \_\_\_\_\_

BY \_\_\_\_\_ DATE 4/19/2004

CHECK \_\_\_\_\_ DATE \_\_\_\_\_

JOB NO. 310.032

Areas and percentage area reported to 4 significant digits by WMS. Actual precision of the data is only 2-3 significant digits

Basin ID	Total Basin Area (sq miles)	Soil Type (WMS)	Soil Type (DDMSW)	Soil Type Area (%)	Soil Type Area (sq-miles)	XKSAT (in/hr)	RTIMP
IW322	1.8923	64576	76	5.64%	0.1067	0.23	0
IW322	1.8923	64577	77	5.16%	0.0976	0.05	0
IW322	1.8923	64578	78	2.76%	0.0522	0.05	0
IW326	0.3331	645112	112	14.69%	0.0489	0.39	0
IW326	0.3331	64558	58	13.99%	0.0466	0.34	0
IW326	0.3331	64576	76	51.05%	0.1700	0.23	0
IW326	0.3331	64578	78	20.28%	0.0675	0.05	0
IW330	1.0372	64529	29	12.36%	0.1282	0.34	0
IW330	1.0372	64553	53	12.80%	0.1328	0.02	0
IW330	1.0372	64555	55	16.05%	0.1665	0.27	0
IW330	1.0372	64558	58	36.44%	0.3779	0.34	0
IW330	1.0372	64570	70	6.29%	0.0652	0.36	0
IW330	1.0372	64576	76	0.65%	0.0067	0.23	0
IW330	1.0372	64578	78	15.40%	0.1597	0.05	0
IW334	1.2571	645112	112	21.30%	0.2678	0.39	0
IW334	1.2571	64522	22	8.48%	0.1066	0.04	0
IW334	1.2571	64553	53	7.22%	0.0908	0.02	0
IW334	1.2571	64576	76	3.61%	0.0454	0.23	0
IW334	1.2571	64578	78	21.30%	0.2678	0.05	0
IW334	1.2571	64598	98	38.09%	0.4788	0.37	0
IW338	0.3390	645112	112	20.98%	0.0711	0.39	0
IW338	0.3390	64522	22	5.59%	0.0190	0.04	0
IW338	0.3390	64576	76	3.50%	0.0119	0.23	0
IW338	0.3390	64580	80	45.45%	0.1541	0.08	0
IW338	0.3390	64598	98	24.48%	0.0830	0.37	0
IW342	0.3535	645112	112	17.11%	0.0605	0.39	0
IW342	0.3535	64558	58	30.92%	0.1093	0.34	0
IW342	0.3535	64580	80	16.45%	0.0582	0.08	0
IW342	0.3535	64598	98	35.53%	0.1256	0.37	0
IW346	3.4432	645112	112	3.25%	0.1119	0.39	0
IW346	3.4432	64522	22	9.88%	0.3402	0.04	0
IW346	3.4432	64523	23	2.93%	0.1009	0.01	0
IW346	3.4432	64529	29	6.63%	0.2283	0.34	0
IW346	3.4432	64555	55	15.41%	0.5306	0.27	0
IW346	3.4432	64558	58	54.16%	1.8648	0.34	0
IW346	3.4432	64570	70	5.01%	0.1725	0.36	0
IW346	3.4432	64576	76	0.07%	0.0024	0.23	0
IW346	3.4432	64578	78	2.60%	0.0895	0.05	0
IW346	3.4432	64598	98	0.07%	0.0024	0.37	0
IW349	0.2498	645113	113	76.15%	0.1902	0.39	0
IW349	0.2498	64548	48	23.85%	0.0596	0.06	0
IW350	2.1751	645110	110	27.21%	0.5918	0.13	0
IW350	2.1751	645119	119	2.80%	0.0609	0.14	0
IW350	2.1751	64522	22	36.14%	0.7861	0.04	0
IW350	2.1751	64529	29	7.17%	0.1560	0.34	0
IW350	2.1751	64553	53	0.42%	0.0091	0.02	0
IW350	2.1751	64555	55	2.49%	0.0542	0.27	0
IW350	2.1751	64570	70	10.49%	0.2282	0.36	0

APPENDIX D.2.1: Soil Type Summary



CLIENT: FCDMC

JOB: Wittmann Area Drainage Master Study Update

SHEET \_\_\_\_\_ OF \_\_\_\_\_

BY \_\_\_\_\_ DATE 4/19/2004

CHECK \_\_\_\_\_ DATE \_\_\_\_\_

JOB NO. 310.032

Areas and percentage area reported to 4 significant digits by WMS. Actual precision of the data is only 2-3 significant digits

Basin ID	Total Basin Area (sq miles)	Soil Type (WMS)	Soil Type (DDMSW)	Soil Type Area (%)	Soil Type Area (sq-miles)	XKSAT (in/hr)	RTIMP
IW350	2.1751	64575	75	13.29%	0.2891	0.23	0
IW351	0.3164	645113	113	2.17%	0.0069	0.39	0
IW351	0.3164	64546	46	5.80%	0.0183	0.03	0
IW351	0.3164	64558	58	20.29%	0.0642	0.34	0
IW351	0.3164	64598	98	71.74%	0.2270	0.37	0
IW352	0.0961	645113	113	48.84%	0.0469	0.39	0
IW352	0.0961	64548	48	11.63%	0.0112	0.06	0
IW352	0.0961	64558	58	37.21%	0.0357	0.34	0
IW352	0.0961	64598	98	2.33%	0.0022	0.37	0
IW353	2.8031	64521	21	1.12%	0.0314	0.38	0
IW353	2.8031	64522	22	50.20%	1.4072	0.04	0
IW353	2.8031	64529	29	23.38%	0.6554	0.34	0
IW353	2.8031	64544	44	4.32%	0.1211	0.03	0
IW353	2.8031	64546	46	10.41%	0.2918	0.03	0
IW353	2.8031	64553	53	0.16%	0.0045	0.02	0
IW353	2.8031	64555	55	0.16%	0.0045	0.27	0
IW353	2.8031	64558	58	6.89%	0.1931	0.34	0
IW353	2.8031	64575	75	2.64%	0.0740	0.23	0
IW353	2.8031	64598	98	0.72%	0.0202	0.37	0
IW354	0.3605	64548	48	92.90%	0.3349	0.06	0
IW354	0.3605	64558	58	7.10%	0.0256	0.34	0
IW356	0.2861	645113	113	24.03%	0.0688	0.39	0
IW356	0.2861	64548	48	75.97%	0.2174	0.06	0
IW357	0.7457	64522	22	0.30%	0.0022	0.04	0
IW357	0.7457	64529	29	3.03%	0.0226	0.34	0
IW357	0.7457	6453	3	4.24%	0.0316	0.58	0
IW357	0.7457	64546	46	63.64%	0.4746	0.03	0
IW357	0.7457	64574	74	6.36%	0.0474	0.08	0
IW357	0.7457	64598	98	22.42%	0.1672	0.37	0
IW358	0.1641	64522	22	7.58%	0.0124	0.04	0
IW358	0.1641	64529	29	12.12%	0.0199	0.34	0
IW358	0.1641	64555	55	25.76%	0.0423	0.27	0
IW358	0.1641	64558	58	54.55%	0.0895	0.34	0
IW359	2.9120	645110	110	2.01%	0.0585	0.13	0
IW359	2.9120	64521	21	1.47%	0.0428	0.38	0
IW359	2.9120	64522	22	0.93%	0.0271	0.04	0
IW359	2.9120	64529	29	45.91%	1.3369	0.34	0
IW359	2.9120	64544	44	6.48%	0.1887	0.03	0
IW359	2.9120	64553	53	19.52%	0.5684	0.02	0
IW359	2.9120	64555	55	15.20%	0.4426	0.27	0
IW359	2.9120	64575	75	4.71%	0.1372	0.23	0
IW359	2.9120	64598	98	3.78%	0.1101	0.37	0
IW360	0.8306	645113	113	29.05%	0.2413	0.39	0
IW360	0.8306	64558	58	3.63%	0.0302	0.34	0
IW360	0.8306	64598	98	67.32%	0.5592	0.37	0
IW361	0.3547	64522	22	2.58%	0.0092	0.04	0
IW361	0.3547	64529	29	7.10%	0.0252	0.34	0
IW361	0.3547	64544	44	17.42%	0.0618	0.03	0

APPENDIX D.2.1: Soil Type Summary



CLIENT: FCDMC

JOB: Wittmann Area Drainage Master Study Update

SHEET \_\_\_\_\_ OF \_\_\_\_\_

BY \_\_\_\_\_ DATE 4/19/2004

CHECK \_\_\_\_\_ DATE \_\_\_\_\_

JOB NO. 310.032

Areas and percentage area reported to 4 significant digits by WMS. Actual precision of the data is only 2-3 significant digits

Basin ID	Total Basin Area (sq miles)	Soil Type (WMS)	Soil Type (DDMSW)	Soil Type Area (%)	Soil Type Area (sq-miles)	XKSAT (in/hr)	RTIMP
IW361	0.3547	64546	46	12.90%	0.0458	0.03	0
IW361	0.3547	64555	55	20.65%	0.0733	0.27	0
IW361	0.3547	64558	58	39.35%	0.1396	0.34	0
IW362	0.1794	64548	48	75.00%	0.1345	0.06	0
IW362	0.1794	64558	58	25.00%	0.0448	0.34	0
IW363	0.6278	645113	113	0.36%	0.0023	0.39	0
IW363	0.6278	64546	46	78.55%	0.4931	0.03	0
IW363	0.6278	64574	74	10.18%	0.0639	0.08	0
IW363	0.6278	64598	98	10.91%	0.0685	0.37	0
IW364	1.1451	645113	113	1.38%	0.0158	0.39	0
IW364	1.1451	64544	44	82.45%	0.9441	0.03	0
IW364	1.1451	64548	48	14.40%	0.1649	0.06	0
IW364	1.1451	64580	80	1.58%	0.0181	0.08	0
IW364	1.1451	64598	98	0.20%	0.0023	0.37	0
IW365	0.4729	64546	46	100.00%	0.4729	0.03	0
IW366	1.0558	64529	29	23.01%	0.2429	0.34	0
IW366	1.0558	64544	44	4.95%	0.0523	0.03	0
IW366	1.0558	64547	47	0.22%	0.0023	0.11	0
IW366	1.0558	64553	53	17.63%	0.1861	0.02	0
IW366	1.0558	64575	75	1.29%	0.0136	0.23	0
IW366	1.0558	64577	77	7.10%	0.0750	0.05	0
IW366	1.0558	64598	98	45.81%	0.4837	0.37	0
IW367	0.0512	64546	46	100.00%	0.0512	0.03	0
IW368	0.2907	645113	113	100.00%	0.2907	0.39	0
IW369	0.7949	645113	113	6.86%	0.0545	0.39	0
IW369	0.7949	64546	46	93.14%	0.7404	0.03	0
IW370	0.8795	645113	113	57.11%	0.5023	0.39	0
IW370	0.8795	6453	3	25.58%	0.2250	0.58	0
IW370	0.8795	64546	46	17.31%	0.1522	0.03	0
IW371	0.0529	64598	98	100.00%	0.0529	0.37	0
IW372	0.3175	645113	113	78.57%	0.2495	0.39	0
IW372	0.3175	6453	3	0.71%	0.0023	0.58	0
IW372	0.3175	64544	44	20.71%	0.0658	0.03	0
IW374	0.3875	64544	44	68.42%	0.2651	0.03	0
IW374	0.3875	64598	98	31.58%	0.1224	0.37	0
IW375	0.6582	645113	113	46.48%	0.3059	0.39	0
IW375	0.6582	64544	44	53.52%	0.3523	0.03	0
IW377	0.1126	645110	110	38.10%	0.0429	0.13	0
IW377	0.1126	64544	44	19.05%	0.0214	0.03	0
IW377	0.1126	64598	98	42.86%	0.0482	0.37	0
IW380	0.4998	6453	3	12.56%	0.0628	0.58	0
IW380	0.4998	64535	35	15.35%	0.0767	0.23	0
IW380	0.4998	64539	39	4.65%	0.0232	0.29	0
IW380	0.4998	64544	44	43.26%	0.2162	0.03	0
IW380	0.4998	64546	46	11.63%	0.0581	0.03	0
IW380	0.4998	64548	48	0.93%	0.0046	0.06	0
IW380	0.4998	6457	7	11.63%	0.0581	0.62	0
IW381	0.3841	64529	29	14.12%	0.0542	0.34	0

APPENDIX D.2.1: Soil Type Summary



CLIENT: FCDMC

JOB: Wittmann Area Drainage Master Study Update

SHEET \_\_\_\_\_ OF \_\_\_\_\_

BY \_\_\_\_\_ DATE 4/19/2004

CHECK \_\_\_\_\_ DATE \_\_\_\_\_

JOB NO. 310.032

Areas and percentage area reported to 4 significant digits by WMS. Actual precision of the data is only 2-3 significant digits

Basin ID	Total Basin Area (sq miles)	Soil Type (WMS)	Soil Type (DDMSW)	Soil Type Area (%)	Soil Type Area (sq-miles)	XKSAT (in/hr)	RTIMP
IW381	0.3841	64544	44	77.06%	0.2960	0.03	0
IW381	0.3841	64574	74	8.82%	0.0339	0.08	0
IW382	0.6196	64513	13	1.82%	0.0113	0.01	0
IW382	0.6196	64529	29	54.74%	0.3392	0.34	0
IW382	0.6196	64544	44	33.94%	0.2103	0.03	0
IW382	0.6196	64574	74	3.28%	0.0203	0.08	0
IW382	0.6196	64598	98	6.20%	0.0384	0.37	0
IW384	0.0826	64529	29	23.53%	0.0194	0.34	0
IW384	0.0826	64544	44	76.47%	0.0631	0.03	0
IW386	0.3732	645110	110	7.41%	0.0277	0.13	0
IW386	0.3732	64513	13	3.09%	0.0115	0.01	0
IW386	0.3732	64544	44	33.95%	0.1267	0.03	0
IW386	0.3732	64548	48	28.40%	0.1060	0.06	0
IW386	0.3732	64549	49	14.20%	0.0530	0.06	0
IW386	0.3732	64574	74	1.23%	0.0046	0.08	0
IW386	0.3732	64598	98	11.73%	0.0438	0.37	0
IW387	1.3509	64536	36	32.72%	0.4420	0.07	0
IW387	1.3509	64539	39	43.85%	0.5924	0.29	0
IW387	1.3509	64564	64	9.80%	0.1324	0.14	25
IW387	1.3509	6457	7	1.66%	0.0224	0.62	0
IW387	1.3509	64594	94	11.96%	0.1616	0.33	0
IW388	1.2257	64513	13	3.49%	0.0428	0.01	0
IW388	1.2257	64529	29	8.26%	0.1012	0.34	0
IW388	1.2257	6453	3	9.54%	0.1169	0.58	0
IW388	1.2257	64544	44	13.39%	0.1641	0.03	0
IW388	1.2257	64546	46	62.20%	0.7624	0.03	0
IW388	1.2257	64598	98	3.12%	0.0382	0.37	0
IW389	0.1312	64546	46	100.00%	0.1312	0.03	0
IW390	0.3110	645110	110	14.60%	0.0454	0.13	0
IW390	0.3110	6453	3	29.20%	0.0908	0.58	0
IW390	0.3110	64544	44	0.73%	0.0023	0.03	0
IW390	0.3110	64548	48	12.41%	0.0386	0.06	0
IW390	0.3110	64549	49	8.76%	0.0272	0.06	0
IW390	0.3110	64574	74	5.84%	0.0182	0.08	0
IW390	0.3110	64598	98	28.47%	0.0885	0.37	0
IW390A	0.9003	64513	13	1.52%	0.0137	0.01	0
IW390A	0.9003	64528	28	5.56%	0.0501	0.02	0
IW390A	0.9003	6453	3	5.81%	0.0523	0.58	0
IW390A	0.9003	64542	42	5.05%	0.0455	0.17	0
IW390A	0.9003	64548	48	2.78%	0.0250	0.06	0
IW390A	0.9003	64564	64	16.16%	0.1455	0.14	25
IW390A	0.9003	64566	66	3.28%	0.0295	0.23	0
IW390A	0.9003	64568	68	3.03%	0.0273	0.63	0
IW390A	0.9003	64573	73	28.03%	0.2524	0.09	30
IW390A	0.9003	64574	74	5.81%	0.0523	0.08	0
IW390A	0.9003	64594	94	22.98%	0.2069	0.33	0
IW392	1.1135	6453	3	2.65%	0.0295	0.58	0
IW392	1.1135	64546	46	28.98%	0.3227	0.03	0

APPENDIX D.2.1: Soil Type Summary



CLIENT: FCDMC

JOB: Wittmann Area Drainage Master Study Update

SHEET \_\_\_\_\_ OF \_\_\_\_\_

BY \_\_\_\_\_ DATE 4/19/2004

CHECK \_\_\_\_\_ DATE \_\_\_\_\_

JOB NO. 310.032

Areas and percentage area reported to 4 significant digits by WMS. Actual precision of the data is only 2-3 significant digits

Basin ID	Total Basin Area (sq miles)	Soil Type (WMS)	Soil Type (DDMSW)	Soil Type Area (%)	Soil Type Area (sq-miles)	XKSAT (in/hr)	RTIMP
IW392	1.1135	64548	48	31.84%	0.3546	0.06	0
IW392	1.1135	64564	64	13.27%	0.1478	0.14	25
IW392	1.1135	64574	74	3.27%	0.0364	0.08	0
IW392	1.1135	64598	98	20.00%	0.2227	0.37	0
IW394	4.1731	645121	121	4.74%	0.1978	0.12	0
IW394	4.1731	64531	31	5.11%	0.2132	0.33	35
IW394	4.1731	64533	33	9.20%	0.3839	0.23	0
IW394	4.1731	64536	36	15.45%	0.6448	0.07	0
IW394	4.1731	64539	39	2.91%	0.1214	0.29	0
IW394	4.1731	64542	42	7.00%	0.2921	0.17	0
IW394	4.1731	64563	63	37.30%	1.5566	0.14	25
IW394	4.1731	64566	66	3.34%	0.1394	0.23	0
IW394	4.1731	6457	7	1.18%	0.0492	0.62	0
IW394	4.1731	64573	73	11.52%	0.4807	0.09	30
IW394	4.1731	64594	94	2.26%	0.0943	0.33	0
IW395	7.8618	637244526	CmD	3.37%	0.2649	0.00	0
IW395	7.8618	6374229	Le	5.23%	0.4112	0.00	0
IW395	7.8618	6374235	Lh	1.14%	0.0896	0.00	0
IW395	7.8618	6375455	Rr	3.26%	0.2563	0.00	0
IW395	7.8618	645104	104	1.23%	0.0967	0.14	60
IW395	7.8618	64516	16	0.23%	0.0181	0.44	15
IW395	7.8618	64531	31	3.49%	0.2744	0.33	35
IW395	7.8618	64534	34	5.12%	0.4025	0.23	0
IW395	7.8618	64536	36	1.40%	0.1101	0.07	0
IW395	7.8618	64543	43	1.06%	0.0833	0.17	0
IW395	7.8618	64562	62	0.17%	0.0134	0.15	0
IW395	7.8618	64563	63	65.28%	5.1322	0.14	25
IW395	7.8618	6457	7	1.20%	0.0943	0.62	0
IW395	7.8618	64572	72	7.01%	0.5511	0.09	30
IW395	7.8618	64597	97	0.80%	0.0629	0.07	0
IW396	0.5863	64535	35	26.54%	0.1556	0.23	0
IW396	0.5863	64536	36	12.69%	0.0744	0.07	0
IW396	0.5863	64539	39	13.85%	0.0812	0.29	0
IW396	0.5863	64543	43	5.38%	0.0315	0.17	0
IW396	0.5863	64544	44	0.38%	0.0022	0.03	0
IW396	0.5863	64564	64	0.77%	0.0045	0.14	25
IW396	0.5863	6457	7	40.38%	0.2367	0.62	0
IW397	4.8148	637244526	CmD	44.82%	2.1580	0.00	0
IW397	4.8148	637244730	CnF	0.28%	0.0135	0.00	0
IW397	4.8148	6374229	Le	16.65%	0.8017	0.00	0
IW397	4.8148	6374235	Lh	16.60%	0.7993	0.00	0
IW397	4.8148	6375455	Rr	21.64%	1.0419	0.00	0
PD700	1.3998	651203222	AGB	1.63%	0.0228	0.40	0
PD700	1.3998	651203424	AHC	10.60%	0.1484	0.38	0
PD700	1.3998	6513244	GM	18.27%	0.2557	0.29	0
PD700	1.3998	651326426	GWD	2.77%	0.0388	0.35	0
PD700	1.3998	651326826	GVD	12.72%	0.1780	0.26	0
PD700	1.3998	651445822	MTB	26.59%	0.3722	0.15	0



Appendix D.2.2

SHEET \_\_\_\_\_ OF \_\_\_\_\_

BY \_\_\_\_\_ DATE \_\_\_\_\_

CHECK \_\_\_\_\_ DATE \_\_\_\_\_

CLIENT: FCDMC

JOB: Wittmann Area Drainage Master Study Update

JOB NO. 310.032

Existing Land Use Summary by Basin

Basin ID	Area (sqmiles)	WMS Land Use Code	Mag Land Use Code	IA (in) Abstractions	Initial RTIMP	% Vegetation	Land Use Definition
IW302	1.09	700	OPEN-1	0.35	5	30	General Open Space (Open space where no detail available)
IW302	0.02	740	WTR	0.00	0	0	Water
IW310	0.33	120	LDR-1	0.30	10	30	Estate Residential (1/5 du per acre to 1 du per acre)
IW310	0.00	130	LDR-2	0.30	15	50	Large Lot Residential - Single Family (1 du per acre to 2 du per acre)
IW310	0.82	700	OPEN-1	0.35	5	30	General Open Space (Open space where no detail available)
IW310	0.02	740	WTR	0.00	0	0	Water
IW312	0.02	600	FWY-1	0.30	20	30	General Transportation (Transportation where no detail available)
IW312	0.01	610	FWY-2	0.10	80	30	Transportation (Includes railroads, railyards, transit centers and freeways)
IW312	0.53	700	OPEN-1	0.35	5	30	General Open Space (Open space where no detail available)
IW314	0.09	120	LDR-1	0.30	10	30	Estate Residential (1/5 du per acre to 1 du per acre)
IW314	0.01	130	LDR-2	0.30	15	50	Large Lot Residential - Single Family (1 du per acre to 2 du per acre)
IW314	2.05	700	OPEN-1	0.35	5	30	General Open Space (Open space where no detail available)
IW314	0.01	740	WTR	0.00	0	0	Water
IW318	0.13	130	LDR-2	0.30	15	50	Large Lot Residential - Single Family (1 du per acre to 2 du per acre)
IW318	0.43	700	OPEN-1	0.35	5	30	General Open Space (Open space where no detail available)
IW322	1.87	700	OPEN-1	0.35	5	30	General Open Space (Open space where no detail available)
IW326	0.32	700	OPEN-1	0.35	5	30	General Open Space (Open space where no detail available)
IW330	1.03	700	OPEN-1	0.35	5	30	General Open Space (Open space where no detail available)
IW334	0.10	120	LDR-1	0.30	10	30	Estate Residential (1/5 du per acre to 1 du per acre)
IW334	1.13	700	OPEN-1	0.35	5	30	General Open Space (Open space where no detail available)
IW334	0.02	740	WTR	0.00	0	0	Water
IW338	0.09	120	LDR-1	0.30	10	30	Estate Residential (1/5 du per acre to 1 du per acre)
IW338	0.23	700	OPEN-1	0.35	5	30	General Open Space (Open space where no detail available)
IW342	0.06	110	VLDR	0.30	5	30	Rural Residential (<= 1/5 du per acre)
IW342	0.28	700	OPEN-1	0.35	5	30	General Open Space (Open space where no detail available)
IW346	0.10	110	VLDR	0.30	5	30	Rural Residential (<= 1/5 du per acre)
IW346	3.35	700	OPEN-1	0.35	5	30	General Open Space (Open space where no detail available)
IW349	0.02	130	LDR-2	0.30	15	50	Large Lot Residential - Single Family (1 du per acre to 2 du per acre)
IW349	0.05	140	MDR-1	0.25	30	50	Medium Lot Residential - Single Family (2-4 du per acre)
IW349	0.01	610	FWY-2	0.10	80	30	Transportation (Includes railroads, railyards, transit centers and freeways)
IW349	0.16	700	OPEN-1	0.35	5	30	General Open Space (Open space where no detail available)
IW350	2.16	700	OPEN-1	0.35	5	30	General Open Space (Open space where no detail available)
IW351	0.04	140	MDR-1	0.25	30	50	Medium Lot Residential - Single Family (2-4 du per acre)
IW351	0.27	700	OPEN-1	0.35	5	30	General Open Space (Open space where no detail available)
IW352	0.10	700	OPEN-1	0.35	5	30	General Open Space (Open space where no detail available)
IW353	0.01	120	LDR-1	0.30	10	30	Estate Residential (1/5 du per acre to 1 du per acre)
IW353	0.00	610	FWY-2	0.10	80	30	Transportation (Includes railroads, railyards, transit centers and freeways)
IW353	2.78	700	OPEN-1	0.35	5	30	General Open Space (Open space where no detail available)



Appendix D.2.2

SHEET \_\_\_\_\_ OF \_\_\_\_\_

BY \_\_\_\_\_ DATE \_\_\_\_\_

CLIENT: FCDMC

CHECK \_\_\_\_\_ DATE \_\_\_\_\_

JOB: Wittmann Area Drainage Master Study Update

JOB NO. 310.032

Existing Land Use Summary by Basin

Basin ID	Area (sqmiles)	WMS Land Use Code	Mag Land Use Code	IA (in) Initial Abstractions	RTIMP	% Vegetation	Land Use Definition
IW354	0.35	700	OPEN-1	0.35	5	30	General Open Space (Open space where no detail available)
IW356	0.29	700	OPEN-1	0.35	5	30	General Open Space (Open space where no detail available)
IW357	0.03	120	LDR-1	0.30	10	30	Estate Residential (1/5 du per acre to 1 du per acre)
IW357	0.03	570	IND-4	0.15	80	25	Other Employment - low (Proving grounds and land fills)
IW357	0.03	610	FWY-2	0.10	80	30	Transportation (Includes railroads, railyards, transit centers and freeways)
IW357	0.65	700	OPEN-1	0.35	5	30	General Open Space (Open space where no detail available)
IW358	0.15	700	OPEN-1	0.35	5	30	General Open Space (Open space where no detail available)
IW359	0.08	120	LDR-1	0.30	10	30	Estate Residential (1/5 du per acre to 1 du per acre)
IW359	0.04	610	FWY-2	0.10	80	30	Transportation (Includes railroads, railyards, transit centers and freeways)
IW359	2.78	700	OPEN-1	0.35	5	30	General Open Space (Open space where no detail available)
IW360	0.22	140	MDR-1	0.25	30	50	Medium Lot Residential - Single Family (2-4 du per acre)
IW360	0.01	610	FWY-2	0.10	80	30	Transportation (Includes railroads, railyards, transit centers and freeways)
IW360	0.57	700	OPEN-1	0.35	5	30	General Open Space (Open space where no detail available)
IW361	0.35	700	OPEN-1	0.35	5	30	General Open Space (Open space where no detail available)
IW362	0.17	700	OPEN-1	0.35	5	30	General Open Space (Open space where no detail available)
IW363	0.02	120	LDR-1	0.30	10	30	Estate Residential (1/5 du per acre to 1 du per acre)
IW363	0.01	140	MDR-1	0.25	30	50	Medium Lot Residential - Single Family (2-4 du per acre)
IW363	0.04	160	VSLR	0.25	40	50	Very Small Lot Residential - Single Family (>6 du per acre-includes mobile home)
IW363	0.03	610	FWY-2	0.10	80	30	Transportation (Includes railroads, railyards, transit centers and freeways)
IW363	0.51	700	OPEN-1	0.35	5	30	General Open Space (Open space where no detail available)
IW364	0.04	610	FWY-2	0.10	80	30	Transportation (Includes railroads, railyards, transit centers and freeways)
IW364	1.10	700	OPEN-1	0.35	5	30	General Open Space (Open space where no detail available)
IW365	0.03	600	FWY-1	0.30	20	30	General Transportation (Transportation where no detail available)
IW365	0.00	610	FWY-2	0.10	80	30	Transportation (Includes railroads, railyards, transit centers and freeways)
IW365	0.44	700	OPEN-1	0.35	5	30	General Open Space (Open space where no detail available)
IW366	0.02	130	LDR-2	0.30	15	50	Large Lot Residential - Single Family (1 du per acre to 2 du per acre)
IW366	0.01	520	INS-1	0.25	45	80	Educational (Public schools, private schools and universities)
IW366	0.02	610	FWY-2	0.10	80	30	Transportation (Includes railroads, railyards, transit centers and freeways)
IW366	0.98	700	OPEN-1	0.35	5	30	General Open Space (Open space where no detail available)
IW367	0.00	610	FWY-2	0.10	80	30	Transportation (Includes railroads, railyards, transit centers and freeways)
IW367	0.04	700	OPEN-1	0.35	5	30	General Open Space (Open space where no detail available)
IW368	0.01	610	FWY-2	0.10	80	30	Transportation (Includes railroads, railyards, transit centers and freeways)
IW368	0.26	700	OPEN-1	0.35	5	30	General Open Space (Open space where no detail available)
IW369	0.02	600	FWY-1	0.30	20	30	General Transportation (Transportation where no detail available)
IW369	0.00	610	FWY-2	0.10	80	30	Transportation (Includes railroads, railyards, transit centers and freeways)
IW369	0.77	700	OPEN-1	0.35	5	30	General Open Space (Open space where no detail available)
IW370	0.16	130	LDR-2	0.30	15	50	Large Lot Residential - Single Family (1 du per acre to 2 du per acre)
IW370	0.03	600	FWY-1	0.30	20	30	General Transportation (Transportation where no detail available)



Appendix D.2.2

SHEET \_\_\_\_\_ OF \_\_\_\_\_

BY \_\_\_\_\_ DATE \_\_\_\_\_

CLIENT: FCDMC

CHECK \_\_\_\_\_ DATE \_\_\_\_\_

JOB: Wittmann Area Drainage Master Study Update

JOB NO. 310.032

Existing Land Use Summary by Basin

Basin ID	Area (sqmiles)	WMS Land Use Code	Mag Land Use Code	IA (in) Initial Abstractions	RTIMP	% Vegetation	Land Use Definition
IW370	0.01	610	FWY-2	0.10	80	30	Transportation (Includes railroads, railyards, transit centers and freeways)
IW370	0.67	700	OPEN-1	0.35	5	30	General Open Space (Open space where no detail available)
IW371	0.01	130	LDR-2	0.30	15	50	Large Lot Residential - Single Family (1 du per acre to 2 du per acre)
IW371	0.01	610	FWY-2	0.10	80	30	Transportation (Includes railroads, railyards, transit centers and freeways)
IW371	0.02	700	OPEN-1	0.35	5	30	General Open Space (Open space where no detail available)
IW372	0.00	130	LDR-2	0.30	15	50	Large Lot Residential - Single Family (1 du per acre to 2 du per acre)
IW372	0.02	610	FWY-2	0.10	80	30	Transportation (Includes railroads, railyards, transit centers and freeways)
IW372	0.29	700	OPEN-1	0.35	5	30	General Open Space (Open space where no detail available)
IW374	0.03	110	VLDR	0.30	5	30	Rural Residential (<= 1/5 du per acre)
IW374	0.14	130	LDR-2	0.30	15	50	Large Lot Residential - Single Family (1 du per acre to 2 du per acre)
IW374	0.04	600	FWY-1	0.30	20	30	General Transportation (Transportation where no detail available)
IW374	0.01	610	FWY-2	0.10	80	30	Transportation (Includes railroads, railyards, transit centers and freeways)
IW374	0.17	700	OPEN-1	0.35	5	30	General Open Space (Open space where no detail available)
IW375	0.00	610	FWY-2	0.10	80	30	Transportation (Includes railroads, railyards, transit centers and freeways)
IW375	0.63	700	OPEN-1	0.35	5	30	General Open Space (Open space where no detail available)
IW377	0.09	110	VLDR	0.30	5	30	Rural Residential (<= 1/5 du per acre)
IW377	0.00	600	FWY-1	0.30	20	30	General Transportation (Transportation where no detail available)
IW377	0.01	700	OPEN-1	0.35	5	30	General Open Space (Open space where no detail available)
IW380	0.00	600	FWY-1	0.30	20	30	General Transportation (Transportation where no detail available)
IW380	0.48	700	OPEN-1	0.35	5	30	General Open Space (Open space where no detail available)
IW381	0.14	110	VLDR	0.30	5	30	Rural Residential (<= 1/5 du per acre)
IW381	0.03	600	FWY-1	0.30	20	30	General Transportation (Transportation where no detail available)
IW381	0.06	610	FWY-2	0.10	80	30	Transportation (Includes railroads, railyards, transit centers and freeways)
IW381	0.15	700	OPEN-1	0.35	5	30	General Open Space (Open space where no detail available)
IW382	0.21	110	VLDR	0.30	5	30	Rural Residential (<= 1/5 du per acre)
IW382	0.02	600	FWY-1	0.30	20	30	General Transportation (Transportation where no detail available)
IW382	0.02	610	FWY-2	0.10	80	30	Transportation (Includes railroads, railyards, transit centers and freeways)
IW382	0.36	700	OPEN-1	0.35	5	30	General Open Space (Open space where no detail available)
IW384	0.00	610	FWY-2	0.10	80	30	Transportation (Includes railroads, railyards, transit centers and freeways)
IW384	0.07	700	OPEN-1	0.35	5	30	General Open Space (Open space where no detail available)
IW386	0.11	110	VLDR	0.30	5	30	Rural Residential (<= 1/5 du per acre)
IW386	0.01	600	FWY-1	0.30	20	30	General Transportation (Transportation where no detail available)
IW386	0.25	700	OPEN-1	0.35	5	30	General Open Space (Open space where no detail available)
IW387	1.35	700	OPEN-1	0.35	5	30	General Open Space (Open space where no detail available)
IW388	0.01	110	VLDR	0.30	5	30	Rural Residential (<= 1/5 du per acre)
IW388	0.05	570	IND-4	0.15	80	25	Other Employment - low (Proving grounds and land fills)
IW388	0.09	600	FWY-1	0.30	20	30	General Transportation (Transportation where no detail available)
IW388	1.07	700	OPEN-1	0.35	5	30	General Open Space (Open space where no detail available)



Appendix D.2.2

SHEET \_\_\_\_\_ OF \_\_\_\_\_

BY \_\_\_\_\_ DATE \_\_\_\_\_

CLIENT: FCDMC

CHECK \_\_\_\_\_ DATE \_\_\_\_\_

JOB: Wittmann Area Drainage Master Study Update

JOB NO. 310.032

Existing Land Use Summary by Basin

Basin ID	Area (sqmiles)	WMS Land Use Code	Mag Land Use Code	IA (in) Initial Abstractions	RTIMP	% Vegetation	Land Use Definition
IW389	0.00	600	FWY-1	0.30	20	30	General Transportation (Transportation where no detail available)
IW389	0.12	700	OPEN-1	0.35	5	30	General Open Space (Open space where no detail available)
IW390	0.00	600	FWY-1	0.30	20	30	General Transportation (Transportation where no detail available)
IW390	0.30	700	OPEN-1	0.35	5	30	General Open Space (Open space where no detail available)
IW390A	0.81	700	OPEN-1	0.35	5	30	General Open Space (Open space where no detail available)
IW390A	0.07	730	OPEN-4	0.35	0	30	Passive Open Space (Includes mountain preserves and washes)
IW392	0.00	600	FWY-1	0.30	20	30	General Transportation (Transportation where no detail available)
IW392	1.10	700	OPEN-1	0.35	5	30	General Open Space (Open space where no detail available)
IW394	1.71	700	OPEN-1	0.35	5	30	General Open Space (Open space where no detail available)
IW394	2.45	730	OPEN-4	0.35	0	30	Passive Open Space (Includes mountain preserves and washes)
IW395	0.97	700	OPEN-1	0.35	5	30	General Open Space (Open space where no detail available)
IW395	6.87	730	OPEN-4	0.35	0	30	Passive Open Space (Includes mountain preserves and washes)
IW396	0.58	700	OPEN-1	0.35	5	30	General Open Space (Open space where no detail available)
IW397	4.79	730	OPEN-4	0.35	0	30	Passive Open Space (Includes mountain preserves and washes)
PD700	1.35	700	OPEN-1	0.35	5	30	General Open Space (Open space where no detail available)
PD700	0.02	740	WTR	0.00	0	0	Water
PD704	0.17	570	IND-4	0.15	80	25	Other Employment - low (Proving grounds and land fills)
PD704	0.18	700	OPEN-1	0.35	5	30	General Open Space (Open space where no detail available)
PD704	0.00	740	WTR	0.00	0	0	Water
PD708	0.01	570	IND-4	0.15	80	25	Other Employment - low (Proving grounds and land fills)
PD708	2.35	700	OPEN-1	0.35	5	30	General Open Space (Open space where no detail available)
PD708	0.05	740	WTR	0.00	0	0	Water
PD712	0.17	300	IND-1	0.15	55	60	General Industrial (Industrial where no detail available)
PD712	0.71	700	OPEN-1	0.35	5	30	General Open Space (Open space where no detail available)
PD712A	0.76	700	OPEN-1	0.35	5	30	General Open Space (Open space where no detail available)
PD716	1.73	700	OPEN-1	0.35	5	30	General Open Space (Open space where no detail available)
PD716	0.01	740	WTR	0.00	0	0	Water
PD720	0.57	700	OPEN-1	0.35	5	30	General Open Space (Open space where no detail available)
PD720	0.02	740	WTR	0.00	0	0	Water
PD726	0.13	120	LDR-1	0.30	10	30	Estate Residential (1/5 du per acre to 1 du per acre)
PD726	0.29	700	OPEN-1	0.35	5	30	General Open Space (Open space where no detail available)
PD726A	0.00	120	LDR-1	0.30	10	30	Estate Residential (1/5 du per acre to 1 du per acre)
PD726A	1.08	700	OPEN-1	0.35	5	30	General Open Space (Open space where no detail available)
PD726A	0.01	740	WTR	0.00	0	0	Water
PD726B	0.56	120	LDR-1	0.30	10	30	Estate Residential (1/5 du per acre to 1 du per acre)
PD726B	0.39	700	OPEN-1	0.35	5	30	General Open Space (Open space where no detail available)
PD732	0.44	700	OPEN-1	0.35	5	30	General Open Space (Open space where no detail available)
PD732	0.03	740	WTR	0.00	0	0	Water

**APPENDIX D.2.3**

**EXISTING CONDITIONS  
BASIN LG CARD SUMMARY**

<b>Basin</b>	<b>IA</b>	<b>DTHETA</b>	<b>PSIF</b>	<b>XKSAT</b>	<b>RTIMP*</b>
AF800	0.276	0.328	6.447	0.135	1.369
AF802	0.342	0.244	5.222	0.251	4.882
AF803	0.253	0.279	5.068	0.289	24.728
AF805	0.325	0.283	5.475	0.217	3.741
AF807	0.358	0.282	6.738	0.150	0.170
AF810	0.325	0.251	4.419	0.389	10.927
AF820	0.350	0.250	4.217	0.423	5.000
AF830	0.350	0.162	6.880	0.130	5.000
AF850	0.213	0.250	5.050	0.307	44.259
AF852	0.211	0.181	6.691	0.168	42.843
AF854	0.208	0.250	5.078	0.304	43.333
AF860	0.229	0.250	4.023	0.518	35.172
AF862	0.273	0.250	4.025	0.487	24.286
AF864	0.150	0.250	5.594	0.280	55.000
AF866	0.204	0.207	6.431	0.192	44.271
IW300	0.333	0.243	5.605	0.213	5.544
IW302	0.314	0.246	4.918	0.296	12.350
IW310	0.331	0.148	8.143	0.081	6.392
IW312	0.344	0.139	9.923	0.042	6.860
IW314	0.346	0.244	6.048	0.180	5.233
IW318	0.338	0.150	8.597	0.070	7.372
IW322	0.350	0.150	8.622	0.067	5.000
IW326	0.350	0.250	5.374	0.235	5.000
IW330	0.350	0.250	5.701	0.208	5.000
IW334	0.341	0.246	5.789	0.196	5.325
IW338	0.336	0.250	5.780	0.198	6.399
IW342	0.341	0.268	4.586	0.346	5.000
IW346	0.349	0.253	5.005	0.281	5.000
IW349	0.315	0.250	4.804	0.321	13.991
IW350	0.350	0.171	6.789	0.135	5.000
IW351	0.338	0.250	4.426	0.393	7.899
IW352	0.350	0.250	4.511	0.364	5.000
IW353	0.350	0.150	7.596	0.098	5.084
IW354	0.350	0.150	8.086	0.083	5.000
IW356	0.350	0.150	7.178	0.115	5.000
IW357	0.330	0.156	8.059	0.083	11.106
IW358	0.350	0.250	4.638	0.333	5.000
IW359	0.345	0.250	5.939	0.187	6.181
IW360	0.319	0.250	4.127	0.480	13.031
IW361	0.350	0.244	6.061	0.180	5.000
IW362	0.350	0.150	7.223	0.113	5.000
IW363	0.328	0.150	9.339	0.055	11.655
IW364	0.342	0.140	9.898	0.043	7.515
IW365	0.345	0.130	10.100	0.037	6.579
IW366	0.343	0.250	5.762	0.202	7.207
IW367	0.324	0.130	10.100	0.037	12.895

\*Ggreen and Ampt parameters are reported to 3 decimal places by WMS, but the actual precision of the data is only 2-3 significant figures.

**APPENDIX D.2.3**

**EXISTING CONDITIONS  
BASIN LG CARD SUMMARY**

<b>Basin</b>	<b>IA</b>	<b>DTHETA</b>	<b>PSIF</b>	<b>XKSAT</b>	<b>RTIMP*</b>
IW368	0.344	0.250	4.000	0.477	6.907
IW369	0.348	0.142	9.869	0.044	5.514
IW370	0.337	0.250	4.616	0.331	7.933
IW371	0.275	0.250	4.150	0.473	26.250
IW372	0.335	0.250	5.001	0.281	9.357
IW374	0.315	0.161	8.147	0.086	12.602
IW375	0.348	0.150	7.035	0.121	5.528
IW377	0.304	0.382	5.921	0.188	5.357
IW380	0.350	0.150	7.161	0.111	5.140
IW381	0.309	0.190	9.152	0.056	11.882
IW382	0.322	0.275	6.319	0.164	8.175
IW384	0.343	0.150	8.675	0.065	7.206
IW386	0.334	0.192	8.439	0.072	5.278
IW387	0.350	0.250	5.656	0.213	7.450
IW388	0.337	0.156	8.775	0.061	9.239
IW389	0.348	0.130	10.100	0.037	5.526
IW390	0.349	0.250	5.071	0.258	5.221
IW390A	0.350	0.250	6.134	0.172	17.033
IW392	0.350	0.150	7.459	0.103	8.347
IW394	0.350	0.327	6.166	0.173	16.632
IW395	0.350	0.374	6.125	0.175	27.313
IW396	0.350	0.250	4.518	0.336	5.192
IW397	0.350	0.381	6.390	0.159	39.233
PD700	0.344	0.246	5.243	0.250	4.918
PD704	0.299	0.297	4.461	0.342	2.516
PD708	0.342	0.245	4.987	0.268	10.121
PD712	0.311	0.250	5.374	0.248	28.595
PD712A	0.350	0.250	4.955	0.280	15.000
PD716	0.348	0.249	5.684	0.209	9.472
PD720	0.341	0.243	5.039	0.275	4.865
PD726	0.335	0.250	4.531	0.353	6.505
PD726A	0.346	0.247	4.501	0.366	8.937
PD726B	0.320	0.250	4.368	0.399	9.751
PD732	0.325	0.232	4.928	0.286	4.645
PD736	0.347	0.248	4.632	0.334	5.248
PD740	0.315	0.242	4.227	0.427	8.181
PD744	0.342	0.249	4.427	0.357	5.989
PD748	0.350	0.283	4.527	0.356	31.290
PD752	0.325	0.277	4.160	0.413	30.417
PD756	0.349	0.346	4.292	0.415	27.514
PD760	0.344	0.336	5.039	0.275	6.919
PI600	0.342	0.244	5.047	0.268	4.887
PI600A	0.345	0.246	5.184	0.252	4.929
PI603	0.344	0.250	4.660	0.316	7.957
PI604	0.307	0.250	4.970	0.285	22.848
PI606	0.324	0.250	4.383	0.419	10.725

\*Ggreen and Ampt parameters are reported to 3 decimal places by WMS, but the actual precision of the data is only 2-3 significant figures.

Table HY-D.2.3 Time of Concentration



CLIENT: FCDMC

JOB NO. 310.032

JOB: Wittmann Area Drainage Master Study Update

Basin	TOC Length [ft]	TOC Length [mi]	US Elev [ft]	DS Elev [ft]	Slope [ft/mi]	Area [mi <sup>2</sup> ]	Adjusted Length [mi]	TOC adjusted slope [ft/mi]	basin adjusted slope [ft/mi]	Adjusted Slope [ft/mi]	Resistance Coefficient Class K <sub>b</sub> <sup>e</sup> [A, B, C, D]	24-hr		6-hr		Existing Time-Area Relation	Future Time-Area Relation
												Tc	R	Tc	R		
												[hr]	[hr]	[hr]	[hr]		
IW326	7473	1.415	1660	1592	48.0	0.3331		48.0		48.0	B	0.588	0.506	0.538	0.459	Natural	Natural
IW330	16152	3.059	1744	1630	37.3	1.0372		37.3		37.3	B	1.058	0.944	1.008	0.895	Natural	Natural
IW334	12266	2.323	1632	1548	36.2	1.2571		36.2		36.2	B	0.838	0.523	0.808	0.503	Natural	Natural
IW338	7098	1.344	1644	1584	44.6	0.3390		44.6		44.6	B	0.575	0.470	0.529	0.428	Natural	Natural
IW342	11257	2.132	1698	1606	43.2	0.3535		43.2		43.2	B	0.888	1.074	0.800	0.957	Natural	Natural
IW346	26458	5.011	1842	1608	46.7	3.4432		46.7		46.7	B	1.254	0.854	1.271	0.866	Natural	Natural
IW349	9959	1.886	1906	1780	66.8	0.2498		66.8		66.8	B	0.650	0.840	0.596	0.763	Natural	Natural
IW350	26115	4.946	1852	1680	34.8	2.1751		34.8		34.8	B	1.363	1.203	1.308	1.150	Natural	Natural
IW351	8979	1.701	1888	1790	57.6	0.3164		57.6		57.6	B	0.642	0.658	0.596	0.615	Natural	Natural
IW352	4164	0.789	1780	1736	55.8	0.0961		55.8		55.8	B	0.421	0.445	0.383	0.401	Natural	Natural
IW353	25008	4.736	1976	1716	54.9	2.8031		54.9		54.9	B	1.004	0.717	0.996	0.710	Natural	Natural
IW354	8758	1.659	1812	1712	60.3	0.3605		60.3		60.3	B	0.550	0.511	0.508	0.468	Natural	Natural
IW356	9366	1.774	1850	1736	64.3	0.2861		64.3		64.3	B	0.579	0.651	0.533	0.594	Natural	Natural
IW357	15742	2.981	2050	1848	67.8	0.7457		67.8		67.8	B	0.742	0.752	0.700	0.705	Natural	Natural
IW358	8063	1.527	1790	1716	48.5	0.1641		48.5		48.5	B	0.683	0.953	0.625	0.863	Natural	Natural
IW359	20045	3.796	1968	1792	46.4	2.9120		46.4		46.4	B	0.979	0.571	0.983	0.574	Natural	Natural
IW360	11142	2.110	1888	1756	62.6	0.8306		62.6		62.6	B	0.683	0.490	0.654	0.467	Urban	Urban
IW361	11395	2.158	1838	1740	45.4	0.3547		45.4		45.4	B	0.804	0.970	0.733	0.876	Urban	Natural
IW362	6905	1.308	1750	1680	53.5	0.1794		53.5		53.5	B	0.525	0.597	0.488	0.550	Natural	Natural
IW363	9983	1.891	1940	1824	61.4	0.6278		61.4		61.4	B	0.563	0.424	0.529	0.396	Natural	Natural
IW364	13961	2.644	1836	1680	59.0	1.1451		59.0		59.0	B	0.683	0.489	0.667	0.475	Natural	Natural
IW365	14686	2.781	2110	1892	78.4	0.4729		78.4		78.4	C	1.113	1.447	1.000	1.285	Natural	Natural

<sup>a</sup> Adjusted length used for Tc calculation

<sup>b</sup> Adjusted slope used

<sup>c</sup> S-Graphs used ( \*\*\*\*\* used to identify S-Graph basins )

<sup>d</sup> Adjusted length and slope used

<sup>e</sup> Roughness classes determined based on Hydrology Manual definitions

Table HY-D.2.3 Time of Concentration



CLIENT: FCDMC

JOB NO. 310.032

JOB: Wittmann Area Drainage Master Study Update

Basin	TOC Length	TOC Length	US Elev	DS Elev	Slope	Area	Adjusted Length	TOC adjusted slope	basin adjusted slope	Adjusted Slope	Resistance Coefficient Class $K_b^e$	24-hr		6-hr		Existing Time-Area Relation	Future Time-Area Relation
												Tc	R	Tc	R		
	[ft]	[mi]	[ft]	[ft]	[ft/mi]	[mi <sup>2</sup> ]	[mi]	[ft/mi]	[ft/mi]	[ft/mi]	[A, B, C, D]	[hr]	[hr]	[hr]	[hr]		
IW366	14409	2.729	2012	1864	54.2	1.0558		54.2		54.2	B	0.796	0.622	0.763	0.593	Natural	Natural
IW367	2549	0.483	1924	1880	91.2	0.0512		91.2		91.2	C	0.371	0.374	0.338	0.337	Natural	Natural
IW368	7212	1.366	1980	1872	79.1	0.2907		79.1		79.1	C	0.833	0.784	0.754	0.702	Natural	Natural
IW369	17914	3.393	2130	1882	73.1	0.7949		73.1		73.1	C	1.271	1.462	1.175	1.340	Natural	Natural
IW370	20797	3.939	2150	1852	75.7	0.8795		75.7		75.7	B	0.967	1.148	0.917	1.082	Natural	Natural
IW371	2857	0.541	2002	1964	70.2	0.0529		70.2		70.2	B	0.308	0.327	0.275	0.288	Urban	Urban
IW372	7532	1.427	1930	1818	78.5	0.3175		78.5		78.5	B	0.479	0.418	0.442	0.382	Natural	Natural
IW374	7657	1.450	2056	1958	67.6	0.3875		67.6		67.6	B	0.475	0.374	0.442	0.345	Natural	Natural
IW375	18208	3.448	2052	1806	71.3	0.6582		71.3		71.3	C	1.392	1.824	1.296	1.685	Natural	Natural
IW377	5891	1.116	2110	2022	78.9	0.1126		78.9		78.9	B	0.442	0.566	0.408	0.519	Natural	Natural
IW380	13534	2.563	2340	2072	104.6	0.4998		104.6		104.6	B	0.575	0.631	0.533	0.581	Natural	Natural
IW381	12702	2.406	2086	1940	60.7	0.3841		60.7		60.7	B	0.704	0.873	0.650	0.799	Natural	Natural
IW382	11526	2.183	2060	1936	56.8	0.6196		56.8		56.8	B	0.688	0.599	0.642	0.555	Urban	Urban
IW384	3456	0.654	1976	1936	61.1	0.0826		61.1		61.1	B	0.338	0.327	0.308	0.296	Natural	Natural
IW386	9528	1.805	2160	2040	66.5	0.3732		66.5		66.5	C	0.917	0.945	0.838	0.855	Natural	Natural
IW387	13341	2.527	2440	2180	102.9	1.3509		102.9		102.9	C	0.921	0.597	0.892	0.576	Natural	Natural
IW388	15575	2.950	2100	1920	61.0	1.2257		61.0		61.0	C	1.200	0.958	1.133	0.900	Natural	Natural
IW389	6318	1.197	2160	2054	88.6	0.1312		88.6		88.6	C	0.638	0.825	0.588	0.753	Natural	Natural
IW390	10762	2.038	2210	2040	83.4	0.3110		83.4		83.4	C	1.021	1.302	0.929	1.173	Natural	Natural
IW390A	15477	2.931	2460	2155	104.1	0.9003		104.1		104.1	C	1.033	0.963	0.971	0.899	Natural	Natural
<sup>b</sup> IW392	10410	1.972	2520	2038	244.5	1.1135		244.5	233.0	233.0	C	0.500	0.278	0.504	0.280	Natural	Natural
<sup>b</sup> IW394	27057	5.124	3380	2260	218.6	4.1731		218.6	212.0	212.0	D	1.367	0.857	1.367	0.857	Natural	Natural

<sup>a</sup> Adjusted length used for Tc calculation

<sup>b</sup> Adjusted slope used

<sup>c</sup> S-Graphs used ( \*\*\*\* used to identify S-Graph basins )

<sup>d</sup> Adjusted length and slope used

<sup>e</sup> Roughness classes determined based on Hydrology Manual definitions



### **D.3 Hydrograph Routing Data**

(Information obtained from July 2005 Wittmann ADMSU Hydrology Final Addendum prepared by Entellus)



## Appendix D.3

BY \_\_\_\_\_ DATE \_\_\_\_\_

CHECK \_\_\_\_\_ DATE \_\_\_\_\_

JOB NO. 310.032CLIENT: FCDMCJOB: Wittmann Area Drainage Master Study UpdateStorm: 100-yr. 6-hr Existing Conditions **Route N-Step vs Velocity**

Route (Model Order)	Previous Time to Peak [hr]	Route Time to Peak [hr]	Difference in Time to peak [hr]	Route Length [ft]	HEC-1 Velocity [fps]	Normal Depth Velocity [fps]	HEC-1 Flow [cfs]	Normal Depth Flow [cfs]	Typical Cross Section	Route Slope [ft/ft]	Manning's "n" Value	HEC-1 N-step
RWT140	5.17	5.42	0.25	13053	14.50	6.45	1849	1857	WTTS20_1	0.019	0.035	3
RWT130	5.25	5.83	0.58	5750	2.75	4.18	2498	2507	Channel	0.001	0.035	5
RWT160	4.58	6.25	1.67	18961	3.15	2.55	501	509	WTTS10_1	0.012	0.035	12
R150A	5.08	5.25	0.17	9557	15.62	9.29	2582	2590	WTTS20_2	0.021	0.035	3
RWT150	5.17	5.58	0.41	16082	10.90	6.22	2752	2758	WTTS20_1	0.013	0.035	5
RWT120	5.67	6.17	0.50	6168	3.43	5.13	5198	5206	Channel	0.001	0.035	7
RWT110	6.17	6.83	0.66	7640	3.22	5.06	4924	4932	Channel	0.001	0.035	5
RWT100	6.75	7.17	0.42	4778	3.16	4.97	4614	4623	Channel	0.001	0.035	1
RTW406	4.08	8.42	4.34	16567	1.06	0.56	40	31	TSR408	0.007	0.04	6
RTW408	4.83	7.75	2.92	11703	1.11	1.49	298	306	TWTS50	0.005	0.04	10
RTW404	7.17	7.50	0.33	14969	12.60	3.48	4499	4500	TSR408	0.008	0.035	2
RIW371	4.08	6.50	2.42	20437	2.35	0.39	15	5	TSR371	0.008	0.04	15
RIW366	4.58	5.00	0.42	6797	4.50	4.87	957	966	TSR375	0.009	0.04	4
RIW377	4.50	5.25	0.75	4964	1.84	1.06	98	89	TSR371	0.010	0.04	6
RD371	4.08	4.33	0.25	1828	2.03	0.96	40	30	WITS130	0.003	0.035	1
RIW374	4.25	5.92	1.67	6588	1.10	1.64	404	411	TSR371	0.010	0.04	15
RIW386	4.58	5.42	0.84	7726	2.55	3.13	301	310	IWTS20_2	0.009	0.035	6
RIW382	4.33	4.83	0.50	10311	5.73	2.31	253	262	IWTS20_1	0.008	0.04	3
RD382A	4.33	4.42	0.09	2942	9.08	2.38	196	205	WITS130	0.009	0.035	1
RIW384	4.33	4.67	0.34	1005	0.82	2.42	310	319	TWTS20	0.014	0.04	3
RIW381	4.50	4.83	0.33	4694	3.95	1.97	574	581	TSR371	0.012	0.04	3
R359*	4.83	5.17	0.34	9280	7.58	5.48	937	946	IWTS30_2	0.010	0.04	4
RIW359 **	5.00	5.33	0.33	17152	14.44	5.93	3233	3241	IWTS30_1	0.006	0.04	4
RIW389	4.42	5.75	1.33	7697	1.61	3.05	102	92	IWTS10_1	0.013	0.04	6
RIW394	5.08	5.25	0.17	6512	10.64	6.00	2613	2620	TWTS10	0.014	0.035	2
R390A	5.25	5.42	0.17	8577	14.01	8.04	2925	2933	IWTS10_1	0.013	0.035	2

+ Time lapse less than time step of 5 minutes, so 1 time step was assumed for HEC-1 Velocity Calculation

\*\* Modified Typical X-Section

x Modified Time To Peak Used



Appendix D.3

BY \_\_\_\_\_ DATE \_\_\_\_\_

CHECK \_\_\_\_\_ DATE \_\_\_\_\_

JOB NO. 310.032

CLIENT: FCDMC

JOB: Wittmann Area Drainage Master Study Update

Storm: 100-yr, 6-hr Existing Conditions      **Route N-Step vs Velocity**

Route (Model Order)	Previous Time to Peak [hr]	Route Time to Peak [hr]	Difference in Time to peak [hr]	Route Length [ft]	HEC-1 Velocity [fps]	Normal Depth Velocity [fps]	HEC-1 Flow [cfs]	Normal Depth Flow [cfs]	Typical Cross Section	Route Slope [ft/ft]	Manning's "n" Value	HEC-1 N-step
RIW390	5.50	5.75	0.25	13324	14.80	6.62	2969	2975	TWTS10	0.017	0.035	3
RIW388	5.67	5.92	0.25	11626	12.92	6.06	3450	3456	IWTS20_1	0.010	0.04	2
RIW357 **	5.92	6.42	0.50	2593	1.44	3.36	2608	2614	TWTS20	0.008	0.04	9
RIW353	5.50	5.75	0.25	2643	2.94	3.06	5328	5326	TSR371	0.009	0.04	2
RIW312	4.75	5.83	1.08	17570	4.52	2.06	238	248	TSR294	0.005	0.04	1
RD312	4.75	4.92	0.17	4244	6.93	2.26	223	232	TWTS20	0.011	0.035	1
RIW365	4.83	5.42	0.59	11648	5.48	3.58	298	307	TSR375	0.010	0.04	5
RD365	4.83	5.00	0.17	1927	3.15	2.13	268	277	WITS130	0.005	0.035	2
RIW369	5.00	5.33	0.33	11583	9.75	1.64	463	471	TSR371	0.009	0.04	4
RD357	5.92	6.00	0.08	3820	13.26	5.03	930	939	IWTS30_1	0.009	0.04	2
RIW363	5.50	6.08	0.58	7351	3.52	5.33	1281	1290	IWTS30_1	0.009	0.04	4
RD369	4.92	5.17	0.25	1409	1.57	1.41	344	353	WITS130	0.001	0.035	1
RIW368	5.08	5.67	0.59	7248	3.41	2.29	438	447	TWTS20	0.010	0.04	6
RIW360	4.33	4.42	0.09	11127	34.34	4.89	751	760	IWTS30_1	0.008	0.035	1
RIW397	4.92	5.42	0.50	2780	1.54	3.99	3662	3668	PDTS40_1	0.006	0.035	8
RIW395	5.25	5.50	0.25	21294	23.66	9.49	7558	7561	TWTS10	0.021	0.035	3
RIW387 +	4.67	4.67	0.08	11459	38.20	2.71	1332	1338	TSR371	0.012	0.035	1
RIW396	5.42	5.58	0.16	5924	10.28	4.75	7911	7917	PDTS40_1	0.006	0.035	1
RIW380 **	5.58	6.67	1.09	4964	1.27	3.87	5586	5582	TSR371	0.012	0.035	11
RIW370 **	6.58	7.17	0.59	15367	7.23	9.16	5504	5511	TSR375	0.010	0.035	3
RIW349	7.17	7.25	0.08	7496	26.03	3.39	2088	2094	IWTS40_1	0.005	0.035	1
RD349 **	7.17	7.42	0.25	6586	7.32	7.04	3318	3326	IWTS30_1	0.008	0.035	1
RIW356 **	7.33	7.42	0.09	13031	40.22	8.57	5405	5413	IWTS30_2	0.006	0.04	1
RIW354 **	7.42	7.75	0.33	16693	14.05	7.27	5317	5324	IWTS30_1	0.008	0.04	2
RIW372	4.33	5.50	1.17	13330	3.16	2.84	153	162	TSR375	0.010	0.04	9
RD372	4.33	4.50	0.17	1142	1.87	1.44	203	212	TWTS20	0.004	0.035	1

+ Time lapse less than time step of 5 minutes, so 1 time step was assumed for HEC-1 Velocity Calculation

\*\* Modified Typical X-Section

x Modified Time To Peak Used



## Appendix D.3

BY \_\_\_\_\_ DATE \_\_\_\_\_

CHECK \_\_\_\_\_ DATE \_\_\_\_\_

CLIENT: FCDMCJOB NO. 310.032JOB: Wittmann Area Drainage Master Study UpdateStorm: 100-yr, 6-hr Existing Conditions      **Route N-Step vs Velocity**

Route (Model Order)	Previous Time to Peak [hr]	Route Time to Peak [hr]	Difference in Time to peak [hr]	Route Length [ft]	HEC-1 Velocity [fps]	Normal Depth Velocity [fps]	HEC-1 Flow [cfs]	Normal Depth Flow [cfs]	Typical Cross Section	Route Slope [ft/ft]	Manning's "n" Value	HEC-1 N-step
RIW375	4.92	5.58	0.66	16914	7.12	2.53	413	421	TWTS20	0.013	0.04	8
RIW364 **	7.67	8.33	0.66	9657	4.06	9.01	5302	5309	IWTS30_2	0.007	0.04	7
RD363	5.50	5.75	0.25	1700	1.89	3.10	598	606	WITS130	0.011	0.04	1
RIW351	5.42	8.25	2.83	4523	0.44	0.90	80	71	TSR371	0.007	0.04	10
RIW346	8.25	8.50	0.25	24311	27.01	9.23	5250	5258	IWTS30_2	0.008	0.04	2
RIW338	8.50	8.92	0.42	4331	2.86	5.20	5177	5177	TWTS20	0.016	0.04	5
RIW326	4.33	4.92	0.59	21501	10.12	2.40	355	364	TWTS40	0.007	0.04	5
RIW334	9.00	9.83	0.83	3706	1.24	4.28	5042	5048	TWTS30	0.005	0.04	12
RD351	5.42	5.75	0.33	2791	2.35	4.22	595	604	IWTS30_1	0.008	0.04	3
RIW361 **	5.92	6.58	0.66	10402	4.38	2.21	1870	1872	TSR371	0.007	0.04	12
R350*	5.92	6.25	0.33	8310	6.99	4.66	6886	6888	IWTS40_1	0.006	0.04	3
RIW330	6.17	6.33	0.16	7227	12.55	4.95	6885	6888	TWTS30	0.006	0.04	1
RIW322	6.33	7.00	0.67	2771	1.15	3.89	7151	7152	IWTS40_2	0.005	0.04	12
RIW318	4.67	6.33	1.66	8066	1.35	2.33	415	424	TWTS30	0.006	0.04	15
RIW310	4.92	5.00	0.08	20068	69.68	2.36	880	888	IWTS50	0.005	0.04	1
RIW314 **	7.42	8.50	1.08	12886	3.31	3.90	6625	6627	IWTS40_2	0.005	0.04	15
RIW300	8.50	8.83	0.33	860	0.72	2.76	6532	6535	IWTS50	0.002	0.04	3
RIW302	9.33	9.67	0.34	8292	6.77	9.70	9794	9800	TSR375	0.010	0.04	3
R264A	5.08	5.50	0.42	4042	2.67	2.26	1691	1699	TSR264A	0.003	0.04	1
RSV264	5.25	5.75	0.50	2946	1.64	2.53	2353	2362	SVTS50_1	0.001	0.04	1
RSV298	5.00	6.25	1.25	6826	1.52	3.85	591	601	Channel	0.004	0.04	14
RSV286	4.75	5.00	0.25	13500	15.00	3.48	1299	1308	TSR294	0.006	0.04	1
RSV284	4.92	5.58	0.66	3544	1.49	3.36	1213	1221	TSR294	0.006	0.04	4
RSV290	4.58	5.67	1.09	14316	3.65	2.79	543	552	TSR294	0.006	0.04	5
RSV272	5.58	5.67	0.09	1016	3.14	3.65	1949	1957	SVTS50_1	0.004	0.04	1
RSV294	5.00	5.75	0.75	15129	5.60	3.71	1815	1822	TSR294	0.006	0.04	5

+ Time lapse less than time step of 5 minutes, so 1 time step was assumed for HEC-1 Velocity Calculation

\*\* Modified Typical X-Section

x Modified Time To Peak Used



BY \_\_\_\_\_ DATE \_\_\_\_\_

CHECK \_\_\_\_\_ DATE \_\_\_\_\_

CLIENT: FCDMC

JOB NO. 310.032

JOB: Wittmann Area Drainage Master Study Update

Route ID	Mannings' n-values		length [ft]	length [miles]	US elev [ft]	DS elev [ft]	slope [ft/ft]	Typical Cross
	Overbank	Channel						Section ID
RD678	0.043	0.035	6108.0	1.16	1574	1550.0	0.0039	WITS110_2
RD744	0.043	0.035	3177.0	0.60	1338	1320.0	0.0057	TSR606
RD800	0.043	0.035	5888.0	1.12	1346	1311.0	0.0059	PDTS40_1
RDCP07	0.043	0.035	6956.4	1.32	1552	1494.0	0.0083	WITS120
RDCP08	0.043	0.035	4229.1	0.80	1556	1508.0	0.0114	WITS120
RDCP09	0.043	0.035	14617.9	2.77	1558	1426.0	0.0090	WITS120
RDCP10	0.043	0.035	6701.3	1.27	1550	1496.0	0.0081	WITS120
RDCP11	0.043	0.035	6376.9	1.21	1550	1490.0	0.0094	PITS10_2
RDCP12	0.043	0.035	6159.0	1.17	1548.4	1490.7	0.0094	PITS30
RDCP13	0.043	0.035	11034.4	2.09	1544	1446.0	0.0089	PITS30
RDCP14	0.043	0.035	20385.8	3.86	1556	1380.0	0.0086	PITS30
RDCP15	0.043	0.035	9601.8	1.82	1556	1454.0	0.0106	PITS30
RDCP16	0.043	0.035	10753.8	2.04	1556	1436.0	0.0112	PDTS30
RDCP17	0.043	0.035	10399.9	1.97	1556	1436.0	0.0115	PDTS30
RDCP18	0.043	0.035	14927.0	2.83	1556	1406.0	0.0100	PDTS30
RDCP19	0.043	0.035	5902.3	1.12	1552	1464.0	0.0149	PDTS30
RDCPOT	0.040	0.04	8638.0	1.64	1538	1474.0	0.0074	WITS70
RIW300	0.040	0.04	6125.0	1.16	1445.1	1418.8	0.0043	TWTS50
RIW302	0.040	0.04	5720.1	1.08	1443.9	1418.8	0.0044	TWTS50
RIW310	0.040	0.04	860.0	0.16	1548.7	1547.0	0.0020	IWTS50
RIW312	0.040	0.04	8292.4	1.57	1908.3	1828.3	0.0096	TSR375
RIW314	0.040	0.04	20067.8	3.80	1547	1445.1	0.0051	IWTS50
RIW318	0.040	0.04	17570.2	3.33	1634.9	1548.7	0.0049	TSR294
RIW322	0.040	0.04	12886.2	2.44	1616.3	1547.0	0.0054	IWTS40_2
RIW326	0.040	0.04	8065.9	1.53	1594.9	1548.7	0.0057	TWTS30
RIW330	0.040	0.04	2770.8	0.52	1630.4	1616.3	0.0051	IWTS40_2
RIW334	0.040	0.04	21501.4	4.07	1588.6	1443.9	0.0067	TWTS40
RIW338	0.040	0.04	7227.0	1.37	1588.6	1548.7	0.0055	TWTS30
RIW346	0.040	0.04	3705.8	0.70	1605.5	1588.6	0.0046	TWTS30
RIW349	0.043	0.035	4331.4	0.82	1782.7	1712.9	0.0161	TWTS20
RIW351	0.040	0.04	24311.3	4.60	1791.5	1606.6	0.0076	IWTS30_2
RIW353	0.040	0.04	7495.9	1.42	1720	1680.0	0.0053	IWTS40_1
RIW354	0.040	0.04	4522.5	0.86	1712.9	1680.3	0.0072	TSR371
RIW356	0.040	0.04	2643.0	0.50	1735.5	1712.9	0.0086	TSR371
RIW357	0.040	0.04	16692.7	3.16	1848	1716.8	0.0079	IWTS30_1
RIW359	0.040	0.04	13031.3	2.47	1793.4	1716.8	0.0059	IWTS30_2
RIW360	0.043	0.035	2593.3	0.49	1756	1735.5	0.0079	TWTS20



BY \_\_\_\_\_ DATE \_\_\_\_\_

CHECK \_\_\_\_\_ DATE \_\_\_\_\_

CLIENT: FCDMC

JOB NO. 310.032

JOB: Wittmann Area Drainage Master Study Update

Route ID	Mannings' n-values		length [ft]	length [miles]	US elev [ft]	DS elev [ft]	slope [ft/ft]	Typical Cross Section ID
	Overbank	Channel						
RIW361	0.040	0.04	17152.4	3.25	1740	1630.0	0.0064	IWTS30_1
RIW363	0.040	0.04	11126.8	2.11	1826	1740.0	0.0077	IWTS30_1
RIW364	0.040	0.04	10401.5	1.97	1682.4	1605.5	0.0074	TSR371
RIW365	0.040	0.04	7350.8	1.39	1893.6	1828.3	0.0089	IWTS30_1
RIW366	0.040	0.04	9656.7	1.83	1864	1796.0	0.0070	IWTS30_2
RIW368	0.040	0.04	11647.7	2.21	1875.5	1756.0	0.0103	TSR375
RIW369	0.040	0.04	6796.8	1.29	1887.1	1828.3	0.0087	TSR375
RIW370	0.043	0.035	7247.6	1.37	1851.8	1782.7	0.0095	TWTS20
RIW371	0.040	0.04	11583.3	2.19	1965.7	1864.0	0.0088	TSR371
RIW372	0.040	0.04	15366.7	2.91	1828	1680.0	0.0096	TSR375
RIW374	0.040	0.04	20436.7	3.87	1962	1796.0	0.0081	TSR371
RIW375	0.040	0.04	13330.0	2.52	1814	1681.5	0.0099	TSR375
RIW377	0.040	0.04	6588.4	1.25	2025.8	1962.6	0.0096	TSR371
RIW380	0.043	0.035	16914.1	3.20	2072.5	1851.8	0.0130	TWTS20
RIW381	0.040	0.04	4964.3	0.94	1939.5	1892.0	0.0096	TSR371
RIW382	0.040	0.04	4964.3	0.94	1947.8	1888.0	0.0120	TSR371
RIW384	0.040	0.04	4694.3	0.89	1940	1882.0	0.0124	TSR371
RIW386	0.043	0.035	10311.1	1.95	2027.8	1947.8	0.0078	IWTS20_1
RIW387	0.043	0.035	1005.1	0.19	2179.3	2165.2	0.0140	TWTS20
RIW388	0.040	0.04	7725.8	1.46	1920.8	1848.0	0.0094	IWTS20_2
RIW389	0.040	0.04	11459.2	2.17	2063.1	1920.8	0.0124	TSR371
RIW390	0.043	0.035	11625.6	2.20	2038.3	1920.8	0.0101	IWTS20_1
RIW394	0.043	0.035	7697.0	1.46	2257	2155.0	0.0133	IWTS10_1
RIW395	0.043	0.035	13324.0	2.52	2389.6	2165.2	0.0168	TWTS10
RIW396	0.043	0.035	6512.2	1.23	2165.2	2072.5	0.0142	TWTS10
RIW397	0.043	0.035	21294.0	4.03	2830	2390.0	0.0207	TWTS10
RPD704	0.043	0.035	5924.4	1.12	1334.9	1302.6	0.0055	PDTS40_1
RPD708	0.043	0.035	2780.0	0.53	1351	1334.9	0.0058	PDTS40_1
RD716	0.043	0.035	4300.0	0.81	1341.9	1302.6	0.0091	PDTS40_1
RPD720	0.045	0.035	1840.0	0.35	1320.9	1302.6	0.0099	Channel
RPD726	0.043	0.035	4959.0	0.94	1342	1320.0	0.0044	PDTS40_1
RPD732	0.043	0.035	10623.0	2.01	1464	1396.0	0.0064	PDTS30
RPD736	0.043	0.035	1894.0	0.36	1406	1396.0	0.0053	PDTS30
RPD740	0.043	0.035	5434.0	1.03	1436	1402.0	0.0063	PDTS30
RPD748	0.043	0.035	6476.1	1.23	1524	1430.0	0.0145	PDTS40_2
RPD756	0.043	0.035	8775.3	1.66	1540	1430.0	0.0125	PDTS40_2
RPD760	0.043	0.035	5148.6	0.98	1516	1464.0	0.0101	PDTS30



**D.4 Reservoir Routing Data**

(Not Applicable to This Study)



**D.5 Flow Splits and Diversion Data**

(Not Applicable to This Study)



Wittmann Phase IV  
Floodplain Delineation Study

**D.6 Hydrologic Calculations**

(Information obtained from July 2005 Wittmann ADMSU Hydrology Final Addendum prepared by Entellus)

Table D.6: Summary of Peak Discharges

Model ID	100-yr 24-hr Storm			100-yr 6-hr Storm			Controlling Storm			
	Drainage Area [mi <sup>2</sup> ]	Exst. Cond.	Fut. Cond.	Drainage Area [mi <sup>2</sup> ]	Exst. Cond.	Fut. Cond.	Exst. Cond.		Fut. Cond.	
		Q [cfs]	Q [cfs]		Q [cfs]	Q [cfs]	Q [cfs]	Storm	Q [cfs]	Storm
C576B	1.69	687	619	1.69	919	837	6-hour	919	6-hour	837
C580A	3.13	2,218	1,878	3.13	2,424	2,129	6-hour	2,424	6-hour	2,129
C600A	8.62	1,288	1,493	9.67	1,221	1,410	24-hour	1,288	24-hour	1,493
C606A	4.08	363	375	3.55	368	396	6-hour	368	6-hour	396
C624A	6.92	2,060	2,199	7.56	1,746	2,115	24-hour	2,060	24-hour	2,199
C700*	302.88	6,286	7,625	302.88	4,027	5,534	24-hour	6,286	24-hour	7,625
C708*	3.70	1,563	1,750	3.70	1,685	2,010	6-hour	1,685	6-hour	2,010
C726*	41.19	3,037	3,389	36.10	2,672	3,211	24-hour	3,037	24-hour	3,389
C726A	21.41	1,812	1,971	18.85	1,657	1,909	24-hour	1,812	24-hour	1,971
C726B	19.77	1,298	1,654	17.24	1,229	1,695	24-hour	1,298	6-hour	1,695
C802*	313.51	7,038	8,537	313.51	4,057	5,638	24-hour	7,038	24-hour	8,537
CAF807	4.49	2,508	2,600	4.49	2,926	2,905	6-hour	2,926	6-hour	2,905
CAF810	320.12	7,163	8,766	320.12	4,048	5,631	24-hour	7,163	24-hour	8,766
CAF820	320.81	7,160	8,755	320.81	4,038	5,615	24-hour	7,160	24-hour	8,755
CAF850	0.72	1,140	1,184	0.72	1,545	1,596	6-hour	1,545	6-hour	1,596
CAF852	0.51	812	817	0.51	1,174	1,179	6-hour	1,174	6-hour	1,179
CAF860	1.08	860	914	1.08	1,151	1,226	6-hour	1,151	6-hour	1,226
CAF862	0.26	373	364	0.26	555	543	6-hour	555	6-hour	543
CAP1*	53.75	14,390	15,575	53.76	13,205	14,655	24-hour	14,390	24-hour	15,575
CAP2*	39.36	8,461	10,018	33.74	6,672	8,797	24-hour	8,461	24-hour	10,018
CIW300	30.57	6,941	7,056	30.55	6,611	6,819	24-hour	6,941	24-hour	7,056
CIW302	24.18	5,078	5,239	24.24	5,042	5,220	24-hour	5,078	24-hour	5,239
CIW310	1.75	708	686	1.75	885	891	6-hour	885	6-hour	891
CIW314	29.09	7,496	7,416	29.07	7,566	7,448	6-hour	7,566	6-hour	7,448
CIW322	25.16	7,021	6,791	25.14	7,314	7,020	6-hour	7,314	6-hour	7,020
CIW330	23.27	6,608	6,392	23.25	6,979	6,677	6-hour	6,979	6-hour	6,677
CIW334	22.85	5,156	5,266	22.91	5,175	5,351	6-hour	5,175	6-hour	5,351
CIW338	21.26	5,213	5,289	21.32	5,248	5,425	6-hour	5,248	6-hour	5,425
CIW342	17.25	5,157	5,226	17.30	5,302	5,474	6-hour	5,302	6-hour	5,474
CIW346	20.92	5,247	5,319	20.98	5,297	5,470	6-hour	5,297	6-hour	5,470
CIW349	12.30	5,399	5,508	12.27	5,506	5,721	6-hour	5,506	6-hour	5,721
CIW350	17.41	5,647	5,297	17.45	6,037	5,645	6-hour	6,037	6-hour	5,645
CIW351	1.71	559	568	1.67	697	711	6-hour	697	6-hour	711
CIW352	6.31	2,192	2,204	6.39	2,291	2,355	6-hour	2,291	6-hour	2,355
CIW353	15.24	5,023	4,662	15.28	5,494	5,051	6-hour	5,494	6-hour	5,051
CIW354	14.59	5,244	5,334	14.64	5,405	5,595	6-hour	5,405	6-hour	5,595
CIW356	14.23	5,308	5,397	14.28	5,452	5,643	6-hour	5,452	6-hour	5,643

\* Drainage areas may differ between the 24-hour and 6-hour storms: the HEC-1 hard coding was performed separately for the 24-hour and 6-hour storms.

Table D.6: Summary of Peak Discharges

Model ID	100-yr 24-hr Storm			100-yr 6-hr Storm			Controlling Storm			
	Drainage Area [mi <sup>2</sup> ]	Exst. Cond. Q [cfs]	Fut. Cond. Q [cfs]	Drainage Area [mi <sup>2</sup> ]	Exst. Cond. Q [cfs]	Fut. Cond. Q [cfs]	Exst. Cond.		Fut. Cond.	
							Storm	Q [cfs]	Storm	Q [cfs]
CIW357	8.60	3,504	3,311	8.60	3,583	3,404	6-hour	3,583	6-hour	3,404
CIW359	5.97	3,105	2,625	5.97	3,423	3,009	6-hour	3,423	6-hour	3,009
CIW360	1.54	666	668	1.62	771	775	6-hour	771	6-hour	775
CIW361	4.82	1,600	1,588	4.76	1,886	1,854	6-hour	1,886	6-hour	1,854
CIW362	14.77	5,141	5,227	14.82	5,316	5,506	6-hour	5,316	6-hour	5,506
CIW363	4.38	1,640	1,570	4.26	1,903	1,805	6-hour	1,903	6-hour	1,805
CIW364	2.13	1,337	1,273	2.13	1,456	1,433	6-hour	1,456	6-hour	1,433
CIW365	0.63	353	330	0.74	583	557	6-hour	583	6-hour	557
CIW366	1.08	881	785	1.07	1,105	989	6-hour	1,105	6-hour	989
CIW367	0.17	80	75	0.43	320	301	6-hour	320	6-hour	301
CIW368	0.71	296	301	0.79	479	498	6-hour	479	6-hour	498
CIW370	12.05	5,663	5,761	12.02	5,750	5,961	6-hour	5,750	6-hour	5,961
CIW374	0.53	558	526	0.54	784	757	6-hour	784	6-hour	757
CIW375	0.82	306	329	0.85	446	493	6-hour	446	6-hour	493
CIW380	15.11	7,968	8,319	15.11	7,995	8,664	6-hour	7,995	6-hour	8,664
CIW381	0.72	493	477	0.64	619	599	6-hour	619	6-hour	599
CIW382	0.99	525	539	0.99	685	708	6-hour	685	6-hour	708
CIW384	0.23	164	157	0.41	390	394	6-hour	390	6-hour	394
CIW388	7.85	3,436	3,268	7.85	3,532	3,396	6-hour	3,532	6-hour	3,396
CIW390	6.50	3,038	2,945	6.50	3,093	3,091	6-hour	3,093	6-hour	3,091
CIW395	12.68	7,572	8,032	12.68	7,766	8,463	6-hour	7,766	6-hour	8,463
CIW396	14.61	7,924	8,342	14.61	8,001	8,699	6-hour	8,001	6-hour	8,699
CPD700	313.12	7,038	8,525	313.12	4,058	5,638	24-hour	7,038	24-hour	8,525
CPD704	8.84	3,060	3,449	8.84	3,265	3,804	6-hour	3,265	6-hour	3,804
CPD708	7.76	2,728	3,185	7.76	2,957	3,529	6-hour	2,957	6-hour	3,529
CPD720	293.30	5,843	7,004	296.13	4,030	5,174	24-hour	5,843	24-hour	7,004
CPD726	44.29	2,998	3,378	39.20	2,581	3,146	24-hour	2,998	24-hour	3,378
CPD732	15.04	1,200	1,349	13.11	1,085	1,319	24-hour	1,200	24-hour	1,349
CPD736	5.27	451	476	4.64	441	489	24-hour	451	6-hour	489
CPD740	18.84	1,273	1,652	16.31	1,249	1,698	24-hour	1,273	6-hour	1,698
CPD748	0.47	605	614	0.47	910	914	6-hour	910	6-hour	914
CPI600	258.01	4,434	4,798	263.07	2,593	3,266	24-hour	4,434	24-hour	4,798
CPI603	15.46	2,450	2,713	15.98	2,200	2,506	24-hour	2,450	24-hour	2,713
CPI604	2.03	321	382	1.82	469	496	6-hour	469	6-hour	496
CPI606	3.49	701	785	3.17	753	881	6-hour	753	6-hour	881
CPI609	8.10	1,197	1,393	9.15	1,118	1,300	24-hour	1,197	24-hour	1,393
CPI612	12.43	1,406	1,586	14.40	1,278	1,425	24-hour	1,406	24-hour	1,586

\* Drainage areas may differ between the 24-hour and 6-hour storms: the HEC-1 hard coding was performed separately for the 24-hour and 6-hour storms.

Wittmann Phase IV  
Floodplain Delineation Study

**100-Year 6-Hour Storm HEC-1 Model**

**(Excerpts from the Wittmann ADSMU model)**

1\*\*\*\*\*
\*
\* FLOOD HYDROGRAPH PACKAGE (HEC-1)
\* JUN 1998
\* VERSION 4.1
\* RUN DATE 03AUG11 TIME 17:11:23
\*\*\*\*\*

\*\*\*\*\*
\*
\* U.S. ARMY CORPS OF ENGINEERS
\* HYDROLOGIC ENGINEERING CENTER
\* 609 SECOND STREET
\* DAVIS, CALIFORNIA 95616
\* (916) 756-1104
\*\*\*\*\*

X X XXXXXXX XXXXX X
X X X X X XX
X X X X X X
XXXXXXXX XXXX X XXXXX X
X X X X X X
X X X X X X
X X XXXXXXX XXXXX XXX

THIS PROGRAM REPLACES ALL PREVIOUS VERSIONS OF HEC-1 KNOWN AS HEC1 (JAN 73), HEC1GS, HEC1DB, AND HEC1KW.

THE DEFINITIONS OF VARIABLES -RTIMP- AND -RTIOR- HAVE CHANGED FROM THOSE USED WITH THE 1973-STYLE INPUT STRUCTURE.
THE DEFINITION OF -AMSKK- ON RM-CARD WAS CHANGED WITH REVISIONS DATED 28 SEP 81. THIS IS THE FORTRAN77 VERSION
NEW OPTIONS: DAMBREAK OUTFLOW SUBMERGENCE , SINGLE EVENT DAMAGE CALCULATION, DSS:WRITE STAGE FREQUENCY,
DSS:READ TIME SERIES AT DESIRED CALCULATION INTERVAL LOSS RATE:GREEN AND AMPT INFILTRATION
KINEMATIC WAVE: NEW FINITE DIFFERENCE ALGORITHM

1

HEC-1 INPUT

PAGE 1

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10
1 ID Wittmann Phase IV Floodplain Delineation Study
2 ID August 2011
3 ID Mod1 removes subbasin iw358 to determine flow at Iona Tributary 1 West
4 ID \*\*\*\*\*
5 ID \*\* -ADDENDUM-ADDENDUM-ADDENDUM-ADDENDUM-ADDENDUM-ADDENDUM-ADDENDUM-ADDENDUM- \*\*
6 ID \*\* -ADDENDUM-ADDENDUM-ADDENDUM-ADDENDUM-ADDENDUM-ADDENDUM-ADDENDUM-ADDENDUM- \*\*
7 ID \*\*\*\*\*
8 ID \*\*
9 ID \*\* ADDENDUM TO THE WITTMANN AREA DRAINAGE MASTER STUDY UPDATE HYDROLOGY \*\*
10 ID \*\*
11 ID \*\*\*\*\*
12 ID \*\* -ADDENDUM-ADDENDUM-ADDENDUM-ADDENDUM-ADDENDUM-ADDENDUM-ADDENDUM-ADDENDUM- \*\*
13 ID \*\* -ADDENDUM-ADDENDUM-ADDENDUM-ADDENDUM-ADDENDUM-ADDENDUM-ADDENDUM-ADDENDUM- \*\*
14 ID \*\*\*\*\*
15 ID \*\*
16 ID \*\* THE FOLLOWING CHANGES HAVE BEEN MADE TO THE WITTMANN HYDROLOGY MODELS \*\*
17 ID \*\* DATED OCTOBER 2004: \*\*
18 ID \*\*
19 ID \*\* -Diversions added after CTW480 \*\*
20 ID \*\* -Main flow (5%) to CTW478 \*\*
21 ID \*\* -Diverted flow (95%) to CWI584 \*\*
22 ID \*\* -Added concentration point CWI584 combining RD480 and WI584 \*\*
23 ID \*\* -Hard coding was added to CTW478 \*\*
24 ID \*\* -Route added after D480 (Diversions Recovery) \*\*
25 ID \*\* -Route RWI584 was adjusted \*\*
26 ID \*\* -Storage Route SCP050 was adjusted \*\*
27 ID \*\* -Route R508\* was adjusted \*\*
28 ID \*\* -Hard coding for CWI510 was adjusted \*\*
29 ID \*\*
30 ID \*\* -Diversions D618 was moved from after CPI618 to before RDCP12 \*\*
31 ID \*\* -Updated route RD618 information \*\*
32 ID \*\* -Hard coding on CPI618 was adjusted \*\*
33 ID \*\* -Hard coding on CPI615 was adjusted \*\*
34 ID \*\*
35 ID \*\* -Route RPI639 changed from following US60 to CPI636 to route \*\*
36 ID \*\* through US60/RR structures and combine with CWI506 \*\*
37 ID \*\* -Cards PI639, D524, RD524, CPI639 and RPI639 were changed from \*\*
38 ID \*\* following R624A to follow RWI524 \*\*
39 ID \*\* -CPI636 was removed-no longer needed to combine PI636 & RPI639 \*\*
40 ID \*\* -RPI639 information was changed to reflect new route \*\*
41 ID \*\* -CWI506 changed from combining 3 to combine 4: \*\*
42 ID \*\* RWI525, RWI524, RPI639 & WI506 \*\*
43 ID \*\* -Hard coding on CWI506 was adjusted \*\*
44 ID \*\* -Hard Coding on CWI500 was adjusted \*\*
45 ID \*\*
46 ID \*\*\*\*\*
47 ID \*\* -ADDENDUM-ADDENDUM-ADDENDUM-ADDENDUM-ADDENDUM-ADDENDUM-ADDENDUM-ADDENDUM- \*\*
48 ID \*\* -ADDENDUM-ADDENDUM-ADDENDUM-ADDENDUM-ADDENDUM-ADDENDUM-ADDENDUM-ADDENDUM- \*\*
49 ID \*\*\*\*\*
50 ID \*\*
51 ID \*\*\*\*\*
52 ID \*\*\*\*\*
53 ID \*\*\*\*\*
54 ID \*\*\*\*\*
55 ID \*\*\*\*\*

1

HEC-1 INPUT

PAGE 2

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10
56 ID \*\* WITTMANN AREA DRAINAGE MASTER STUDY UPDATE-HYDROLOGY MODEL 2004 \*\*
57 ID \*\*
58 ID \*\*\*\*\*
59 ID \*\*\*\*\*
60 ID \*\*
61 ID PROJECT: Wittmann ADMS Update
62 ID CLIENT: Flood Control District of Maricopa County
63 ID PREPARED BY: Entellus, Inc.
64 ID PROJECT No: FCD 2002C029 Entellus 310.032

```

65      ID      wtec6-addendum-mod1.out
66      ID      FILE NAME: WTEC6-addendum.hc1      CREATED DATE: JUL 01, 2004
67      ID      MODIFIED DATE: MAY 04, 2005
68      ID
69      ID      STORM:
70      ID      100-year 6-hour Storm
71      ID
72      ID      DEVELOPMENT CONDITIONS:
73      ID      Existing Conditions
74      ID
75      ID      MODELING ASSUMPTIONS:
76      ID      It was assumed that the US60 did not have adequate
77      ID      storage to cause any significant attenuation.
78      ID      The assumption was made that the CAP Canal embankment
79      ID      would not be breached under a large flood event.
80      ID
81      ID      The assumption was made that the Beardsley Canal would
82      ID      not fail under a large storm event (Per District
83      ID      Instruction). In addition, the berm north of the
84      ID      Beardsley canal and east of US60 was assumed to fail
85      ID      (per district Instruction).
86      ID
87      ID      For both the CAP and Beardsley Canals, once the berm
88      ID      elevation was reached weir flow was assumed. It was
89      ID      also assumed that any weir flow over the canal that
90      ID      might enter the canal and be diverted out of the study
91      ID      area was insignificant, and thus was ignored. In other
92      ID      words all weir flow over the canal embankment reaches
93      ID      the downstream concentration point.
94      ID
95      ID      Typical X-sects were developed, and it was assumed
96      ID      that a typical x-sect could adequately represent
97      ID      various reaches.
98      ID
99      ID      Time-Area Relations were used base on the District's
100     ID      Hydrology Manual criteria. Two Time-Area Relation
101     ID      Curves were utilized:
102     ID      -Urban
103     ID      -Natural
104     ID      The Time-Area Relation Curves were taken directly
105     ID      from the manual
106     ID
107     ID      MODELING METHODS:
108     ID      This model utilizes QI cards to input the Padelford
109     ID      hydrographs from the 100-year 6-hour existing
110     ID      conditions model developed by A-N west Inc. for the
111     ID      HEC-1 INPUT

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PAGE 3

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1
LINE      ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10
111     ID      Padelford wash Floodplain Delineation Study. The
112     ID      hydrograph was altered from its original form
113     ID      (2-minute interval to 5-minute interval) through
114     ID      simple interpolation.
115     ID
116     ID      Clark Unit Hydrographs were used for all subbasins
117     ID      except the two subbasins directly upstream of the
118     ID      Bonita Dam (PD726B and PD740 use S-graphs). The UC
119     ID      parameters were calculated using the WMS7.0 software.
120     ID
121     ID      For Basins PD726B and PD740 S-Graphs were utilized per
122     ID      the request of the FCDMC. Limited details regarding
123     ID      the calculations of the S-graphs can be found in the
124     ID      model by the basin KK card. For full details of the
125     ID      S-graph calculations refer to the Appendix.
126     ID
127     ID      Normal Depth routing was used for all routing reaches.
128     ID
129     ID      Hard coding was used to account for the percentage of
130     ID      area associated with a diversion. Because of the use
131     ID      of JD cards and areal reduction, hard coding was
132     ID      necessary to properly account for area. For a given
133     ID      diversion a percentage of the flow is routed to two
134     ID      different locations. The same percentage of area
135     ID      follows that diverted flow. In addition the area
136     ID      downstream of the main path is reduced or increased and
137     ID      is hard coded to account for the loss or gain of area.
138     ID      Hard coding was performed based on the 6-hour existing
139     ID      conditions model.
140     ID
141     ID      Stage-storage was developed for all the structures
142     ID      along the CAP Canal, as well as along the Beardsley
143     ID      Canal. In addition several stage-storage locations
144     ID      were developed for areas with significant storage
145     ID      along the SR74. No storage was modeled along the us60
146     ID      and railroad, but the culverts were analyzed for
147     ID      diversion potential. Diversions were placed in the
148     ID      model where deemed appropriate. See appendix for
149     ID      details.
150     ID
151     ID      FLO-2D was utilized to calculate the split flows at
152     ID      concentration points CIW351, CIW357, CIW363 and CWI576.
153     ID      See appendix for modeling details.
154     ID
155     ID      *****
156     ID      *****
157     ID
158     ID      *DIAGRAM
159     ID      IT      5      0      1500
160     ID      IO      5
161     ID      IN      15      0
162     ID      JD      3.40  0.01
163     ID      * 6-hour distribution
164     ID      PC      0.000  0.008  0.016  0.025  0.033  0.041  0.050  0.058  0.066  0.074
165     ID      PC      0.087  0.099  0.118  0.138  0.216  0.377  0.834  0.911  0.931  0.950
166     ID      HEC-1 INPUT

```

PAGE 4

wtec6-addendum-mod1.out

LINE	ID	1	2	3	4	5	6	7	8	9	10
164	PC	0.962	0.972	0.983	0.991	1.000					
165	JD	3.378	0.50								
166	JD	3.314	2.80								
167	PC	0.000	0.009	0.016	0.025	0.034	0.042	0.051	0.059	0.067	0.076
168	PC	0.087	0.100	0.120	0.163	0.252	0.451	0.694	0.837	0.900	0.938
169	PC	0.950	0.963	0.975	0.988	1.000					
170	JD	3.135	16.00								
171	PC	0.000	0.009	0.020	0.030	0.048	0.063	0.076	0.090	0.105	0.119
172	PC	0.135	0.152	0.175	0.222	0.304	0.472	0.670	0.796	0.868	0.912
173	PC	0.946	0.960	0.973	0.987	1.000					
174	JD	2.761	90.00								
175	PC	0.000	0.021	0.035	0.051	0.071	0.087	0.105	0.125	0.143	0.160
176	PC	0.179	0.201	0.232	0.281	0.364	0.500	0.658	0.773	0.841	0.888
177	PC	0.927	0.945	0.964	0.982	1.000					
178	JD	1.938	500.00								
179	PC	0.000	0.024	0.043	0.059	0.078	0.098	0.119	0.141	0.162	0.186
180	PC	0.212	0.239	0.271	0.321	0.408	0.515	0.627	0.735	0.814	0.864
181	PC	0.907	0.930	0.954	0.977	1.000					

\*\*\*\*\*  
 \*\*\*\*\*  
 \*\*\*\*\* BEGINNING OF IONA WASH SUBAREA \*\*\*\*\*  
 \*\*\*\*\*  
 \*\*\*\*\*  
 \*\*\*\*\* BEGINNING OF IONA WASH REGION 1 \*\*\*\*\*  
 \*\*\*\*\*  
 \*\*\*\*\*

182	KK	Iw371									
183	KO	0	0	0.0	1	22					
184	BA	0.0529									
185	LG	0.275	0.25	4.15	0.473	26.25					
186	UC	0.275	0.288								
		* Urban Time-Area Relation									
187	UA	0	5	16	30	65	77	84	90	94	97
188	UA	100									

\* Railroad Diversion: Main path to CIW366, Diversion to CIW374  
 \* From Structure RR40, See Appendix D.5 for analysis  
 HEC-1 INPUT

PAGE 5

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

189	KK	D371									
190	KO	0	0	0.0	0	22					
191	DT	D0371									
192	DI	0.0	14	79	208	383	823	1374	2014	2724	
193	DQ	0.0	0	55	173	335	763	1302	1930	2632	

\* ROUTE UPDATED 4/14/4

194	KK	RIW371									
195	KO	0	0	0.0	0	22					
196	RS	15	FLOW	0.0	0.0						
197	RC	0.04	0.04	0.04	11583	0.0088					
		* TSR371									
198	RX	0	142	486	525	651	770	1000	1518		
199	RY	1900	1900	1899	1897	1896	1899	1899.5	1900		

\*

200	KK	Iw366									
201	KO	0	0	0.0	1	22					
202	BA	1.0558									
203	LG	0.343	0.25	5.762	0.202	7.207					
204	UC	0.763	0.593								
		* Natural Time-Area Relation									
205	UA	0	3	5	8	12	20	43	75	90	96
206	UA	100									

\* Combines Iw366 and RIW371

207	KK	CIW366									
208	KO	0	0	0.0	0	22					
		* hard coded									
209	HC	2	1.07								

\* ROUTE UPDATED 4/14/4

210	KK	RIW366									
211	KO	0	0	0.0	0	22					
212	RS	4	FLOW	0.0	0.0						
213	RC	0.04	0.04	0.04	9657	0.007					
		* IWTS30_2									
214	RX	0	6.5	13	57	102	104	109	110		
215	RY	1728	1725.5	1724	1723	1724	1725.5	1727	1728		

\*

1  
LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

216 KK IW377  
217 KO 0 0 0.0 1 22  
218 BA 0.1125  
219 LG 0.304 0.382 5.921 0.188 5.357  
220 UC 0.408 0.519  
\* Natural Time-Area Relation  
221 UA 0 3 5 8 12 20 43 75 90 96  
222 UA 100

\* \*\*\* STAGE DISCHARGE / STORAGE BEHIND STRUCTURE SR010 (HWY74 CULVERT)

223 KK SSR010  
224 RS 1 STOR 0  
225 SQ 0 36 100 160  
226 SA 0 .05 .41 1.98  
227 SE 2021.3 2024 2026 2028

\* ROUTE UPDATED 4/14/4

228 KK RIW377  
229 KO 0 0 0.0 0 22  
230 RS 6 FLOW 0.0 0.0  
231 RC 0.04 0.04 0.04 6588 0.0096  
\* TSR371  
232 RX 0 142 486 525 651 770 1000 1518  
233 RY 1900 1900 1899 1897 1896 1899 1899.5 1900

234 KK IW374  
235 KO 0 0 0.0 1 22  
236 BA 0.3874  
237 LG 0.315 0.161 8.147 0.086 12.602  
238 UC 0.442 0.345  
\* Natural Time-Area Relation  
239 UA 0 3 5 8 12 20 43 75 90 96  
240 UA 100

\* RAILROAD DIVERSION RECOVERY: ALONG RAILROAD FROM STRUCTURE RR40

241 KK D371  
242 KO 0 0 0.0 0 22  
243 DR D0371

\* ROUTE UPDATED 4/14/4

1  
LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

244 KK RD371  
245 KO 0 0 0.0 0 22  
246 RS 1 FLOW 0.0 0.0  
247 RC 0.043 0.035 0.043 1409 0.0014  
\* WITS130  
248 RX 0 400 456 575 620 650 660 682  
249 RY 1396.5 1394 1393.5 1393 1392 1394 1395 1396.5

\* Combines RD371, IW374 and RIW377

250 KK CIW374  
251 KO 0 0 0.0 0 22  
\* Hard Coded  
252 HC 3 0.54

\* ROUTE UPDATED 4/14/4

253 KK RIW374  
254 KO 0 0 0.0 0 22  
255 RS 15 FLOW 0.0 0.0  
256 RC 0.04 0.04 0.04 20437 0.0081  
\* TSR371  
257 RX 0 142 486 525 651 770 1000 1518  
258 RY 1900 1900 1899 1897 1896 1899 1899.5 1900

259 KK IW386  
260 KO 0 0 0.0 1 22  
261 BA 0.3731  
262 LG 0.334 0.192 8.439 0.072 5.278  
263 UC 0.838 0.855  
\* Natural Time-Area Relation  
264 UA 0 3 5 8 12 20 43 75 90 96  
265 UA 100

\* ROUTE UPDATED 4/14/4

266 KK RIW386  
267 KO 0 0 0.0 0 22  
268 RS 6 FLOW 0.0 0.0  
269 RC 0.043 0.035 0.043 10311 0.0078  
\* IWTS20\_1  
270 RX 0 21 75 93 234 280 301 391

271 RY 1952 1949 1948 1946 1945 1948 1948.5 1952

\*  
\*

HEC-1 INPUT

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

272 KK IW382  
 273 KO 0 0 0.0 1 22  
 274 BA 0.6196  
 275 LG 0.322 0.275 6.319 0.164 8.175  
 276 UC 0.642 0.555  
 \* Urban Time-Area Relation  
 277 UA 0 5 16 30 65 77 84 90 94 97  
 278 UA 100

\* Combines IW382 and RIW386

279 KK CIW382  
 280 KO 0 0 0.0 0 22  
 281 HC 2

\* US60 Diversion: Main path to D382A, Diversion to CIW381  
\* From Structures US006 and US007, See Appendix D.5 for analysis

282 KK D382B  
 283 KO 0 0 0.0 0 22  
 284 DT D0382B  
 285 DI 0 24 180 10000  
 286 DQ 0 24 180 180

\* Railroad Diversion: Main path to C359\*, Diversion to CIW384  
\* From Structure RR90 and RR100, See Appendix D.5 for analysis

287 KK D382A  
 288 KO 0 0 0.0 0 22  
 289 DT D0382A  
 290 DI 0 14 60 200 315 448 629 1371  
 291 DQ 0 0 0 0 55 174 338 1045

\* ROUTE UPDATED 4/14/4

292 KK RIW382  
 293 KO 0 0 0.0 0 22  
 294 RS 3 FLOW 0.0 0.0  
 295 RC 0.04 0.04 0.04 4964 0.012  
 \* TSR371  
 296 RX 0 142 486 525 651 770 1000 1518  
 297 RY 1900 1900 1899 1897 1896 1899 1899.5 1900

298 KK IW384  
 299 KO 0 0 0.0 1 22  
 300 BA 0.0825  
 301 LG 0.343 0.15 8.675 0.065 7.206  
 302 UC 0.308 0.296  
 \* Natural Time-Area Relation  
 303 UA 0 3 5 8 12 20 43 75 90 96

HEC-1 INPUT

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

304 UA 100

\* Railroad Diversion Recovery: Outflow from structures RR90 AND RR100

305 KK D382A  
 306 KO 0 0 0.0 0 22  
 307 DR D0382A

\* ROUTE UPDATED 4/14/4

308 KK RD382A  
 309 KO 0 0 0.0 0 22  
 310 RS 1 FLOW 0.0 0.0  
 311 RC 0.043 0.035 0.043 1172 0.0068  
 \* WITS130  
 312 RX 0 400 456 575 620 650 660 682  
 313 RY 1396.5 1394 1393.5 1393 1392 1394 1395 1396.5

\* Combines RD382A and IW384

314 KK CIW384  
 315 KO 0 0 0.0 0 22  
 \* Hard Coded  
 316 HC 2 0.41

\* ROUTE UPDATED 4/14/4

317 KK RIW384  
 318 KO 0 0 0.0 0 22  
 319 RS 3 FLOW 0.0 0.0  
 320 RC 0.04 0.04 0.04 4694 0.0124  
 \* TSR371  
 321 RX 0 142 486 525 651 770 1000 1518  
 322 RY 1900 1900 1899 1897 1896 1899 1899.5 1900

323 KK IW381  
 324 KO 0 0 0.0 1 22  
 325 BA 0.3841  
 326 LG 0.309 0.19 9.152 0.056 11.882  
 327 UC 0.650 0.799  
 \* Natural Time-Area Relation  
 328 UA 0 3 5 8 12 20 43 75 90 96  
 329 UA 100

\*  
 \*  
 \* This diversion is not routed because the distance traveled from the actual  
 \* point of diversion (not CIW382) to CIW382 is approximately the same as the  
 \* distance from the same point of diversion to CIW381. Thus to avoid double  
 \* counting storage in the routes, a route for the diversion was not included  
 \* in the model.  
 \*  
 \*  
 \* Us60 Diversion Recovery: Outflow from structures US006 and US007  
 HEC-1 INPUT

PAGE 10

1  
 LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

330 KK D382B  
 331 KO 0 0 0.0 0 22  
 332 DR D0382B  
 \*  
 \*  
 \* Combines D382B and IW381  
 333 KK CIW381  
 \* Hard Coded  
 334 HC 2 0.64  
 \*  
 \*  
 \* ROUTE UPDATED 4/14/4  
 335 KK RIW381  
 336 KO 0 0 0.0 0 22  
 337 RS 3 FLOW 0.0 0.0  
 338 RC 0.04 0.04 0.04 4964 0.0096  
 \* TSR371  
 339 RX 0 142 486 525 651 770 1000 1518  
 340 RY 1900 1900 1899 1897 1896 1899 1899.5 1900

\* Combines RIW381, RIW382 and RIW384

341 KK C359\*  
 342 KO 0 0 0.0 0 22  
 \* Hard Coded  
 343 HC 3 1.45  
 \*  
 \*  
 \* ROUTE UPDATED 4/14/4  
 344 KK R359\*  
 345 KO 0 0 0.0 0 22  
 346 RS 4 FLOW 0.0 0.0  
 347 RC 0.04 0.04 0.04 9280 0.0103  
 \* IWTS30.2  
 348 RX 0 6.5 13 57 102 104 109 110  
 349 RY 1728 1725.5 1724 1723 1724 1725.5 1727 1728

\*  
 \*  
 350 KK IW359  
 351 KO 0 0 0.0 1 22  
 352 BA 2.912  
 353 LG 0.345 0.25 5.939 0.187 6.181  
 354 UC 0.983 0.574  
 \* Natural Time-Area Relation  
 355 UA 0 3 5 8 12 20 43 75 90 96  
 356 UA 100

\* Combines IW359, R359\*, RIW374 and RIW366  
 HEC-1 INPUT

PAGE 11

1  
 LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

357 KK CIW359  
 358 KO 0 0 0.0 0 22  
 359 HC 4  
 \*  
 \*  
 \* ROUTE UPDATED 4/14/4  
 360 KK RIW359  
 361 KO 0 0 0.0 0 22  
 362 RS 4 FLOW 0.0 0.0  
 363 RC 0.04 0.04 0.04 13031 0.0059  
 \* IWTS30.2 modified  
 364 RX 0 6.5 13 57 102 104 109 110  
 365 RY 1730 1725.5 1724 1723 1724 1725.5 1727 1730

\*  
 \*  
 \* \*\*\*\*\*  
 \* \*\*\*\*\* END OF IONA WASH REGION 1 \*\*\*\*\*  
 \* \*\*\*\*\*  
 \*  
 \*  
 \*

\*\*\*\*\*  
\*\*\*\*\* BEGINNING OF IONA WASH REGION 2 \*\*\*\*\*  
\*\*\*\*\*

366 KK IW389  
367 KO 0 0 0.0 1 22  
368 BA 0.1312  
369 LG 0.348 0.13 10.1 0.037 5.526  
370 UC 0.588 0.753  
\* Natural Time-Area Relation  
371 UA 0 3 5 8 12 20 43 75 90 96  
372 UA 100

\* \*\*\* STAGE DISCHARGE / STORAGE BEHIND STRUCTURE SR190 (HWY74 CULVERT)

373 KK SSR190  
374 KO 0 0 0.0 1 22  
375 RS 1 STOR 0  
376 SQ 0 69 161 184 207 230  
377 SE 2053.8 2055.5 2056.83 2057.1 2057.4 2057.7  
378 SA 0 .01 .08 .22 .39 1.13  
379 SE 2052 2054 2056 2058 2060 2062

\* ROUTE UPDATED 4/14/4

HEC-1 INPUT

PAGE 12

1  
LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

380 KK RIW389  
381 KO 0 0 0.0 0 22  
382 RS 6 FLOW 0.0 0.0  
383 RC 0.04 0.04 0.04 11459 0.0124  
\* TSR371  
384 RX 0 142 486 525 651 770 1000 1518  
385 RY 1900 1900 1899 1897 1896 1899 1899.5 1900

386 KK IW394  
387 KO 0 0 0.0 1 22  
388 BA 4.1731  
389 LG 0.35 0.327 6.166 0.173 16.632  
390 UC 1.367 0.857  
\* Natural Time-Area Relation  
391 UA 0 3 5 8 12 20 43 75 90 96  
392 UA 100

\* ROUTE UPDATED 4/14/4

393 KK RIW394  
394 KO 0 0 0.0 0 22  
395 RS 2 FLOW 0.0 0.0  
396 RC 0.043 0.035 0.043 7697 0.0133  
\* IWTS10\_1  
397 RX 0 34 89 134 161 198 298 333  
398 RY 2160 2158 2154 2152 2154 2156 2158 2160

399 KK IW390A  
400 KO 0 0 0.0 1 22  
401 BA 0.9003  
402 LG 0.35 0.25 6.134 0.172 17.033  
403 UC 0.971 0.899  
\* Natural Time-Area Relation  
404 UA 0 3 5 8 12 20 43 75 90 96  
405 UA 100

\* Combines Iw390A and RIW394

406 KK C390A  
407 KO 0 0 0.0 0 22  
408 HC 2

\* ROUTE UPDATED 4/14/4

HEC-1 INPUT

PAGE 13

1  
LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

409 KK R390A  
410 KO 0 0 0.0 0 22  
411 RS 2 FLOW 0.0 0.0  
412 RC 0.043 0.035 0.043 8577 0.0134  
\* IWTS10\_1  
413 RX 0 34 89 134 161 198 298 333  
414 RY 2160 2158 2154 2152 2154 2156 2158 2160

415 KK IW390  
416 KO 0 0 0.0 1 22  
417 BA 0.3110  
418 LG 0.349 0.25 5.071 0.258 5.221  
419 UC 0.929 1.173  
\* Natural Time-Area Relation  
420 UA 0 3 5 8 12 20 43 75 90 96

421 UA 100  
 \*  
 \*  
 422 KK IW392  
 423 KO 0 0 0.0 1 22  
 424 BA 1.1135  
 425 LG 0.35 0.15 7.459 0.103 8.347  
 426 UC 0.504 0.280  
 \* Natural Time-Area Relation  
 427 UA 0 3 5 8 12 20 43 75 90 96  
 428 UA 100

\* Combines IW392, IW390 and R390A

429 KK CIW390  
 430 KO 0 0 0.0 0 22  
 431 HC 3

\* \*\*\* STAGE DISCHARGE / STORAGE BEHIND STRUCTURE SR160 (HWY74 CULVERT)

432 KK SSR160  
 433 RS 1 STOR 0  
 434 SQ 0 762 1440 2640 3840 5040  
 435 SA 0.0 0.04 0.36 3.17 4.85 6.69  
 436 SE 2033 2036 2038 2040 2042 2044

\* ROUTE UPDATED 4/14/4

HEC-1 INPUT

PAGE 14

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

437 KK RIW390  
 438 KO 0 0 0.0 0 22  
 439 RS 3 FLOW 0.0 0.0  
 440 RC 0.043 0.035 0.043 11626 0.0101  
 \* IWTS20.1  
 441 RX 0 21 75 93 234 280 301 391  
 442 RY 1952 1949 1948 1946 1945 1948 1948.5 1952

443 KK IW388  
 444 KO 0 0 0.0 1 22  
 445 BA 1.2256  
 446 LG 0.337 0.156 8.775 0.061 9.239  
 447 UC 1.133 0.900  
 \* Natural Time-Area Relation  
 448 UA 0 3 5 8 12 20 43 75 90 96  
 449 UA 100

\* Combines IW388, RIW390 and RIW389

450 KK CIW388  
 451 KO 0 0 0.0 0 22  
 452 HC 3

\* ROUTE UPDATED 4/14/4

453 KK RIW388  
 454 KO 0 0 0.0 0 22  
 455 RS 2 FLOW 0.0 0.0  
 456 RC 0.04 0.04 0.04 7726 0.0094  
 \* IWTS20.2  
 457 RX 0 79 201 242 303 327 346 350  
 458 RY 1940 1938 1936 1933.5 1936 1936.5 1940 1940

459 KK IW357  
 460 KO 0 0 0.0 1 22  
 461 BA 0.7456  
 462 LG 0.33 0.156 8.059 0.083 11.106  
 463 UC 0.700 0.705  
 \* Natural Time-Area Relation  
 464 UA 0 3 5 8 12 20 43 75 90 96  
 465 UA 100

\* Combines IW357 and RIW388

HEC-1 INPUT

PAGE 15

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

466 KK CIW357  
 467 KO 0 0 0.0 0 22  
 468 HC 2

\* This split was determined using FLO-2D. The model is titled "WEST" and details and documentation about the model are contained in the Appendix D 5.4.

469 KK D357  
 470 DT DO357  
 471 DI 0 258 530 766 975 1140 1340 1556 1789 1937  
 472 DI 2120 2357 2514 2736 2992 3255 3301 3472 3610 5000  
 473 DQ 0 56 144 211 291 357 437 383 451 502  
 474 DQ 532 601 633 705 737 793 854 930 947 1312

\*  
\* ROUTE UPDATED 4/14/4

```

475 KK RIW357
476 KO 0 0 0.0 0 22
477 RS 9 FLOW 0.0 0.0
478 RC 0.04 0.04 0.04 16693 0.0079
* IWTS30_1 modified
479 RX 0 50 96 135 160 177 190 206
480 RY 1881 1879 1878 1876 1876 1878 1879 1881

```

\*\*\*\*\*  
\*\*\*\*\* END OF IONA WASH REGION 2 \*\*\*\*\*  
\*\*\*\*\*

\*\*\*\*\*  
\*\*\*\*\* BEGINNING OF IONA WASH REGION 3 \*\*\*\*\*  
\*\*\*\*\*

```

481 KK IW353
482 KO 0 0 0.0 1 22
483 BA 2.8030
484 LG 0.35 0.15 7.596 0.098 5.084
485 UC 0.996 0.710
* Natural Time-Area Relation
486 UA 0 3 5 8 12 20 43 75 90 96
487 UA 100

```

\* Combines IW353, RIW357, and RIW359  
HEC-1 INPUT

PAGE 16

1 LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

```

488 KK CIW353
489 KO 0 0 0.0 0 22
* Hard Coded
490 HC 3 15.12

```

\* ROUTE UPDATED 4/14/4

```

491 KK RIW353
492 KO 0 0 0.0 0 22
493 RS 2 FLOW 0.0 0.0
494 RC 0.04 0.04 0.04 7496 0.0053
* IWTS40_1
495 RX 0 147 714 840 930 1073 1087 1417
496 RY 1708 1704 1700 1696 1696 1700 1704 1708

```

```

497 KK IW350
498 KO 0 0 0.0 1 22
499 BA 2.1750
500 LG 0.35 0.171 6.789 0.135 5.0
501 UC 1.308 1.150
* Natural Time-Area Relation
502 UA 0 3 5 8 12 20 43 75 90 96
503 UA 100

```

\* Combines IW350 and RIW353

```

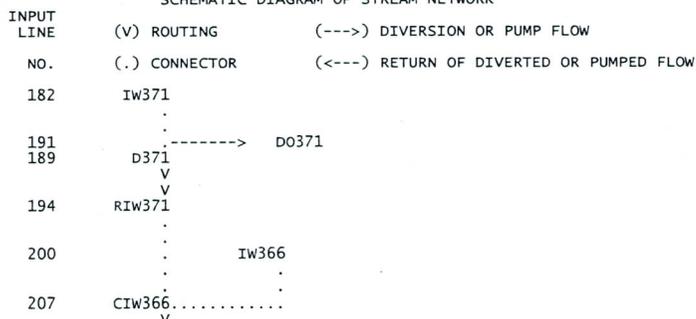
504 KK CIW350
505 KO 0 0 0.0 0 22
506 HC 2

```

\*\*\*\*\*  
\*\*\*\*\* END OF IONA WASH REGION 3 \*\*\*\*\*  
\*\*\*\*\*

507 ZZ

SCHMATIC DIAGRAM OF STREAM NETWORK



```

210      V
      RIW366
216      .
      IW377
      V
223      .
      SSR010
      V
228      .
      RIW377
      V
234      .
      IW374
      .
243      .
      D371 <----- D0371
241      .
      V
244      .
      RD371
      V
250      .
      CIW374 .....
      V
253      .
      RIW374
      V
259      .
      IW386
      V
266      .
      RIW386
      V
272      .
      IW382
      .
279      .
      CIW382 .....
      .
284      .
      D382B -----> D0382B
282      .
      .
289      .
      D382A -----> D0382A
287      .
      V
292      .
      RIW382
      V
298      .
      IW384
      .
307      .
      D382A <----- D0382A
305      .
      V
308      .
      RD382A
      V
314      .
      CIW384 .....
      V
317      .
      RIW384
      V
323      .
      IW381
      .
332      .
      D382B <----- D0382B
330      .
      .
333      .
      CIW381 .....
      V
335      .
      RIW381
      V
341      .
      C359* .....
      V
344      .
      R359*
      V
350      .
      IW359
      .
357      .
      CIW359 .....
      V
360      .
      RIW359
      V
366      .
      IW389
      V
373      .
      SSR190
      V
380      .
      RIW389
      V
386      .
      IW394

```



```

wtec6-addendum-mod1.out
** -Route R508* was adjusted **
** -Hard coding for CWI510 was adjusted **
**
** -Diversión D618 was moved from after CPI618 to before RDCP12 **
** -Updated route RD618 information **
** -Hard coding on CPI618 was adjusted **
** -Hard coding on CPI615 was adjusted **
**
** -Route RPI639 changed from following US60 to CPI636 to route **
** through US60/RR structures and combine with CWI506 **
** -Cards PI639, D524, RD524, CPI639 and RPI639 were changed from **
** following R624A to follow RWI524 **
** -CPI636 was removed-no longer needed to combine PI636 & RPI639 **
** -RPI639 information was changed to reflect new route **
** -CWI506 changed from combining 3 to combine 4: **
** RWI525, RWI524, RPI639 & WI506 **
** -Hard coding on CWI506 was adjusted **
** -Hard Coding on CWI500 was adjusted **
**

```

```

*****
**-ADDENDUM-ADDENDUM-ADDENDUM-ADDENDUM-ADDENDUM-ADDENDUM-ADDENDUM-ADDENDUM-**
**-ADDENDUM-ADDENDUM-ADDENDUM-ADDENDUM-ADDENDUM-ADDENDUM-ADDENDUM-ADDENDUM-**
*****

```

```

*****
**
** WITTMANN AREA DRAINAGE MASTER STUDY UPDATE-HYDROLOGY MODEL 2004 **
**
*****

```

```

PROJECT: Wittmann ADMS Update
CLIENT: Flood Control District of Maricopa County
PREPARED BY: Entellus, Inc.
PROJECT No: FCD 2002C029 Entellus 310.032
FILE NAME: WTEC6-addendum.hcl CREATED DATE: JUL 01, 2004
MODIFIED DATE: MAY 04, 2005

```

STORM: 100-year 6-hour Storm

DEVELOPMENT CONDITIONS: Existing Conditions

MODELING ASSUMPTIONS:

It was assumed that the US60 did not have adequate storage to cause any significant attenuation.

The assumption was made that the CAP Canal embankment would not be breached under a large flood event.

The assumption was made that the Beardsley Canal would not fail under a large storm event (Per District Instruction). In addition, the berm north of the Beardsley canal and east of US60 was assumed to fail (per district Instruction).

For both the CAP and Beardsley Canals, once the berm elevation was reached weir flow was assumed. It was also assumed that any weir flow over the canal that might enter the canal and be diverted out of the study area was insignificant, and thus was ignored. In other words all weir flow over the canal embankment reaches the downstream concentration point.

Typical x-sects were developed, and it was assumed that a typical x-sect could adequately represent various reaches.

Time-Area Relations were used base on the District's Hydrology Manual criteria. Two Time-Area Relation Curves were utilized:  
 -Urban  
 -Natural

The Time-Area Relation Curves were taken directly from the manual

MODELING METHODS:

This model utilizes QI cards to input the Padelford hydrographs from the 100-year 6-hour existing conditions model developed by A-N West Inc. for the Padelford wash Floodplain Delineation Study. The hydrograph was altered from its original form (2-minute interval to 5-minute interval) through simple interpolation.

Clark Unit Hydrographs were used for all subbasins except the two subbasins directly upstream of the Bonita Dam (PD726B and PD740 use S-graphs). The UC parameters were calculated using the WMS7.0 software.

For Basins PD726B and PD740 S-Graphs were utilized per the request of the FCDMC. Limited details regarding the calculations of the S-graphs can be found in the model by the basin KK card. For full details of the S-graph calculations refer to the Appendix.

Normal Depth routing was used for all routing reaches.

Hard coding was used to account for the percentage of area associated with a diversion. Because of the use of JD cards and aerial reduction, hard coding was necessary to properly account for area. For a given

wtec6-addendum-mod1.out  
 diversion a percentage of the flow is routed to two different locations. The same percentage of area follows that diverted flow. In addition the area downstream of the main path is reduced or increased and is hard coded to account for the loss or gain of area. Hard coding was performed based on the 6-hour existing conditions model.

Stage-storage was developed for all the structures along the CAP Canal, as well as along the Beardsley Canal. In addition several stage-storage locations were developed for areas with significant storage along the SR74. No storage was modeled along the US60 and railroad, but the culverts were analyzed for diversion potential. Diversions were placed in the model where deemed appropriate. See appendix for details.

FLO-2D was utilized to calculate the split flows at concentration points CIW351, CIW357, CIW363 and CWI576. See appendix for modeling details.

\*\*\*\*\*  
 \*\*\*\*\*

159 IO

OUTPUT CONTROL VARIABLES  
 IPRNT 5 PRINT CONTROL  
 IPLOT 0 PLOT CONTROL  
 QSCAL 0. HYDROGRAPH PLOT SCALE

IT

HYDROGRAPH TIME DATA  
 NMIN 5 MINUTES IN COMPUTATION INTERVAL  
 IDATE 1 0 STARTING DATE  
 ITIME 0000 STARTING TIME  
 NQ 1500 NUMBER OF HYDROGRAPH ORDINATES  
 NDDATE 6 0 ENDING DATE  
 NDTIME 0455 ENDING TIME  
 ICENT 19 CENTURY MARK  
 COMPUTATION INTERVAL 0.08 HOURS  
 TOTAL TIME BASE 124.92 HOURS

ENGLISH UNITS

DRAINAGE AREA SQUARE MILES  
 PRECIPITATION DEPTH INCHES  
 LENGTH, ELEVATION FEET  
 FLOW CUBIC FEET PER SECOND  
 STORAGE VOLUME ACRE-Feet  
 SURFACE AREA ACRES  
 TEMPERATURE DEGREES FAHRENHEIT

161 JD

INDEX STORM NO. 1  
 STRM 3.40 PRECIPITATION DEPTH  
 TRDA 0.01 TRANSPOSITION DRAINAGE AREA

162 PI

PRECIPITATION PATTERN  
 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00  
 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00  
 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00  
 0.00 0.00 0.00 0.01 0.01 0.01 0.01 0.01 0.01 0.01  
 0.03 0.03 0.05 0.05 0.05 0.15 0.15 0.15 0.03 0.03  
 0.03 0.01 0.01 0.01 0.01 0.01 0.01 0.01 0.00 0.00  
 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00  
 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00

165 JD

INDEX STORM NO. 2  
 STRM 3.38 PRECIPITATION DEPTH  
 TRDA 0.50 TRANSPOSITION DRAINAGE AREA

0 PI

PRECIPITATION PATTERN  
 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00  
 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00  
 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00  
 0.00 0.00 0.00 0.01 0.01 0.01 0.01 0.01 0.01 0.01  
 0.03 0.03 0.05 0.05 0.05 0.15 0.15 0.15 0.03 0.03  
 0.03 0.01 0.01 0.01 0.01 0.01 0.01 0.01 0.00 0.00  
 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00  
 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00

166 JD

INDEX STORM NO. 3  
 STRM 3.31 PRECIPITATION DEPTH  
 TRDA 2.80 TRANSPOSITION DRAINAGE AREA

167 PI

PRECIPITATION PATTERN  
 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00  
 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00  
 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00  
 0.00 0.00 0.00 0.01 0.01 0.01 0.01 0.01 0.01 0.01  
 0.03 0.03 0.07 0.07 0.07 0.08 0.08 0.08 0.05 0.05  
 0.05 0.02 0.02 0.02 0.01 0.01 0.01 0.01 0.00 0.00  
 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00  
 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00

170 JD

INDEX STORM NO. 4  
 STRM 3.13 PRECIPITATION DEPTH  
 TRDA 16.00 TRANSPOSITION DRAINAGE AREA

171 PI

PRECIPITATION PATTERN  
 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.01  
 0.01 0.01 0.00 0.01 0.00 0.00 0.00 0.00 0.00 0.00  
 0.00 0.00 0.01 0.00 0.00 0.00 0.00 0.01 0.01 0.01  
 0.01 0.01 0.01 0.01 0.01 0.01 0.02 0.02 0.02 0.03  
 0.03 0.03 0.06 0.06 0.06 0.07 0.07 0.07 0.04 0.04  
 0.04 0.02 0.02 0.02 0.01 0.01 0.01 0.01 0.01 0.01  
 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00

0.00 0.00

174 JD INDEX STORM NO. 5  
STRM 2.76 PRECIPITATION DEPTH  
TRDA 90.00 TRANSPOSITION DRAINAGE AREA

175 PI PRECIPITATION PATTERN  
0.01 0.01 0.01 0.00 0.00 0.00 0.01 0.01 0.01 0.01  
0.01 0.01 0.01 0.01 0.01 0.01 0.01 0.01 0.01 0.01  
0.01 0.01 0.01 0.01 0.01 0.01 0.01 0.01 0.01 0.01  
0.01 0.01 0.01 0.01 0.01 0.01 0.02 0.02 0.02 0.03  
0.03 0.03 0.05 0.05 0.05 0.05 0.05 0.05 0.04 0.04  
0.04 0.02 0.02 0.02 0.02 0.02 0.02 0.01 0.01 0.01  
0.01 0.01 0.01 0.01 0.01 0.01 0.01 0.01 0.01 0.01  
0.01 0.01

178 JD INDEX STORM NO. 6  
STRM 1.94 PRECIPITATION DEPTH  
TRDA 500.00 TRANSPOSITION DRAINAGE AREA

179 PI PRECIPITATION PATTERN  
0.01 0.01 0.01 0.01 0.01 0.01 0.01 0.01 0.01 0.01  
0.01 0.01 0.01 0.01 0.01 0.01 0.01 0.01 0.01 0.01  
0.01 0.01 0.01 0.01 0.01 0.01 0.01 0.01 0.01 0.01  
0.01 0.01 0.01 0.01 0.01 0.01 0.02 0.02 0.02 0.03  
0.03 0.03 0.04 0.04 0.04 0.04 0.04 0.04 0.04 0.04  
0.04 0.03 0.03 0.03 0.02 0.02 0.02 0.01 0.01 0.01  
0.01 0.01 0.01 0.01 0.01 0.01 0.01 0.01 0.01 0.01  
0.01 0.01

\*\*\* \*\*

\*\*\*\*\*  
\* \*  
\* IW371 \*  
\* \*  
\*\*\*\*\*

183 KO OUTPUT CONTROL VARIABLES  
IPRNT 5 PRINT CONTROL  
IPLOT 0 PLOT CONTROL  
QSCAL 0. HYDROGRAPH PLOT SCALE  
IPNCH 1 PUNCH COMPUTED HYDROGRAPH  
IOUT 22 SAVE HYDROGRAPH ON THIS UNIT  
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
ISAV2 1500 LAST ORDINATE PUNCHED OR SAVED  
TIMINT 0.083 TIME INTERVAL IN HOURS

\*\*\* \*\*

\*\*\*\*\*  
\* \*  
\* D371 \*  
\* \*  
\*\*\*\*\*

190 KO OUTPUT CONTROL VARIABLES  
IPRNT 5 PRINT CONTROL  
IPLOT 0 PLOT CONTROL  
QSCAL 0. HYDROGRAPH PLOT SCALE  
IPNCH 0 PUNCH COMPUTED HYDROGRAPH  
IOUT 22 SAVE HYDROGRAPH ON THIS UNIT  
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
ISAV2 1500 LAST ORDINATE PUNCHED OR SAVED  
TIMINT 0.083 TIME INTERVAL IN HOURS

\*\*\* \*\*

\*\*\*\*\*  
\* \*  
\* RIW371 \*  
\* \*  
\*\*\*\*\*

195 KO OUTPUT CONTROL VARIABLES  
IPRNT 5 PRINT CONTROL  
IPLOT 0 PLOT CONTROL  
QSCAL 0. HYDROGRAPH PLOT SCALE  
IPNCH 0 PUNCH COMPUTED HYDROGRAPH  
IOUT 22 SAVE HYDROGRAPH ON THIS UNIT  
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
ISAV2 1500 LAST ORDINATE PUNCHED OR SAVED  
TIMINT 0.083 TIME INTERVAL IN HOURS

\*\*\* \*\*

\*\*\*\*\*  
\* \*  
\* IW366 \*  
\* \*  
\*\*\*\*\*

201 KO        OUTPUT CONTROL VARIABLES  
 IPRNT        5    PRINT CONTROL  
 IPLOT        0    PLOT CONTROL  
 QSCAL        0.    HYDROGRAPH PLOT SCALE  
 IPNCH        1    PUNCH COMPUTED HYDROGRAPH  
 IOUT         22    SAVE HYDROGRAPH ON THIS UNIT  
 ISAV1        1    FIRST ORDINATE PUNCHED OR SAVED  
 ISAV2        1500 LAST ORDINATE PUNCHED OR SAVED  
 TIMINT       0.083 TIME INTERVAL IN HOURS

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 \*    CIW366   \*  
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208 KO        OUTPUT CONTROL VARIABLES  
 IPRNT        5    PRINT CONTROL  
 IPLOT        0    PLOT CONTROL  
 QSCAL        0.    HYDROGRAPH PLOT SCALE  
 IPNCH        0    PUNCH COMPUTED HYDROGRAPH  
 IOUT         22    SAVE HYDROGRAPH ON THIS UNIT  
 ISAV1        1    FIRST ORDINATE PUNCHED OR SAVED  
 ISAV2        1500 LAST ORDINATE PUNCHED OR SAVED  
 TIMINT       0.083 TIME INTERVAL IN HOURS

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 \*            \*  
 \*    RIW366   \*  
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211 KO        OUTPUT CONTROL VARIABLES  
 IPRNT        5    PRINT CONTROL  
 IPLOT        0    PLOT CONTROL  
 QSCAL        0.    HYDROGRAPH PLOT SCALE  
 IPNCH        0    PUNCH COMPUTED HYDROGRAPH  
 IOUT         22    SAVE HYDROGRAPH ON THIS UNIT  
 ISAV1        1    FIRST ORDINATE PUNCHED OR SAVED  
 ISAV2        1500 LAST ORDINATE PUNCHED OR SAVED  
 TIMINT       0.083 TIME INTERVAL IN HOURS

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 \*            \*  
 \*    IW377    \*  
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217 KO        OUTPUT CONTROL VARIABLES  
 IPRNT        5    PRINT CONTROL  
 IPLOT        0    PLOT CONTROL  
 QSCAL        0.    HYDROGRAPH PLOT SCALE  
 IPNCH        1    PUNCH COMPUTED HYDROGRAPH  
 IOUT         22    SAVE HYDROGRAPH ON THIS UNIT  
 ISAV1        1    FIRST ORDINATE PUNCHED OR SAVED  
 ISAV2        1500 LAST ORDINATE PUNCHED OR SAVED  
 TIMINT       0.083 TIME INTERVAL IN HOURS

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 \*            \*  
 \*    RIW377   \*  
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229 KO        OUTPUT CONTROL VARIABLES  
 IPRNT        5    PRINT CONTROL  
 IPLOT        0    PLOT CONTROL  
 QSCAL        0.    HYDROGRAPH PLOT SCALE  
 IPNCH        0    PUNCH COMPUTED HYDROGRAPH  
 IOUT         22    SAVE HYDROGRAPH ON THIS UNIT  
 ISAV1        1    FIRST ORDINATE PUNCHED OR SAVED  
 ISAV2        1500 LAST ORDINATE PUNCHED OR SAVED  
 TIMINT       0.083 TIME INTERVAL IN HOURS

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234 KK \* IW374 \*  
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235 KO OUTPUT CONTROL VARIABLES  
IPRNT 5 PRINT CONTROL  
IPLOT 0 PLOT CONTROL  
QSCAL 0. HYDROGRAPH PLOT SCALE  
IPNCH 1 PUNCH COMPUTED HYDROGRAPH  
IOUT 22 SAVE HYDROGRAPH ON THIS UNIT  
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
ISAV2 1500 LAST ORDINATE PUNCHED OR SAVED  
TIMINT 0.083 TIME INTERVAL IN HOURS

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241 KK \* D371 \*  
\*\*\*\*\*

242 KO OUTPUT CONTROL VARIABLES  
IPRNT 5 PRINT CONTROL  
IPLOT 0 PLOT CONTROL  
QSCAL 0. HYDROGRAPH PLOT SCALE  
IPNCH 0 PUNCH COMPUTED HYDROGRAPH  
IOUT 22 SAVE HYDROGRAPH ON THIS UNIT  
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
ISAV2 1500 LAST ORDINATE PUNCHED OR SAVED  
TIMINT 0.083 TIME INTERVAL IN HOURS

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\*\*\*\*\*  
244 KK \* RD371 \*  
\*\*\*\*\*

245 KO OUTPUT CONTROL VARIABLES  
IPRNT 5 PRINT CONTROL  
IPLOT 0 PLOT CONTROL  
QSCAL 0. HYDROGRAPH PLOT SCALE  
IPNCH 0 PUNCH COMPUTED HYDROGRAPH  
IOUT 22 SAVE HYDROGRAPH ON THIS UNIT  
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
ISAV2 1500 LAST ORDINATE PUNCHED OR SAVED  
TIMINT 0.083 TIME INTERVAL IN HOURS

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\*\*\*\*\*  
250 KK \* CIW374 \*  
\*\*\*\*\*

251 KO OUTPUT CONTROL VARIABLES  
IPRNT 5 PRINT CONTROL  
IPLOT 0 PLOT CONTROL  
QSCAL 0. HYDROGRAPH PLOT SCALE  
IPNCH 0 PUNCH COMPUTED HYDROGRAPH  
IOUT 22 SAVE HYDROGRAPH ON THIS UNIT  
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
ISAV2 1500 LAST ORDINATE PUNCHED OR SAVED  
TIMINT 0.083 TIME INTERVAL IN HOURS

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\*\*\*\*\*  
253 KK \* RIW374 \*  
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254 KO OUTPUT CONTROL VARIABLES  
IPRNT 5 PRINT CONTROL  
IPLOT 0 PLOT CONTROL  
QSCAL 0. HYDROGRAPH PLOT SCALE  
IPNCH 0 PUNCH COMPUTED HYDROGRAPH  
IOUT 22 SAVE HYDROGRAPH ON THIS UNIT  
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
ISAV2 1500 LAST ORDINATE PUNCHED OR SAVED  
TIMINT 0.083 TIME INTERVAL IN HOURS

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\* Iw386 \*  
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260 KO OUTPUT CONTROL VARIABLES  
IPRNT 5 PRINT CONTROL  
IPLOT 0 PLOT CONTROL  
QSCAL 0. HYDROGRAPH PLOT SCALE  
IPNCH 1 PUNCH COMPUTED HYDROGRAPH  
IOUT 22 SAVE HYDROGRAPH ON THIS UNIT  
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
ISAV2 1500 LAST ORDINATE PUNCHED OR SAVED  
TIMINT 0.083 TIME INTERVAL IN HOURS

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\* RIW386 \*  
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267 KO OUTPUT CONTROL VARIABLES  
IPRNT 5 PRINT CONTROL  
IPLOT 0 PLOT CONTROL  
QSCAL 0. HYDROGRAPH PLOT SCALE  
IPNCH 0 PUNCH COMPUTED HYDROGRAPH  
IOUT 22 SAVE HYDROGRAPH ON THIS UNIT  
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
ISAV2 1500 LAST ORDINATE PUNCHED OR SAVED  
TIMINT 0.083 TIME INTERVAL IN HOURS

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\* Iw382 \*  
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273 KO OUTPUT CONTROL VARIABLES  
IPRNT 5 PRINT CONTROL  
IPLOT 0 PLOT CONTROL  
QSCAL 0. HYDROGRAPH PLOT SCALE  
IPNCH 1 PUNCH COMPUTED HYDROGRAPH  
IOUT 22 SAVE HYDROGRAPH ON THIS UNIT  
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
ISAV2 1500 LAST ORDINATE PUNCHED OR SAVED  
TIMINT 0.083 TIME INTERVAL IN HOURS

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\* CIW382 \*  
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280 KO OUTPUT CONTROL VARIABLES  
IPRNT 5 PRINT CONTROL  
IPLOT 0 PLOT CONTROL  
QSCAL 0. HYDROGRAPH PLOT SCALE  
IPNCH 0 PUNCH COMPUTED HYDROGRAPH  
IOUT 22 SAVE HYDROGRAPH ON THIS UNIT  
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
ISAV2 1500 LAST ORDINATE PUNCHED OR SAVED  
TIMINT 0.083 TIME INTERVAL IN HOURS

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\* D382B \*  
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283 KO OUTPUT CONTROL VARIABLES  
IPRNT 5 PRINT CONTROL  
IPLOT 0 PLOT CONTROL  
QSCAL 0. HYDROGRAPH PLOT SCALE  
IPNCH 0 PUNCH COMPUTED HYDROGRAPH  
IOUT 22 SAVE HYDROGRAPH ON THIS UNIT  
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
ISAV2 1500 LAST ORDINATE PUNCHED OR SAVED  
TIMINT 0.083 TIME INTERVAL IN HOURS

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\* \*  
287 KK \* D382A \*  
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288 KO OUTPUT CONTROL VARIABLES  
IPRNT 5 PRINT CONTROL  
IPLOT 0 PLOT CONTROL  
QSCAL 0. HYDROGRAPH PLOT SCALE  
IPNCH 0 PUNCH COMPUTED HYDROGRAPH  
IOUT 22 SAVE HYDROGRAPH ON THIS UNIT  
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
ISAV2 1500 LAST ORDINATE PUNCHED OR SAVED  
TIMINT 0.083 TIME INTERVAL IN HOURS

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\* \*  
292 KK \* RIW382 \*  
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293 KO OUTPUT CONTROL VARIABLES  
IPRNT 5 PRINT CONTROL  
IPLOT 0 PLOT CONTROL  
QSCAL 0. HYDROGRAPH PLOT SCALE  
IPNCH 0 PUNCH COMPUTED HYDROGRAPH  
IOUT 22 SAVE HYDROGRAPH ON THIS UNIT  
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
ISAV2 1500 LAST ORDINATE PUNCHED OR SAVED  
TIMINT 0.083 TIME INTERVAL IN HOURS

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298 KK \* IW384 \*  
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299 KO OUTPUT CONTROL VARIABLES  
IPRNT 5 PRINT CONTROL  
IPLOT 0 PLOT CONTROL  
QSCAL 0. HYDROGRAPH PLOT SCALE  
IPNCH 1 PUNCH COMPUTED HYDROGRAPH  
IOUT 22 SAVE HYDROGRAPH ON THIS UNIT  
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
ISAV2 1500 LAST ORDINATE PUNCHED OR SAVED  
TIMINT 0.083 TIME INTERVAL IN HOURS

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\* \*  
305 KK \* D382A \*  
\* \*  
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306 KO OUTPUT CONTROL VARIABLES  
IPRNT 5 PRINT CONTROL  
IPLOT 0 PLOT CONTROL  
QSCAL 0. HYDROGRAPH PLOT SCALE  
IPNCH 0 PUNCH COMPUTED HYDROGRAPH  
IOUT 22 SAVE HYDROGRAPH ON THIS UNIT  
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
ISAV2 1500 LAST ORDINATE PUNCHED OR SAVED  
TIMINT 0.083 TIME INTERVAL IN HOURS

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\* \*  
308 KK \* RD382A \*  
\* \*  
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309 KO OUTPUT CONTROL VARIABLES  
IPRNT 5 PRINT CONTROL  
IPLOT 0 PLOT CONTROL  
QSCAL 0. HYDROGRAPH PLOT SCALE  
IPNCH 0 PUNCH COMPUTED HYDROGRAPH

IOUT	22	SAVE HYDROGRAPH ON THIS UNIT
ISAV1	1	FIRST ORDINATE PUNCHED OR SAVED
ISAV2	1500	LAST ORDINATE PUNCHED OR SAVED
TIMINT	0.083	TIME INTERVAL IN HOURS

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* 314 KK      CIW384 *
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315 KO      OUTPUT CONTROL VARIABLES
            IPRNT      5  PRINT CONTROL
            IPLOT      0  PLOT CONTROL
            QSCAL      0. HYDROGRAPH PLOT SCALE
            IPNCH      0  PUNCH COMPUTED HYDROGRAPH
            IOUT       22 SAVE HYDROGRAPH ON THIS UNIT
            ISAV1      1  FIRST ORDINATE PUNCHED OR SAVED
            ISAV2      1500 LAST ORDINATE PUNCHED OR SAVED
            TIMINT     0.083 TIME INTERVAL IN HOURS

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*****
*
* 317 KK      RIW384 *
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318 KO      OUTPUT CONTROL VARIABLES
            IPRNT      5  PRINT CONTROL
            IPLOT      0  PLOT CONTROL
            QSCAL      0. HYDROGRAPH PLOT SCALE
            IPNCH      0  PUNCH COMPUTED HYDROGRAPH
            IOUT       22 SAVE HYDROGRAPH ON THIS UNIT
            ISAV1      1  FIRST ORDINATE PUNCHED OR SAVED
            ISAV2      1500 LAST ORDINATE PUNCHED OR SAVED
            TIMINT     0.083 TIME INTERVAL IN HOURS

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*****
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* 323 KK      IW381 *
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324 KO      OUTPUT CONTROL VARIABLES
            IPRNT      5  PRINT CONTROL
            IPLOT      0  PLOT CONTROL
            QSCAL      0. HYDROGRAPH PLOT SCALE
            IPNCH      1  PUNCH COMPUTED HYDROGRAPH
            IOUT       22 SAVE HYDROGRAPH ON THIS UNIT
            ISAV1      1  FIRST ORDINATE PUNCHED OR SAVED
            ISAV2      1500 LAST ORDINATE PUNCHED OR SAVED
            TIMINT     0.083 TIME INTERVAL IN HOURS

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*****
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* 330 KK      D382B *
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331 KO      OUTPUT CONTROL VARIABLES
            IPRNT      5  PRINT CONTROL
            IPLOT      0  PLOT CONTROL
            QSCAL      0. HYDROGRAPH PLOT SCALE
            IPNCH      0  PUNCH COMPUTED HYDROGRAPH
            IOUT       22 SAVE HYDROGRAPH ON THIS UNIT
            ISAV1      1  FIRST ORDINATE PUNCHED OR SAVED
            ISAV2      1500 LAST ORDINATE PUNCHED OR SAVED
            TIMINT     0.083 TIME INTERVAL IN HOURS

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*****
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* 335 KK      RIW381 *
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*****

```

336 KO OUTPUT CONTROL VARIABLES

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IPRNT      5  PRINT CONTROL
IPLOT      0  PLOT CONTROL
QSCAL      0. HYDROGRAPH PLOT SCALE
IPNCH      0  PUNCH COMPUTED HYDROGRAPH
IOUT       22 SAVE HYDROGRAPH ON THIS UNIT
ISAV1      1  FIRST ORDINATE PUNCHED OR SAVED
ISAV2     1500 LAST ORDINATE PUNCHED OR SAVED
TIMINT     0.083 TIME INTERVAL IN HOURS

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341 KK      C359*

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342 KO      OUTPUT CONTROL VARIABLES
IPRNT      5  PRINT CONTROL
IPLOT      0  PLOT CONTROL
QSCAL      0. HYDROGRAPH PLOT SCALE
IPNCH      0  PUNCH COMPUTED HYDROGRAPH
IOUT       22 SAVE HYDROGRAPH ON THIS UNIT
ISAV1      1  FIRST ORDINATE PUNCHED OR SAVED
ISAV2     1500 LAST ORDINATE PUNCHED OR SAVED
TIMINT     0.083 TIME INTERVAL IN HOURS

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344 KK      R359*

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345 KO      OUTPUT CONTROL VARIABLES
IPRNT      5  PRINT CONTROL
IPLOT      0  PLOT CONTROL
QSCAL      0. HYDROGRAPH PLOT SCALE
IPNCH      0  PUNCH COMPUTED HYDROGRAPH
IOUT       22 SAVE HYDROGRAPH ON THIS UNIT
ISAV1      1  FIRST ORDINATE PUNCHED OR SAVED
ISAV2     1500 LAST ORDINATE PUNCHED OR SAVED
TIMINT     0.083 TIME INTERVAL IN HOURS

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350 KK      Iw359

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```

351 KO      OUTPUT CONTROL VARIABLES
IPRNT      5  PRINT CONTROL
IPLOT      0  PLOT CONTROL
QSCAL      0. HYDROGRAPH PLOT SCALE
IPNCH      1  PUNCH COMPUTED HYDROGRAPH
IOUT       22 SAVE HYDROGRAPH ON THIS UNIT
ISAV1      1  FIRST ORDINATE PUNCHED OR SAVED
ISAV2     1500 LAST ORDINATE PUNCHED OR SAVED
TIMINT     0.083 TIME INTERVAL IN HOURS

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357 KK      CIW359

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358 KO      OUTPUT CONTROL VARIABLES
IPRNT      5  PRINT CONTROL
IPLOT      0  PLOT CONTROL
QSCAL      0. HYDROGRAPH PLOT SCALE
IPNCH      0  PUNCH COMPUTED HYDROGRAPH
IOUT       22 SAVE HYDROGRAPH ON THIS UNIT
ISAV1      1  FIRST ORDINATE PUNCHED OR SAVED
ISAV2     1500 LAST ORDINATE PUNCHED OR SAVED
TIMINT     0.083 TIME INTERVAL IN HOURS

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360 KK      RIW359

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361 KO      OUTPUT CONTROL VARIABLES  
          IPRNT        5    PRINT CONTROL  
          IPLOT        0    PLOT CONTROL  
          QSCAL        0.    HYDROGRAPH PLOT SCALE  
          IPNCH        0    PUNCH COMPUTED HYDROGRAPH  
          IOUT        22    SAVE HYDROGRAPH ON THIS UNIT  
          ISAV1        1    FIRST ORDINATE PUNCHED OR SAVED  
          ISAV2        1500 LAST ORDINATE PUNCHED OR SAVED  
          TIMINT       0.083 TIME INTERVAL IN HOURS

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\*      Iw389    \*  
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367 KO      OUTPUT CONTROL VARIABLES  
          IPRNT        5    PRINT CONTROL  
          IPLOT        0    PLOT CONTROL  
          QSCAL        0.    HYDROGRAPH PLOT SCALE  
          IPNCH        1    PUNCH COMPUTED HYDROGRAPH  
          IOUT        22    SAVE HYDROGRAPH ON THIS UNIT  
          ISAV1        1    FIRST ORDINATE PUNCHED OR SAVED  
          ISAV2        1500 LAST ORDINATE PUNCHED OR SAVED  
          TIMINT       0.083 TIME INTERVAL IN HOURS

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\*      SSR190   \*  
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374 KO      OUTPUT CONTROL VARIABLES  
          IPRNT        5    PRINT CONTROL  
          IPLOT        0    PLOT CONTROL  
          QSCAL        0.    HYDROGRAPH PLOT SCALE  
          IPNCH        1    PUNCH COMPUTED HYDROGRAPH  
          IOUT        22    SAVE HYDROGRAPH ON THIS UNIT  
          ISAV1        1    FIRST ORDINATE PUNCHED OR SAVED  
          ISAV2        1500 LAST ORDINATE PUNCHED OR SAVED  
          TIMINT       0.083 TIME INTERVAL IN HOURS

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\*      RIW389   \*  
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381 KO      OUTPUT CONTROL VARIABLES  
          IPRNT        5    PRINT CONTROL  
          IPLOT        0    PLOT CONTROL  
          QSCAL        0.    HYDROGRAPH PLOT SCALE  
          IPNCH        0    PUNCH COMPUTED HYDROGRAPH  
          IOUT        22    SAVE HYDROGRAPH ON THIS UNIT  
          ISAV1        1    FIRST ORDINATE PUNCHED OR SAVED  
          ISAV2        1500 LAST ORDINATE PUNCHED OR SAVED  
          TIMINT       0.083 TIME INTERVAL IN HOURS

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\*      Iw394    \*  
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387 KO      OUTPUT CONTROL VARIABLES  
          IPRNT        5    PRINT CONTROL  
          IPLOT        0    PLOT CONTROL  
          QSCAL        0.    HYDROGRAPH PLOT SCALE  
          IPNCH        1    PUNCH COMPUTED HYDROGRAPH  
          IOUT        22    SAVE HYDROGRAPH ON THIS UNIT  
          ISAV1        1    FIRST ORDINATE PUNCHED OR SAVED  
          ISAV2        1500 LAST ORDINATE PUNCHED OR SAVED  
          TIMINT       0.083 TIME INTERVAL IN HOURS

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\* RIW394 \*  
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394 KO        OUTPUT CONTROL VARIABLES  
              IPRNT        5    PRINT CONTROL  
              IPLOT        0    PLOT CONTROL  
              QSCAL        0.    HYDROGRAPH PLOT SCALE  
              IPNCH        0    PUNCH COMPUTED HYDROGRAPH  
              IOUT         22    SAVE HYDROGRAPH ON THIS UNIT  
              ISAV1        1    FIRST ORDINATE PUNCHED OR SAVED  
              ISAV2        1500    LAST ORDINATE PUNCHED OR SAVED  
              TIMINT       0.083    TIME INTERVAL IN HOURS

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\* Iw390A \*  
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400 KO        OUTPUT CONTROL VARIABLES  
              IPRNT        5    PRINT CONTROL  
              IPLOT        0    PLOT CONTROL  
              QSCAL        0.    HYDROGRAPH PLOT SCALE  
              IPNCH        1    PUNCH COMPUTED HYDROGRAPH  
              IOUT         22    SAVE HYDROGRAPH ON THIS UNIT  
              ISAV1        1    FIRST ORDINATE PUNCHED OR SAVED  
              ISAV2        1500    LAST ORDINATE PUNCHED OR SAVED  
              TIMINT       0.083    TIME INTERVAL IN HOURS

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\* C390A \*  
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407 KO        OUTPUT CONTROL VARIABLES  
              IPRNT        5    PRINT CONTROL  
              IPLOT        0    PLOT CONTROL  
              QSCAL        0.    HYDROGRAPH PLOT SCALE  
              IPNCH        0    PUNCH COMPUTED HYDROGRAPH  
              IOUT         22    SAVE HYDROGRAPH ON THIS UNIT  
              ISAV1        1    FIRST ORDINATE PUNCHED OR SAVED  
              ISAV2        1500    LAST ORDINATE PUNCHED OR SAVED  
              TIMINT       0.083    TIME INTERVAL IN HOURS

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\* R390A \*  
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410 KO        OUTPUT CONTROL VARIABLES  
              IPRNT        5    PRINT CONTROL  
              IPLOT        0    PLOT CONTROL  
              QSCAL        0.    HYDROGRAPH PLOT SCALE  
              IPNCH        0    PUNCH COMPUTED HYDROGRAPH  
              IOUT         22    SAVE HYDROGRAPH ON THIS UNIT  
              ISAV1        1    FIRST ORDINATE PUNCHED OR SAVED  
              ISAV2        1500    LAST ORDINATE PUNCHED OR SAVED  
              TIMINT       0.083    TIME INTERVAL IN HOURS

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\* Iw390 \*  
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416 KO        OUTPUT CONTROL VARIABLES  
              IPRNT        5    PRINT CONTROL  
              IPLOT        0    PLOT CONTROL  
              QSCAL        0.    HYDROGRAPH PLOT SCALE  
              IPNCH        1    PUNCH COMPUTED HYDROGRAPH  
              IOUT         22    SAVE HYDROGRAPH ON THIS UNIT  
              ISAV1        1    FIRST ORDINATE PUNCHED OR SAVED  
              ISAV2        1500    LAST ORDINATE PUNCHED OR SAVED  
              TIMINT       0.083    TIME INTERVAL IN HOURS

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\* IW392 \*  
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423 KO OUTPUT CONTROL VARIABLES  
IPRNT 5 PRINT CONTROL  
IPLOT 0 PLOT CONTROL  
QSCAL 0. HYDROGRAPH PLOT SCALE  
IPNCH 1 PUNCH COMPUTED HYDROGRAPH  
IOUT 22 SAVE HYDROGRAPH ON THIS UNIT  
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
ISAV2 1500 LAST ORDINATE PUNCHED OR SAVED  
TIMINT 0.083 TIME INTERVAL IN HOURS

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\* CIW390 \*  
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430 KO OUTPUT CONTROL VARIABLES  
IPRNT 5 PRINT CONTROL  
IPLOT 0 PLOT CONTROL  
QSCAL 0. HYDROGRAPH PLOT SCALE  
IPNCH 0 PUNCH COMPUTED HYDROGRAPH  
IOUT 22 SAVE HYDROGRAPH ON THIS UNIT  
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
ISAV2 1500 LAST ORDINATE PUNCHED OR SAVED  
TIMINT 0.083 TIME INTERVAL IN HOURS

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\* RIW390 \*  
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438 KO OUTPUT CONTROL VARIABLES  
IPRNT 5 PRINT CONTROL  
IPLOT 0 PLOT CONTROL  
QSCAL 0. HYDROGRAPH PLOT SCALE  
IPNCH 0 PUNCH COMPUTED HYDROGRAPH  
IOUT 22 SAVE HYDROGRAPH ON THIS UNIT  
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
ISAV2 1500 LAST ORDINATE PUNCHED OR SAVED  
TIMINT 0.083 TIME INTERVAL IN HOURS

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\* IW388 \*  
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444 KO OUTPUT CONTROL VARIABLES  
IPRNT 5 PRINT CONTROL  
IPLOT 0 PLOT CONTROL  
QSCAL 0. HYDROGRAPH PLOT SCALE  
IPNCH 1 PUNCH COMPUTED HYDROGRAPH  
IOUT 22 SAVE HYDROGRAPH ON THIS UNIT  
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
ISAV2 1500 LAST ORDINATE PUNCHED OR SAVED  
TIMINT 0.083 TIME INTERVAL IN HOURS

\*\*\* \*\*

\*\*\*\*\*  
\*  
\* CIW388 \*  
\*  
\*\*\*\*\*

451 KO OUTPUT CONTROL VARIABLES  
IPRNT 5 PRINT CONTROL  
IPLOT 0 PLOT CONTROL  
QSCAL 0. HYDROGRAPH PLOT SCALE  
IPNCH 0 PUNCH COMPUTED HYDROGRAPH  
IOUT 22 SAVE HYDROGRAPH ON THIS UNIT  
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED

ISAV2 1500 LAST ORDINATE PUNCHED OR SAVED  
TIMINT 0.083 TIME INTERVAL IN HOURS

\*\*\* \*\*

\*\*\*\*\*  
\* \*  
453 KK \* RIW388 \*  
\* \*  
\*\*\*\*\*

454 KO OUTPUT CONTROL VARIABLES  
IPRNT 5 PRINT CONTROL  
IPLOT 0 PLOT CONTROL  
QSCAL 0. HYDROGRAPH PLOT SCALE  
IPNCH 0 PUNCH COMPUTED HYDROGRAPH  
IOUT 22 SAVE HYDROGRAPH ON THIS UNIT  
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
ISAV2 1500 LAST ORDINATE PUNCHED OR SAVED  
TIMINT 0.083 TIME INTERVAL IN HOURS

\*\*\* \*\*

\*\*\*\*\*  
\* \*  
459 KK \* IW357 \*  
\* \*  
\*\*\*\*\*

460 KO OUTPUT CONTROL VARIABLES  
IPRNT 5 PRINT CONTROL  
IPLOT 0 PLOT CONTROL  
QSCAL 0. HYDROGRAPH PLOT SCALE  
IPNCH 1 PUNCH COMPUTED HYDROGRAPH  
IOUT 22 SAVE HYDROGRAPH ON THIS UNIT  
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
ISAV2 1500 LAST ORDINATE PUNCHED OR SAVED  
TIMINT 0.083 TIME INTERVAL IN HOURS

\*\*\* \*\*

\*\*\*\*\*  
\* \*  
466 KK \* CIW357 \*  
\* \*  
\*\*\*\*\*

467 KO OUTPUT CONTROL VARIABLES  
IPRNT 5 PRINT CONTROL  
IPLOT 0 PLOT CONTROL  
QSCAL 0. HYDROGRAPH PLOT SCALE  
IPNCH 0 PUNCH COMPUTED HYDROGRAPH  
IOUT 22 SAVE HYDROGRAPH ON THIS UNIT  
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
ISAV2 1500 LAST ORDINATE PUNCHED OR SAVED  
TIMINT 0.083 TIME INTERVAL IN HOURS

\*\*\* \*\*

\*\*\*\*\*  
\* \*  
475 KK \* RIW357 \*  
\* \*  
\*\*\*\*\*

476 KO OUTPUT CONTROL VARIABLES  
IPRNT 5 PRINT CONTROL  
IPLOT 0 PLOT CONTROL  
QSCAL 0. HYDROGRAPH PLOT SCALE  
IPNCH 0 PUNCH COMPUTED HYDROGRAPH  
IOUT 22 SAVE HYDROGRAPH ON THIS UNIT  
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
ISAV2 1500 LAST ORDINATE PUNCHED OR SAVED  
TIMINT 0.083 TIME INTERVAL IN HOURS

\*\*\* \*\*

\*\*\*\*\*  
\* \*  
481 KK \* IW353 \*  
\* \*  
\*\*\*\*\*

482 KO OUTPUT CONTROL VARIABLES  
IPRNT 5 PRINT CONTROL  
IPLOT 0 PLOT CONTROL

```

                                wtec6-addendum-mod1.out
QSCAL      0.  HYDROGRAPH PLOT SCALE
IPNCH      1  PUNCH COMPUTED HYDROGRAPH
IOUT       22 SAVE HYDROGRAPH ON THIS UNIT
ISAV1      1  FIRST ORDINATE PUNCHED OR SAVED
ISAV2     1500 LAST ORDINATE PUNCHED OR SAVED
TIMINT     0.083 TIME INTERVAL IN HOURS

```

\*\*\* \*\*

```

*****
*
* CIW353 *
*
*****

```

```

489 KO      OUTPUT CONTROL VARIABLES
            IPRNT      5  PRINT CONTROL
            IPLOT      0  PLOT CONTROL
            QSCAL      0.  HYDROGRAPH PLOT SCALE
            IPNCH      0  PUNCH COMPUTED HYDROGRAPH
            IOUT       22 SAVE HYDROGRAPH ON THIS UNIT
            ISAV1      1  FIRST ORDINATE PUNCHED OR SAVED
            ISAV2     1500 LAST ORDINATE PUNCHED OR SAVED
            TIMINT     0.083 TIME INTERVAL IN HOURS

```

\*\*\* \*\*

```

*****
*
* RIW353 *
*
*****

```

```

492 KO      OUTPUT CONTROL VARIABLES
            IPRNT      5  PRINT CONTROL
            IPLOT      0  PLOT CONTROL
            QSCAL      0.  HYDROGRAPH PLOT SCALE
            IPNCH      0  PUNCH COMPUTED HYDROGRAPH
            IOUT       22 SAVE HYDROGRAPH ON THIS UNIT
            ISAV1      1  FIRST ORDINATE PUNCHED OR SAVED
            ISAV2     1500 LAST ORDINATE PUNCHED OR SAVED
            TIMINT     0.083 TIME INTERVAL IN HOURS

```

\*\*\* \*\*

```

*****
*
* IW350 *
*
*****

```

```

498 KO      OUTPUT CONTROL VARIABLES
            IPRNT      5  PRINT CONTROL
            IPLOT      0  PLOT CONTROL
            QSCAL      0.  HYDROGRAPH PLOT SCALE
            IPNCH      1  PUNCH COMPUTED HYDROGRAPH
            IOUT       22 SAVE HYDROGRAPH ON THIS UNIT
            ISAV1      1  FIRST ORDINATE PUNCHED OR SAVED
            ISAV2     1500 LAST ORDINATE PUNCHED OR SAVED
            TIMINT     0.083 TIME INTERVAL IN HOURS

```

\*\*\* \*\*

```

*****
*
* CIW350 *
*
*****

```

```

505 KO      OUTPUT CONTROL VARIABLES
            IPRNT      5  PRINT CONTROL
            IPLOT      0  PLOT CONTROL
            QSCAL      0.  HYDROGRAPH PLOT SCALE
            IPNCH      0  PUNCH COMPUTED HYDROGRAPH
            IOUT       22 SAVE HYDROGRAPH ON THIS UNIT
            ISAV1      1  FIRST ORDINATE PUNCHED OR SAVED
            ISAV2     1500 LAST ORDINATE PUNCHED OR SAVED
            TIMINT     0.083 TIME INTERVAL IN HOURS

```

1

RUNOFF SUMMARY  
FLOW IN CUBIC FEET PER SECOND  
TIME IN HOURS, AREA IN SQUARE MILES

OPERATION	STATION	PEAK FLOW	TIME OF PEAK	AVERAGE FLOW FOR MAXIMUM PERIOD			BASIN AREA	MAXIMUM STAGE	TIME OF MAX STAGE
				6-HOUR	24-HOUR	72-HOUR			
+	HYDROGRAPH AT								
+	IW371	103.	4.08	12.	3.	1.	0.05		

+	DIVERSION TO	DO371	77.	4.08	6.	2.	1.	0.05
+	HYDROGRAPH AT	D371	26.	4.08	5.	1.	0.	0.05
+	ROUTED TO	RIW371	15.	6.50	5.	1.	0.	0.05
+	HYDROGRAPH AT	IW366	1107.	4.58	209.	52.	17.	1.06
+	2 COMBINED AT	CIW366	1105.	4.58	213.	54.	18.	1.07
+	ROUTED TO	RIW366	957.	5.00	212.	54.	18.	1.07
+	HYDROGRAPH AT	IW377	153.	4.25	22.	6.	2.	0.11
+	ROUTED TO	SSR010	121.	4.50	22.	6.	2.	0.11
+	ROUTED TO	RIW377	98.	5.25	22.	6.	2.	0.11
+	HYDROGRAPH AT	IW374	758.	4.25	97.	24.	8.	0.39
+	HYDROGRAPH AT	D371	77.	4.08	6.	2.	1.	0.05
+	ROUTED TO	RD371	40.	4.33	6.	2.	1.	0.05
+	3 COMBINED AT	CIW374	784.	4.25	124.	31.	10.	0.54
+	ROUTED TO	RIW374	404.	5.92	121.	31.	10.	0.54
+	HYDROGRAPH AT	IW386	395.	4.58	90.	23.	8.	0.37
+	ROUTED TO	RIW386	301.	5.42	89.	23.	8.	0.37
+	HYDROGRAPH AT	IW382	738.	4.33	131.	33.	11.	0.62
+	2 COMBINED AT	CIW382	685.	4.33	211.	54.	18.	0.99
+	DIVERSION TO	DO382B	180.	3.92	112.	29.	10.	0.99
+	HYDROGRAPH AT	D382B	505.	4.33	99.	25.	8.	0.99
+	DIVERSION TO	DO382A	226.	4.33	20.	5.	2.	0.99
+	HYDROGRAPH AT	D382A	279.	4.33	79.	20.	7.	0.99
+	ROUTED TO	RIW382	253.	4.83	79.	20.	7.	0.99
+	HYDROGRAPH AT	IW384	184.	4.17	21.	5.	2.	0.08
+	HYDROGRAPH AT	D382A	226.	4.33	20.	5.	2.	0.99
+	ROUTED TO	RD382A	196.	4.42	20.	5.	2.	0.99
+	2 COMBINED AT	CIW384	390.	4.33	47.	12.	4.	0.41
+	ROUTED TO	RIW384	310.	4.67	47.	12.	4.	0.41
+	HYDROGRAPH AT	IW381	454.	4.50	98.	25.	8.	0.38
+	HYDROGRAPH AT	D382B	180.	3.92	112.	29.	10.	0.99
+	2 COMBINED AT	CIW381	619.	4.50	208.	54.	18.	0.64
+	ROUTED TO	RIW381	574.	4.83	207.	54.	18.	0.64
+	3 COMBINED AT	C359*	990.	4.83	320.	82.	27.	1.45
+	ROUTED TO	R359*	937.	5.17	317.	82.	27.	1.45
+	HYDROGRAPH AT	IW359	2496.	4.75	537.	135.	45.	2.91

+	4 COMBINED AT	CIW359	3423.	5.00	1059.	274.	91.	5.97
+	ROUTED TO	RIW359	3233.	5.33	1045.	274.	91.	5.97
+	HYDROGRAPH AT	IW389	168.	4.42	35.	9.	3.	0.13
+	ROUTED TO	SSR190	166.	4.42	35.	9.	3.	0.13
+	ROUTED TO	RIW389	102.	5.75	34.	9.	3.	0.13
+	HYDROGRAPH AT	IW394	2700.	5.08	777.	198.	66.	4.17
+	ROUTED TO	RIW394	2613.	5.25	776.	198.	66.	4.17
+	HYDROGRAPH AT	IW390A	771.	4.75	197.	50.	17.	0.90
+	2 COMBINED AT	C390A	3030.	5.25	936.	239.	80.	5.07
+	ROUTED TO	R390A	2925.	5.42	935.	239.	80.	5.07
+	HYDROGRAPH AT	IW390	221.	4.75	61.	16.	5.	0.31
+	HYDROGRAPH AT	IW392	1985.	4.33	262.	66.	22.	1.11
+	3 COMBINED AT	CIW390	3093.	5.42	1193.	305.	102.	6.50
+	ROUTED TO	SSR160	3068.	5.50	1193.	305.	102.	6.50
+	ROUTED TO	RIW390	2969.	5.75	1183.	305.	102.	6.50
+	HYDROGRAPH AT	IW388	1107.	4.83	303.	76.	25.	1.23
+	3 COMBINED AT	CIW388	3532.	5.67	1464.	378.	126.	7.85
+	ROUTED TO	RIW388	3450.	5.92	1457.	378.	126.	7.85
+	HYDROGRAPH AT	IW357	879.	4.50	184.	46.	15.	0.75
+	2 COMBINED AT	CIW357	3583.	5.92	1595.	416.	139.	8.60
+	DIVERSION TO	DO357	937.	5.92	416.	108.	36.	8.60
+	HYDROGRAPH AT	D357	2646.	5.92	1179.	308.	103.	8.60
+	ROUTED TO	RIW357	2608.	6.42	1169.	308.	103.	8.60
+	HYDROGRAPH AT	IW353	2512.	4.83	629.	158.	53.	2.80
+	3 COMBINED AT	CIW353	5467.	5.50	2558.	682.	227.	15.12
+	ROUTED TO	RIW353	5298.	5.83	2534.	682.	227.	15.12
+	HYDROGRAPH AT	IW350	1410.	5.08	457.	116.	39.	2.17
+	2 COMBINED AT	CIW350	6011.	5.75	2855.	768.	256.	17.30

\*\*\* NORMAL END OF HEC-1 \*\*\*

1\*\*\*\*\*
\*
\* FLOOD HYDROGRAPH PACKAGE (HEC-1)
\* JUN 1998
\* VERSION 4.1
\* RUN DATE 06AUG11 TIME 13:25:46
\*\*\*\*\*

\*\*\*\*\*
\*
\* U.S. ARMY CORPS OF ENGINEERS
\* HYDROLOGIC ENGINEERING CENTER
\* 609 SECOND STREET
\* DAVIS, CALIFORNIA 95616
\* (916) 756-1104
\*\*\*\*\*

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THIS PROGRAM REPLACES ALL PREVIOUS VERSIONS OF HEC-1 KNOWN AS HEC1 (JAN 73), HEC1GS, HEC1DB, AND HEC1KW.

THE DEFINITIONS OF VARIABLES -RTIMP- AND -RTIOR- HAVE CHANGED FROM THOSE USED WITH THE 1973-STYLE INPUT STRUCTURE.
THE DEFINITION OF -AMSKK- ON RM-CARD WAS CHANGED WITH REVISIONS DATED 28 SEP 81. THIS IS THE FORTRAN77 VERSION
NEW OPTIONS: DAMBREAK OUTFLOW SUBMERGENCE, SINGLE EVENT DAMAGE CALCULATION, DSS:WRITE STAGE FREQUENCY,
DSS:READ TIME SERIES AT DESIRED CALCULATION INTERVAL LOSS RATE:GREEN AND AMPT INFILTRATION
KINEMATIC WAVE: NEW FINITE DIFFERENCE ALGORITHM

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10
1 ID Wittmann Phase IV Floodplain Delineation Study
2 ID August 2011
3 ID Mod2 removes subbasin Iw358 and CIW357
4 ID Mod2 replaces Iw353 with Iw353A and updates RIW359
5 ID Mod2 truncates model at CW353A
6 ID \*\*\*\*\*
7 ID \*\* -ADDENDUM-ADDENDUM-ADDENDUM-ADDENDUM-ADDENDUM-ADDENDUM-ADDENDUM-ADDENDUM- \*\*
8 ID \*\* -ADDENDUM-ADDENDUM-ADDENDUM-ADDENDUM-ADDENDUM-ADDENDUM-ADDENDUM-ADDENDUM- \*\*
9 ID \*\*\*\*\*
10 ID \*\*
11 ID \*\* ADDENDUM TO THE WITTMANN AREA DRAINAGE MASTER STUDY UPDATE HYDROLOGY \*\*
12 ID \*\*
13 ID \*\*\*\*\*
14 ID \*\* -ADDENDUM-ADDENDUM-ADDENDUM-ADDENDUM-ADDENDUM-ADDENDUM-ADDENDUM-ADDENDUM- \*\*
15 ID \*\* -ADDENDUM-ADDENDUM-ADDENDUM-ADDENDUM-ADDENDUM-ADDENDUM-ADDENDUM-ADDENDUM- \*\*
16 ID \*\*\*\*\*
17 ID \*\*
18 ID \*\* THE FOLLOWING CHANGES HAVE BEEN MADE TO THE WITTMANN HYDROLOGY MODELS \*\*
19 ID \*\* DATED OCTOBER 2004: \*\*
20 ID \*\*
21 ID \*\* -Diversions added after CTW480 \*\*
22 ID \*\* -Main flow (5%) to CTW478 \*\*
23 ID \*\* -Diverted flow (95%) to CWI584 \*\*
24 ID \*\* -Added concentration point CWI584 combining RD480 and WI584 \*\*
25 ID \*\* -Hard coding was added to CTW478 \*\*
26 ID \*\* -Route added after D480 (Diversions Recovery) \*\*
27 ID \*\* -Route RWI584 was adjusted \*\*
28 ID \*\* -Storage Route SCP050 was adjusted \*\*
29 ID \*\* -Route R508\* was adjusted \*\*
30 ID \*\* -Hard coding for CWI510 was adjusted \*\*
31 ID \*\*
32 ID \*\* -Diversions D618 was moved from after CPI618 to before RDCP12 \*\*
33 ID \*\* -Updated route RD618 information \*\*
34 ID \*\* -Hard coding on CPI618 was adjusted \*\*
35 ID \*\* -Hard coding on CPI615 was adjusted \*\*
36 ID \*\*
37 ID \*\* -Route RPI639 changed from following US60 to CPI636 to route \*\*
38 ID \*\* through US60/RR structures and combine with CWI506 \*\*
39 ID \*\* -Cards PI639, D524, RD524, CPI639 and RPI639 were changed from \*\*
40 ID \*\* following R624A to follow RWI524 \*\*
41 ID \*\* -CPI636 was removed-no longer needed to combine PI636 & RPI639 \*\*
42 ID \*\* -RPI639 information was changed to reflect new route \*\*
43 ID \*\* -CWI506 changed from combining 3 to combine 4: \*\*
44 ID \*\* RWI525, RWI524, RPI639 & WI506 \*\*
45 ID \*\* -Hard coding on CWI506 was adjusted \*\*
46 ID \*\* -Hard Coding on CWI500 was adjusted \*\*
47 ID \*\*
48 ID \*\*\*\*\*
49 ID \*\* -ADDENDUM-ADDENDUM-ADDENDUM-ADDENDUM-ADDENDUM-ADDENDUM-ADDENDUM-ADDENDUM- \*\*
50 ID \*\* -ADDENDUM-ADDENDUM-ADDENDUM-ADDENDUM-ADDENDUM-ADDENDUM-ADDENDUM-ADDENDUM- \*\*
51 ID \*\*\*\*\*
52 ID \*\*
53 ID \*\*
54 ID \*\*
55 ID \*\*\*\*\*

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10
56 ID \*\*\*\*\*
57 ID \*\*
58 ID \*\* WITTMANN AREA DRAINAGE MASTER STUDY UPDATE-HYDROLOGY MODEL 2004 \*\*
59 ID \*\*
60 ID \*\*\*\*\*
61 ID \*\*
62 ID \*\*
63 ID PROJECT: Wittmann ADMS Update
64 ID CLIENT: Flood Control District of Maricopa County
Page 1

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65 ID wtec6-addendum-mod2.out
66 ID PREPARED BY: Entellus, Inc.
67 ID PROJECT No: FCD 2002C029 Entellus 310.032
68 ID FILE NAME: WTEC6-addendum.hcl CREATED DATE: JUL 01, 2004
69 ID MODIFIED DATE: MAY 04, 2005
70 ID
71 ID STORM:
72 ID 100-year 6-hour Storm
73 ID
74 ID DEVELOPMENT CONDITIONS:
75 ID Existing Conditions
76 ID
77 ID MODELING ASSUMPTIONS:
78 ID It was assumed that the US60 did not have adequate
79 ID storage to cause any significant attenuation.
80 ID
81 ID The assumption was made that the CAP Canal embankment
82 ID would not be breached under a large flood event.
83 ID
84 ID The assumption was made that the Beardsley Canal would
85 ID not fail under a large storm event (Per District
86 ID Instruction). In addition, the berm north of the
87 ID Beardsley canal and east of US60 was assumed to fail
88 ID (per district Instruction).
89 ID
90 ID For both the CAP and Beardsley Canals, once the berm
91 ID elevation was reached weir flow was assumed. It was
92 ID also assumed that any weir flow over the canal that
93 ID might enter the canal and be diverted out of the study
94 ID area was insignificant, and thus was ignored. In other
95 ID words all weir flow over the canal embankment reaches
96 ID the downstream concentration point.
97 ID
98 ID Typical x-sects were developed, and it was assumed
99 ID that a typical x-sect could adequately represent
100 ID various reaches.
101 ID
102 ID Time-Area Relations were used base on the District's
103 ID Hydrology Manual criteria. Two Time-Area Relation
104 ID Curves were utilized:
105 ID -Urban
106 ID -Natural
107 ID The Time-Area Relation Curves were taken directly
108 ID from the manual
109 ID
110 ID MODELING METHODS:
110 ID This model utilizes QI cards to input the Padelford
110 ID HEC-1 INPUT

```

```

1 LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10
111 ID hydrographs from the 100-year 6-hour existing
112 ID conditions model developed by A-N West Inc. for the
113 ID Padelford Wash Floodplain Delineation Study. The
114 ID hydrograph was altered from its original form
115 ID (2-minute interval to 5-minute interval) through
116 ID simple interpolation.
117 ID
118 ID Clark Unit Hydrographs were used for all subbasins
119 ID except the two subbasins directly upstream of the
120 ID Bonita Dam (PD726B and PD740 use S-graphs). The UC
121 ID parameters were calculated using the WMS7.0 software.
122 ID
123 ID For Basins PD726B and PD740 S-Graphs were utilized per
124 ID the request of the FCDMC. Limited details regarding
125 ID the calculations of the S-graphs can be found in the
126 ID model by the basin KK card. For full details of the
127 ID S-graph calculations refer to the Appendix.
128 ID
129 ID Normal Depth routing was used for all routing reaches.
130 ID
131 ID Hard coding was used to account for the percentage of
132 ID area associated with a diversion. Because of the use
133 ID of JD cards and aerial reduction, hard coding was
134 ID necessary to properly account for area. For a given
135 ID diversion a percentage of the flow is routed to two
136 ID different locations. The same percentage of area
137 ID follows that diverted flow. In addition the area
138 ID downstream of the main path is reduced or increased and
139 ID is hard coded to account for the loss or gain of area.
140 ID Hard coding was performed based on the 6-hour existing
141 ID conditions model.
142 ID
143 ID Stage-storage was developed for all the structures
144 ID along the CAP Canal, as well as along the Beardsley
145 ID Canal. In addition several stage-storage locations
146 ID were developed for areas with significant storage
147 ID along the SR74. No storage was modeled along the US60
148 ID and railroad, but the culverts were analyzed for
149 ID diversion potential. Diversions were placed in the
150 ID model where deemed appropriate. See appendix for
151 ID details.
152 ID
153 ID FLO-2D was utilized to calculate the split flows at
154 ID concentration points CIW351, CIW357, CIW363 and CWI576.
155 ID See appendix for modeling details.
156 ID
157 ID *****
158 ID *****
159 ID

```

```

160 *DIAGRAM
161 IT 5 0 1500
162 IO 5
163 IN 15 0
164 JD 3.40 0.01
165 * 6-hour distribution

```

LINE	ID	1	2	3	4	5	6	7	8	9	10
164	PC	0.000	0.008	0.016	0.025	0.033	0.041	0.050	0.058	0.066	0.074
165	PC	0.087	0.099	0.118	0.138	0.216	0.377	0.834	0.911	0.931	0.950
166	PC	0.962	0.972	0.983	0.991	1.000					
	*										
167	JD	3.378	0.50								
168	JD	3.314	2.80								
169	PC	0.000	0.009	0.016	0.025	0.034	0.042	0.051	0.059	0.067	0.076
170	PC	0.087	0.100	0.120	0.163	0.252	0.451	0.694	0.837	0.900	0.938
171	PC	0.950	0.963	0.975	0.988	1.000					
	*										
172	JD	3.135	16.00								
173	PC	0.000	0.009	0.020	0.030	0.048	0.063	0.076	0.090	0.105	0.119
174	PC	0.135	0.152	0.175	0.222	0.304	0.472	0.670	0.796	0.868	0.912
175	PC	0.946	0.960	0.973	0.987	1.000					
	*										
176	JD	2.761	90.00								
177	PC	0.000	0.021	0.035	0.051	0.071	0.087	0.105	0.125	0.143	0.160
178	PC	0.179	0.201	0.232	0.281	0.364	0.500	0.658	0.773	0.841	0.888
179	PC	0.927	0.945	0.964	0.982	1.000					
	*										
180	JD	1.938	500.00								
181	PC	0.000	0.024	0.043	0.059	0.078	0.098	0.119	0.141	0.162	0.186
182	PC	0.212	0.239	0.271	0.321	0.408	0.515	0.627	0.735	0.814	0.864
183	PC	0.907	0.930	0.954	0.977	1.000					

\*\*\*\*\*  
 \*\*\*\*\*  
 \*\*\*\*\* BEGINNING OF IONA WASH SUBAREA \*\*\*\*\*  
 \*\*\*\*\*  
 \*\*\*\*\*  
 \*\*\*\*\* BEGINNING OF IONA WASH REGION 1 \*\*\*\*\*  
 \*\*\*\*\*  
 \*\*\*\*\*

184	KK	Iw371									
185	KO	0	0	0.0	1	22					
186	BA	0.0529									
187	LG	0.275	0.25	4.15	0.473	26.25					
188	UC	0.275	0.288								
	*	* Urban Time-Area Relation									
189	UA	0	5	16	30	65	77	84	90	94	97
190	UA	100									

\* Railroad Diversion: Main path to CIW366, Diversion to CIW374  
 \* From Structure RR40, See Appendix D.5 for analysis  
 HEC-1 INPUT

1

LINE	ID	1	2	3	4	5	6	7	8	9	10
191	KK	D371									
192	KO	0	0	0.0	0	22					
193	DT	DO371									
194	DI	0.0	14	79	208	383	823	1374	2014	2724	
195	DQ	0.0	0	55	173	335	763	1302	1930	2632	
	*										
	*										
	*	* ROUTE UPDATED 4/14/4									
196	KK	RIW371									
197	KO	0	0	0.0	0	22					
198	RS	15	FLOW	0.0	0.0						
199	RC	0.04	0.04	0.04	11583	0.0088					
	*	* TSR371									
200	RX	0	142	486	525	651	770	1000	1518		
201	RY	1900	1900	1899	1897	1896	1899	1899.5	1900		
	*										
	*										
202	KK	Iw366									
203	KO	0	0	0.0	1	22					
204	BA	1.0558									
205	LG	0.343	0.25	5.762	0.202	7.207					
206	UC	0.763	0.593								
	*	* Natural Time-Area Relation									
207	UA	0	3	5	8	12	20	43	75	90	96
208	UA	100									
	*										
	*										
	*	* Combines Iw366 and RIW371									
209	KK	CIW366									
210	KO	0	0	0.0	0	22					
	*	* hard coded									
211	HC	2	1.07								
	*										
	*										
	*	* ROUTE UPDATED 4/14/4									
212	KK	RIW366									
213	KO	0	0	0.0	0	22					
214	RS	4	FLOW	0.0	0.0						
215	RC	0.04	0.04	0.04	9657	0.007					
	*	* IWTS30_2									
216	RX	0	6.5	13	57	102	104	109	110		
217	RY	1728	1725.5	1724	1723	1724	1725.5	1727	1728		

\*

1

HEC-1 INPUT

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

218 KK IW377  
 219 KO 0 0 0.0 1 22  
 220 BA 0.1125  
 221 LG 0.304 0.382 5.921 0.188 5.357  
 222 UC 0.408 0.519  
 \* Natural Time-Area Relation  
 223 UA 0 3 5 8 12 20 43 75 90 96  
 224 UA 100

\* \*\*\* STAGE DISCHARGE / STORAGE BEHIND STRUCTURE SR010 (HWY74 CULVERT)

225 KK SSR010  
 226 RS 1 STOR 0  
 227 SQ 0 36 100 160  
 228 SA 0 .05 .41 1.98  
 229 SE 2021.3 2024 2026 2028

\* ROUTE UPDATED 4/14/4

230 KK RIW377  
 231 KO 0 0 0.0 0 22  
 232 RS 6 FLOW 0.0 0.0  
 233 RC 0.04 0.04 0.04 6588 0.0096  
 \* TSR371  
 234 RX 0 142 486 525 651 770 1000 1518  
 235 RY 1900 1900 1899 1897 1896 1899 1899.5 1900

236 KK IW374  
 237 KO 0 0 0.0 1 22  
 238 BA 0.3874  
 239 LG 0.315 0.161 8.147 0.086 12.602  
 240 UC 0.442 0.345  
 \* Natural Time-Area Relation  
 241 UA 0 3 5 8 12 20 43 75 90 96  
 242 UA 100

\* RAILROAD DIVERSION RECOVERY: ALONG RAILROAD FROM STRUCTURE RR40

243 KK D371  
 244 KO 0 0 0.0 0 22  
 245 DR D0371

\* ROUTE UPDATED 4/14/4

HEC-1 INPUT

1

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

246 KK RD371  
 247 KO 0 0 0.0 0 22  
 248 RS 1 FLOW 0.0 0.0  
 249 RC 0.043 0.035 0.043 1409 0.0014  
 \* WITS130  
 250 RX 0 400 456 575 620 650 660 682  
 251 RY 1396.5 1394 1393.5 1393 1392 1394 1395 1396.5

\* Combines RD371, IW374 and RIW377

252 KK CIW374  
 253 KO 0 0 0.0 0 22  
 \* Hard Coded  
 254 HC 3 0.54

\* ROUTE UPDATED 4/14/4

255 KK RIW374  
 256 KO 0 0 0.0 0 22  
 257 RS 15 FLOW 0.0 0.0  
 258 RC 0.04 0.04 0.04 20437 0.0081  
 \* TSR371  
 259 RX 0 142 486 525 651 770 1000 1518  
 260 RY 1900 1900 1899 1897 1896 1899 1899.5 1900

261 KK IW386  
 262 KO 0 0 0.0 1 22  
 263 BA 0.3731  
 264 LG 0.334 0.192 8.439 0.072 5.278  
 265 UC 0.838 0.855  
 \* Natural Time-Area Relation  
 266 UA 0 3 5 8 12 20 43 75 90 96  
 267 UA 100

\* ROUTE UPDATED 4/14/4

268 KK RIW386  
 269 KO 0 0 0.0 0 22  
 270 RS 6 FLOW 0.0 0.0  
 271 RC 0.043 0.035 0.043 10311 0.0078

\* IWTS20\_1  
 RX 0 21 75 93 234 280 301 391  
 RY 1952 1949 1948 1946 1945 1948 1948.5 1952  
 \*  
 \*

HEC-1 INPUT

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

274 KK IW382  
 275 KO 0 0 0.0 1 22  
 276 BA 0.6196  
 277 LG 0.322 0.275 6.319 0.164 8.175  
 278 UC 0.642 0.555  
 \* Urban Time-Area Relation  
 279 UA 0 5 16 30 65 77 84 90 94 97  
 280 UA 100  
 \*  
 \*

\* Combines IW382 and RIW386

281 KK CIW382  
 282 KO 0 0 0.0 0 22  
 283 HC 2  
 \*

\* US60 Diversion: Main path to D382A, Diversion to CIW381  
 \* From Structures US006 and US007, See Appendix D.5 for analysis

284 KK D382B  
 285 KO 0 0 0.0 0 22  
 286 DT D0382B  
 287 DI 0 24 180 10000  
 288 DQ 0 24 180 180  
 \*

\* Railroad Diversion: Main path to C359\*, Diversion to CIW384  
 \* From Structure RR90 and RR100, See Appendix D.5 for analysis

289 KK D382A  
 290 KO 0 0 0.0 0 22  
 291 DT D0382A  
 292 DI 0 14 60 200 315 448 629 1371  
 293 DQ 0 0 0 0 55 174 338 1045  
 \*

\* ROUTE UPDATED 4/14/4

294 KK RIW382  
 295 KO 0 0 0.0 0 22  
 296 RS 3 FLOW 0.0 0.0  
 297 RC 0.04 0.04 0.04 4964 0.012  
 \* TSR371  
 298 RX 0 142 486 525 651 770 1000 1518  
 299 RY 1900 1900 1899 1897 1896 1899 1899.5 1900  
 \*

300 KK IW384  
 301 KO 0 0 0.0 1 22  
 302 BA 0.0825  
 303 LG 0.343 0.15 8.675 0.065 7.206  
 304 UC 0.308 0.296  
 \* Natural Time-Area Relation  
 305 UA 0 3 5 8 12 20 43 75 90 96  
 HEC-1 INPUT

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

306 UA 100  
 \*  
 \*

\* Railroad Diversion Recovery: Outflow from structures RR90 AND RR100

307 KK D382A  
 308 KO 0 0 0.0 0 22  
 309 DR D0382A  
 \*

\* ROUTE UPDATED 4/14/4

310 KK RD382A  
 311 KO 0 0 0.0 0 22  
 312 RS 1 FLOW 0.0 0.0  
 313 RC 0.043 0.035 0.043 1172 0.0068  
 \* WITS130  
 314 RX 0 400 456 575 620 650 660 682  
 315 RY 1396.5 1394 1393.5 1393 1392 1394 1395 1396.5  
 \*

\* Combines RD382A and IW384

316 KK CIW384  
 317 KO 0 0 0.0 0 22  
 \* Hard Coded  
 318 HC 2 0.41  
 \*

\* ROUTE UPDATED 4/14/4

319 KK RIW384  
 320 KO 0 0 0.0 0 22  
 321 RS 3 FLOW 0.0 0.0  
 322 RC 0.04 0.04 0.04 4694 0.0124  
 \* TSR371  
 323 RX 0 142 486 525 651 770 1000 1518  
 324 RY 1900 1900 1899 1897 1896 1899 1899.5 1900

```

*
*
325 KK IW381
326 KO 0 0 0.0 1 22
327 BA 0.3841
328 LG 0.309 0.19 9.152 0.056 11.882
329 UC 0.650 0.799
* Natural Time-Area Relation
330 UA 0 3 5 8 12 20 43 75 90 96
331 UA 100

```

```

*
* This diversion is not routed because the distance traveled from the actual
* point of diversion (not CIW382) to CIW382 is approximately the same as the
* distance from the same point of diversion to CIW381. Thus to avoid double
* counting storage in the routes, a route for the diversion was not included
* in the model.

```

```

* US60 Diversion Recovery: Outflow from structures US006 and US007
HEC-1 INPUT

```

```

1 LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

```

```

332 KK D382B
333 KO 0 0 0.0 0 22
334 DR D0382B
*
* Combines D382B and IW381
335 KK CIW381
* Hard Coded
336 HC 2 0.64
*
* ROUTE UPDATED 4/14/4
337 KK RIW381
338 KO 0 0 0.0 0 22
339 RS 3 FLOW 0.0 0.0
340 RC 0.04 0.04 0.04 4964 0.0096
*
* TSR371
341 RX 0 142 486 525 651 770 1000 1518
342 RY 1900 1900 1899 1897 1896 1899 1899.5 1900

```

```

* Combines RIW381, RIW382 and RIW384

```

```

343 KK C359*
344 KO 0 0 0.0 0 22
345 HC 3 1.45
*
* ROUTE UPDATED 4/14/4
346 KK R359*
347 KO 0 0 0.0 0 22
348 RS 4 FLOW 0.0 0.0
349 RC 0.04 0.04 0.04 9280 0.0103
*
* IWTS30_2
350 RX 0 6.5 13 57 102 104 109 110
351 RY 1728 1725.5 1724 1723 1724 1725.5 1727 1728

```

```

352 KK IW359
353 KO 0 0 0.0 1 22
354 BA 2.912
355 LG 0.345 0.25 5.939 0.187 6.181
356 UC 0.983 0.574
* Natural Time-Area Relation
357 UA 0 3 5 8 12 20 43 75 90 96
358 UA 100

```

```

* Combines IW359, R359*, RIW374 and RIW366
HEC-1 INPUT

```

```

1 LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

```

```

359 KK CIW359
360 KO 0 0 0.0 0 22
361 HC 4
*
* ROUTE UPDATED 4/14/4
362 KK RIW359
363 KO 0 0 0.0 0 22
364 RS 1 FLOW 0.0 0.0
365 RC 0.04 0.04 0.04 4667 0.0051
*
* IWTS30_2 modified
366 RX 0 6.5 13 57 102 104 109 110
367 RY 1730 1725.5 1724 1723 1724 1725.5 1727 1730

```

```

*
* *****
* ***** END OF IONA WASH REGION 1 *****
* *****
*

```

\*\*\*\*\*  
 \*\*\*\*\* BEGINNING OF IONA WASH REGION 3 \*\*\*\*\*  
 \*\*\*\*\*

368	KK	Iw353A												
369	KO	0	0	0.0	1	22								
370	BA	0.7164												
371	LG	0.35	0.25	5.876	0.191	5.0								
372	UC	0.629	0.412											

\* Natural Time-Area Relation  
 UA 0 8 12 20 43 75 90 96  
 UA 100 3 5

\* Combines Iw353A and RIW359

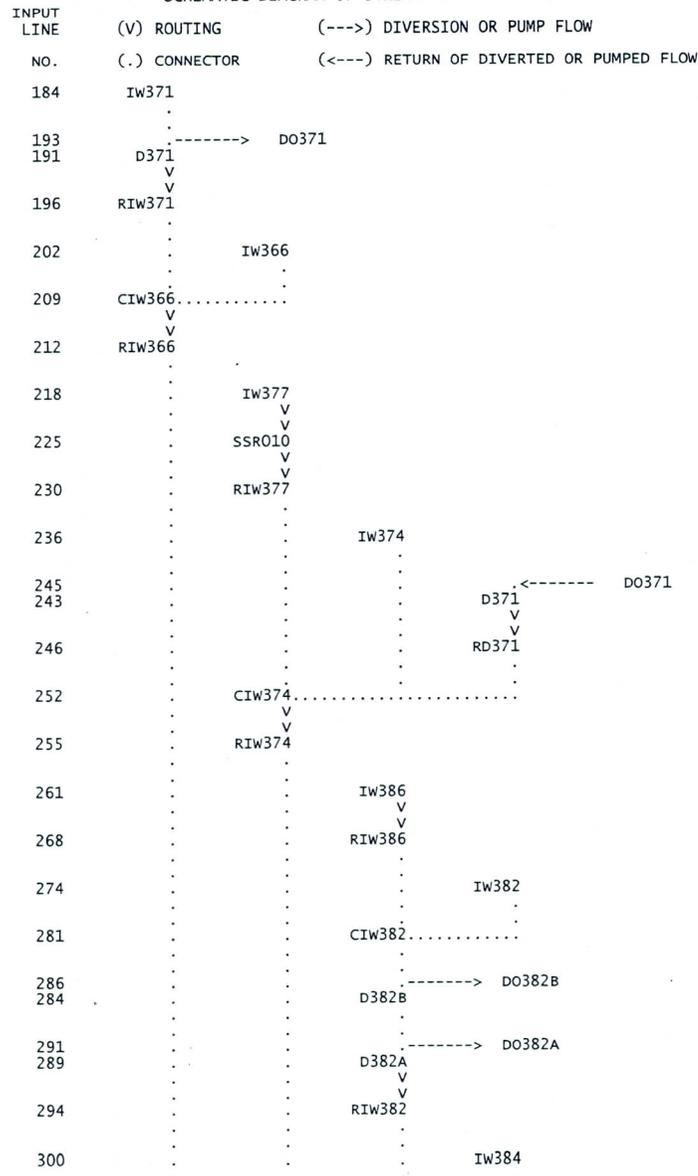
375	KK	Cw353A												
376	KO	0	0	0.0	0	22								
377	HC	2												

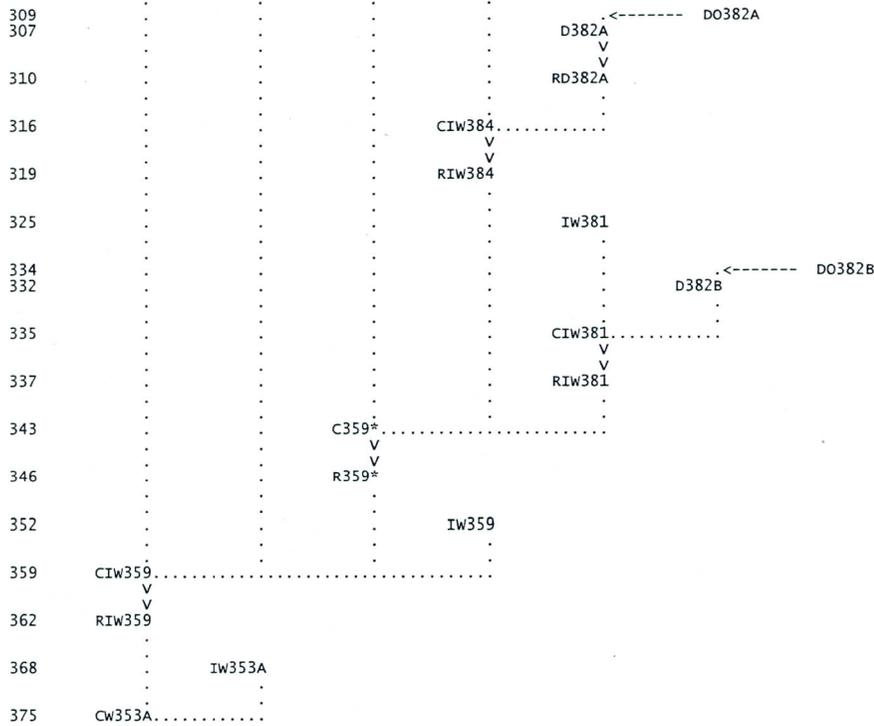
\*\*\*\*\*  
 \*\*\*\*\* END OF IONA WASH REGION 3 \*\*\*\*\*  
 \*\*\*\*\*

378 ZZ

1

SCHEMATIC DIAGRAM OF STREAM NETWORK





(\*\*\*) RUNOFF ALSO COMPUTED AT THIS LOCATION

```

1*****
* FLOOD HYDROGRAPH PACKAGE (HEC-1) *
* JUN 1998 *
* VERSION 4.1 *
* RUN DATE 06AUG11 TIME 13:25:46 *
*****

```

```

*****
* U.S. ARMY CORPS OF ENGINEERS *
* HYDROLOGIC ENGINEERING CENTER *
* 609 SECOND STREET *
* DAVIS, CALIFORNIA 95616 *
* (916) 756-1104 *
*****

```

Wittmann Phase IV Floodplain Delineation Study  
August 2011  
Mod2 removes subbasin Iw358 and CIW357  
Mod2 replaces Iw353 with Iw353A and updates RIW359  
Mod2 truncates model at CW353A

\*\*\*\*\*  
\*\*\_ADDENDUM-ADDENDUM-ADDENDUM-ADDENDUM-ADDENDUM-ADDENDUM-ADDENDUM-ADDENDUM-\*\*  
\*\*\_ADDENDUM-ADDENDUM-ADDENDUM-ADDENDUM-ADDENDUM-ADDENDUM-ADDENDUM-ADDENDUM-\*\*  
\*\*\*\*\*

\*\* ADDENDUM TO THE WITTMANN AREA DRAINAGE MASTER STUDY UPDATE HYDROLOGY \*\*

\*\*\*\*\*  
\*\*\_ADDENDUM-ADDENDUM-ADDENDUM-ADDENDUM-ADDENDUM-ADDENDUM-ADDENDUM-ADDENDUM-\*\*  
\*\*\_ADDENDUM-ADDENDUM-ADDENDUM-ADDENDUM-ADDENDUM-ADDENDUM-ADDENDUM-ADDENDUM-\*\*  
\*\*\*\*\*

\*\* THE FOLLOWING CHANGES HAVE BEEN MADE TO THE WITTMANN HYDROLOGY MODELS \*\*  
DATED OCTOBER 2004:

- \*\* -Diversion added after CTW480 \*\*
- \*\* -Main flow (5%) to CTW478 \*\*
- \*\* -Diverted flow (95%) to CWI584 \*\*
- \*\* -Added concentration point CWI584 combining RD480 and WI584 \*\*
- \*\* -Hard coding was added to CTW478 \*\*
- \*\* -Route added after D480 (Diversion Recovery) \*\*
- \*\* -Route RWI584 was adjusted \*\*
- \*\* -Storage Route SCP050 was adjusted \*\*
- \*\* -Route R508\* was adjusted \*\*
- \*\* -Hard coding for CWI510 was adjusted \*\*
- \*\* -Diversion D618 was moved from after CPI618 to before RDCP12 \*\*
- \*\* -Updated route RD618 information \*\*
- \*\* -Hard coding on CPI618 was adjusted \*\*
- \*\* -Hard coding on CPI615 was adjusted \*\*
- \*\* -Route RPI639 changed from following US60 to CPI636 to route \*\*
- \*\* through US60/RR structures and combine with CWI506 \*\*
- \*\* -Cards PI639, D524, RD524, CPI639 and RPI639 were changed from \*\*
- \*\* following R624A to follow RWI524 \*\*
- \*\* -CPI636 was removed-no longer needed to combine PI636 & RPI639 \*\*
- \*\* -RPI639 information was changed to reflect new route \*\*
- \*\* -CWI506 changed from combining 3 to combine 4: \*\*
- \*\* RWI525, RWI524, RPI639 & WI506 \*\*



wtec6-addendum-mod2.out  
 details.

FLO-2D was utilized to calculate the split flows at  
 concentration points CIW351, CIW357, CIW363 and CWI576.  
 See appendix for modeling details.

\*\*\*\*\*  
 \*\*\*\*\*

161 IO      OUTPUT CONTROL VARIABLES  
           IPRNT        5    PRINT CONTROL  
           IPLOT        0    PLOT CONTROL  
           QSCAL        0.    HYDROGRAPH PLOT SCALE

IT            HYDROGRAPH TIME DATA  
               NMIN        5    MINUTES IN COMPUTATION INTERVAL  
               IDATE        1    0    STARTING DATE  
               ITIME        0000 STARTING TIME  
               NQ           1500 NUMBER OF HYDROGRAPH ORDINATES  
               NDDATE       6    0    ENDING DATE  
               NDTIME       0455 ENDING TIME  
               ICENT        19    CENTURY MARK

              COMPUTATION INTERVAL    0.08 HOURS  
               TOTAL TIME BASE    124.92 HOURS

ENGLISH UNITS  
 DRAINAGE AREA        SQUARE MILES  
 PRECIPITATION DEPTH    INCHES  
 LENGTH, ELEVATION    FEET  
 FLOW                CUBIC FEET PER SECOND  
 STORAGE VOLUME        ACRE-FEET  
 SURFACE AREA        ACRES  
 TEMPERATURE        DEGREES FAHRENHEIT

163 JD      INDEX STORM NO. 1  
           STRM        3.40 PRECIPITATION DEPTH  
           TRDA        0.01 TRANSPOSITION DRAINAGE AREA

164 PI      PRECIPITATION PATTERN  
           0.00    0.00    0.00    0.00    0.00    0.00    0.00    0.00    0.00    0.00  
           0.00    0.00    0.00    0.00    0.00    0.00    0.00    0.00    0.00    0.00  
           0.00    0.00    0.00    0.00    0.00    0.00    0.00    0.00    0.00    0.00  
           0.00    0.00    0.00    0.01    0.01    0.01    0.01    0.01    0.01    0.03  
           0.03    0.03    0.05    0.05    0.05    0.15    0.15    0.15    0.03    0.03  
           0.03    0.01    0.01    0.01    0.01    0.01    0.01    0.00    0.00    0.00  
           0.00    0.00    0.00    0.00    0.00    0.00    0.00    0.00    0.00    0.00  
           0.00    0.00    0.00    0.00    0.00    0.00    0.00    0.00    0.00    0.00

167 JD      INDEX STORM NO. 2  
           STRM        3.38 PRECIPITATION DEPTH  
           TRDA        0.50 TRANSPOSITION DRAINAGE AREA

0 PI      PRECIPITATION PATTERN  
           0.00    0.00    0.00    0.00    0.00    0.00    0.00    0.00    0.00    0.00  
           0.00    0.00    0.00    0.00    0.00    0.00    0.00    0.00    0.00    0.00  
           0.00    0.00    0.00    0.00    0.00    0.00    0.00    0.00    0.00    0.00  
           0.00    0.00    0.00    0.01    0.01    0.01    0.01    0.01    0.01    0.03  
           0.03    0.03    0.05    0.05    0.05    0.15    0.15    0.15    0.03    0.03  
           0.03    0.01    0.01    0.01    0.01    0.01    0.01    0.00    0.00    0.00  
           0.00    0.00    0.00    0.00    0.00    0.00    0.00    0.00    0.00    0.00  
           0.00    0.00    0.00    0.00    0.00    0.00    0.00    0.00    0.00    0.00

168 JD      INDEX STORM NO. 3  
           STRM        3.31 PRECIPITATION DEPTH  
           TRDA        2.80 TRANSPOSITION DRAINAGE AREA

169 PI      PRECIPITATION PATTERN  
           0.00    0.00    0.00    0.00    0.00    0.00    0.00    0.00    0.00    0.00  
           0.00    0.00    0.00    0.00    0.00    0.00    0.00    0.00    0.00    0.00  
           0.00    0.00    0.00    0.00    0.00    0.00    0.00    0.00    0.00    0.00  
           0.00    0.00    0.00    0.01    0.01    0.01    0.01    0.01    0.01    0.03  
           0.03    0.03    0.07    0.07    0.07    0.08    0.08    0.08    0.05    0.05  
           0.05    0.02    0.02    0.02    0.01    0.01    0.01    0.00    0.00    0.00  
           0.00    0.00    0.00    0.00    0.00    0.00    0.00    0.00    0.00    0.00  
           0.00    0.00    0.00    0.00    0.00    0.00    0.00    0.00    0.00    0.00

172 JD      INDEX STORM NO. 4  
           STRM        3.13 PRECIPITATION DEPTH  
           TRDA        16.00 TRANSPOSITION DRAINAGE AREA

173 PI      PRECIPITATION PATTERN  
           0.00    0.00    0.00    0.00    0.00    0.00    0.00    0.00    0.00    0.01  
           0.01    0.01    0.00    0.01    0.00    0.00    0.00    0.00    0.00    0.00  
           0.00    0.00    0.01    0.00    0.00    0.00    0.00    0.01    0.01    0.01  
           0.01    0.01    0.01    0.01    0.01    0.01    0.02    0.02    0.02    0.03  
           0.03    0.03    0.06    0.06    0.06    0.07    0.07    0.07    0.04    0.04  
           0.04    0.02    0.02    0.02    0.01    0.01    0.01    0.01    0.01    0.01  
           0.00    0.00    0.00    0.00    0.00    0.00    0.00    0.00    0.00    0.00  
           0.00    0.00    0.00    0.00    0.00    0.00    0.00    0.00    0.00    0.00

176 JD      INDEX STORM NO. 5  
           STRM        2.76 PRECIPITATION DEPTH  
           TRDA        90.00 TRANSPOSITION DRAINAGE AREA

177 PI      PRECIPITATION PATTERN  
           0.01    0.01    0.01    0.00    0.00    0.00    0.01    0.01    0.01    0.01  
           0.01    0.01    0.01    0.01    0.01    0.01    0.01    0.01    0.01    0.01  
           0.01    0.01    0.01    0.01    0.01    0.01    0.01    0.01    0.01    0.01  
           0.01    0.01    0.01    0.01    0.01    0.01    0.02    0.02    0.02    0.03  
           0.03    0.03    0.05    0.05    0.05    0.05    0.05    0.05    0.04    0.04  
           0.04    0.02    0.02    0.02    0.02    0.02    0.02    0.01    0.01    0.01  
           0.01    0.01    0.01    0.01    0.01    0.01    0.01    0.01    0.01    0.01  
           0.01    0.01    0.01    0.01    0.01    0.01    0.01    0.01    0.01    0.01

180 JD INDEX STORM NO. 6  
STRM 1.94 PRECIPITATION DEPTH  
TRDA 500.00 TRANSPOSITION DRAINAGE AREA

181 PI PRECIPITATION PATTERN  
0.01 0.01 0.01 0.01 0.01 0.01 0.01 0.01 0.01 0.01  
0.01 0.01 0.01 0.01 0.01 0.01 0.01 0.01 0.01 0.01  
0.01 0.01 0.01 0.01 0.01 0.01 0.01 0.01 0.01 0.01  
0.01 0.01 0.01 0.01 0.01 0.01 0.02 0.02 0.02 0.03  
0.03 0.03 0.04 0.04 0.04 0.04 0.04 0.04 0.04 0.04  
0.04 0.03 0.03 0.03 0.02 0.02 0.02 0.01 0.01 0.01  
0.01 0.01 0.01 0.01 0.01 0.01 0.01 0.01 0.01 0.01  
0.01 0.01

\*\*\* \*\*

\*\*\*\*\*  
\* \*  
184 KK \* IW371 \*  
\* \*  
\*\*\*\*\*

185 KO OUTPUT CONTROL VARIABLES  
IPRNT 5 PRINT CONTROL  
IPLOT 0 PLOT CONTROL  
QSCAL 0. HYDROGRAPH PLOT SCALE  
IPNCH 1 PUNCH COMPUTED HYDROGRAPH  
IOUT 22 SAVE HYDROGRAPH ON THIS UNIT  
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
ISAV2 1500 LAST ORDINATE PUNCHED OR SAVED  
TIMINT 0.083 TIME INTERVAL IN HOURS

\*\*\* \*\*

\*\*\*\*\*  
\* \*  
191 KK \* D371 \*  
\* \*  
\*\*\*\*\*

192 KO OUTPUT CONTROL VARIABLES  
IPRNT 5 PRINT CONTROL  
IPLOT 0 PLOT CONTROL  
QSCAL 0. HYDROGRAPH PLOT SCALE  
IPNCH 0 PUNCH COMPUTED HYDROGRAPH  
IOUT 22 SAVE HYDROGRAPH ON THIS UNIT  
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
ISAV2 1500 LAST ORDINATE PUNCHED OR SAVED  
TIMINT 0.083 TIME INTERVAL IN HOURS

\*\*\* \*\*

\*\*\*\*\*  
\* \*  
196 KK \* RIW371 \*  
\* \*  
\*\*\*\*\*

197 KO OUTPUT CONTROL VARIABLES  
IPRNT 5 PRINT CONTROL  
IPLOT 0 PLOT CONTROL  
QSCAL 0. HYDROGRAPH PLOT SCALE  
IPNCH 0 PUNCH COMPUTED HYDROGRAPH  
IOUT 22 SAVE HYDROGRAPH ON THIS UNIT  
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
ISAV2 1500 LAST ORDINATE PUNCHED OR SAVED  
TIMINT 0.083 TIME INTERVAL IN HOURS

\*\*\* \*\*

\*\*\*\*\*  
\* \*  
202 KK \* IW366 \*  
\* \*  
\*\*\*\*\*

203 KO OUTPUT CONTROL VARIABLES  
IPRNT 5 PRINT CONTROL  
IPLOT 0 PLOT CONTROL  
QSCAL 0. HYDROGRAPH PLOT SCALE  
IPNCH 1 PUNCH COMPUTED HYDROGRAPH  
IOUT 22 SAVE HYDROGRAPH ON THIS UNIT  
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
ISAV2 1500 LAST ORDINATE PUNCHED OR SAVED  
TIMINT 0.083 TIME INTERVAL IN HOURS

\*\*\* \*\*



\*\*\* \*\*

\*\*\*\*\*  
 \*  
 \* D371 \*  
 \*  
 \*\*\*\*\*

244 KO OUTPUT CONTROL VARIABLES  
 IPRNT 5 PRINT CONTROL  
 IPLOT 0 PLOT CONTROL  
 QSCAL 0. HYDROGRAPH PLOT SCALE  
 IPNCH 0 PUNCH COMPUTED HYDROGRAPH  
 IOUT 22 SAVE HYDROGRAPH ON THIS UNIT  
 ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
 ISAV2 1500 LAST ORDINATE PUNCHED OR SAVED  
 TIMINT 0.083 TIME INTERVAL IN HOURS

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 \*  
 \* RD371 \*  
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247 KO OUTPUT CONTROL VARIABLES  
 IPRNT 5 PRINT CONTROL  
 IPLOT 0 PLOT CONTROL  
 QSCAL 0. HYDROGRAPH PLOT SCALE  
 IPNCH 0 PUNCH COMPUTED HYDROGRAPH  
 IOUT 22 SAVE HYDROGRAPH ON THIS UNIT  
 ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
 ISAV2 1500 LAST ORDINATE PUNCHED OR SAVED  
 TIMINT 0.083 TIME INTERVAL IN HOURS

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 \*  
 \* CIW374 \*  
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253 KO OUTPUT CONTROL VARIABLES  
 IPRNT 5 PRINT CONTROL  
 IPLOT 0 PLOT CONTROL  
 QSCAL 0. HYDROGRAPH PLOT SCALE  
 IPNCH 0 PUNCH COMPUTED HYDROGRAPH  
 IOUT 22 SAVE HYDROGRAPH ON THIS UNIT  
 ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
 ISAV2 1500 LAST ORDINATE PUNCHED OR SAVED  
 TIMINT 0.083 TIME INTERVAL IN HOURS

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 \*  
 \* RIW374 \*  
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256 KO OUTPUT CONTROL VARIABLES  
 IPRNT 5 PRINT CONTROL  
 IPLOT 0 PLOT CONTROL  
 QSCAL 0. HYDROGRAPH PLOT SCALE  
 IPNCH 0 PUNCH COMPUTED HYDROGRAPH  
 IOUT 22 SAVE HYDROGRAPH ON THIS UNIT  
 ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
 ISAV2 1500 LAST ORDINATE PUNCHED OR SAVED  
 TIMINT 0.083 TIME INTERVAL IN HOURS

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 \*  
 \* IW386 \*  
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262 KO OUTPUT CONTROL VARIABLES  
 IPRNT 5 PRINT CONTROL  
 IPLOT 0 PLOT CONTROL  
 QSCAL 0. HYDROGRAPH PLOT SCALE  
 IPNCH 1 PUNCH COMPUTED HYDROGRAPH  
 IOUT 22 SAVE HYDROGRAPH ON THIS UNIT  
 ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED

ISAV2 1500 LAST ORDINATE PUNCHED OR SAVED  
TIMINT 0.083 TIME INTERVAL IN HOURS

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\* \*  
268 KK \* RIW386 \*  
\* \*  
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269 KO OUTPUT CONTROL VARIABLES  
IPRNT 5 PRINT CONTROL  
IPLOT 0 PLOT CONTROL  
QSCAL 0. HYDROGRAPH PLOT SCALE  
IPNCH 0 PUNCH COMPUTED HYDROGRAPH  
IOUT 22 SAVE HYDROGRAPH ON THIS UNIT  
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
ISAV2 1500 LAST ORDINATE PUNCHED OR SAVED  
TIMINT 0.083 TIME INTERVAL IN HOURS

\*\*\* \*\*

\*\*\*\*\*  
\* \*  
274 KK \* IW382 \*  
\* \*  
\*\*\*\*\*

275 KO OUTPUT CONTROL VARIABLES  
IPRNT 5 PRINT CONTROL  
IPLOT 0 PLOT CONTROL  
QSCAL 0. HYDROGRAPH PLOT SCALE  
IPNCH 1 PUNCH COMPUTED HYDROGRAPH  
IOUT 22 SAVE HYDROGRAPH ON THIS UNIT  
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
ISAV2 1500 LAST ORDINATE PUNCHED OR SAVED  
TIMINT 0.083 TIME INTERVAL IN HOURS

\*\*\* \*\*

\*\*\*\*\*  
\* \*  
281 KK \* CIW382 \*  
\* \*  
\*\*\*\*\*

282 KO OUTPUT CONTROL VARIABLES  
IPRNT 5 PRINT CONTROL  
IPLOT 0 PLOT CONTROL  
QSCAL 0. HYDROGRAPH PLOT SCALE  
IPNCH 0 PUNCH COMPUTED HYDROGRAPH  
IOUT 22 SAVE HYDROGRAPH ON THIS UNIT  
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
ISAV2 1500 LAST ORDINATE PUNCHED OR SAVED  
TIMINT 0.083 TIME INTERVAL IN HOURS

\*\*\* \*\*

\*\*\*\*\*  
\* \*  
284 KK \* D382B \*  
\* \*  
\*\*\*\*\*

285 KO OUTPUT CONTROL VARIABLES  
IPRNT 5 PRINT CONTROL  
IPLOT 0 PLOT CONTROL  
QSCAL 0. HYDROGRAPH PLOT SCALE  
IPNCH 0 PUNCH COMPUTED HYDROGRAPH  
IOUT 22 SAVE HYDROGRAPH ON THIS UNIT  
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
ISAV2 1500 LAST ORDINATE PUNCHED OR SAVED  
TIMINT 0.083 TIME INTERVAL IN HOURS

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\*\*\*\*\*  
\* \*  
289 KK \* D382A \*  
\* \*  
\*\*\*\*\*

290 KO OUTPUT CONTROL VARIABLES  
IPRNT 5 PRINT CONTROL  
IPLOT 0 PLOT CONTROL

QSCAL	0.	HYDROGRAPH PLOT SCALE
IPNCH	0	PUNCH COMPUTED HYDROGRAPH
IOUT	22	SAVE HYDROGRAPH ON THIS UNIT
ISAV1	1	FIRST ORDINATE PUNCHED OR SAVED
ISAV2	1500	LAST ORDINATE PUNCHED OR SAVED
TIMINT	0.083	TIME INTERVAL IN HOURS

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*****
*
* 294 KK      * RIW382 *
*
*****

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295 KO      OUTPUT CONTROL VARIABLES
            IPRNT      5  PRINT CONTROL
            IPLOT      0  PLOT CONTROL
            QSCAL      0. HYDROGRAPH PLOT SCALE
            IPNCH      0  PUNCH COMPUTED HYDROGRAPH
            IOUT       22 SAVE HYDROGRAPH ON THIS UNIT
            ISAV1      1  FIRST ORDINATE PUNCHED OR SAVED
            ISAV2     1500 LAST ORDINATE PUNCHED OR SAVED
            TIMINT     0.083 TIME INTERVAL IN HOURS

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\*\*\* \*\*

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*****
*
* 300 KK      * IW384 *
*
*****

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```

301 KO      OUTPUT CONTROL VARIABLES
            IPRNT      5  PRINT CONTROL
            IPLOT      0  PLOT CONTROL
            QSCAL      0. HYDROGRAPH PLOT SCALE
            IPNCH      1  PUNCH COMPUTED HYDROGRAPH
            IOUT       22 SAVE HYDROGRAPH ON THIS UNIT
            ISAV1      1  FIRST ORDINATE PUNCHED OR SAVED
            ISAV2     1500 LAST ORDINATE PUNCHED OR SAVED
            TIMINT     0.083 TIME INTERVAL IN HOURS

```

\*\*\* \*\*

```

*****
*
* 307 KK      * D382A *
*
*****

```

```

308 KO      OUTPUT CONTROL VARIABLES
            IPRNT      5  PRINT CONTROL
            IPLOT      0  PLOT CONTROL
            QSCAL      0. HYDROGRAPH PLOT SCALE
            IPNCH      0  PUNCH COMPUTED HYDROGRAPH
            IOUT       22 SAVE HYDROGRAPH ON THIS UNIT
            ISAV1      1  FIRST ORDINATE PUNCHED OR SAVED
            ISAV2     1500 LAST ORDINATE PUNCHED OR SAVED
            TIMINT     0.083 TIME INTERVAL IN HOURS

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*****
*
* 310 KK      * RD382A *
*
*****

```

```

311 KO      OUTPUT CONTROL VARIABLES
            IPRNT      5  PRINT CONTROL
            IPLOT      0  PLOT CONTROL
            QSCAL      0. HYDROGRAPH PLOT SCALE
            IPNCH      0  PUNCH COMPUTED HYDROGRAPH
            IOUT       22 SAVE HYDROGRAPH ON THIS UNIT
            ISAV1      1  FIRST ORDINATE PUNCHED OR SAVED
            ISAV2     1500 LAST ORDINATE PUNCHED OR SAVED
            TIMINT     0.083 TIME INTERVAL IN HOURS

```

\*\*\* \*\*

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*****
*
* 316 KK      * CIW384 *
*
*****

```

```

317 KO      OUTPUT CONTROL VARIABLES
            IPRNT      5  PRINT CONTROL
            IPLOT      0  PLOT CONTROL
            QSCAL      0. HYDROGRAPH PLOT SCALE
            IPNCH      0  PUNCH COMPUTED HYDROGRAPH
            IOUT       22 SAVE HYDROGRAPH ON THIS UNIT
            ISAV1      1  FIRST ORDINATE PUNCHED OR SAVED
            ISAV2     1500 LAST ORDINATE PUNCHED OR SAVED
            TIMINT     0.083 TIME INTERVAL IN HOURS

```

\*\*\* \*\*

```

*****
*
* RIW384 *
*
*****

```

```

320 KO      OUTPUT CONTROL VARIABLES
            IPRNT      5  PRINT CONTROL
            IPLOT      0  PLOT CONTROL
            QSCAL      0. HYDROGRAPH PLOT SCALE
            IPNCH      0  PUNCH COMPUTED HYDROGRAPH
            IOUT       22 SAVE HYDROGRAPH ON THIS UNIT
            ISAV1      1  FIRST ORDINATE PUNCHED OR SAVED
            ISAV2     1500 LAST ORDINATE PUNCHED OR SAVED
            TIMINT     0.083 TIME INTERVAL IN HOURS

```

\*\*\* \*\*

```

*****
*
* IW381 *
*
*****

```

```

326 KO      OUTPUT CONTROL VARIABLES
            IPRNT      5  PRINT CONTROL
            IPLOT      0  PLOT CONTROL
            QSCAL      0. HYDROGRAPH PLOT SCALE
            IPNCH      1  PUNCH COMPUTED HYDROGRAPH
            IOUT       22 SAVE HYDROGRAPH ON THIS UNIT
            ISAV1      1  FIRST ORDINATE PUNCHED OR SAVED
            ISAV2     1500 LAST ORDINATE PUNCHED OR SAVED
            TIMINT     0.083 TIME INTERVAL IN HOURS

```

\*\*\* \*\*

```

*****
*
* D382B *
*
*****

```

```

333 KO      OUTPUT CONTROL VARIABLES
            IPRNT      5  PRINT CONTROL
            IPLOT      0  PLOT CONTROL
            QSCAL      0. HYDROGRAPH PLOT SCALE
            IPNCH      0  PUNCH COMPUTED HYDROGRAPH
            IOUT       22 SAVE HYDROGRAPH ON THIS UNIT
            ISAV1      1  FIRST ORDINATE PUNCHED OR SAVED
            ISAV2     1500 LAST ORDINATE PUNCHED OR SAVED
            TIMINT     0.083 TIME INTERVAL IN HOURS

```

\*\*\* \*\*

```

*****
*
* RIW381 *
*
*****

```

```

338 KO      OUTPUT CONTROL VARIABLES
            IPRNT      5  PRINT CONTROL
            IPLOT      0  PLOT CONTROL
            QSCAL      0. HYDROGRAPH PLOT SCALE
            IPNCH      0  PUNCH COMPUTED HYDROGRAPH
            IOUT       22 SAVE HYDROGRAPH ON THIS UNIT
            ISAV1      1  FIRST ORDINATE PUNCHED OR SAVED
            ISAV2     1500 LAST ORDINATE PUNCHED OR SAVED
            TIMINT     0.083 TIME INTERVAL IN HOURS

```

\*\*\* \*\*

\*\*\*\*\*

343 KK \* \*  
\* C359\* \*  
\* \*  
\*\*\*\*\*

344 KO OUTPUT CONTROL VARIABLES  
IPRNT 5 PRINT CONTROL  
IPLOT 0 PLOT CONTROL  
QSCAL 0. HYDROGRAPH PLOT SCALE  
IPNCH 0 PUNCH COMPUTED HYDROGRAPH  
IOUT 22 SAVE HYDROGRAPH ON THIS UNIT  
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
ISAV2 1500 LAST ORDINATE PUNCHED OR SAVED  
TIMINT 0.083 TIME INTERVAL IN HOURS

\*\*\* \*\*

\*\*\*\*\*  
\* \*  
346 KK \* R359\* \*  
\* \*  
\*\*\*\*\*

347 KO OUTPUT CONTROL VARIABLES  
IPRNT 5 PRINT CONTROL  
IPLOT 0 PLOT CONTROL  
QSCAL 0. HYDROGRAPH PLOT SCALE  
IPNCH 0 PUNCH COMPUTED HYDROGRAPH  
IOUT 22 SAVE HYDROGRAPH ON THIS UNIT  
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
ISAV2 1500 LAST ORDINATE PUNCHED OR SAVED  
TIMINT 0.083 TIME INTERVAL IN HOURS

\*\*\* \*\*

\*\*\*\*\*  
\* \*  
352 KK \* IW359 \*  
\* \*  
\*\*\*\*\*

353 KO OUTPUT CONTROL VARIABLES  
IPRNT 5 PRINT CONTROL  
IPLOT 0 PLOT CONTROL  
QSCAL 0. HYDROGRAPH PLOT SCALE  
IPNCH 1 PUNCH COMPUTED HYDROGRAPH  
IOUT 22 SAVE HYDROGRAPH ON THIS UNIT  
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
ISAV2 1500 LAST ORDINATE PUNCHED OR SAVED  
TIMINT 0.083 TIME INTERVAL IN HOURS

\*\*\* \*\*

\*\*\*\*\*  
\* \*  
359 KK \* CIW359 \*  
\* \*  
\*\*\*\*\*

360 KO OUTPUT CONTROL VARIABLES  
IPRNT 5 PRINT CONTROL  
IPLOT 0 PLOT CONTROL  
QSCAL 0. HYDROGRAPH PLOT SCALE  
IPNCH 0 PUNCH COMPUTED HYDROGRAPH  
IOUT 22 SAVE HYDROGRAPH ON THIS UNIT  
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
ISAV2 1500 LAST ORDINATE PUNCHED OR SAVED  
TIMINT 0.083 TIME INTERVAL IN HOURS

\*\*\* \*\*

\*\*\*\*\*  
\* \*  
362 KK \* RIW359 \*  
\* \*  
\*\*\*\*\*

363 KO OUTPUT CONTROL VARIABLES  
IPRNT 5 PRINT CONTROL  
IPLOT 0 PLOT CONTROL  
QSCAL 0. HYDROGRAPH PLOT SCALE  
IPNCH 0 PUNCH COMPUTED HYDROGRAPH  
IOUT 22 SAVE HYDROGRAPH ON THIS UNIT  
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
ISAV2 1500 LAST ORDINATE PUNCHED OR SAVED  
TIMINT 0.083 TIME INTERVAL IN HOURS

\*\*\* \*\*

\*\*\*\*\*  
 \*  
 368 KK \* IW353A \*  
 \*  
 \*\*\*\*\*

369 KO OUTPUT CONTROL VARIABLES  
 IPRNT 5 PRINT CONTROL  
 IPLOT 0 PLOT CONTROL  
 QSCAL 0. HYDROGRAPH PLOT SCALE  
 IPNCH 1 PUNCH COMPUTED HYDROGRAPH  
 IOUT 22 SAVE HYDROGRAPH ON THIS UNIT  
 ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
 ISAV2 1500 LAST ORDINATE PUNCHED OR SAVED  
 TIMINT 0.083 TIME INTERVAL IN HOURS

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\*\*\*\*\*  
 \*  
 375 KK \* CW353A \*  
 \*  
 \*\*\*\*\*

376 KO OUTPUT CONTROL VARIABLES  
 IPRNT 5 PRINT CONTROL  
 IPLOT 0 PLOT CONTROL  
 QSCAL 0. HYDROGRAPH PLOT SCALE  
 IPNCH 0 PUNCH COMPUTED HYDROGRAPH  
 IOUT 22 SAVE HYDROGRAPH ON THIS UNIT  
 ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
 ISAV2 1500 LAST ORDINATE PUNCHED OR SAVED  
 TIMINT 0.083 TIME INTERVAL IN HOURS

1

RUNOFF SUMMARY  
 FLOW IN CUBIC FEET PER SECOND  
 TIME IN HOURS, AREA IN SQUARE MILES

	OPERATION	STATION	PEAK FLOW	TIME OF PEAK	AVERAGE FLOW FOR MAXIMUM PERIOD			BASIN AREA	MAXIMUM STAGE	TIME OF MAX STAGE
					6-HOUR	24-HOUR	72-HOUR			
+	HYDROGRAPH AT	IW371	103.	4.08	12.	3.	1.	0.05		
+	DIVERSION TO	DO371	77.	4.08	6.	2.	1.	0.05		
+	HYDROGRAPH AT	D371	26.	4.08	5.	1.	0.	0.05		
+	ROUTED TO	RIW371	15.	6.50	5.	1.	0.	0.05		
+	HYDROGRAPH AT	IW366	1107.	4.58	209.	52.	17.	1.06		
+	2 COMBINED AT	CIW366	1105.	4.58	213.	54.	18.	1.07		
+	ROUTED TO	RIW366	957.	5.00	212.	54.	18.	1.07		
+	HYDROGRAPH AT	IW377	153.	4.25	22.	6.	2.	0.11		
+	ROUTED TO	SSR010	121.	4.50	22.	6.	2.	0.11		
+	ROUTED TO	RIW377	98.	5.25	22.	6.	2.	0.11		
+	HYDROGRAPH AT	IW374	758.	4.25	97.	24.	8.	0.39		
+	HYDROGRAPH AT	D371	77.	4.08	6.	2.	1.	0.05		
+	ROUTED TO	RD371	40.	4.33	6.	2.	1.	0.05		
+	3 COMBINED AT	CIW374	784.	4.25	124.	31.	10.	0.54		
+	ROUTED TO	RIW374	404.	5.92	121.	31.	10.	0.54		
+	HYDROGRAPH AT	IW386	395.	4.58	90.	23.	8.	0.37		
+	ROUTED TO	RIW386	301.	5.42	89.	23.	8.	0.37		
+	HYDROGRAPH AT	IW382	738.	4.33	131.	33.	11.	0.62		
+	2 COMBINED AT	CIW382	685.	4.33	211.	54.	18.	0.99		

+	DIVERSION TO	D0382B	180.	3.92	112.	29.	10.	0.99
+	HYDROGRAPH AT	D382B	505.	4.33	99.	25.	8.	0.99
+	DIVERSION TO	D0382A	226.	4.33	20.	5.	2.	0.99
+	HYDROGRAPH AT	D382A	279.	4.33	79.	20.	7.	0.99
+	ROUTED TO	RIW382	253.	4.83	79.	20.	7.	0.99
+	HYDROGRAPH AT	IW384	184.	4.17	21.	5.	2.	0.08
+	HYDROGRAPH AT	D382A	226.	4.33	20.	5.	2.	0.99
+	ROUTED TO	RD382A	196.	4.42	20.	5.	2.	0.99
+	2 COMBINED AT	CIW384	390.	4.33	47.	12.	4.	0.41
+	ROUTED TO	RIW384	310.	4.67	47.	12.	4.	0.41
+	HYDROGRAPH AT	IW381	454.	4.50	98.	25.	8.	0.38
+	HYDROGRAPH AT	D382B	180.	3.92	112.	29.	10.	0.99
+	2 COMBINED AT	CIW381	619.	4.50	208.	54.	18.	0.64
+	ROUTED TO	RIW381	574.	4.83	207.	54.	18.	0.64
+	3 COMBINED AT	C359*	990.	4.83	320.	82.	27.	1.45
+	ROUTED TO	R359*	937.	5.17	317.	82.	27.	1.45
+	HYDROGRAPH AT	IW359	2496.	4.75	537.	135.	45.	2.91
+	4 COMBINED AT	CIW359	3423.	5.00	1059.	274.	91.	5.97
+	ROUTED TO	RIW359	3323.	5.08	1054.	274.	91.	5.97
+	HYDROGRAPH AT	IW353A	1029.	4.42	145.	36.	12.	0.72
+	2 COMBINED AT	CW353A	3561.	5.08	1156.	301.	100.	6.69

\*\*\* NORMAL END OF HEC-1 \*\*\*

1\*\*\*\*\*
\* FLOOD HYDROGRAPH PACKAGE (HEC-1) \*
\* JUN 1998 \*
\* VERSION 4.1 \*
\* RUN DATE 05AUG11 TIME 16:03:06 \*
\*\*\*\*\*

\*\*\*\*\*
\* U.S. ARMY CORPS OF ENGINEERS \*
\* HYDROLOGIC ENGINEERING CENTER \*
\* 609 SECOND STREET \*
\* DAVIS, CALIFORNIA 95616 \*
\* (916) 756-1104 \*
\*\*\*\*\*

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THIS PROGRAM REPLACES ALL PREVIOUS VERSIONS OF HEC-1 KNOWN AS HEC1 (JAN 73), HEC1GS, HEC1DB, AND HEC1KW.
THE DEFINITIONS OF VARIABLES -RTIMP- AND -RTIOR- HAVE CHANGED FROM THOSE USED WITH THE 1973-STYLE INPUT STRUCTURE.
THE DEFINITION OF -AMSK- ON RM-CARD WAS CHANGED WITH REVISIONS DATED 28 SEP 81. THIS IS THE FORTRAN77 VERSION
NEW OPTIONS: DAMBREAK OUTFLOW SUBMERGENCE, SINGLE EVENT DAMAGE CALCULATION, DSS:WRITE STAGE FREQUENCY,
DSS:READ TIME SERIES AT DESIRED CALCULATION INTERVAL LOSS RATE:GREEN AND AMPT INFILTRATION
KINEMATIC WAVE: NEW FINITE DIFFERENCE ALGORITHM

1

HEC-1 INPUT

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10
1 ID Wittmann Phase IV Floodplain Delineation Study
2 ID August 2011
3 ID Mod3 removes subbasin Iw358, Iw353, CIW357, and C359
4 ID Mod3 replaces Iw359 with Iw359A and updates RIW366 and RIW374
5 ID Mod3 truncates model at CW359A
6 ID \*\*\*\*\*
7 ID \*\*-ADDENDUM-ADDENDUM-ADDENDUM-ADDENDUM-ADDENDUM-ADDENDUM-ADDENDUM-ADDENDUM-\*\*
8 ID \*\*-ADDENDUM-ADDENDUM-ADDENDUM-ADDENDUM-ADDENDUM-ADDENDUM-ADDENDUM-ADDENDUM-\*\*
9 ID \*\*\*\*\*
10 ID \*\*
11 ID \*\* ADDENDUM TO THE WITTMANN AREA DRAINAGE MASTER STUDY UPDATE HYDROLOGY \*\*
12 ID \*\*
13 ID \*\*\*\*\*
14 ID \*\*-ADDENDUM-ADDENDUM-ADDENDUM-ADDENDUM-ADDENDUM-ADDENDUM-ADDENDUM-ADDENDUM-\*\*
15 ID \*\*-ADDENDUM-ADDENDUM-ADDENDUM-ADDENDUM-ADDENDUM-ADDENDUM-ADDENDUM-ADDENDUM-\*\*
16 ID \*\*\*\*\*
17 ID \*\*
18 ID \*\* THE FOLLOWING CHANGES HAVE BEEN MADE TO THE WITTMANN HYDROLOGY MODELS \*\*
19 ID \*\* DATED OCTOBER 2004: \*\*
20 ID \*\*
21 ID \*\* -Diversion added after CTW480 \*\*
22 ID \*\* -Main flow (5%) to CTW478 \*\*
23 ID \*\* -Diverted flow (95%) to CWI584 \*\*
24 ID \*\* -Added concentration point CWI584 combining RD480 and WI584 \*\*
25 ID \*\* -Hard coding was added to CTW478 \*\*
26 ID \*\* -Route added after D480 (Diversion Recovery) \*\*
27 ID \*\* -Route RWI584 was adjusted \*\*
28 ID \*\* -Storage Route SCP050 was adjusted \*\*
29 ID \*\* -Route R508\* was adjusted \*\*
30 ID \*\* -Hard coding for CWI510 was adjusted \*\*
31 ID \*\*
32 ID \*\* -Diversion D618 was moved from after CPI618 to before RDCP12 \*\*
33 ID \*\* -Updated route RD618 information \*\*
34 ID \*\* -Hard coding on CPI618 was adjusted \*\*
35 ID \*\* -Hard coding on CPI615 was adjusted \*\*
36 ID \*\*
37 ID \*\* -Route RPI639 changed from following US60 to CPI636 to route \*\*
38 ID \*\* through US60/RR structures and combine with CWI506 \*\*
39 ID \*\* -Cards PI639, D524, RD524, CPI639 and RPI639 were changed from \*\*
40 ID \*\* following R624A to follow RWI524 \*\*
41 ID \*\* -CPI636 was removed-no longer needed to combine PI636 & RPI639 \*\*
42 ID \*\* -RPI639 information was changed to reflect new route \*\*
43 ID \*\* -CWI506 changed from combining 3 to combine 4: \*\*
44 ID \*\* RWI525, RWI524, RPI639 & WI506 \*\*
45 ID \*\* -Hard coding on CWI506 was adjusted \*\*
46 ID \*\* -Hard Coding on CWI500 was adjusted \*\*
47 ID \*\*
48 ID \*\* \*\*\*\*\*
49 ID \*\*-ADDENDUM-ADDENDUM-ADDENDUM-ADDENDUM-ADDENDUM-ADDENDUM-ADDENDUM-ADDENDUM-\*\*
50 ID \*\*-ADDENDUM-ADDENDUM-ADDENDUM-ADDENDUM-ADDENDUM-ADDENDUM-ADDENDUM-ADDENDUM-\*\*
51 ID \*\*\*\*\*
52 ID \*\*
53 ID \*\*
54 ID \*\*
55 ID \*\*\*\*\*

1

HEC-1 INPUT

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10
56 ID \*\*\*\*\*
57 ID \*\*
58 ID \*\* WITTMANN AREA DRAINAGE MASTER STUDY UPDATE-HYDROLOGY MODEL 2004 \*\*
59 ID \*\*
60 ID \*\*\*\*\*
61 ID \*\*
62 ID \*\*
63 ID PROJECT: Wittmann ADMS Update
64 ID CLIENT: Flood Control District of Maricopa County
Page 1

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65 ID wtec6-addendum-mod3.out
66 ID PREPARED BY: Entellus, Inc.
67 ID PROJECT No: FCD 2002C029 Entellus 310.032
68 ID FILE NAME: WTEC6-addendum.hcl CREATED DATE: JUL 01, 2004
69 ID MODIFIED DATE: MAY 04, 2005
70 ID
71 ID STORM:
72 ID 100-year 6-hour Storm
73 ID
74 ID DEVELOPMENT CONDITIONS:
75 ID Existing Conditions
76 ID
77 ID MODELING ASSUMPTIONS:
78 ID It was assumed that the US60 did not have adequate
79 ID storage to cause any significant attenuation.
80 ID
81 ID The assumption was made that the CAP Canal embankment
82 ID would not be breached under a large flood event.
83 ID
84 ID The assumption was made that the Beardsley Canal would
85 ID not fail under a large storm event (Per District
86 ID Instruction). In addition, the berm north of the
87 ID Beardsley canal and east of US60 was assumed to fail
88 ID (per district Instruction).
89 ID
90 ID For both the CAP and Beardsley Canals, once the berm
91 ID elevation was reached weir flow was assumed. It was
92 ID also assumed that any weir flow over the canal that
93 ID might enter the canal and be diverted out of the study
94 ID area was insignificant, and thus was ignored. In other
95 ID words all weir flow over the canal embankment reaches
96 ID the downstream concentration point.
97 ID
98 ID Typical X-sects were developed, and it was assumed
99 ID that a typical x-sect could adequately represent
100 ID various reaches.
101 ID
102 ID Time-Area Relations were used base on the District's
103 ID Hydrology Manual criteria. Two Time-Area Relation
104 ID Curves were utilized:
105 ID -Urban
106 ID -Natural
107 ID The Time-Area Relation Curves were taken directly
108 ID from the manual
109 ID
110 ID MODELING METHODS: This model utilizes QI cards to input the Padelford
HEC-1 INPUT

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LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10
111 ID hydrographs from the 100-year 6-hour existing
112 ID conditions model developed by A-N West Inc. for the
113 ID Padelford Wash Floodplain Delineation Study. The
114 ID hydrograph was altered from its original form
115 ID (2-minute interval to 5-minute interval) through
116 ID simple interpolation.
117 ID
118 ID Clark Unit Hydrographs were used for all subbasins
119 ID except the two subbasins directly upstream of the
120 ID Bonita Dam (PD726B and PD740 use S-graphs). The UC
121 ID parameters were calculated using the WMS7.0 software.
122 ID
123 ID For Basins PD726B and PD740 S-Graphs were utilized per
124 ID the request of the FCDMC. Limited details regarding
125 ID the calculations of the S-graphs can be found in the
126 ID model by the basin KK card. For full details of the
127 ID S-graph calculations refer to the Appendix.
128 ID
129 ID Normal Depth routing was used for all routing reaches.
130 ID
131 ID Hard coding was used to account for the percentage of
132 ID area associated with a diversion. Because of the use
133 ID of JD cards and areal reduction, hard coding was
134 ID necessary to properly account for area. For a given
135 ID diversion a percentage of the flow is routed to two
136 ID different locations. The same percentage of area
137 ID follows that diverted flow. In addition the area
138 ID downstream of the main path is reduced or increased and
139 ID is hard coded to account for the loss or gain of area.
140 ID Hard coding was performed based on the 6-hour existing
141 ID conditions model.
142 ID
143 ID Stage-storage was developed for all the structures
144 ID along the CAP Canal, as well as along the Beardsley
145 ID Canal. In addition several stage-storage locations
146 ID were developed for areas with significant storage
147 ID along the SR74. No storage was modeled along the US60
148 ID and railroad, but the culverts were analyzed for
149 ID diversion potential. Diversions were placed in the
150 ID model where deemed appropriate. See appendix for
151 ID details.
152 ID
153 ID FLO-2D was utilized to calculate the split flows at
154 ID concentration points CIW351, CIW357, CIW363 and CIW576.
155 ID See appendix for modeling details.
156 ID
157 ID *****
158 ID *****
159 ID

```

```

160 ID *DIAGRAM
161 ID IT 5 0 1500
162 ID IO 5
163 ID IN 15 0
164 ID JD 3.40 0.01
165 ID * 6-hour distribution

```

LINE	ID	1	2	3	4	5	6	7	8	9	10
164	PC	0.000	0.008	0.016	0.025	0.033	0.041	0.050	0.058	0.066	0.074
165	PC	0.087	0.099	0.118	0.138	0.216	0.377	0.834	0.911	0.931	0.950
166	PC	0.962	0.972	0.983	0.991	1.000					
	*										
167	JD	3.378	0.50								
168	JD	3.314	2.80								
169	PC	0.000	0.009	0.016	0.025	0.034	0.042	0.051	0.059	0.067	0.076
170	PC	0.087	0.100	0.120	0.163	0.252	0.451	0.694	0.837	0.900	0.938
171	PC	0.950	0.963	0.975	0.988	1.000					
	*										
172	JD	3.135	16.00								
173	PC	0.000	0.009	0.020	0.030	0.048	0.063	0.076	0.090	0.105	0.119
174	PC	0.135	0.152	0.175	0.222	0.304	0.472	0.670	0.796	0.868	0.912
175	PC	0.946	0.960	0.973	0.987	1.000					
	*										
176	JD	2.761	90.00								
177	PC	0.000	0.021	0.035	0.051	0.071	0.087	0.105	0.125	0.143	0.160
178	PC	0.179	0.201	0.232	0.281	0.364	0.500	0.658	0.773	0.841	0.888
179	PC	0.927	0.945	0.964	0.982	1.000					
	*										
180	JD	1.938	500.00								
181	PC	0.000	0.024	0.043	0.059	0.078	0.098	0.119	0.141	0.162	0.186
182	PC	0.212	0.239	0.271	0.321	0.408	0.515	0.627	0.735	0.814	0.864
183	PC	0.907	0.930	0.954	0.977	1.000					
	*										
	*										
	*										

\*\*\*\*\*  
 \*\*\*\*\*  
 \*\*\*\*\* BEGINNING OF IONA WASH SUBAREA \*\*\*\*\*  
 \*\*\*\*\*  
 \*\*\*\*\*  
 \*\*\*\*\* BEGINNING OF IONA WASH REGION 1 \*\*\*\*\*  
 \*\*\*\*\*  
 \*\*\*\*\*

184	KK	Iw371									
185	KO	0	0	0.0	1	22					
186	BA	0.0529									
187	LG	0.275	0.25	4.15	0.473	26.25					
188	UC	0.275	0.288								
	*	* Urban Time-Area Relation									
189	UA	0	5	16	30	65	77	84	90	94	97
190	UA	100									
	*										

\* Railroad Diversion: Main path to CIW366, Diversion to CIW374  
 \* From Structure RR40, See Appendix D.5 for analysis  
 HEC-1 INPUT

1

LINE	ID	1	2	3	4	5	6	7	8	9	10
191	KK	D371									
192	KO	0	0	0.0	0	22					
193	DT	Do371									
194	DI	0.0	14	79	208	383	823	1374	2014	2724	
195	DQ	0.0	0	55	173	335	763	1302	1930	2632	
	*										
	*										
	*	* ROUTE UPDATED 4/14/4									
196	KK	RIW371									
197	KO	0	0	0.0	0	22					
198	RS	15	FLOW	0.0	0.0						
199	RC	0.04	0.04	0.04	11583	0.0088					
	*	* TSR371									
200	RX	0	142	486	525	651	770	1000	1518		
201	RY	1900	1900	1899	1897	1896	1899	1899.5	1900		
	*										
	*										
202	KK	Iw366									
203	KO	0	0	0.0	1	22					
204	BA	1.0558									
205	LG	0.343	0.25	5.762	0.202	7.207					
206	UC	0.763	0.593								
	*	* Natural Time-Area Relation									
207	UA	0	3	5	8	12	20	43	75	90	96
208	UA	100									
	*										
	*										
	*	* Combines Iw366 and RIW371									
209	KK	CIW366									
210	KO	0	0	0.0	0	22					
	*	* hard coded									
211	HC	2	1.07								
	*										
	*										
	*	* ROUTE UPDATED 4/14/4									
212	KK	RIW366									
213	KO	0	0	0.0	0	22					
214	RS	4	FLOW	0.0	0.0						
215	RC	0.04	0.04	0.04	8105	0.007					
	*	* IWTS30_2									
216	RX	0	6.5	13	57	102	104	109	110		
217	RY	1728	1725.5	1724	1723	1724	1725.5	1727	1728		

\*  
\*

HEC-1 INPUT

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

218 KK IW377  
 219 KO 0 0 0.0 1 22  
 220 BA 0.1125  
 221 LG 0.304 0.382 5.921 0.188 5.357  
 222 UC 0.408 0.519  
 \* Natural Time-Area Relation  
 223 UA 0 3 5 8 12 20 43 75 90 96  
 224 UA 100

\* \*\* STAGE DISCHARGE / STORAGE BEHIND STRUCTURE SR010 (HWY74 CULVERT)

225 KK SSR010  
 226 RS 1 STOR 0  
 227 SQ 0 36 100 160  
 228 SA 0 .05 .41 1.98  
 229 SE 2021.3 2024 2026 2028

\* ROUTE UPDATED 4/14/4

230 KK RIW377  
 231 KO 0 0 0.0 0 22  
 232 RS 6 FLOW 0.0 0.0  
 233 RC 0.04 0.04 0.04 6588 0.0096  
 \* TSR371  
 234 RX 0 142 486 525 651 770 1000 1518  
 235 RY 1900 1900 1899 1897 1896 1899 1899.5 1900

236 KK IW374  
 237 KO 0 0 0.0 1 22  
 238 BA 0.3874  
 239 LG 0.315 0.161 8.147 0.086 12.602  
 240 UC 0.442 0.345  
 \* Natural Time-Area Relation  
 241 UA 0 3 5 8 12 20 43 75 90 96  
 242 UA 100

\* RAILROAD DIVERSION RECOVERY: ALONG RAILROAD FROM STRUCTURE RR40

243 KK D371  
 244 KO 0 0 0.0 0 22  
 245 DR D0371

\* ROUTE UPDATED 4/14/4

HEC-1 INPUT

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

246 KK RD371  
 247 KO 0 0 0.0 0 22  
 248 RS 1 FLOW 0.0 0.0  
 249 RC 0.043 0.035 0.043 1409 0.0014  
 \* WITS130  
 250 RX 0 400 456 575 620 650 660 682  
 251 RY 1396.5 1394 1393.5 1393 1392 1394 1395 1396.5

\* Combines RD371, IW374 and RIW377

252 KK CIW374  
 253 KO 0 0 0.0 0 22  
 \* Hard Coded  
 254 HC 3 0.54

\* ROUTE UPDATED 4/14/4

255 KK RIW374  
 256 KO 0 0 0.0 0 22  
 257 RS 15 FLOW 0.0 0.0  
 258 RC 0.04 0.04 0.04 18876 0.0081  
 \* TSR371  
 259 RX 0 142 486 525 651 770 1000 1518  
 260 RY 1900 1900 1899 1897 1896 1899 1899.5 1900

261 KK IW359A  
 262 KO 0 0 0.0 1 22  
 263 BA 1.893  
 264 LG 0.348 0.21 6.399 0.159 5.491  
 265 UC 0.946 0.659  
 \* Natural Time-Area Relation  
 266 UA 0 3 5 8 12 20 43 75 90 96  
 267 UA 100

\* Combines IW359A, RIW374, and RIW366

268 KK CW359A  
 269 KO 0 0 0.0 0 22  
 270 HC 3



```

wtec6-addendum-mod3.out
** -Updated route RD618 information **
** -Hard coding on CPI618 was adjusted **
** -Hard coding on CPI615 was adjusted **
**
** -Route RPI639 changed from following US60 to CPI636 to route **
** through US60/RR structures and combine with CWI506 **
** -Cards PI639, D524, RD524, CPI639 and RPI639 were changed from **
** following R624A to follow RWI524 **
** -CPI636 was removed-no longer needed to combine PI636 & RPI639 **
** -RPI639 information was changed to reflect new route **
** -CWI506 changed from combining 3 to combine 4: **
** RWI525, RWI524, RPI639 & WI506 **
** -Hard coding on CWI506 was adjusted **
** -Hard Coding on CWI500 was adjusted **
**

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** -ADDENDUM-ADDENDUM-ADDENDUM-ADDENDUM-ADDENDUM-ADDENDUM-ADDENDUM-ADDENDUM- **
** -ADDENDUM-ADDENDUM-ADDENDUM-ADDENDUM-ADDENDUM-ADDENDUM-ADDENDUM-ADDENDUM- **
*****

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**
** WITTMANN AREA DRAINAGE MASTER STUDY UPDATE-HYDROLOGY MODEL 2004 **
**
*****

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```

PROJECT: Wittmann ADMS Update
CLIENT: Flood Control District of Maricopa County
PREPARED BY: Entellus, Inc.
PROJECT NO: FCD 2002C029 Entellus 310.032
FILE NAME: WTEC6-addendum.hcl CREATED DATE: JUL 01, 2004
MODIFIED DATE: MAY 04, 2005

```

STORM: 100-year 6-hour Storm

DEVELOPMENT CONDITIONS: Existing Conditions

MODELING ASSUMPTIONS:

It was assumed that the US60 did not have adequate storage to cause any significant attenuation.

The assumption was made that the CAP Canal embankment would not be breached under a large flood event.

The assumption was made that the Beardsley Canal would not fail under a large storm event (Per District Instruction). In addition, the berm north of the Beardsley canal and east of US60 was assumed to fail (per district Instruction).

For both the CAP and Beardsley Canals, once the berm elevation was reached weir flow was assumed. It was also assumed that any weir flow over the canal that might enter the canal and be diverted out of the study area was insignificant, and thus was ignored. In other words all weir flow over the canal embankment reaches the downstream concentration point.

Typical X-sections were developed, and it was assumed that a typical x-sect could adequately represent various reaches.

Time-Area Relations were used base on the District's Hydrology Manual criteria. Two Time-Area Relation Curves were utilized:  
 -Urban  
 -Natural

The Time-Area Relation Curves were taken directly from the manual

MODELING METHODS:

This model utilizes OI cards to input the Padelford hydrographs from the 100-year 6-hour existing conditions model developed by A-N west Inc. for the Padelford wash Floodplain Delineation Study. The hydrograph was altered from its original form (2-minute interval to 5-minute interval) through simple interpolation.

Clark unit Hydrographs were used for all subbasins except the two subbasins directly upstream of the Bonita Dam (PD726B and PD740 use S-graphs). The UC parameters were calculated using the WMS7.0 software.

For Basins PD726B and PD740 S-Graphs were utilized per the request of the FCDMC. Limited details regarding the calculations of the S-graphs can be found in the model by the basin KK card. For full details of the S-graph calculations refer to the Appendix.

Normal Depth routing was used for all routing reaches.

Hard coding was used to account for the percentage of area associated with a diversion. Because of the use of JD cards and aerial reduction, hard coding was necessary to properly account for area. For a given diversion a percentage of the flow is routed to two different locations. The same percentage of area follows that diverted flow. In addition the area downstream of the main path is reduced or increased and

wtec6-addendum-mod3.out  
 is hard coded to account for the loss or gain of area.  
 Hard coding was performed based on the 6-hour existing  
 conditions model.

Stage-storage was developed for all the structures  
 along the CAP Canal, as well as along the Beardsley  
 Canal. In addition several stage-storage locations  
 were developed for areas with significant storage  
 along the SR74. No storage was modeled along the US60  
 and railroad, but the culverts were analyzed for  
 diversion potential. Diversions were placed in the  
 model where deemed appropriate. See appendix for  
 details.

FLO-2D was utilized to calculate the split flows at  
 concentration points CIW351, CIW357, CIW363 and CWI576.  
 See appendix for modeling details.

\*\*\*\*\*  
 \*\*\*\*\*

161 IO OUTPUT CONTROL VARIABLES  
 IPRNT 5 PRINT CONTROL  
 IPLOT 0 PLOT CONTROL  
 QSCAL 0. HYDROGRAPH PLOT SCALE

IT HYDROGRAPH TIME DATA  
 NMIN 5 MINUTES IN COMPUTATION INTERVAL  
 IDATE 1 0 STARTING DATE  
 ITIME 0000 STARTING TIME  
 NQ 1500 NUMBER OF HYDROGRAPH ORDINATES  
 NDDATE 6 0 ENDING DATE  
 NDTIME 0455 ENDING TIME  
 ICENT 19 CENTURY MARK

COMPUTATION INTERVAL 0.08 HOURS  
 TOTAL TIME BASE 124.92 HOURS

ENGLISH UNITS  
 DRAINAGE AREA SQUARE MILES  
 PRECIPITATION DEPTH INCHES  
 LENGTH, ELEVATION FEET  
 FLOW CUBIC FEET PER SECOND  
 STORAGE VOLUME ACRE-FEET  
 SURFACE AREA ACRES  
 TEMPERATURE DEGREES FAHRENHEIT

163 JD INDEX STORM NO. 1  
 STRM 3.40 PRECIPITATION DEPTH  
 TRDA 0.01 TRANSPOSITION DRAINAGE AREA

164 PI PRECIPITATION PATTERN  
 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00  
 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00  
 0.00 0.00 0.00 0.01 0.01 0.01 0.01 0.01 0.01 0.01 0.03  
 0.03 0.03 0.05 0.05 0.05 0.15 0.15 0.15 0.03 0.03  
 0.03 0.01 0.01 0.01 0.01 0.01 0.01 0.01 0.00 0.00 0.00  
 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00

167 JD INDEX STORM NO. 2  
 STRM 3.38 PRECIPITATION DEPTH  
 TRDA 0.50 TRANSPOSITION DRAINAGE AREA

0 PI PRECIPITATION PATTERN  
 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00  
 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00  
 0.00 0.00 0.00 0.01 0.01 0.01 0.01 0.01 0.01 0.01 0.03  
 0.03 0.03 0.05 0.05 0.05 0.15 0.15 0.15 0.03 0.03  
 0.03 0.01 0.01 0.01 0.01 0.01 0.01 0.01 0.00 0.00 0.00  
 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00

168 JD INDEX STORM NO. 3  
 STRM 3.31 PRECIPITATION DEPTH  
 TRDA 2.80 TRANSPOSITION DRAINAGE AREA

169 PI PRECIPITATION PATTERN  
 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00  
 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00  
 0.00 0.00 0.00 0.01 0.01 0.01 0.01 0.01 0.01 0.01 0.03  
 0.03 0.03 0.07 0.07 0.07 0.08 0.08 0.08 0.05 0.05  
 0.05 0.02 0.02 0.01 0.01 0.01 0.01 0.01 0.00 0.00 0.00  
 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00

172 JD INDEX STORM NO. 4  
 STRM 3.13 PRECIPITATION DEPTH  
 TRDA 16.00 TRANSPOSITION DRAINAGE AREA

173 PI PRECIPITATION PATTERN  
 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.01  
 0.01 0.01 0.00 0.01 0.00 0.00 0.00 0.00 0.00 0.00  
 0.00 0.00 0.01 0.00 0.00 0.00 0.00 0.01 0.01 0.01  
 0.01 0.01 0.01 0.01 0.01 0.01 0.02 0.02 0.02 0.03  
 0.03 0.03 0.06 0.06 0.06 0.07 0.07 0.07 0.04 0.04  
 0.04 0.02 0.02 0.02 0.01 0.01 0.01 0.01 0.01 0.01  
 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00

176 JD INDEX STORM NO. 5  
 STRM 2.76 PRECIPITATION DEPTH

TRDA 90.00 TRANSPOSITION DRAINAGE AREA

177 PI PRECIPITATION PATTERN

0.01	0.01	0.01	0.00	0.00	0.00	0.01	0.01	0.01	0.01
0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01
0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01
0.01	0.01	0.01	0.01	0.01	0.01	0.02	0.02	0.02	0.03
0.03	0.03	0.05	0.05	0.05	0.05	0.05	0.05	0.04	0.04
0.04	0.02	0.02	0.02	0.02	0.02	0.02	0.01	0.01	0.01
0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01
0.01	0.01								

180 JD INDEX STORM NO. 6

STRM 1.94 PRECIPITATION DEPTH

TRDA 500.00 TRANSPOSITION DRAINAGE AREA

181 PI PRECIPITATION PATTERN

0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01
0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01
0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01
0.01	0.01	0.01	0.01	0.01	0.01	0.02	0.02	0.02	0.03
0.03	0.03	0.04	0.04	0.04	0.04	0.04	0.04	0.04	0.04
0.04	0.03	0.03	0.03	0.02	0.02	0.02	0.01	0.01	0.01
0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01
0.01	0.01								

\*\*\* \*\*

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\* \* \*

184 KK IW371 \* \*

\* \* \*

\*\*\*\*\*

185 KO OUTPUT CONTROL VARIABLES

IPRNT	5	PRINT CONTROL
IPLOT	0	PLOT CONTROL
QSCAL	0.	HYDROGRAPH PLOT SCALE
IPNCH	1	PUNCH COMPUTED HYDROGRAPH
IOUT	22	SAVE HYDROGRAPH ON THIS UNIT
ISAV1	1	FIRST ORDINATE PUNCHED OR SAVED
ISAV2	1500	LAST ORDINATE PUNCHED OR SAVED
TIMINT	0.083	TIME INTERVAL IN HOURS

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\* \* \*

191 KK D371 \* \*

\* \* \*

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192 KO OUTPUT CONTROL VARIABLES

IPRNT	5	PRINT CONTROL
IPLOT	0	PLOT CONTROL
QSCAL	0.	HYDROGRAPH PLOT SCALE
IPNCH	0	PUNCH COMPUTED HYDROGRAPH
IOUT	22	SAVE HYDROGRAPH ON THIS UNIT
ISAV1	1	FIRST ORDINATE PUNCHED OR SAVED
ISAV2	1500	LAST ORDINATE PUNCHED OR SAVED
TIMINT	0.083	TIME INTERVAL IN HOURS

\*\*\* \*\*

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\* \* \*

196 KK RIW371 \* \*

\* \* \*

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197 KO OUTPUT CONTROL VARIABLES

IPRNT	5	PRINT CONTROL
IPLOT	0	PLOT CONTROL
QSCAL	0.	HYDROGRAPH PLOT SCALE
IPNCH	0	PUNCH COMPUTED HYDROGRAPH
IOUT	22	SAVE HYDROGRAPH ON THIS UNIT
ISAV1	1	FIRST ORDINATE PUNCHED OR SAVED
ISAV2	1500	LAST ORDINATE PUNCHED OR SAVED
TIMINT	0.083	TIME INTERVAL IN HOURS

\*\*\* \*\*

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\* \* \*

202 KK IW366 \* \*

\* \* \*

\*\*\*\*\*

203 KO OUTPUT CONTROL VARIABLES

IPRNT	5	PRINT CONTROL
IPLOT	0	PLOT CONTROL

QSCAL	0.	HYDROGRAPH PLOT SCALE
IPNCH	1	PUNCH COMPUTED HYDROGRAPH
IOUT	22	SAVE HYDROGRAPH ON THIS UNIT
ISAV1	1	FIRST ORDINATE PUNCHED OR SAVED
ISAV2	1500	LAST ORDINATE PUNCHED OR SAVED
TIMINT	0.083	TIME INTERVAL IN HOURS

\*\*\* \*\*

```

*****
*
*
*   CIW366
*
*****

```

```

210 KO      OUTPUT CONTROL VARIABLES
            IPRNT      5  PRINT CONTROL
            IPLOT      0  PLOT CONTROL
            QSCAL      0. HYDROGRAPH PLOT SCALE
            IPNCH      0  PUNCH COMPUTED HYDROGRAPH
            IOUT       22 SAVE HYDROGRAPH ON THIS UNIT
            ISAV1      1  FIRST ORDINATE PUNCHED OR SAVED
            ISAV2     1500 LAST ORDINATE PUNCHED OR SAVED
            TIMINT     0.083 TIME INTERVAL IN HOURS

```

\*\*\* \*\*

```

*****
*
*
*   RIW366
*
*****

```

```

213 KO      OUTPUT CONTROL VARIABLES
            IPRNT      5  PRINT CONTROL
            IPLOT      0  PLOT CONTROL
            QSCAL      0. HYDROGRAPH PLOT SCALE
            IPNCH      0  PUNCH COMPUTED HYDROGRAPH
            IOUT       22 SAVE HYDROGRAPH ON THIS UNIT
            ISAV1      1  FIRST ORDINATE PUNCHED OR SAVED
            ISAV2     1500 LAST ORDINATE PUNCHED OR SAVED
            TIMINT     0.083 TIME INTERVAL IN HOURS

```

\*\*\* \*\*

```

*****
*
*
*   IW377
*
*****

```

```

219 KO      OUTPUT CONTROL VARIABLES
            IPRNT      5  PRINT CONTROL
            IPLOT      0  PLOT CONTROL
            QSCAL      0. HYDROGRAPH PLOT SCALE
            IPNCH      0  PUNCH COMPUTED HYDROGRAPH
            IOUT       22 SAVE HYDROGRAPH ON THIS UNIT
            ISAV1      1  FIRST ORDINATE PUNCHED OR SAVED
            ISAV2     1500 LAST ORDINATE PUNCHED OR SAVED
            TIMINT     0.083 TIME INTERVAL IN HOURS

```

\*\*\* \*\*

```

*****
*
*
*   RIW377
*
*****

```

```

231 KO      OUTPUT CONTROL VARIABLES
            IPRNT      5  PRINT CONTROL
            IPLOT      0  PLOT CONTROL
            QSCAL      0. HYDROGRAPH PLOT SCALE
            IPNCH      0  PUNCH COMPUTED HYDROGRAPH
            IOUT       22 SAVE HYDROGRAPH ON THIS UNIT
            ISAV1      1  FIRST ORDINATE PUNCHED OR SAVED
            ISAV2     1500 LAST ORDINATE PUNCHED OR SAVED
            TIMINT     0.083 TIME INTERVAL IN HOURS

```

\*\*\* \*\*

```

*****
*
*
*   IW374
*
*****

```

237 KO            OUTPUT CONTROL VARIABLES  
                  IPRNT            5    PRINT CONTROL  
                  IPLOT            0    PLOT CONTROL  
                  QSCAL            0.    HYDROGRAPH PLOT SCALE  
                  IPNCH            1    PUNCH COMPUTED HYDROGRAPH  
                  IOUT             22    SAVE HYDROGRAPH ON THIS UNIT  
                  ISAV1            1    FIRST ORDINATE PUNCHED OR SAVED  
                  ISAV2            1500 LAST ORDINATE PUNCHED OR SAVED  
                  TIMINT           0.083 TIME INTERVAL IN HOURS

\*\*\* \*\*

\*\*\*\*\*  
 \*            \*  
 243 KK       \*        D371       \*  
 \*            \*  
 \*\*\*\*\*

244 KO            OUTPUT CONTROL VARIABLES  
                  IPRNT            5    PRINT CONTROL  
                  IPLOT            0    PLOT CONTROL  
                  QSCAL            0.    HYDROGRAPH PLOT SCALE  
                  IPNCH            0    PUNCH COMPUTED HYDROGRAPH  
                  IOUT             22    SAVE HYDROGRAPH ON THIS UNIT  
                  ISAV1            1    FIRST ORDINATE PUNCHED OR SAVED  
                  ISAV2            1500 LAST ORDINATE PUNCHED OR SAVED  
                  TIMINT           0.083 TIME INTERVAL IN HOURS

\*\*\* \*\*

\*\*\*\*\*  
 \*            \*  
 246 KK       \*        RD371      \*  
 \*            \*  
 \*\*\*\*\*

247 KO            OUTPUT CONTROL VARIABLES  
                  IPRNT            5    PRINT CONTROL  
                  IPLOT            0    PLOT CONTROL  
                  QSCAL            0.    HYDROGRAPH PLOT SCALE  
                  IPNCH            0    PUNCH COMPUTED HYDROGRAPH  
                  IOUT             22    SAVE HYDROGRAPH ON THIS UNIT  
                  ISAV1            1    FIRST ORDINATE PUNCHED OR SAVED  
                  ISAV2            1500 LAST ORDINATE PUNCHED OR SAVED  
                  TIMINT           0.083 TIME INTERVAL IN HOURS

\*\*\* \*\*

\*\*\*\*\*  
 \*            \*  
 252 KK       \*        CIW374    \*  
 \*            \*  
 \*\*\*\*\*

253 KO            OUTPUT CONTROL VARIABLES  
                  IPRNT            5    PRINT CONTROL  
                  IPLOT            0    PLOT CONTROL  
                  QSCAL            0.    HYDROGRAPH PLOT SCALE  
                  IPNCH            0    PUNCH COMPUTED HYDROGRAPH  
                  IOUT             22    SAVE HYDROGRAPH ON THIS UNIT  
                  ISAV1            1    FIRST ORDINATE PUNCHED OR SAVED  
                  ISAV2            1500 LAST ORDINATE PUNCHED OR SAVED  
                  TIMINT           0.083 TIME INTERVAL IN HOURS

\*\*\* \*\*

\*\*\*\*\*  
 \*            \*  
 255 KK       \*        RIW374    \*  
 \*            \*  
 \*\*\*\*\*

256 KO            OUTPUT CONTROL VARIABLES  
                  IPRNT            5    PRINT CONTROL  
                  IPLOT            0    PLOT CONTROL  
                  QSCAL            0.    HYDROGRAPH PLOT SCALE  
                  IPNCH            0    PUNCH COMPUTED HYDROGRAPH  
                  IOUT             22    SAVE HYDROGRAPH ON THIS UNIT  
                  ISAV1            1    FIRST ORDINATE PUNCHED OR SAVED  
                  ISAV2            1500 LAST ORDINATE PUNCHED OR SAVED  
                  TIMINT           0.083 TIME INTERVAL IN HOURS

\*\*\* \*\*

\*\*\*\*\*

261 KK \*  
\* Iw359A \*  
\*  
\*\*\*\*\*

262 KO OUTPUT CONTROL VARIABLES  
 IPRNT 5 PRINT CONTROL  
 IPLOT 0 PLOT CONTROL  
 QSCAL 0. HYDROGRAPH PLOT SCALE  
 IPNCH 1 PUNCH COMPUTED HYDROGRAPH  
 IOUT 22 SAVE HYDROGRAPH ON THIS UNIT  
 ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
 ISAV2 1500 LAST ORDINATE PUNCHED OR SAVED  
 TIMINT 0.083 TIME INTERVAL IN HOURS

\*\*\* \*\*

\*\*\*\*\*  
 \*  
 \* CW359A \*  
 \*  
 \*\*\*\*\*

269 KO OUTPUT CONTROL VARIABLES  
 IPRNT 5 PRINT CONTROL  
 IPLOT 0 PLOT CONTROL  
 QSCAL 0. HYDROGRAPH PLOT SCALE  
 IPNCH 0 PUNCH COMPUTED HYDROGRAPH  
 IOUT 22 SAVE HYDROGRAPH ON THIS UNIT  
 ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
 ISAV2 1500 LAST ORDINATE PUNCHED OR SAVED  
 TIMINT 0.083 TIME INTERVAL IN HOURS

1

RUNOFF SUMMARY  
 FLOW IN CUBIC FEET PER SECOND  
 TIME IN HOURS, AREA IN SQUARE MILES

OPERATION	STATION	PEAK FLOW	TIME OF PEAK	AVERAGE FLOW FOR MAXIMUM PERIOD			BASIN AREA	MAXIMUM STAGE	TIME OF MAX STAGE
				6-HOUR	24-HOUR	72-HOUR			
+	HYDROGRAPH AT								
+		IW371	103.	4.08	12.	3.	1.	0.05	
+	DIVERSION TO								
+		DO371	77.	4.08	6.	2.	1.	0.05	
+	HYDROGRAPH AT								
+		D371	26.	4.08	5.	1.	0.	0.05	
+	ROUTED TO								
+		RIW371	15.	6.50	5.	1.	0.	0.05	
+	HYDROGRAPH AT								
+		IW366	1107.	4.58	209.	52.	17.	1.06	
+	2 COMBINED AT								
+		CIW366	1105.	4.58	213.	54.	18.	1.07	
+	ROUTED TO								
+		RIW366	1002.	4.92	212.	54.	18.	1.07	
+	HYDROGRAPH AT								
+		IW377	153.	4.25	22.	6.	2.	0.11	
+	ROUTED TO								
+		SSR010	121.	4.50	22.	6.	2.	0.11	
+	ROUTED TO								
+		RIW377	98.	5.25	22.	6.	2.	0.11	
+	HYDROGRAPH AT								
+		IW374	758.	4.25	97.	24.	8.	0.39	
+	HYDROGRAPH AT								
+		D371	77.	4.08	6.	2.	1.	0.05	
+	ROUTED TO								
+		RD371	40.	4.33	6.	2.	1.	0.05	
+	3 COMBINED AT								
+		CIW374	784.	4.25	124.	31.	10.	0.54	
+	ROUTED TO								
+		RIW374	434.	5.75	121.	31.	10.	0.54	
+	HYDROGRAPH AT								
+		IW359A	1716.	4.75	382.	96.	32.	1.89	
+	3 COMBINED AT								
+		CW359A	2334.	4.83	662.	169.	56.	3.50	

\*\*\* NORMAL END OF HEC-1 \*\*\*

Wittmann Phase IV  
Floodplain Delineation Study

**100-Year 24-Hour Storm HEC-1 Model**

**(Excerpts from the Wittmann ADMSU model)**

1\*\*\*\*\*
\*
\* FLOOD HYDROGRAPH PACKAGE (HEC-1)
\* JUN 1998
\* VERSION 4.1
\* RUN DATE 03AUG11 TIME 17:14:59
\*\*\*\*\*

\*\*\*\*\*
\*
\* U.S. ARMY CORPS OF ENGINEERS
\* HYDROLOGIC ENGINEERING CENTER
\* 609 SECOND STREET
\* DAVIS, CALIFORNIA 95616
\* (916) 756-1104
\*\*\*\*\*

X X XXXXXXX XXXXX X
X X X X X XX
X X X X X X
XXXXXXXX XXXX X XXXXX X
X X X X X X
X X X X X X
X X XXXXXXX XXXXX XXX

THIS PROGRAM REPLACES ALL PREVIOUS VERSIONS OF HEC-1 KNOWN AS HEC1 (JAN 73), HEC1GS, HEC1DB, AND HEC1KW.
THE DEFINITIONS OF VARIABLES -RTIMP- AND -RTIOR- HAVE CHANGED FROM THOSE USED WITH THE 1973-STYLE INPUT STRUCTURE.
THE DEFINITION OF -AMSK- ON RM-CARD WAS CHANGED WITH REVISIONS DATED 28 SEP 81. THIS IS THE FORTRAN77 VERSION
NEW OPTIONS: DAMBREAK OUTFLOW SUBMERGENCE, SINGLE EVENT DAMAGE CALCULATION, DSS:WRITE STAGE FREQUENCY,
DSS:READ TIME SERIES AT DESIRED CALCULATION INTERVAL LOSS RATE:GREEN AND AMPT INFILTRATION
KINEMATIC WAVE: NEW FINITE DIFFERENCE ALGORITHM

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10
1 ID \*\*\*\*\*
2 ID \*\* -ADDENDUM-ADDENDUM-ADDENDUM-ADDENDUM-ADDENDUM-ADDENDUM-ADDENDUM-ADDENDUM-ADDENDUM-ADDENDUM- \*\*
3 ID \*\* -ADDENDUM-ADDENDUM-ADDENDUM-ADDENDUM-ADDENDUM-ADDENDUM-ADDENDUM-ADDENDUM-ADDENDUM-ADDENDUM- \*\*
4 ID \*\*\*\*\*
5 ID \*\*
6 ID \*\* ADDENDUM TO THE WITTMANN AREA DRAINAGE MASTER STUDY UPDATE HYDROLOGY \*\*
7 ID \*\*
8 ID \*\*\*\*\*
9 ID \*\* -ADDENDUM-ADDENDUM-ADDENDUM-ADDENDUM-ADDENDUM-ADDENDUM-ADDENDUM-ADDENDUM-ADDENDUM-ADDENDUM- \*\*
10 ID \*\* -ADDENDUM-ADDENDUM-ADDENDUM-ADDENDUM-ADDENDUM-ADDENDUM-ADDENDUM-ADDENDUM-ADDENDUM-ADDENDUM- \*\*
11 ID \*\*\*\*\*
12 ID \*\*
13 ID \*\* THE FOLLOWING CHANGES HAVE BEEN MADE TO THE WITTMANN HYDROLOGY MODELS \*\*
14 ID \*\* DATED OCTOBER 2004: \*\*
15 ID \*\*
16 ID \*\* -Diversion added after CTW480 \*\*
17 ID \*\* -Main flow (5%) to CTW478 \*\*
18 ID \*\* -Diverted flow (95%) to CWI584 \*\*
19 ID \*\* -Added concentration point CWI584 combining RD480 and WI584 \*\*
20 ID \*\* -Hard coding was added to CTW478 \*\*
21 ID \*\* -Route added after D480 (Diversion Recovery) \*\*
22 ID \*\* -Route RWI584 was adjusted \*\*
23 ID \*\* -Storage Route SCP050 was adjusted \*\*
24 ID \*\* -Route R508\* was adjusted \*\*
25 ID \*\* -Hard coding for CWI510 was adjusted \*\*
26 ID \*\*
27 ID \*\* -Diversion D618 was moved from after CPI618 to before RDCP12 \*\*
28 ID \*\* -Updated route RD618 information \*\*
29 ID \*\* -Hard coding on CPI618 was adjusted \*\*
30 ID \*\* -Hard coding on CPI615 was adjusted \*\*
31 ID \*\*
32 ID \*\* -Route RPI639 changed from following US60 to CPI636 to route \*\*
33 ID \*\* through US60/RR structures and combine with CWI506 \*\*
34 ID \*\* -Cards PI639, D524, RD524, CPI639 and RPI639 were changed from \*\*
35 ID \*\* following R624A to follow RWI524 \*\*
36 ID \*\* -CPI636 was removed-no longer needed to combine PI636 & RPI639 \*\*
37 ID \*\* -RPI639 information was changed to reflect new route \*\*
38 ID \*\* -CWI506 changed from combining 3 to combine 4: \*\*
39 ID \*\* RWI525, RWI524, RPI639 & WI506 \*\*
40 ID \*\* -Hard coding on CWI506 was adjusted \*\*
41 ID \*\* -Hard Coding on CWI500 was adjusted \*\*
42 ID \*\*
43 ID \*\*\*\*\*
44 ID \*\* -ADDENDUM-ADDENDUM-ADDENDUM-ADDENDUM-ADDENDUM-ADDENDUM-ADDENDUM-ADDENDUM-ADDENDUM-ADDENDUM- \*\*
45 ID \*\* -ADDENDUM-ADDENDUM-ADDENDUM-ADDENDUM-ADDENDUM-ADDENDUM-ADDENDUM-ADDENDUM-ADDENDUM-ADDENDUM- \*\*
46 ID \*\*\*\*\*
47 ID \*\*
48 ID \*\*
49 ID \*\*
50 ID \*\*\*\*\*
51 ID \*\*\*\*\*
52 ID \*\*
53 ID \*\* WITTMANN AREA DRAINAGE MASTER STUDY UPDATE-HYDROLOGY MODEL 2004 \*\*
54 ID \*\*
55 ID \*\*\*\*\*

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10
56 ID \*\*\*\*\*
57 ID \*\*\*\*\*
58 ID PROJECT: Wittmann ADMS Update
59 ID CLIENT: Flood Control District of Maricopa County
60 ID PREPARED BY: Entellus, Inc.
61 ID PROJECT No: FCD 2002C029 Entellus 310.032
62 ID FILE NAME: WTEC24-addendum.hcl CREATED DATE: FEB 01, 2004
63 ID MODIFIED DATE: MAY 04, 2005
64 ID \*\*\*\*\*

65 ID STORM: 100-year 24-hour Storm  
66 ID  
67 ID  
68 ID DEVELOPMENT CONDITIONS:  
69 ID Existing Conditions  
70 ID  
71 ID MODELING ASSUMPTIONS:  
72 ID It was assumed that the US60 did not have adequate  
73 ID storage to cause any significant attenuation.  
74 ID  
75 ID The assumption was made that the CAP Canal embankment  
76 ID would not be breached under a large flood event.  
77 ID  
78 ID The assumption was made that the Beardsley Canal would  
79 ID not fail under a large storm event (Per District  
80 ID Instruction). In addition, the berm north of the  
81 ID Beardsley canal and east of US60 was assumed to fail  
82 ID (per district instruction).  
83 ID  
84 ID For both the CAP and Beardsley Canals, once the berm  
85 ID elevation was reached weir flow was assumed. It was  
86 ID also assumed that any weir flow over the canal that  
87 ID might enter the canal and be diverted out of the study  
88 ID area was insignificant, and thus was ignored. In other  
89 ID words all weir flow over the canal embankment reaches  
90 ID the downstream concentration point.  
91 ID  
92 ID Typical X-sects were developed, and it was assumed  
93 ID that a typical x-sect could adequately represent  
94 ID various reaches.  
95 ID  
96 ID Time-Area Relations were used base on the District's  
97 ID Hydrology Manual criteria. Two Time-Area Relation  
98 ID Curves were utilized:  
99 ID -Urban  
100 ID -Natural  
101 ID The Time-Area Relation Curves were taken directly  
102 ID from the manual  
103 ID  
104 ID MODELING METHODS:  
105 ID  
106 ID This model utilizes QI cards to input the Padelford  
107 ID hydrographs from the 100-year 6-hour existing  
108 ID conditions model developed by A-N West Inc. for the  
109 ID Padelford wash Floodplain Delineation Study. The  
110 ID hydrograph was altered from its original form  
(2-minute interval to 5-minute interval) through  
HEC-1 INPUT

1

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

111 ID simple interpolation.  
112 ID  
113 ID Clark Unit Hydrographs were used for all subbasins  
114 ID except the two subbasins directly upstream of the  
115 ID Bonita Dam (PD726B and PD740 use S-graphs). The UC  
116 ID parameters were calculated using the WMS7.0 software.  
117 ID  
118 ID For Basins PD726B and PD740 S-Graphs were utilized per  
119 ID the request of the FCDMC. Limited details regarding  
120 ID the calculations of the S-graphs can be found in the  
121 ID model by the basin KK card. For full details of the  
122 ID S-graph calculations refer to the Appendix.  
123 ID  
124 ID Normal Depth routing was used for all routing reaches.  
125 ID  
126 ID Hard coding was used to account for the percentage of  
127 ID area associated with a diversion. Because of the use  
128 ID of JD cards and areal reduction, hard coding was  
129 ID necessary to properly account for area. For a given  
130 ID diversion a percentage of the flow is routed to two  
131 ID different locations. The same percentage of area  
132 ID follows that diverted flow. In addition the area  
133 ID downstream of the main path is reduced or increased and  
134 ID is hard coded to account for the loss or gain of area.  
135 ID Hard coding was performed based on the 24-hour existing  
136 ID conditions model.  
137 ID  
138 ID Stage-storage was developed for all the structures  
139 ID along the CAP Canal, as well as along the Beardsley  
140 ID Canal. In addition several stage-storage locations  
141 ID were developed for areas with significant storage  
142 ID along the SR74. No storage was modeled along the US60  
143 ID and railroad, but the culverts were analyzed for  
144 ID diversion potential. Diversions were placed in the  
145 ID model where deemed appropriate. See appendix for  
146 ID details.  
147 ID  
148 ID FLO-2D was utilized to calculate the split flows at  
149 ID concentration points CIW351, CIW357, CIW363 and CWI576.  
150 ID See appendix for modeling details.  
151 ID  
152 ID \*\*\*\*\*  
153 ID \*\*\*\*\*  
154 ID  
155 ID \*DIAGRAM  
IT 5 0 1500  
156 ID IO 5  
157 ID IN 15 0  
158 ID JD 4.18 0.01  
\* 24-hour distribution  
159 PC 0.0 0.002 0.005 0.008 0.011 0.014 0.017 0.02 0.023 0.026  
160 PC 0.029 0.032 0.035 0.038 0.041 0.044 0.048 0.052 0.056 0.06  
161 PC 0.064 0.068 0.072 0.076 0.08 0.085 0.09 0.095 0.1 0.105  
162 PC 0.11 0.115 0.12 0.126 0.133 0.14 0.147 0.155 0.163 0.172  
163 PC 0.181 0.191 0.203 0.218 0.236 0.257 0.283 0.387 0.663 0.707  
HEC-1 INPUT

1

LINE	ID	1	2	3	4	5	6	7	8	9	10
164	PC	0.735	0.758	0.776	0.791	0.804	0.815	0.825	0.834	0.842	0.849
165	PC	0.856	0.863	0.869	0.875	0.881	0.887	0.893	0.898	0.903	0.908
166	PC	0.913	0.918	0.922	0.926	0.93	0.934	0.938	0.942	0.946	0.95
167	PC	0.953	0.956	0.959	0.962	0.965	0.968	0.971	0.974	0.977	0.98
168	PC	0.983	0.986	0.989	0.992	0.995	0.998	1.0			
169	JD	3.929	10.0								
170	JD	3.856	20.0								
171	JD	3.725	40.0								
172	JD	3.599	80.0								
173	JD	3.532	120.0								
174	JD	3.444	200.0								
175	JD	3.385	300.0								
176	JD	3.289	500.0								

\*\*\*\*\*  
 \*\*\*\*\*  
 \*\*\*\*\* BEGINNING OF IONA WASH SUBAREA \*\*\*\*\*  
 \*\*\*\*\*  
 \*\*\*\*\*  
 \*\*\*\*\* BEGINNING OF IONA WASH REGION 1 \*\*\*\*\*  
 \*\*\*\*\*  
 \*\*\*\*\*

177	KK	IW371									
178	KO	0	0	0.0	1	22					
179	BA	0.0529									
180	LG	0.275	0.25	4.15	0.473	26.25					
181	UC	0.308	0.327								
		* Urban Time-Area Relation									
182	UA	0	5	16	30	65	77	84	90	94	97
183	UA	100									
		* Railroad Diversion: Main path to CIW366, Diversion to CIW374									
		* From Structure RR40, See Appendix D.5 for analysis									

184	KK	D371									
185	KO	0	0	0.0	0	22					
186	DT	D0371	0.0	0.0							
187	DI	0.0	14	79	208	383	823	1374	2014	2724	
188	DQ	0.0	0	55	173	335	763	1302	1930	2632	

\* ROUTE UPDATED 4/14/4  
 HEC-1 INPUT

LINE	ID	1	2	3	4	5	6	7	8	9	10
------	----	---	---	---	---	---	---	---	---	---	----

189	KK	RIW371									
190	KO	0			0	22					
191	RS	15	FLOW	0.0	0.0						
192	RC	0.04	0.04	0.04	11583	0.0088					
		* TSR371									
193	RX	0	142	486	525	651	770	1000	1518		
194	RY	1900	1900	1899	1897	1896	1899	1899.5	1900		

195	KK	IW366									
196	KO	0	0	0.0	1	22					
197	BA	1.0558									
198	LG	0.343	0.25	5.762	0.202	7.207					
199	UC	0.796	0.622								
		* Natural Time-Area Relation									
200	UA	0	3	5	8	12	20	43	75	90	96
201	UA	100									

\* Combines IW366 and RIW371

202	KK	CIW366									
203	KO	0	0	0.0	0	22					
		* hard coded									
204	HC	2	1.08								

\* ROUTE UPDATED 4/14/4

205	KK	RIW366									
206	KO	0			0	22					
207	RS	6	FLOW	0.0	0.0						
208	RC	0.04	0.04	0.04	9657	0.007					
		* IWTS30_2									
209	RX	0	6.5	13	57	102	104	109	110		
210	RY	1728	1725.5	1724	1723	1724	1725.5	1727	1728		

211	KK	IW377									
212	KO	0	0	0.0	1	22					
213	BA	0.1125									
214	LG	0.304	0.382	5.921	0.188	5.357					
215	UC	0.442	0.566								
		* Natural Time-Area Relation									
216	UA	0	3	5	8	12	20	43	75	90	96
217	UA	100									

\* \*\* STAGE DISCHARGE / STORAGE BEHIND STRUCTURE SR010 (HWY74 CULVERT)  
HEC-1 INPUT

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

218 KK SSR010  
219 RS 1 STOR 0  
220 SQ 0 36 100 160  
221 SA 0 .05 .41 1.98  
222 SE 2021.3 2024 2026 2028

\* ROUTE UPDATED 4/14/4

223 KK RIW377  
224 KO 0 0 0.0 0 22  
225 RS 6 FLOW 0.0 0.0  
226 RC 0.04 0.04 0.04 6588 0.0096  
\* TSR371  
227 RX 0 142 486 525 651 770 1000 1518  
228 RY 1900 1900 1899 1897 1896 1899 1899.5 1900

229 KK IW374  
230 KO 0 0 0.0 1 22  
231 BA 0.3874  
232 LG 0.315 0.161 8.147 0.086 12.602  
233 UC 0.475 0.374  
\* Natural Time-Area Relation  
234 UA 0 3 5 8 12 20 43 75 90 96  
235 UA 100

\* RAILROAD DIVERSION RECOVERY: ALONG RAILROAD FROM STRUCTURE RR40

236 KK D371  
237 KO 0 0 0.0 0 22  
238 DR DO371

\* ROUTE UPDATED 4/14/4

239 KK RD371  
240 KO 0 0 0.0 0 22  
241 RS 1 FLOW 0.0 0.0  
242 RC 0.043 0.035 0.043 1409 0.0014  
\* WITS130  
243 RX 0 400 456 575 620 650 660 682  
244 RY 1396.5 1394 1393.5 1393 1392 1394 1395 1396.5

\* Combines RD371, IW374 and RIW377  
HEC-1 INPUT

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

245 KK CIW374  
246 KO 0 0 0.0 0 22  
247 \* Hard Coded  
HC 3 0.53

\* ROUTE UPDATED 4/14/4

248 KK RIW374  
249 KO 0 0 0.0 0 22  
250 RS 15 FLOW 0.0 0.0  
251 RC 0.04 0.04 0.04 20437 0.0081  
\* TSR371  
252 RX 0 142 486 525 651 770 1000 1518  
253 RY 1900 1900 1899 1897 1896 1899 1899.5 1900

254 KK IW386  
255 KO 0 0 0.0 1 22  
256 BA 0.3731  
257 LG 0.334 0.192 8.439 0.072 5.278  
258 UC 0.917 0.945  
\* Natural Time-Area Relation  
259 UA 0 3 5 8 12 20 43 75 90 96  
260 UA 100

\* ROUTE UPDATED 4/14/4

261 KK RIW386  
262 KO 0 0 0.0 0 22  
263 RS 6 FLOW 0.0 0.0  
264 RC 0.043 0.035 0.043 10311 0.0078  
\* IWTS20\_1  
265 RX 0 21 75 93 234 280 301 391  
266 RY 1952 1949 1948 1946 1945 1948 1948.5 1952

267 KK IW382  
268 KO 0 0 0.0 1 22  
269 BA 0.6196  
270 LG 0.322 0.275 6.319 0.164 8.175  
271 UC 0.688 0.599  
\* Urban Time-Area Relation

272 UA 0 5 16 30 65 77 84 90 94 97  
273 UA 100  
\*  
\*  
\* Combines IW382 and RIW386

HEC-1 INPUT

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

274 KK CIW382  
275 KO 0 0 0.0 0 22  
276 HC 2

\* US60 Diversion: Main path to D382A, Diversion to CIW381  
\* From Structures US006 and US007, See Appendix D.5 for analysis

277 KK D382B  
278 KO 0 0 0.0 0 22  
279 DT D0382B 0.0 0.0  
280 DI 0 24 180 10000  
281 DQ 0 24 180 180

\* Railroad Diversion: Main path to C359\*, Diversion to CIW384  
\* From Structure RR90 and RR100, See Appendix D.5 for analysis

282 KK D382A  
283 KO 0 0 0.0 0 22  
284 DT D0382A 0.0 0.0  
285 DI 0 14 60 200 315 448 629 1371  
286 DQ 0 0 0 0 55 174 338 1045

\* ROUTE UPDATED 4/14/4

287 KK RIW382  
288 KO 0 0 0.0 0 22  
289 RS 3 FLOW 0.0 0.0  
290 RC 0.04 0.04 0.04 4964 0.012  
\* TSR371  
291 RX 0 142 486 525 651 770 1000 1518  
292 RY 1900 1900 1899 1897 1896 1899 1899.5 1900

293 KK IW384  
294 KO 0 0 0.0 1 22  
295 BA 0.0825  
296 LG 0.343 0.15 8.675 0.065 7.206  
297 UC 0.338 0.327  
\* Natural Time-Area Relation  
298 UA 0 3 5 8 12 20 43 75 90 96  
299 UA 100

\* Railroad Diversion Recovery: Outflow from structures RR90 AND RR100

300 KK D382A  
301 KO 0 0 0.0 0 22  
302 DR D0382A

\* ROUTE UPDATED 4/14/4

HEC-1 INPUT

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

303 KK RD382A  
304 KO 0 0 0.0 0 22  
305 RS 1 FLOW 0.0 0.0  
306 RC 0.043 0.035 0.043 1172 0.0068  
\* WITS130  
307 RX 0 400 456 575 620 650 660 682  
308 RY 1396.5 1394 1393.5 1393 1392 1394 1395 1396.5

\* Combines RD382A and IW384

309 KK CIW384  
310 KO 0 0 0.0 0 22  
\* Hard Coded  
311 HC 2 0.23

\* ROUTE UPDATED 4/14/4

312 KK RIW384  
313 KO 0 0 0.0 0 22  
314 RS 3 FLOW 0.0 0.0  
315 RC 0.04 0.04 0.04 4694 0.0124  
\* TSR371  
316 RX 0 142 486 525 651 770 1000 1518  
317 RY 1900 1900 1899 1897 1896 1899 1899.5 1900

318 KK IW381  
319 KO 0 0 0.0 1 22  
320 BA 0.3841  
321 LG 0.309 0.19 9.152 0.056 11.882  
322 UC 0.704 0.873  
\* Natural Time-Area Relation  
323 UA 0 3 5 8 12 20 43 75 90 96  
324 UA 100

\*
\* This diversion is not routed because the distance traveled from the actual
\* point of diversion (not CIW382) to CIW382 is approximately the same as the
\* distance from the same point of diversion to CIW381. Thus to avoid double
\* counting storage in the routes, a route for the diversion was not included
\* in the model.

\* US60 Diversion Recovery: Outflow from structures US006 and US007

325 KK D382B
326 KO 0 0 0.0 0 22
327 DR D0382B

\* Combines D382B and IW381

HEC-1 INPUT

PAGE 10

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

328 KK CIW381
\* Hard Coded
329 HC 2 0.72

\* ROUTE UPDATED 4/14/4

330 KK RIW381
331 KO 0 0 0.0 0 22
332 RS 3 FLOW 0.0 0.0
333 RC 0.04 0.04 0.04 4964 0.0096
\* TSR371
334 RX 0 142 486 525 651 770 1000 1518
335 RY 1900 1900 1899 1897 1896 1899 1899.5 1900

\* Combines RIW381, RIW382 and RIW384

336 KK C359\*
337 KO 0 0 0.0 0 22
\* Hard Coded
338 HC 3 1.45

\* ROUTE UPDATED 4/14/4

339 KK R359\*
340 KO 0 0 0.0 0 22
341 RS 4 FLOW 0.0 0.0
342 RC 0.04 0.04 0.04 9280 0.0103
\* IWTS30\_2
343 RX 0 6.5 13 57 102 104 109 110
344 RY 1728 1725.5 1724 1723 1724 1725.5 1727 1728

345 KK IW359
346 KO 0 0 0.0 1 22
347 BA 2.912
348 LG 0.345 0.25 5.939 0.187 6.181
349 UC 0.979 0.571
\* Natural Time-Area Relation
350 UA 0 3 5 8 12 20 43 75 90 96
351 UA 100

\* Combines IW359, R359\*, RIW374 and RIW366

352 KK CIW359
353 KO 0 0 0.0 0 22
354 HC 4

\* ROUTE UPDATED 4/14/4

HEC-1 INPUT

PAGE 11

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

355 KK RIW359
356 KO 0 0 0.0 0 22
357 RS 5 FLOW 0.0 0.0
358 RC 0.04 0.04 0.04 13031 0.0059
\* IWTS30\_2 modified
359 RX 0 6.5 13 57 102 104 109 110
360 RY 1728.5 1725.5 1724 1723 1724 1725.5 1727 1728.5

\*\*\*\*\*
\* END OF IONA WASH REGION 1 \*\*\*\*\*

\*\*\*\*\*
\* BEGINNING OF IONA WASH REGION 2 \*\*\*\*\*

361 KK IW389
362 KO 0 0 0.0 1 22
363 BA 0.1312

wtec24-addendum-mod1.out  
 364 LG 0.348 0.13 10.1 0.037 5.526  
 365 UC 0.638 0.825  
 \* Natural Time-Area Relation  
 366 UA 0 3 5 8 12 20 43 75 90 96  
 367 UA 100

\*\*\* STAGE DISCHARGE / STORAGE BEHIND STRUCTURE SR190 (HWY74 CULVERT)

368 KK SSR190  
 369 RS 1 STOR 0  
 370 SQ 0 69 161 184 207 230  
 371 SE 2053.8 2055.5 2056.83 2057.1 2057.4 2057.7  
 372 SA 0 .01 .08 .22 .39 1.13  
 373 SE 2052 2054 2056 2058 2060 2062

\* ROUTE UPDATED 4/14/4

374 KK RIW389  
 375 KO 0 0 0.0 0 22  
 376 RS 6 FLOW 0.0 0.0  
 377 RC 0.04 0.04 0.04 11459 0.0124  
 \* TSR371  
 378 RX 0 142 486 525 651 770 1000 1518  
 379 RY 1900 1900 1899 1897 1896 1899 1899.5 1900

HEC-1 INPUT

PAGE 12

1  
 LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

380 KK IW394  
 381 KO 0 0 0.0 1 22  
 382 BA 4.1731  
 383 LG 0.35 0.327 6.166 0.173 16.632  
 384 UC 1.367 0.857  
 \* Natural Time-Area Relation  
 385 UA 0 3 5 8 12 20 43 75 90 96  
 386 UA 100

\* ROUTE UPDATED 4/14/4

387 KK RIW394  
 388 KO 0 0 0.0 0 22  
 389 RS 3 FLOW 0.0 0.0  
 390 RC 0.043 0.035 0.043 7697 0.0133  
 \* IWTS10\_1  
 391 RX 0 34 89 134 161 198 298 333  
 392 RY 2160 2158 2154 2152 2154 2156 2158 2160

393 KK IW390A  
 394 KO 0 0 0.0 1 22  
 395 BA 0.9003  
 396 LG 0.35 0.25 6.134 0.172 17.033  
 397 UC 1.033 0.963  
 \* Natural Time-Area Relation  
 398 UA 0 3 5 8 12 20 43 75 90 96  
 399 UA 100

\* Combines IW390A and RIW394

400 KK C390A  
 401 KO 0 0 0.0 0 22  
 402 HC 2

\* ROUTE UPDATED 4/14/4

403 KK R390A  
 404 KO 0 0 0.0 0 22  
 405 RS 4 FLOW 0.0 0.0  
 406 RC 0.043 0.035 0.043 8577 0.0134  
 \* IWTS10\_1  
 407 RX 0 34 89 134 161 198 298 333  
 408 RY 2160 2158 2154 2152 2154 2156 2158 2160

HEC-1 INPUT

PAGE 13

1  
 LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

409 KK IW390  
 410 KO 0 0 0.0 1 22  
 411 BA 0.3110  
 412 LG 0.349 0.25 5.071 0.258 5.221  
 413 UC 1.021 1.302  
 \* Natural Time-Area Relation  
 414 UA 0 3 5 8 12 20 43 75 90 96  
 415 UA 100

416 KK IW392  
 417 KO 0 0 0.0 1 22  
 418 BA 1.1135  
 419 LG 0.35 0.15 7.459 0.103 8.347  
 420 UC 0.500 0.278  
 \* Natural Time-Area Relation  
 421 UA 0 3 5 8 12 20 43 75 90 96

422 UA 100  
 \*  
 \*  
 \* Combines Iw392, Iw390 and R390A  
 423 KK CIW390  
 424 KO 0 0 0.0 0 22  
 425 HC 3  
 \*  
 \*  
 \* \*\*\* STAGE DISCHARGE / STORAGE BEHIND STRUCTURE SR160 (HWY74 CULVERT)  
 426 KK SSR160  
 427 RS 1 STOR 0  
 428 SQ 0 762 1440 2640 3840 5040  
 429 SA 0.0 0.04 0.36 3.17 4.85 6.69  
 430 SE 2033 2036 2038 2040 2042 2044  
 \*  
 \*  
 \* ROUTE UPDATED 4/14/4  
 431 KK RIW390  
 432 KO 0 0.0 0 22  
 433 RS 5 FLOW 0.0 0.0  
 434 RC 0.043 0.035 0.043 11626 0.0101  
 \* IWTS20\_1  
 435 RX 0 21 75 93 234 280 301 391  
 436 RY 1952 1949 1948 1946 1945 1948 1948.5 1952  
 \*

1

HEC-1 INPUT

PAGE 14

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

437 KK Iw388  
 438 KO 0 0.0 1 22  
 439 BA 1.2256  
 440 LG 0.337 0.156 8.775 0.061 9.239  
 441 UC 1.200 0.958  
 \* Natural Time-Area Relation  
 442 UA 0 3 5 8 12 20 43 75 90 96  
 443 UA 100  
 \*

\* Combines Iw388, RIW390 and RIW389

444 KK CIW388  
 445 KO 0 0.0 0 22  
 446 HC 3  
 \*

\* ROUTE UPDATED 4/14/4

447 KK RIW388  
 448 KO 0 0.0 0 22  
 449 RS 4 FLOW 0.0 0.0  
 450 RC 0.04 0.04 0.04 7726 0.0094  
 \* IWTS20\_2  
 451 RX 0 79 201 242 303 327 346 350  
 452 RY 1940 1938 1936 1933.5 1936 1936.5 1940 1940  
 \*

\* Combines Iw357 and RIW388

453 KK Iw357  
 454 KO 0 0.0 1 22  
 455 BA 0.7456  
 456 LG 0.33 0.156 8.059 0.083 11.106  
 457 UC 0.742 0.752  
 \* Natural Time-Area Relation  
 458 UA 0 3 5 8 12 20 43 75 90 96  
 459 UA 100  
 \*

\* Combines Iw357 and RIW388

460 KK CIW357  
 461 KO 0 0.0 0 22  
 462 HC 2  
 \*

\* This split was determined using FLO-2D. The model is titled "WEST" and details and documentation about the model are contained in the Appendix D 5.4.  
 HEC-1 INPUT

1

PAGE 15

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

463 KK D357  
 464 DT D0357 0.0 0.0  
 465 DI 0 258 530 766 975 1140 1340 1556 1789 1937  
 466 DI 2120 2357 2514 2736 2992 3255 3301 3472 3610 5000  
 467 DQ 0 56 144 211 291 357 437 383 451 502  
 468 DQ 532 601 633 705 737 793 854 930 947 1312  
 \*

\* ROUTE UPDATED 4/14/4

469 KK RIW357  
 470 KO 0 0.0 0 22  
 471 RS 9 FLOW 0.0 0.0  
 472 RC 0.04 0.04 0.04 16693 0.0079  
 \* IWTS30\_1 modified  
 473 RX 0 50 96 135 160 177 190 206  
 474 RY 1881 1879 1878 1876 1876 1878 1879 1881  
 \*

\*\*\*\*\*  
 \*\*\*\*\* END OF IONA WASH REGION 2 \*\*\*\*\*  
 \*\*\*\*\*

\*\*\*\*\*  
 \*\*\*\*\* BEGINNING OF IONA WASH REGION 3 \*\*\*\*\*  
 \*\*\*\*\*

475 KK IW353  
 476 KO 0 0 0.0 1 22  
 477 BA 2.8030  
 478 LG 0.35 0.15 7.596 0.098 5.084  
 479 UC 1.004 0.717  
 \* Natural Time-Area Relation  
 480 UA 0 3 5 8 12 20 43 75 90 96  
 481 UA 100

\* Combines IW353, RIW357, and RIW359

482 KK CIW353  
 483 KO 0 0 0.0 0 22  
 \* Hard Coded  
 484 HC 3 15.08

\* ROUTE UPDATED 4/14/4

HEC-1 INPUT

PAGE 16

1 LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

485 KK RIW353  
 486 KO 0 0 0.0 0 22  
 487 RS 2 FLOW 0.0 0.0  
 488 RC 0.04 0.04 0.04 7496 0.0053  
 \* IWTS40\_1  
 489 RX 0 147 714 840 930 1073 1087 1417  
 490 RY 1708 1704 1700 1696 1696 1700 1704 1708

491 KK IW350  
 492 KO 0 0 0.0 1 22  
 493 BA 2.1750  
 494 LG 0.35 0.171 6.789 0.135 5.0  
 495 UC 1.363 1.203  
 \* Natural Time-Area Relation  
 496 UA 0 3 5 8 12 20 43 75 90 96  
 497 UA 100

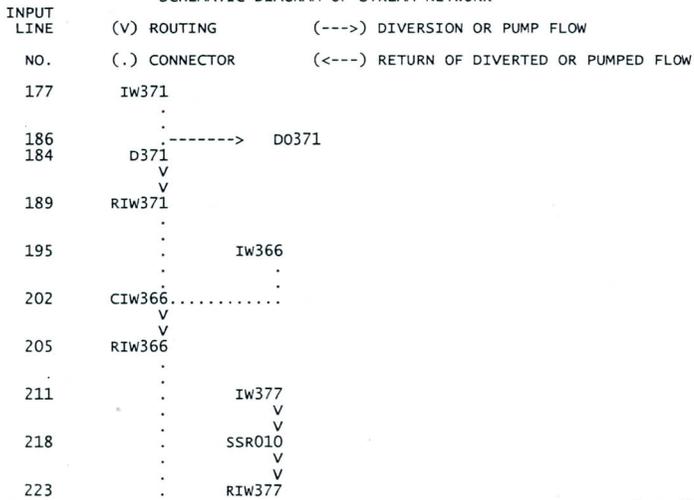
\* Combines IW350 and RIW353

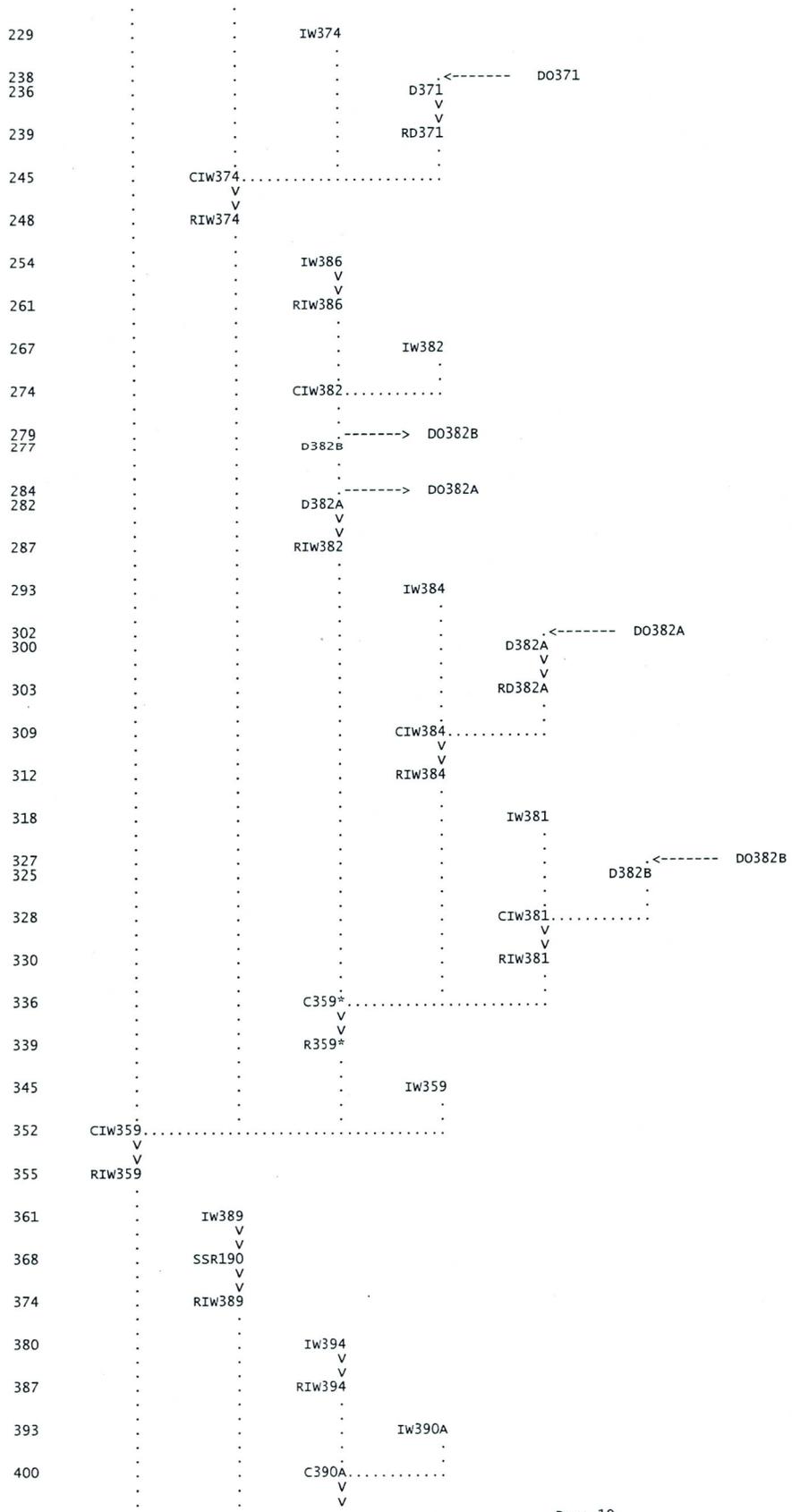
498 KK CIW350  
 499 KO 0 0 0.0 0 22  
 500 HC 2

\*\*\*\*\*  
 \*\*\*\*\* END OF IONA WASH REGION 3 \*\*\*\*\*  
 \*\*\*\*\*

501 ZZ

SCHEMATIC DIAGRAM OF STREAM NETWORK





```

403 . . . . . R390A
. . . . .
409 . . . . . IW390
. . . . .
416 . . . . . IW392
. . . . .
423 . . . . . CIW390.....
. . . . . V
. . . . . V
426 . . . . . SSR160
. . . . . V
. . . . . V
431 . . . . . RIW390
. . . . .
437 . . . . . IW388
. . . . .
444 . . . . . CIW388.....
. . . . . V
. . . . . V
447 . . . . . RIW388
. . . . .
453 . . . . . IW357
. . . . .
460 . . . . . CIW357.....
. . . . .
464 . . . . . -----> D0357
463 . . . . . D357
. . . . . V
. . . . . V
469 . . . . . RIW357
. . . . .
475 . . . . . IW353
. . . . .
482 . . . . . CIW353.....
. . . . . V
. . . . . V
485 . . . . . RIW353
. . . . .
491 . . . . . IW350
. . . . .
498 . . . . . CIW350.....

```

(\*\*\* RUNOFF ALSO COMPUTED AT THIS LOCATION

```

*****
* FLOOD HYDROGRAPH PACKAGE (HEC-1) *
* JUN 1998 *
* VERSION 4.1 *
* RUN DATE 03AUG11 TIME 17:14:59 *
*****

```

```

*****
* U.S. ARMY CORPS OF ENGINEERS *
* HYDROLOGIC ENGINEERING CENTER *
* 609 SECOND STREET *
* DAVIS, CALIFORNIA 95616 *
* (916) 756-1104 *
*****

```

```

*****
**--ADDENDUM--ADDENDUM--ADDENDUM--ADDENDUM--ADDENDUM--ADDENDUM--ADDENDUM--ADDENDUM--**
**--ADDENDUM--ADDENDUM--ADDENDUM--ADDENDUM--ADDENDUM--ADDENDUM--ADDENDUM--ADDENDUM--**
*****
**
** ADDENDUM TO THE WITTMANN AREA DRAINAGE MASTER STUDY UPDATE HYDROLOGY **
**
*****
**--ADDENDUM--ADDENDUM--ADDENDUM--ADDENDUM--ADDENDUM--ADDENDUM--ADDENDUM--ADDENDUM--**
**--ADDENDUM--ADDENDUM--ADDENDUM--ADDENDUM--ADDENDUM--ADDENDUM--ADDENDUM--ADDENDUM--**
*****
**
** THE FOLLOWING CHANGES HAVE BEEN MADE TO THE WITTMANN HYDROLOGY MODELS **
** DATED OCTOBER 2004: **
**
** -Diversions added after CTW480 **
** -Main flow (5%) to CTW478 **
** -Diverted flow (95%) to CWI584 **
** -Added concentration point CWI584 combining RD480 and WI584 **
** -Hard coding was added to CTW478 **
** -Route added after D480 (Diversions Recovery) **
** -Route RWI584 was adjusted **
** -Storage Route SCP050 was adjusted **
** -Route R508* was adjusted **
** -Hard coding for CWI510 was adjusted **
**
** -Diversions D618 was moved from after CPI618 to before RDPC12 **
** -Updated route RD618 information **
** -Hard coding on CPI618 was adjusted **
** -Hard coding on CPI615 was adjusted **
**
** -Route RPI639 changed from following US60 to CPI636 to route **
** through US60/RR structures and combine with CWI506 **
** -Cards PI639, D524, RD524, CPI639 and RPI639 were changed from **
** following R624A to follow RWI524 **
** -CPI636 was removed-no longer needed to combine PI636 & RPI639 **
** -RPI639 information was changed to reflect new route **
**
Page 11

```











\*\*\*\*\*  
\*  
195 KK \* Iw366 \*  
\*  
\*\*\*\*\*

196 KO OUTPUT CONTROL VARIABLES  
IPRNT 5 PRINT CONTROL  
IPLOT 0 PLOT CONTROL  
QSCAL 0. HYDROGRAPH PLOT SCALE  
IPNCH 1 PUNCH COMPUTED HYDROGRAPH  
IOUT 22 SAVE HYDROGRAPH ON THIS UNIT  
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
ISAV2 1500 LAST ORDINATE PUNCHED OR SAVED  
TIMINT 0.083 TIME INTERVAL IN HOURS

\*\*\* \*\*

\*\*\*\*\*  
\*  
202 KK \* CIW366 \*  
\*  
\*\*\*\*\*

203 KO OUTPUT CONTROL VARIABLES  
IPRNT 5 PRINT CONTROL  
IPLOT 0 PLOT CONTROL  
QSCAL 0. HYDROGRAPH PLOT SCALE  
IPNCH 0 PUNCH COMPUTED HYDROGRAPH  
IOUT 22 SAVE HYDROGRAPH ON THIS UNIT  
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
ISAV2 1500 LAST ORDINATE PUNCHED OR SAVED  
TIMINT 0.083 TIME INTERVAL IN HOURS

\*\*\* \*\*

\*\*\*\*\*  
\*  
205 KK \* RIW366 \*  
\*  
\*\*\*\*\*

206 KO OUTPUT CONTROL VARIABLES  
IPRNT 5 PRINT CONTROL  
IPLOT 0 PLOT CONTROL  
QSCAL 0. HYDROGRAPH PLOT SCALE  
IPNCH 0 PUNCH COMPUTED HYDROGRAPH  
IOUT 22 SAVE HYDROGRAPH ON THIS UNIT  
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
ISAV2 1500 LAST ORDINATE PUNCHED OR SAVED  
TIMINT 0.083 TIME INTERVAL IN HOURS

\*\*\* \*\*

\*\*\*\*\*  
\*  
211 KK \* Iw377 \*  
\*  
\*\*\*\*\*

212 KO OUTPUT CONTROL VARIABLES  
IPRNT 5 PRINT CONTROL  
IPLOT 0 PLOT CONTROL  
QSCAL 0. HYDROGRAPH PLOT SCALE  
IPNCH 1 PUNCH COMPUTED HYDROGRAPH  
IOUT 22 SAVE HYDROGRAPH ON THIS UNIT  
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
ISAV2 1500 LAST ORDINATE PUNCHED OR SAVED  
TIMINT 0.083 TIME INTERVAL IN HOURS

\*\*\* \*\*

\*\*\*\*\*  
\*  
223 KK \* RIW377 \*  
\*  
\*\*\*\*\*

224 KO OUTPUT CONTROL VARIABLES  
IPRNT 5 PRINT CONTROL  
IPLOT 0 PLOT CONTROL  
QSCAL 0. HYDROGRAPH PLOT SCALE  
IPNCH 0 PUNCH COMPUTED HYDROGRAPH  
IOUT 22 SAVE HYDROGRAPH ON THIS UNIT  
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
ISAV2 1500 LAST ORDINATE PUNCHED OR SAVED  
TIMINT 0.083 TIME INTERVAL IN HOURS

\*\*\* \*\*

\*\*\*\*\*  
\*  
\* Iw374 \*  
\*  
\*\*\*\*\*

230 KO        OUTPUT CONTROL VARIABLES  
          IPRNT        5    PRINT CONTROL  
          IPLOT        0    PLOT CONTROL  
          QSCAL        0.    HYDROGRAPH PLOT SCALE  
          IPNCH        1    PUNCH COMPUTED HYDROGRAPH  
          IOUT        22    SAVE HYDROGRAPH ON THIS UNIT  
          ISAV1        1    FIRST ORDINATE PUNCHED OR SAVED  
          ISAV2        1500    LAST ORDINATE PUNCHED OR SAVED  
          TIMINT        0.083    TIME INTERVAL IN HOURS

\*\*\* \*\*

\*\*\*\*\*  
\*  
\* D371 \*  
\*  
\*\*\*\*\*

237 KO        OUTPUT CONTROL VARIABLES  
          IPRNT        5    PRINT CONTROL  
          IPLOT        0    PLOT CONTROL  
          QSCAL        0.    HYDROGRAPH PLOT SCALE  
          IPNCH        0    PUNCH COMPUTED HYDROGRAPH  
          IOUT        22    SAVE HYDROGRAPH ON THIS UNIT  
          ISAV1        1    FIRST ORDINATE PUNCHED OR SAVED  
          ISAV2        1500    LAST ORDINATE PUNCHED OR SAVED  
          TIMINT        0.083    TIME INTERVAL IN HOURS

\*\*\* \*\*

\*\*\*\*\*  
\*  
\* RD371 \*  
\*  
\*\*\*\*\*

240 KO        OUTPUT CONTROL VARIABLES  
          IPRNT        5    PRINT CONTROL  
          IPLOT        0    PLOT CONTROL  
          QSCAL        0.    HYDROGRAPH PLOT SCALE  
          IPNCH        0    PUNCH COMPUTED HYDROGRAPH  
          IOUT        22    SAVE HYDROGRAPH ON THIS UNIT  
          ISAV1        1    FIRST ORDINATE PUNCHED OR SAVED  
          ISAV2        1500    LAST ORDINATE PUNCHED OR SAVED  
          TIMINT        0.083    TIME INTERVAL IN HOURS

\*\*\* \*\*

\*\*\*\*\*  
\*  
\* CIw374 \*  
\*  
\*\*\*\*\*

246 KO        OUTPUT CONTROL VARIABLES  
          IPRNT        5    PRINT CONTROL  
          IPLOT        0    PLOT CONTROL  
          QSCAL        0.    HYDROGRAPH PLOT SCALE  
          IPNCH        0    PUNCH COMPUTED HYDROGRAPH  
          IOUT        22    SAVE HYDROGRAPH ON THIS UNIT  
          ISAV1        1    FIRST ORDINATE PUNCHED OR SAVED  
          ISAV2        1500    LAST ORDINATE PUNCHED OR SAVED  
          TIMINT        0.083    TIME INTERVAL IN HOURS

\*\*\* \*\*

\*\*\*\*\*  
\*  
\* RIw374 \*  
\*  
\*\*\*\*\*

249 KO        OUTPUT CONTROL VARIABLES  
          IPRNT        5    PRINT CONTROL  
          IPLOT        0    PLOT CONTROL  
          QSCAL        0.    HYDROGRAPH PLOT SCALE  
          IPNCH        0    PUNCH COMPUTED HYDROGRAPH  
          IOUT        22    SAVE HYDROGRAPH ON THIS UNIT  
          ISAV1        1    FIRST ORDINATE PUNCHED OR SAVED  
          ISAV2        1500    LAST ORDINATE PUNCHED OR SAVED

TIMINT 0.083 TIME INTERVAL IN HOURS wtec24-addendum-mod1.out

\*\*\* \*\*

\*\*\*\*\*  
\*  
254 KK \* IW386 \*  
\*  
\*\*\*\*\*

255 KO OUTPUT CONTROL VARIABLES  
IPRNT 5 PRINT CONTROL  
IPLOT 0 PLOT CONTROL  
QSCAL 0. HYDROGRAPH PLOT SCALE  
IPNCH 1 PUNCH COMPUTED HYDROGRAPH  
IOUT 22 SAVE HYDROGRAPH ON THIS UNIT  
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
ISAV2 1500 LAST ORDINATE PUNCHED OR SAVED  
TIMINT 0.083 TIME INTERVAL IN HOURS

\*\*\* \*\*

\*\*\*\*\*  
\*  
261 KK \* RIW386 \*  
\*  
\*\*\*\*\*

262 KO OUTPUT CONTROL VARIABLES  
IPRNT 5 PRINT CONTROL  
IPLOT 0 PLOT CONTROL  
QSCAL 0. HYDROGRAPH PLOT SCALE  
IPNCH 0 PUNCH COMPUTED HYDROGRAPH  
IOUT 22 SAVE HYDROGRAPH ON THIS UNIT  
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
ISAV2 1500 LAST ORDINATE PUNCHED OR SAVED  
TIMINT 0.083 TIME INTERVAL IN HOURS

\*\*\* \*\*

\*\*\*\*\*  
\*  
267 KK \* IW382 \*  
\*  
\*\*\*\*\*

268 KO OUTPUT CONTROL VARIABLES  
IPRNT 5 PRINT CONTROL  
IPLOT 0 PLOT CONTROL  
QSCAL 0. HYDROGRAPH PLOT SCALE  
IPNCH 1 PUNCH COMPUTED HYDROGRAPH  
IOUT 22 SAVE HYDROGRAPH ON THIS UNIT  
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
ISAV2 1500 LAST ORDINATE PUNCHED OR SAVED  
TIMINT 0.083 TIME INTERVAL IN HOURS

\*\*\* \*\*

\*\*\*\*\*  
\*  
274 KK \* CIW382 \*  
\*  
\*\*\*\*\*

275 KO OUTPUT CONTROL VARIABLES  
IPRNT 5 PRINT CONTROL  
IPLOT 0 PLOT CONTROL  
QSCAL 0. HYDROGRAPH PLOT SCALE  
IPNCH 0 PUNCH COMPUTED HYDROGRAPH  
IOUT 22 SAVE HYDROGRAPH ON THIS UNIT  
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
ISAV2 1500 LAST ORDINATE PUNCHED OR SAVED  
TIMINT 0.083 TIME INTERVAL IN HOURS

\*\*\* \*\*

\*\*\*\*\*  
\*  
277 KK \* D382B \*  
\*  
\*\*\*\*\*

278 KO OUTPUT CONTROL VARIABLES  
IPRNT 5 PRINT CONTROL  
IPLOT 0 PLOT CONTROL  
QSCAL 0. HYDROGRAPH PLOT SCALE

```

IPNCH      0 PUNCH COMPUTED HYDROGRAPH
IOUT      22 SAVE HYDROGRAPH ON THIS UNIT
ISAV1     1 FIRST ORDINATE PUNCHED OR SAVED
ISAV2    1500 LAST ORDINATE PUNCHED OR SAVED
TIMINT    0.083 TIME INTERVAL IN HOURS

```

\*\*\* \*\*

```

*****
*
282 KK * D382A *
*
*****

```

```

283 KO OUTPUT CONTROL VARIABLES
      IPRNT      5 PRINT CONTROL
      IPLOT      0 PLOT CONTROL
      QSCAL      0 HYDROGRAPH PLOT SCALE
      IPNCH      0 PUNCH COMPUTED HYDROGRAPH
      IOUT      22 SAVE HYDROGRAPH ON THIS UNIT
      ISAV1     1 FIRST ORDINATE PUNCHED OR SAVED
      ISAV2    1500 LAST ORDINATE PUNCHED OR SAVED
      TIMINT    0.083 TIME INTERVAL IN HOURS

```

\*\*\* \*\*

```

*****
*
287 KK * RIW382 *
*
*****

```

```

288 KO OUTPUT CONTROL VARIABLES
      IPRNT      5 PRINT CONTROL
      IPLOT      0 PLOT CONTROL
      QSCAL      0 HYDROGRAPH PLOT SCALE
      IPNCH      0 PUNCH COMPUTED HYDROGRAPH
      IOUT      22 SAVE HYDROGRAPH ON THIS UNIT
      ISAV1     1 FIRST ORDINATE PUNCHED OR SAVED
      ISAV2    1500 LAST ORDINATE PUNCHED OR SAVED
      TIMINT    0.083 TIME INTERVAL IN HOURS

```

\*\*\* \*\*

```

*****
*
293 KK * IW384 *
*
*****

```

```

294 KO OUTPUT CONTROL VARIABLES
      IPRNT      5 PRINT CONTROL
      IPLOT      0 PLOT CONTROL
      QSCAL      0 HYDROGRAPH PLOT SCALE
      IPNCH      1 PUNCH COMPUTED HYDROGRAPH
      IOUT      22 SAVE HYDROGRAPH ON THIS UNIT
      ISAV1     1 FIRST ORDINATE PUNCHED OR SAVED
      ISAV2    1500 LAST ORDINATE PUNCHED OR SAVED
      TIMINT    0.083 TIME INTERVAL IN HOURS

```

\*\*\* \*\*

```

*****
*
300 KK * D382A *
*
*****

```

```

301 KO OUTPUT CONTROL VARIABLES
      IPRNT      5 PRINT CONTROL
      IPLOT      0 PLOT CONTROL
      QSCAL      0 HYDROGRAPH PLOT SCALE
      IPNCH      0 PUNCH COMPUTED HYDROGRAPH
      IOUT      22 SAVE HYDROGRAPH ON THIS UNIT
      ISAV1     1 FIRST ORDINATE PUNCHED OR SAVED
      ISAV2    1500 LAST ORDINATE PUNCHED OR SAVED
      TIMINT    0.083 TIME INTERVAL IN HOURS

```

\*\*\* \*\*

```

*****
*
303 KK * RD382A *
*
*****

```

```

304 KO      OUTPUT CONTROL VARIABLES
            IPRNT      5  PRINT CONTROL
            IPLOT      0  PLOT CONTROL
            QSCAL      0. HYDROGRAPH PLOT SCALE
            IPNCH      0  PUNCH COMPUTED HYDROGRAPH
            IOUT       22  SAVE HYDROGRAPH ON THIS UNIT
            ISAV1      1  FIRST ORDINATE PUNCHED OR SAVED
            ISAV2     1500 LAST ORDINATE PUNCHED OR SAVED
            TIMINT     0.083 TIME INTERVAL IN HOURS

```

\*\*\* \*\*

```

*****
*
* CIW384
*
*****

```

```

310 KO      OUTPUT CONTROL VARIABLES
            IPRNT      5  PRINT CONTROL
            IPLOT      0  PLOT CONTROL
            QSCAL      0. HYDROGRAPH PLOT SCALE
            IPNCH      0  PUNCH COMPUTED HYDROGRAPH
            IOUT       22  SAVE HYDROGRAPH ON THIS UNIT
            ISAV1      1  FIRST ORDINATE PUNCHED OR SAVED
            ISAV2     1500 LAST ORDINATE PUNCHED OR SAVED
            TIMINT     0.083 TIME INTERVAL IN HOURS

```

\*\*\* \*\*

```

*****
*
* RIW384
*
*****

```

```

313 KO      OUTPUT CONTROL VARIABLES
            IPRNT      5  PRINT CONTROL
            IPLOT      0  PLOT CONTROL
            QSCAL      0. HYDROGRAPH PLOT SCALE
            IPNCH      0  PUNCH COMPUTED HYDROGRAPH
            IOUT       22  SAVE HYDROGRAPH ON THIS UNIT
            ISAV1      1  FIRST ORDINATE PUNCHED OR SAVED
            ISAV2     1500 LAST ORDINATE PUNCHED OR SAVED
            TIMINT     0.083 TIME INTERVAL IN HOURS

```

\*\*\* \*\*

```

*****
*
* IW381
*
*****

```

```

319 KO      OUTPUT CONTROL VARIABLES
            IPRNT      5  PRINT CONTROL
            IPLOT      0  PLOT CONTROL
            QSCAL      0. HYDROGRAPH PLOT SCALE
            IPNCH      1  PUNCH COMPUTED HYDROGRAPH
            IOUT       22  SAVE HYDROGRAPH ON THIS UNIT
            ISAV1      1  FIRST ORDINATE PUNCHED OR SAVED
            ISAV2     1500 LAST ORDINATE PUNCHED OR SAVED
            TIMINT     0.083 TIME INTERVAL IN HOURS

```

\*\*\* \*\*

```

*****
*
* D382B
*
*****

```

```

326 KO      OUTPUT CONTROL VARIABLES
            IPRNT      5  PRINT CONTROL
            IPLOT      0  PLOT CONTROL
            QSCAL      0. HYDROGRAPH PLOT SCALE
            IPNCH      0  PUNCH COMPUTED HYDROGRAPH
            IOUT       22  SAVE HYDROGRAPH ON THIS UNIT
            ISAV1      1  FIRST ORDINATE PUNCHED OR SAVED
            ISAV2     1500 LAST ORDINATE PUNCHED OR SAVED
            TIMINT     0.083 TIME INTERVAL IN HOURS

```

\*\*\* \*\*

```

*****
*

```

330 KK \* RIW381 \*  
\* \*  
\*\*\*\*\*

331 KO OUTPUT CONTROL VARIABLES  
IPRNT 5 PRINT CONTROL  
IPLOT 0 PLOT CONTROL  
QSCAL 0. HYDROGRAPH PLOT SCALE  
IPNCH 0 PUNCH COMPUTED HYDROGRAPH  
IOUT 22 SAVE HYDROGRAPH ON THIS UNIT  
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
ISAV2 1500 LAST ORDINATE PUNCHED OR SAVED  
TIMINT 0.083 TIME INTERVAL IN HOURS

\*\*\* \*\*

\*\*\*\*\*  
\* \*  
336 KK \* C359\* \*  
\* \*  
\*\*\*\*\*

337 KO OUTPUT CONTROL VARIABLES  
IPRNT 5 PRINT CONTROL  
IPLOT 0 PLOT CONTROL  
QSCAL 0. HYDROGRAPH PLOT SCALE  
IPNCH 0 PUNCH COMPUTED HYDROGRAPH  
IOUT 22 SAVE HYDROGRAPH ON THIS UNIT  
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
ISAV2 1500 LAST ORDINATE PUNCHED OR SAVED  
TIMINT 0.083 TIME INTERVAL IN HOURS

\*\*\* \*\*

\*\*\*\*\*  
\* \*  
339 KK \* R359\* \*  
\* \*  
\*\*\*\*\*

340 KO OUTPUT CONTROL VARIABLES  
IPRNT 5 PRINT CONTROL  
IPLOT 0 PLOT CONTROL  
QSCAL 0. HYDROGRAPH PLOT SCALE  
IPNCH 0 PUNCH COMPUTED HYDROGRAPH  
IOUT 22 SAVE HYDROGRAPH ON THIS UNIT  
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
ISAV2 1500 LAST ORDINATE PUNCHED OR SAVED  
TIMINT 0.083 TIME INTERVAL IN HOURS

\*\*\* \*\*

\*\*\*\*\*  
\* \*  
345 KK \* IW359 \*  
\* \*  
\*\*\*\*\*

346 KO OUTPUT CONTROL VARIABLES  
IPRNT 5 PRINT CONTROL  
IPLOT 0 PLOT CONTROL  
QSCAL 0. HYDROGRAPH PLOT SCALE  
IPNCH 1 PUNCH COMPUTED HYDROGRAPH  
IOUT 22 SAVE HYDROGRAPH ON THIS UNIT  
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
ISAV2 1500 LAST ORDINATE PUNCHED OR SAVED  
TIMINT 0.083 TIME INTERVAL IN HOURS

\*\*\* \*\*

\*\*\*\*\*  
\* \*  
352 KK \* CIW359 \*  
\* \*  
\*\*\*\*\*

353 KO OUTPUT CONTROL VARIABLES  
IPRNT 5 PRINT CONTROL  
IPLOT 0 PLOT CONTROL  
QSCAL 0. HYDROGRAPH PLOT SCALE  
IPNCH 0 PUNCH COMPUTED HYDROGRAPH  
IOUT 22 SAVE HYDROGRAPH ON THIS UNIT  
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
ISAV2 1500 LAST ORDINATE PUNCHED OR SAVED  
TIMINT 0.083 TIME INTERVAL IN HOURS

\*\*\* \*\*

\*\*\*\*\*  
\*  
355 KK \* RIW359 \*  
\*  
\*\*\*\*\*

356 KO OUTPUT CONTROL VARIABLES  
IPRNT 5 PRINT CONTROL  
IPLOT 0 PLOT CONTROL  
QSCAL 0. HYDROGRAPH PLOT SCALE  
IPNCH 0 PUNCH COMPUTED HYDROGRAPH  
IOUT 22 SAVE HYDROGRAPH ON THIS UNIT  
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
ISAV2 1500 LAST ORDINATE PUNCHED OR SAVED  
TIMINT 0.083 TIME INTERVAL IN HOURS

\*\*\* \*\*

\*\*\*\*\*  
\*  
361 KK \* IW389 \*  
\*  
\*\*\*\*\*

362 KO OUTPUT CONTROL VARIABLES  
IPRNT 5 PRINT CONTROL  
IPLOT 0 PLOT CONTROL  
QSCAL 0. HYDROGRAPH PLOT SCALE  
IPNCH 1 PUNCH COMPUTED HYDROGRAPH  
IOUT 22 SAVE HYDROGRAPH ON THIS UNIT  
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
ISAV2 1500 LAST ORDINATE PUNCHED OR SAVED  
TIMINT 0.083 TIME INTERVAL IN HOURS

WARNING EXCESS AT PONDING LESS THAN ZERO FOR PERIOD. EXCESS SET TO ZERO

\*\*\* \*\*

\*\*\*\*\*  
\*  
374 KK \* RIW389 \*  
\*  
\*\*\*\*\*

375 KO OUTPUT CONTROL VARIABLES  
IPRNT 5 PRINT CONTROL  
IPLOT 0 PLOT CONTROL  
QSCAL 0. HYDROGRAPH PLOT SCALE  
IPNCH 0 PUNCH COMPUTED HYDROGRAPH  
IOUT 22 SAVE HYDROGRAPH ON THIS UNIT  
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
ISAV2 1500 LAST ORDINATE PUNCHED OR SAVED  
TIMINT 0.083 TIME INTERVAL IN HOURS

\*\*\* \*\*

\*\*\*\*\*  
\*  
380 KK \* IW394 \*  
\*  
\*\*\*\*\*

381 KO OUTPUT CONTROL VARIABLES  
IPRNT 5 PRINT CONTROL  
IPLOT 0 PLOT CONTROL  
QSCAL 0. HYDROGRAPH PLOT SCALE  
IPNCH 1 PUNCH COMPUTED HYDROGRAPH  
IOUT 22 SAVE HYDROGRAPH ON THIS UNIT  
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
ISAV2 1500 LAST ORDINATE PUNCHED OR SAVED  
TIMINT 0.083 TIME INTERVAL IN HOURS

\*\*\* \*\*

\*\*\*\*\*  
\*  
387 KK \* RIW394 \*  
\*  
\*\*\*\*\*

388 KO OUTPUT CONTROL VARIABLES  
IPRNT 5 PRINT CONTROL  
IPLOT 0 PLOT CONTROL  
QSCAL 0. HYDROGRAPH PLOT SCALE  
IPNCH 0 PUNCH COMPUTED HYDROGRAPH  
IOUT 22 SAVE HYDROGRAPH ON THIS UNIT  
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
ISAV2 1500 LAST ORDINATE PUNCHED OR SAVED  
TIMINT 0.083 TIME INTERVAL IN HOURS

\*\*\* \*\* \*\* \*\* \*\*

\*\*\*\*\*  
\* \*  
\* Iw390A \*  
\* \*  
\*\*\*\*\*

394 KO        OUTPUT CONTROL VARIABLES  
          IPRNT        5    PRINT CONTROL  
          IPLOT        0    PLOT CONTROL  
          QSCAL        0.    HYDROGRAPH PLOT SCALE  
          IPNCH        1    PUNCH COMPUTED HYDROGRAPH  
          IOUT         22    SAVE HYDROGRAPH ON THIS UNIT  
          ISAV1        1    FIRST ORDINATE PUNCHED OR SAVED  
          ISAV2        1500 LAST ORDINATE PUNCHED OR SAVED  
          TIMINT       0.083 TIME INTERVAL IN HOURS

\*\*\* \*\* \*\* \*\* \*\*

\*\*\*\*\*  
\* \*  
\* C390A \*  
\* \*  
\*\*\*\*\*

401 KO        OUTPUT CONTROL VARIABLES  
          IPRNT        5    PRINT CONTROL  
          IPLOT        0    PLOT CONTROL  
          QSCAL        0.    HYDROGRAPH PLOT SCALE  
          IPNCH        0    PUNCH COMPUTED HYDROGRAPH  
          IOUT         22    SAVE HYDROGRAPH ON THIS UNIT  
          ISAV1        1    FIRST ORDINATE PUNCHED OR SAVED  
          ISAV2        1500 LAST ORDINATE PUNCHED OR SAVED  
          TIMINT       0.083 TIME INTERVAL IN HOURS

\*\*\* \*\* \*\* \*\*~

\*\*\*\*\*  
\* \*  
\* R390A \*  
\* \*  
\*\*\*\*\*

404 KO        OUTPUT CONTROL VARIABLES  
          IPRNT        5    PRINT CONTROL  
          IPLOT        0    PLOT CONTROL  
          QSCAL        0.    HYDROGRAPH PLOT SCALE  
          IPNCH        0    PUNCH COMPUTED HYDROGRAPH  
          IOUT         22    SAVE HYDROGRAPH ON THIS UNIT  
          ISAV1        1    FIRST ORDINATE PUNCHED OR SAVED  
          ISAV2        1500 LAST ORDINATE PUNCHED OR SAVED  
          TIMINT       0.083 TIME INTERVAL IN HOURS

\*\*\* \*\* \*\* \*\*~

\*\*\*\*\*  
\* \*  
\* Iw390 \*  
\* \*  
\*\*\*\*\*

410 KO        OUTPUT CONTROL VARIABLES  
          IPRNT        5    PRINT CONTROL  
          IPLOT        0    PLOT CONTROL  
          QSCAL        0.    HYDROGRAPH PLOT SCALE  
          IPNCH        1    PUNCH COMPUTED HYDROGRAPH  
          IOUT         22    SAVE HYDROGRAPH ON THIS UNIT  
          ISAV1        1    FIRST ORDINATE PUNCHED OR SAVED  
          ISAV2        1500 LAST ORDINATE PUNCHED OR SAVED  
          TIMINT       0.083 TIME INTERVAL IN HOURS

\*\*\* \*\* \*\* \*\*~

\*\*\*\*\*  
\* \*  
\* Iw392 \*  
\* \*  
\*\*\*\*\*

417 KO        OUTPUT CONTROL VARIABLES  
          IPRNT        5    PRINT CONTROL  
          IPLOT        0    PLOT CONTROL  
          QSCAL        0.    HYDROGRAPH PLOT SCALE  
          IPNCH        1    PUNCH COMPUTED HYDROGRAPH

IOUT	22	SAVE HYDROGRAPH ON THIS UNIT
ISAV1	1	FIRST ORDINATE PUNCHED OR SAVED
ISAV2	1500	LAST ORDINATE PUNCHED OR SAVED
TIMINT	0.083	TIME INTERVAL IN HOURS

\*\*\* \*\*

```
*****
*
* CIW390 *
*
*****
```

```
424 KO      OUTPUT CONTROL VARIABLES
            IPRNT      5  PRINT CONTROL
            IPLOT      0  PLOT CONTROL
            QSCAL      0. HYDROGRAPH PLOT SCALE
            IPNCH      0  PUNCH COMPUTED HYDROGRAPH
            IOUT       22  SAVE HYDROGRAPH ON THIS UNIT
            ISAV1      1  FIRST ORDINATE PUNCHED OR SAVED
            ISAV2     1500 LAST ORDINATE PUNCHED OR SAVED
            TIMINT     0.083 TIME INTERVAL IN HOURS
```

\*\*\* \*\*

```
*****
*
* RIW390 *
*
*****
```

```
432 KO      OUTPUT CONTROL VARIABLES
            IPRNT      5  PRINT CONTROL
            IPLOT      0  PLOT CONTROL
            QSCAL      0. HYDROGRAPH PLOT SCALE
            IPNCH      0  PUNCH COMPUTED HYDROGRAPH
            IOUT       22  SAVE HYDROGRAPH ON THIS UNIT
            ISAV1      1  FIRST ORDINATE PUNCHED OR SAVED
            ISAV2     1500 LAST ORDINATE PUNCHED OR SAVED
            TIMINT     0.083 TIME INTERVAL IN HOURS
```

\*\*\* \*\*

```
*****
*
* IW388 *
*
*****
```

```
438 KO      OUTPUT CONTROL VARIABLES
            IPRNT      5  PRINT CONTROL
            IPLOT      0  PLOT CONTROL
            QSCAL      0. HYDROGRAPH PLOT SCALE
            IPNCH      1  PUNCH COMPUTED HYDROGRAPH
            IOUT       22  SAVE HYDROGRAPH ON THIS UNIT
            ISAV1      1  FIRST ORDINATE PUNCHED OR SAVED
            ISAV2     1500 LAST ORDINATE PUNCHED OR SAVED
            TIMINT     0.083 TIME INTERVAL IN HOURS
```

\*\*\* \*\*

```
*****
*
* CIW388 *
*
*****
```

```
445 KO      OUTPUT CONTROL VARIABLES
            IPRNT      5  PRINT CONTROL
            IPLOT      0  PLOT CONTROL
            QSCAL      0. HYDROGRAPH PLOT SCALE
            IPNCH      0  PUNCH COMPUTED HYDROGRAPH
            IOUT       22  SAVE HYDROGRAPH ON THIS UNIT
            ISAV1      1  FIRST ORDINATE PUNCHED OR SAVED
            ISAV2     1500 LAST ORDINATE PUNCHED OR SAVED
            TIMINT     0.083 TIME INTERVAL IN HOURS
```

\*\*\* \*\*

```
*****
*
* RIW388 *
*
*****
```

448 KO OUTPUT CONTROL VARIABLES

```

IPRNT      5  PRINT CONTROL
IPLOT      0  PLOT CONTROL
QSCAL      0. HYDROGRAPH PLOT SCALE
IPNCH      0  PUNCH COMPUTED HYDROGRAPH
IOUT       22  SAVE HYDROGRAPH ON THIS UNIT
ISAV1      1  FIRST ORDINATE PUNCHED OR SAVED
ISAV2     1500 LAST ORDINATE PUNCHED OR SAVED
TIMINT     0.083 TIME INTERVAL IN HOURS

```

\*\*\* \*\*

```

*****
*
* IW357 *
*
*****

```

```

454 KO      OUTPUT CONTROL VARIABLES
IPRNT      5  PRINT CONTROL
IPLOT      0  PLOT CONTROL
QSCAL      0. HYDROGRAPH PLOT SCALE
IPNCH      1  PUNCH COMPUTED HYDROGRAPH
IOUT       22  SAVE HYDROGRAPH ON THIS UNIT
ISAV1      1  FIRST ORDINATE PUNCHED OR SAVED
ISAV2     1500 LAST ORDINATE PUNCHED OR SAVED
TIMINT     0.083 TIME INTERVAL IN HOURS

```

\*\*\* \*\*

```

*****
*
* CIW357 *
*
*****

```

```

461 KO      OUTPUT CONTROL VARIABLES
IPRNT      5  PRINT CONTROL
IPLOT      0  PLOT CONTROL
QSCAL      0. HYDROGRAPH PLOT SCALE
IPNCH      0  PUNCH COMPUTED HYDROGRAPH
IOUT       22  SAVE HYDROGRAPH ON THIS UNIT
ISAV1      1  FIRST ORDINATE PUNCHED OR SAVED
ISAV2     1500 LAST ORDINATE PUNCHED OR SAVED
TIMINT     0.083 TIME INTERVAL IN HOURS

```

\*\*\* \*\*

```

*****
*
* RIW357 *
*
*****

```

```

470 KO      OUTPUT CONTROL VARIABLES
IPRNT      5  PRINT CONTROL
IPLOT      0  PLOT CONTROL
QSCAL      0. HYDROGRAPH PLOT SCALE
IPNCH      0  PUNCH COMPUTED HYDROGRAPH
IOUT       22  SAVE HYDROGRAPH ON THIS UNIT
ISAV1      1  FIRST ORDINATE PUNCHED OR SAVED
ISAV2     1500 LAST ORDINATE PUNCHED OR SAVED
TIMINT     0.083 TIME INTERVAL IN HOURS

```

\*\*\* \*\*

```

*****
*
* IW353 *
*
*****

```

```

476 KO      OUTPUT CONTROL VARIABLES
IPRNT      5  PRINT CONTROL
IPLOT      0  PLOT CONTROL
QSCAL      0. HYDROGRAPH PLOT SCALE
IPNCH      1  PUNCH COMPUTED HYDROGRAPH
IOUT       22  SAVE HYDROGRAPH ON THIS UNIT
ISAV1      1  FIRST ORDINATE PUNCHED OR SAVED
ISAV2     1500 LAST ORDINATE PUNCHED OR SAVED
TIMINT     0.083 TIME INTERVAL IN HOURS

```

\*\*\* \*\*

```

*****
*
* CIW353 *

```

```

*
*****
483 KO      OUTPUT CONTROL VARIABLES
            IPRNT      5  PRINT CONTROL
            IPLOT      0  PLOT CONTROL
            QSCAL      0  HYDROGRAPH PLOT SCALE
            IPNCH      0  PUNCH COMPUTED HYDROGRAPH
            IOUT       22 SAVE HYDROGRAPH ON THIS UNIT
            ISAV1      1  FIRST ORDINATE PUNCHED OR SAVED
            ISAV2     1500 LAST ORDINATE PUNCHED OR SAVED
            TIMINT     0.083 TIME INTERVAL IN HOURS
    
```

\*\*\* \*\*

```

*****
*
*      RIW353
*
*****
    
```

```

486 KO      OUTPUT CONTROL VARIABLES
            IPRNT      5  PRINT CONTROL
            IPLOT      0  PLOT CONTROL
            QSCAL      0  HYDROGRAPH PLOT SCALE
            IPNCH      0  PUNCH COMPUTED HYDROGRAPH
            IOUT       22 SAVE HYDROGRAPH ON THIS UNIT
            ISAV1      1  FIRST ORDINATE PUNCHED OR SAVED
            ISAV2     1500 LAST ORDINATE PUNCHED OR SAVED
            TIMINT     0.083 TIME INTERVAL IN HOURS
    
```

\*\*\* \*\*

```

*****
*
*      IW350
*
*****
    
```

```

492 KO      OUTPUT CONTROL VARIABLES
            IPRNT      5  PRINT CONTROL
            IPLOT      0  PLOT CONTROL
            QSCAL      0  HYDROGRAPH PLOT SCALE
            IPNCH      1  PUNCH COMPUTED HYDROGRAPH
            IOUT       22 SAVE HYDROGRAPH ON THIS UNIT
            ISAV1      1  FIRST ORDINATE PUNCHED OR SAVED
            ISAV2     1500 LAST ORDINATE PUNCHED OR SAVED
            TIMINT     0.083 TIME INTERVAL IN HOURS
    
```

\*\*\* \*\*

```

*****
*
*      CIW350
*
*****
    
```

```

499 KO      OUTPUT CONTROL VARIABLES
            IPRNT      5  PRINT CONTROL
            IPLOT      0  PLOT CONTROL
            QSCAL      0  HYDROGRAPH PLOT SCALE
            IPNCH      0  PUNCH COMPUTED HYDROGRAPH
            IOUT       22 SAVE HYDROGRAPH ON THIS UNIT
            ISAV1      1  FIRST ORDINATE PUNCHED OR SAVED
            ISAV2     1500 LAST ORDINATE PUNCHED OR SAVED
            TIMINT     0.083 TIME INTERVAL IN HOURS
    
```

1

RUNOFF SUMMARY  
 FLOW IN CUBIC FEET PER SECOND  
 TIME IN HOURS, AREA IN SQUARE MILES

+	OPERATION	STATION	PEAK FLOW	TIME OF PEAK	AVERAGE FLOW FOR MAXIMUM PERIOD			BASIN AREA	MAXIMUM STAGE	TIME OF MAX STAGE
					6-HOUR	24-HOUR	72-HOUR			
+	HYDROGRAPH AT	IW371	68.	12.08	9.	3.	1.	0.05		
+	DIVERSION TO	D0371	46.	12.08	4.	1.	0.	0.05		
+	HYDROGRAPH AT	D371	22.	12.08	5.	2.	1.	0.05		
+	ROUTED TO	RIW371	13.	14.58	5.	2.	1.	0.05		
+	HYDROGRAPH AT	IW366	881.	12.58	162.	43.	14.	1.06		
+	2 COMBINED AT	CIW366	881.	12.58	167.	45.	15.	1.08		

+	ROUTED TO	RIW366	771.	13.00	166.	45.	15.	1.08
+	HYDROGRAPH AT	IW377	106.	12.33	17.	4.	1.	0.11
+	ROUTED TO	SSR010	99.	12.42	17.	4.	1.	0.11
+	ROUTED TO	RIW377	67.	13.25	17.	4.	1.	0.11
+	HYDROGRAPH AT	IW374	534.	12.33	79.	21.	7.	0.39
+	HYDROGRAPH AT	D371	46.	12.08	4.	1.	0.	0.05
+	ROUTED TO	RD371	26.	12.33	4.	1.	0.	0.05
+	3 COMBINED AT	CIW374	558.	12.33	98.	26.	9.	0.53
+	ROUTED TO	RIW374	279.	14.17	96.	26.	9.	0.53
+	HYDROGRAPH AT	IW386	271.	12.67	73.	19.	6.	0.37
+	ROUTED TO	RIW386	208.	13.75	72.	19.	6.	0.37
+	HYDROGRAPH AT	IW382	526.	12.33	100.	26.	9.	0.62
+	2 COMBINED AT	CIW382	525.	12.33	168.	45.	15.	0.99
+	DIVERSION TO	DO382B	180.	12.00	112.	31.	10.	0.99
+	HYDROGRAPH AT	D382B	345.	12.33	56.	14.	5.	0.99
+	DIVERSION TO	DO382A	82.	12.33	5.	1.	0.	0.99
+	HYDROGRAPH AT	D382A	263.	12.33	51.	13.	4.	0.99
+	ROUTED TO	RIW382	212.	12.83	51.	13.	4.	0.99
+	HYDROGRAPH AT	IW384	128.	12.25	18.	5.	2.	0.08
+	HYDROGRAPH AT	D382A	82.	12.33	5.	1.	0.	0.99
+	ROUTED TO	RD382A	60.	12.42	5.	1.	0.	0.99
+	2 COMBINED AT	CIW384	164.	12.33	23.	6.	2.	0.23
+	ROUTED TO	RIW384	120.	12.75	23.	6.	2.	0.23
+	HYDROGRAPH AT	IW381	315.	12.50	83.	22.	7.	0.38
+	HYDROGRAPH AT	D382B	180.	12.00	112.	31.	10.	0.99
+	2 COMBINED AT	CIW381	493.	12.50	194.	53.	18.	0.72
+	ROUTED TO	RIW381	460.	12.92	193.	53.	18.	0.72
+	3 COMBINED AT	C359*	778.	12.83	264.	71.	24.	1.45
+	ROUTED TO	R359*	719.	13.25	261.	71.	24.	1.45
+	HYDROGRAPH AT	IW359	2442.	12.67	444.	116.	39.	2.91
+	4 COMBINED AT	CIW359	3105.	12.92	939.	253.	85.	5.97
+	ROUTED TO	RIW359	2943.	13.25	928.	253.	85.	5.97
+	HYDROGRAPH AT	IW389	118.	12.50	32.	8.	3.	0.13
+	ROUTED TO	SSR190	118.	12.50	32.	8.	3.	0.13
+	ROUTED TO	RIW389	76.	13.92	31.	8.	3.	0.13
+	HYDROGRAPH AT							

					wtec24-addendum-mod1.out			
+		IW394	2569.	13.00	682.	192.	64.	4.17
+	ROUTED TO	RIW394	2473.	13.17	682.	192.	64.	4.17
+	HYDROGRAPH AT	IW390A	576.	12.75	156.	44.	15.	0.90
+	2' COMBINED AT	C390A	2903.	13.17	832.	235.	78.	5.07
+	ROUTED TO	R390A	2827.	13.42	832.	234.	78.	5.07
+	HYDROGRAPH AT	IW390	146.	12.75	45.	12.	4.	0.31
+	HYDROGRAPH AT	IW392	1713.	12.33	213.	56.	19.	1.11
+	3 COMBINED AT	CIW390	3038.	13.33	1076.	300.	100.	6.50
+	ROUTED TO	SSR160	2996.	13.42	1076.	300.	100.	6.50
+	ROUTED TO	RIW390	2891.	13.75	1070.	299.	100.	6.50
+	HYDROGRAPH AT	IW388	867.	12.92	259.	68.	23.	1.23
+	3 COMBINED AT	CIW388	3436.	13.75	1343.	373.	125.	7.85
+	ROUTED TO	RIW388	3364.	14.00	1341.	373.	125.	7.85
+	HYDROGRAPH AT	IW357	647.	12.50	151.	40.	14.	0.75
+	2 COMBINED AT	CIW357	3504.	14.00	1474.	411.	138.	8.60
+	DIVERSION TO	DO357	935.	14.00	387.	106.	36.	8.60
+	HYDROGRAPH AT	D357	2569.	14.00	1087.	305.	102.	8.60
+	ROUTED TO	RIW357	2494.	14.50	1080.	305.	102.	8.60
+	HYDROGRAPH AT	IW353	2307.	12.75	524.	135.	45.	2.80
+	3 COMBINED AT	CIW353	4986.	13.42	2430.	677.	227.	15.08
+	ROUTED TO	RIW353	4862.	13.75	2410.	677.	227.	15.08
+	HYDROGRAPH AT	IW350	1154.	13.00	368.	96.	32.	2.17
+	2 COMBINED AT	CIW350	5607.	13.67	2739.	765.	257.	17.25

\*\*\* NORMAL END OF HEC-1 \*\*\*

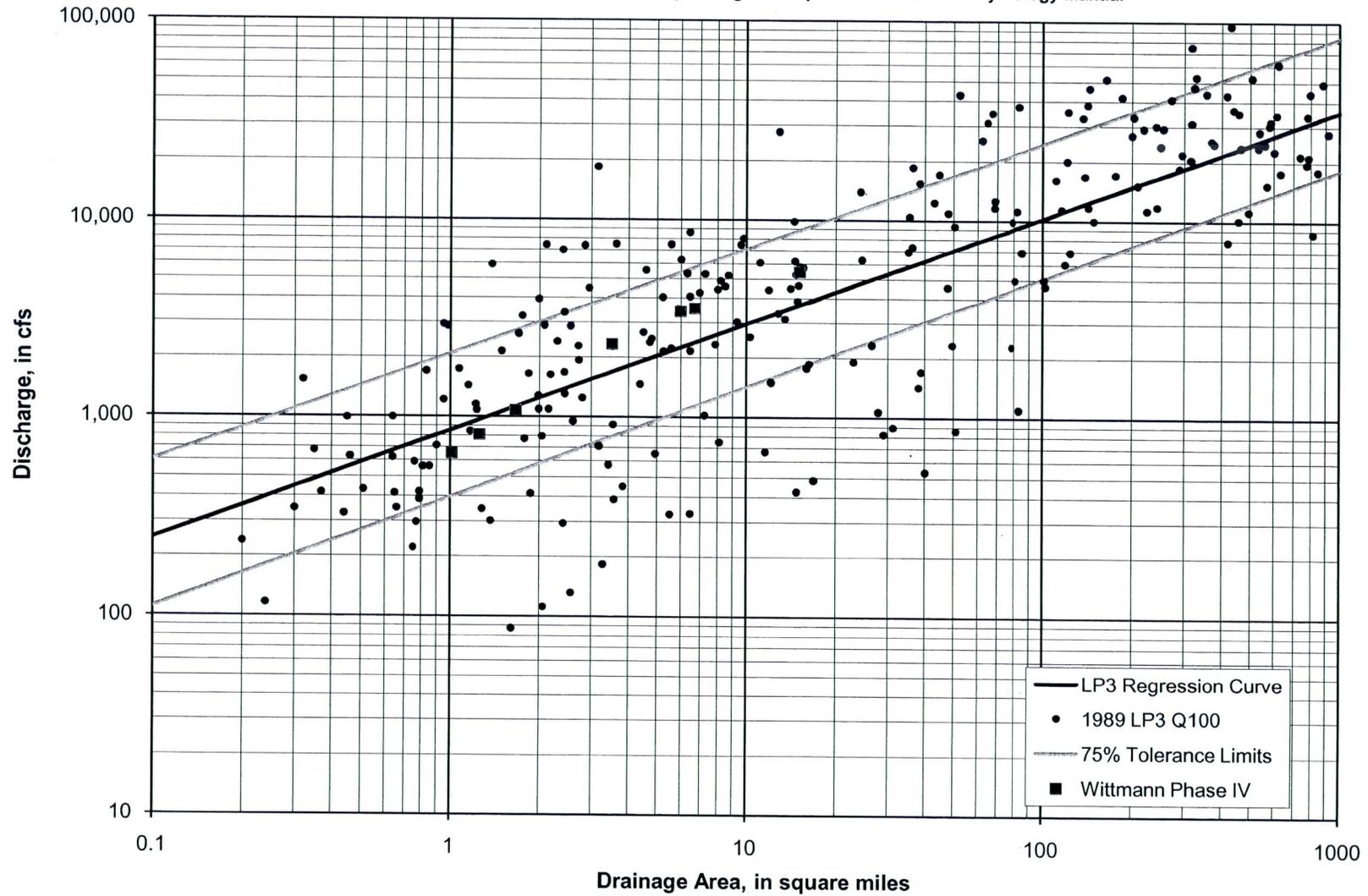
Wittmann Phase IV  
Floodplain Delineation Study

**Indirect Method Comparison Graphs**



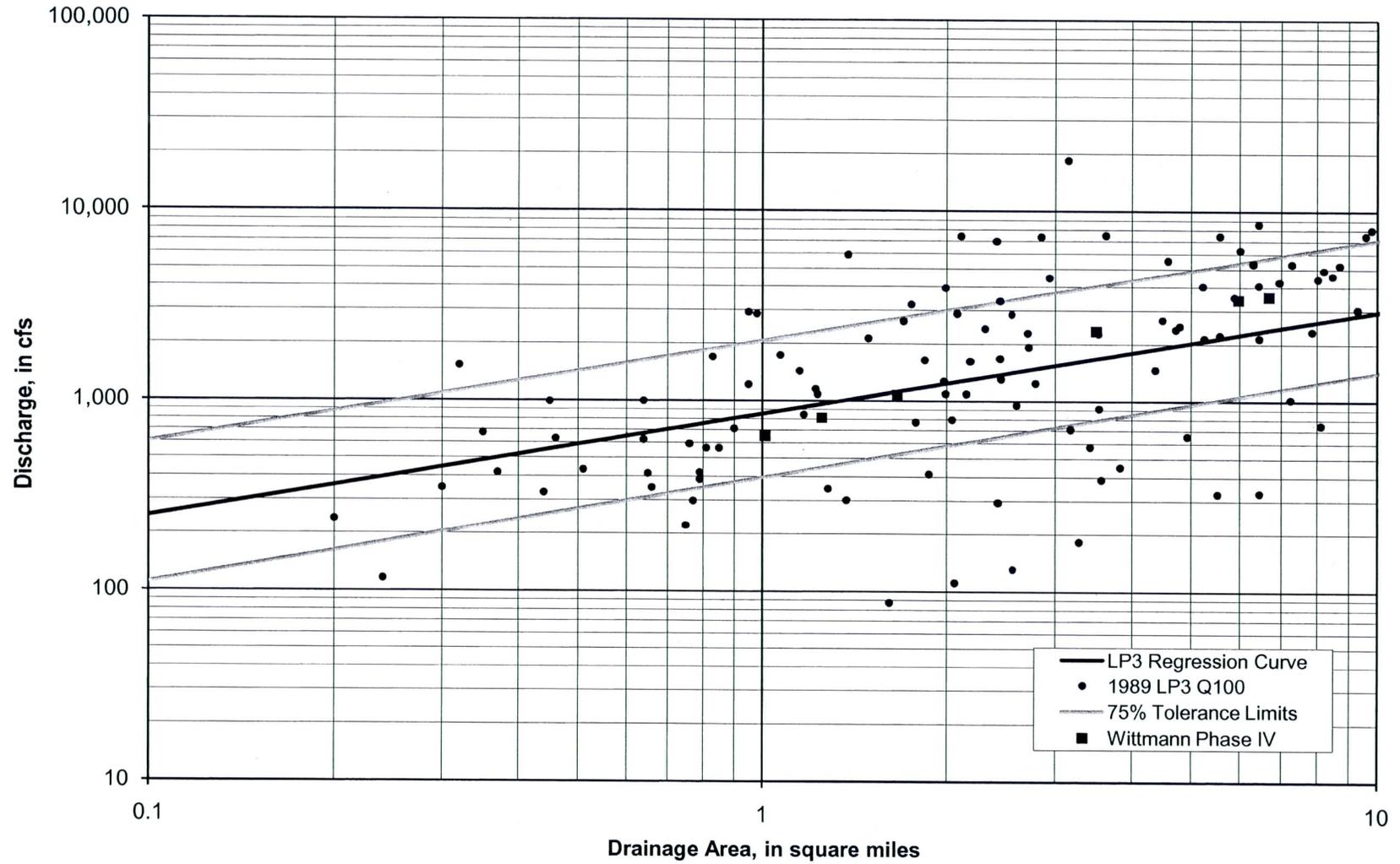
# 100-Year Peak Discharge by LP3 Analysis

Source: 1989 USGS Basin Characteristic Report, Figure Adapted from the ADOT Hydrology Manual



### 100-Year Peak Discharge by LP3 Analysis

Source: 1989 USGS Basin Characteristic Report, Figure Adapted from the ADOT Hydrology Manual





**Appendix E Hydraulic Analysis Supporting  
Documentation**



**E.1 Roughness Coefficient Estimation**

### Manning's "n" Value Determination

The procedure used to determine Manning's roughness coefficient, "n", is outlined in the USGS publication "Selection of Manning's Roughness Coefficients for Natural and Constructed Vegetated and Non-Vegetated Channels, and Vegetation Maintenance Plan Guidelines for Vegetated Channels in Central Arizona" (2006). The following equation was used:

$$n = (n_b + n_1 + n_2 + n_3 + n_4)m$$

Where n = estimated Manning's roughness coefficient

- $n_b$  = base value of n for a straight, uniform channel,
- $n_1$  = value for surface irregularities,
- $n_2$  = value for obstruction,
- $n_3$  = value for vegetation,
- $n_4$  = value for variation in channel cross section, and
- m = degree of meandering.

"Guidelines and Specifications for Flood Hazard Mapping Partners" recommends that the number of cross sections be minimized to one or two sections that are representative of the entire flooding source. In addition, Manning roughness coefficients should be estimated from field inspection and should be minimized by choosing values that are representative of the entire flooding source.

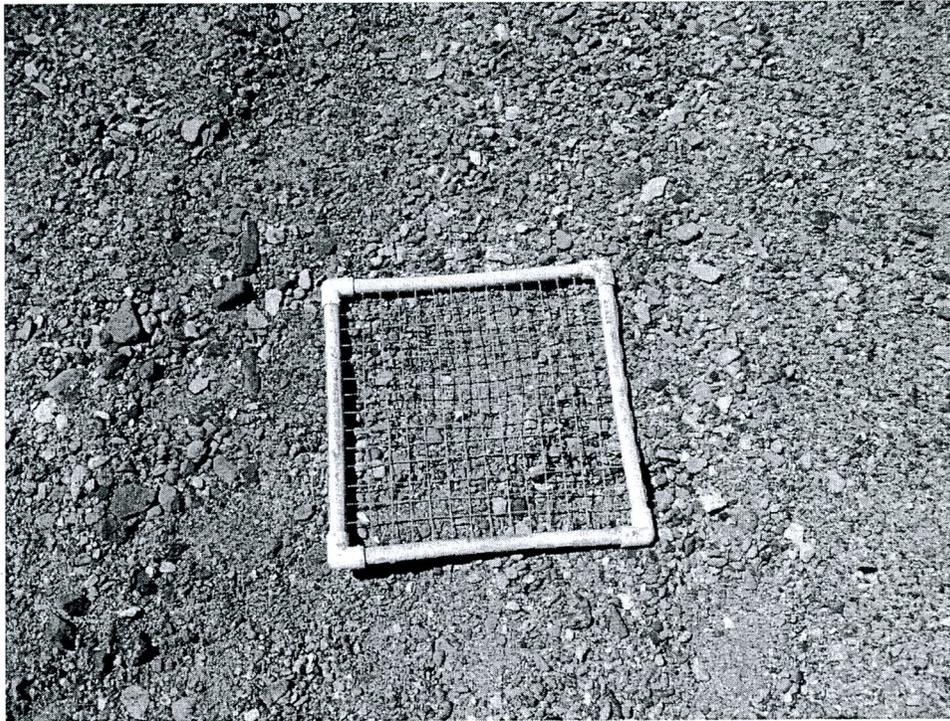
Based on the site visit the washes within this study are very similar. The base n,  $n_b$ , is established by the size of the bed material of the channel. Material described as coarse sand has a base n value ranging from 0.026-0.035. Material described as gravel ranges from 0.028-0.035. The value 0.026 was chosen for washes with sand and 0.028-0.030 for washes with gravel. The degree of irregularity was smooth for the channel and overbanks. The  $n_1$  value for the channel and for the overbanks was 0.000. The effect of obstruction in the channel and for the overbanks is negligible (few scattered obstructions, less than 5% of the cross-sectional area). The values of  $n_2$  chosen were 0.002 for the channel and for the overbanks. The channels have little vegetation and the overbanks have medium to large vegetation. The value of  $n_3$  for the channel was chosen to be 0.002 and the overbanks ranges from 0.010 to 0.025. The variation in channel cross section size and shape changes gradually so  $n_4$  is 0.000. The degree of meandering is minor so a value of 1 was used for m. The Table below summarizes the Manning's roughness coefficients.

Wash Name	Location	$n_b$	$n_1$	$n_2$	$n_3$	$n_4$	m	n
Iona Tributary 1 West (Location 1)	Channel	0.028	0.000	0.002	0.002	0.000	1.00	<b>0.032</b>
	Overbanks	0.026	0.000	0.002	0.025	0.000	1.00	<b>0.053</b>
Iona Tributary 1 West (Location 2)	Channel	0.028	0.000	0.002	0.002	0.000	1.00	<b>0.032</b>
	Overbanks	0.026	0.000	0.002	0.025	0.000	1.00	<b>0.053</b>

Iona Tributary 1 West Extension (Location 3)	Channel	0.030	0.000	0.002	0.002	0.000	1.00	<b>0.034</b>
	Overbanks	0.026	0.000	0.002	0.025	0.000	1.00	<b>0.053</b>
Iona Tributary 1 West Extension (Location 4)	Channel	0.026	0.000	0.002	0.002	0.000	1.00	<b>0.030</b>
	Overbanks	0.026	0.000	0.002	0.025	0.000	1.00	<b>0.053</b>
Iona Tributary 1 West Extension (Location 5)	Channel	0.026	0.000	0.002	0.002	0.000	1.00	<b>0.030</b>
	Overbanks	0.026	0.000	0.002	0.018	0.000	1.00	<b>0.046</b>
Iona Tributary 1 West Extension (Location 6)	Channel	0.026	0.000	0.002	0.002	0.000	1.00	<b>0.030</b>
	Overbanks	0.026	0.000	0.002	0.018	0.000	1.00	<b>0.046</b>
Iona Tributary 2 West (Location 7)	Channel	0.026	0.000	0.002	0.002	0.000	1.00	<b>0.030</b>
	Overbanks	0.026	0.000	0.002	0.025	0.000	1.00	<b>0.053</b>
Iona Tributary 2 West (Location 8)	Channel	0.026	0.000	0.002	0.002	0.000	1.00	<b>0.030</b>
	Overbanks	0.026	0.000	0.002	0.010	0.000	1.00	<b>0.038</b>
Iona Tributary 2 West (Location 9)	Channel	0.026	0.000	0.002	0.002	0.000	1.00	<b>0.030</b>
	Overbanks	0.026	0.000	0.002	0.010	0.000	1.00	<b>0.038</b>



Iona Tributary 1 West Looking Upstream  
South of Dove Valley Road Alignment (Location 1)



Iona Tributary 1 West Channel  
South of Dove Valley Road Alignment (Location 1)



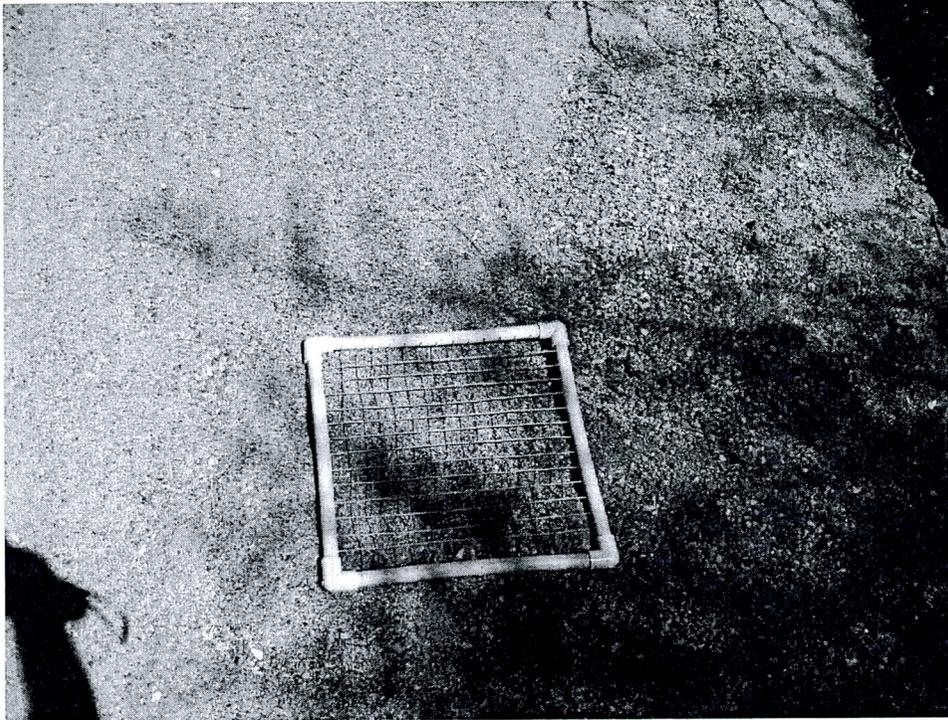
Iona Tributary 1 West Looking Upstream at Left Bank  
South of Dove Valley Road Alignment (Location 1)



Iona Tributary 1 West Looking Upstream at Right Bank  
South of Dove Valley Road Alignment (Location 1)



Iona Tributary 1 West Looking Upstream  
Near Carefree Highway Alignment (Location 2)



Iona Tributary 1 West Channel  
Near Carefree Highway Alignment (Location 2)



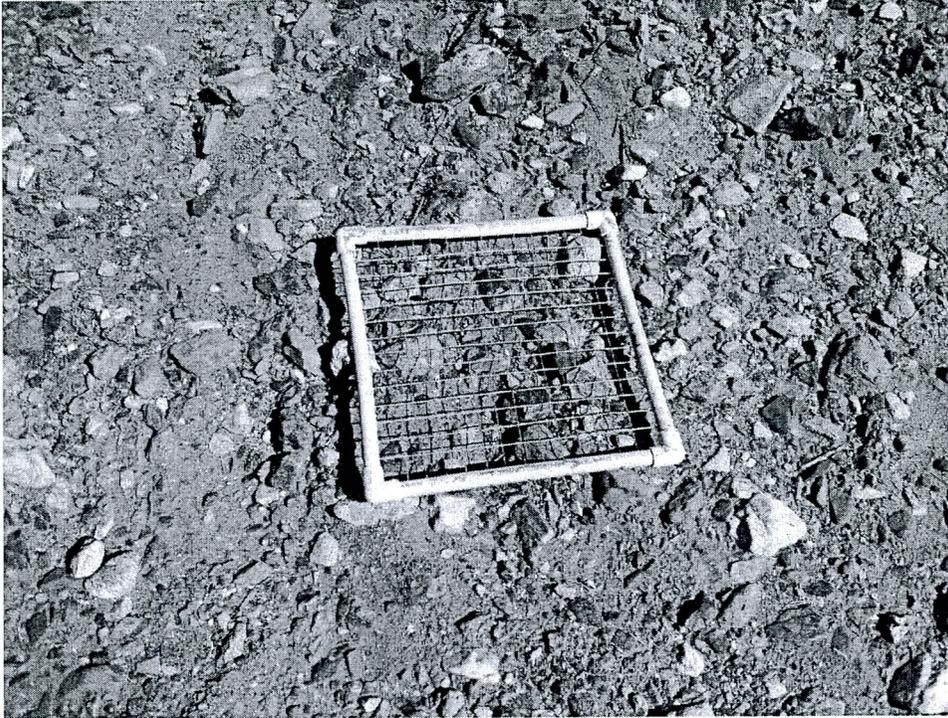
Iona Tributary 1 West Looking Upstream at Left Bank  
Near Carefree Highway Alignment (Location 2)



Iona Tributary 1 West Looking Upstream at Right Bank  
Near Carefree Highway Alignment (Location 2)



Iona Tributary 1 West Extension Looking Upstream  
Downstream of Stock Tank (Location 3)



Iona Tributary 1 West Extension Channel  
Downstream of Stock Tank (Location 3)



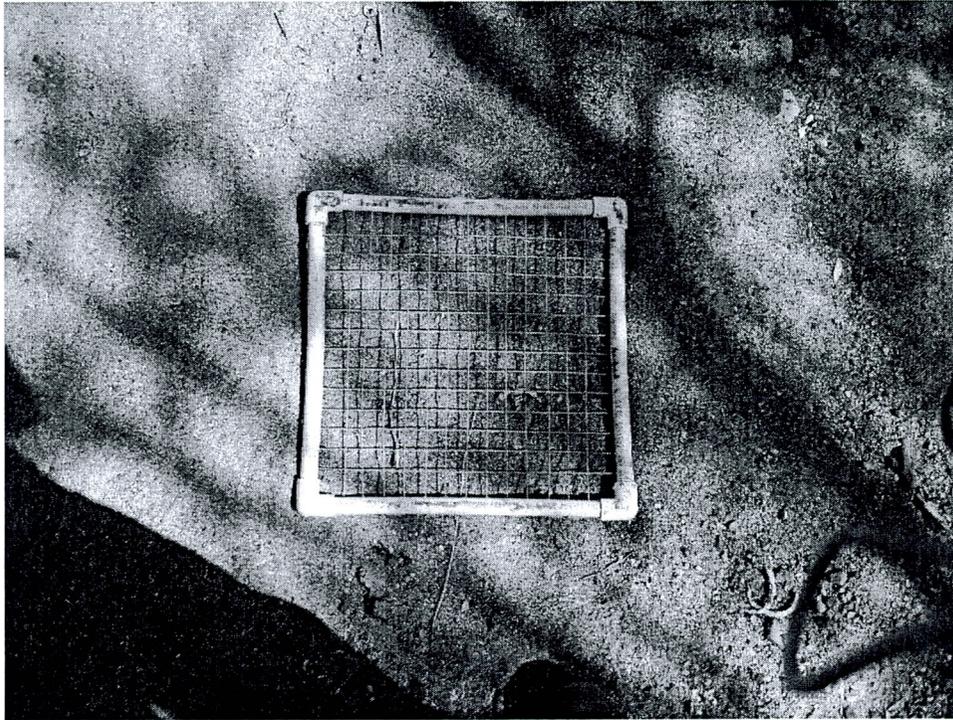
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Downstream of Stock Tank (Location 3)



Iona Tributary 1 West Extension Looking Upstream at Right Bank  
Downstream of Stock Tank (Location 3)



Iona Tributary 1 West Extension Looking Upstream  
Upstream of Stock Tank (Location 4)



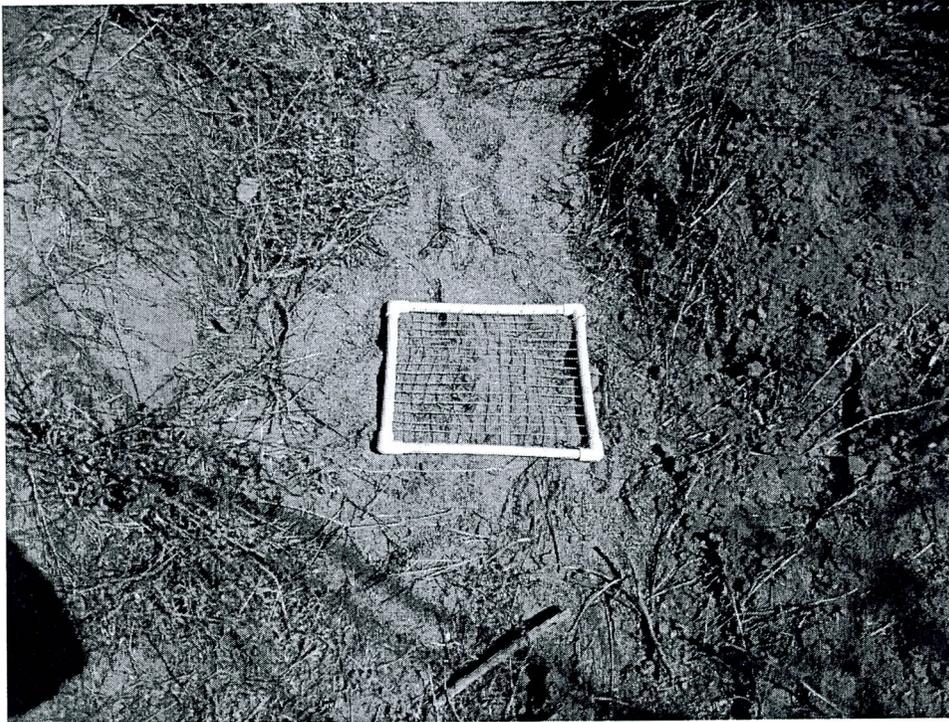
Iona Tributary 1 West Extension Channel  
Upstream of Stock Tank (Location 4)



Iona Tributary 1 West Extension Looking Upstream at Left Bank  
Upstream of Stock Tank (Location 4)



Iona Tributary 1 West Extension Looking Upstream at Right Bank  
Upstream of Stock Tank (Location 4)



Iona Tributary 1 West Extension Channel Bottom  
Near Black Mountain Road Alignment (Location 5)



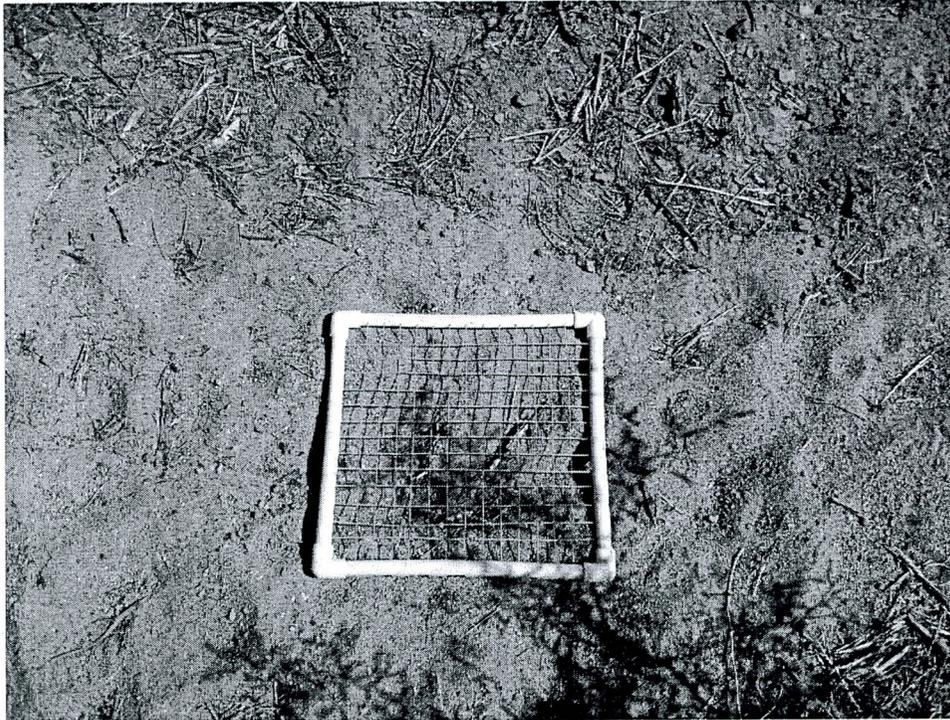
Iona Tributary 1 West Extension Looking Upstream  
Near Black Mountain Road Alignment (Location 5)



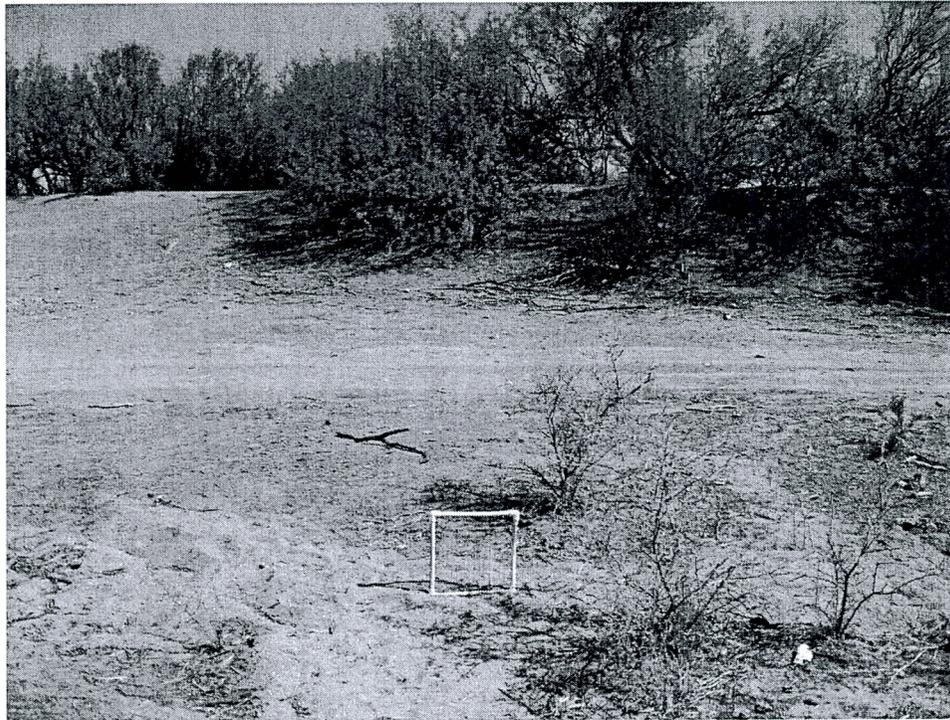
Iona Tributary 1 West Extension Looking Upstream at Left Bank  
Near Black Mountain Road Alignment (Location 5)



Iona Tributary 1 West Extension Looking Upstream at Right Bank  
Near Black Mountain Road Alignment (Location 5)



Iona Tributary 1 West Extension Channel Bottom  
Limit of Study Downstream of Stock Tank (Location 6)



Iona Tributary 1 West Extension Looking Upstream  
Limit of Study Downstream of Stock Tank (Location 6)



Iona Tributary 1 West Extension Looking Upstream at Left Bank  
Limit of Study Downstream of Stock Tank (Location 6)



Iona Tributary 1 West Extension Looking Upstream at Right Bank  
Limit of Study Downstream of Stock Tank (Location 6)



Iona Tributary 2 West Looking Upstream  
Near the Confluence with Iona Wash (Location 7)



Iona Tributary 2 West Channel  
Near the Confluence with Iona Wash (Location 7)



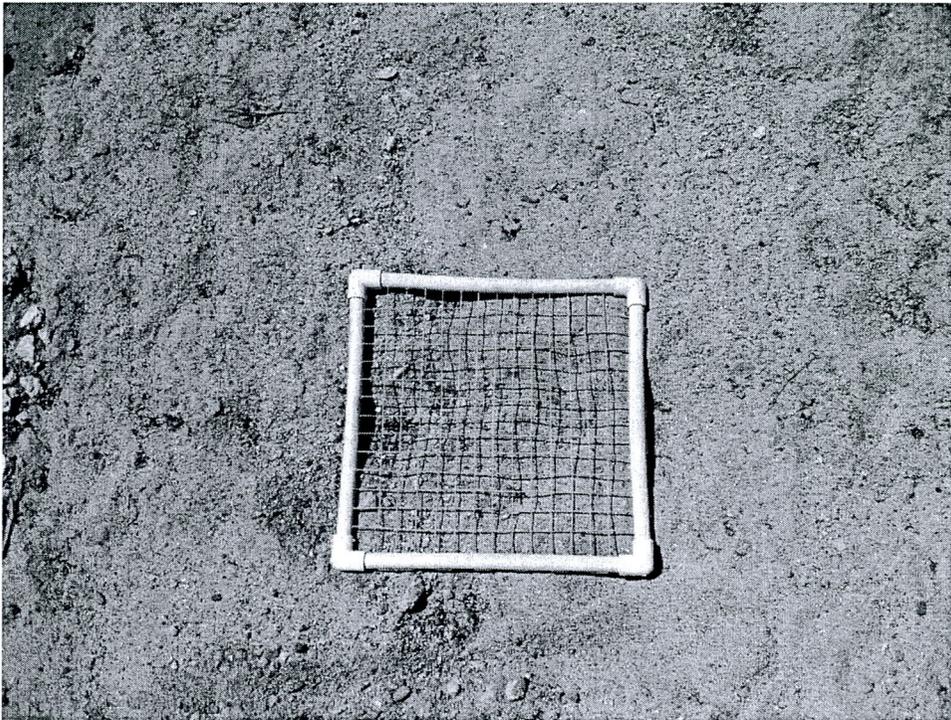
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Near the Confluence with Iona Wash (Location 7)



Iona Tributary 2 West Looking Upstream at Right Bank  
Near the Confluence with Iona Wash (Location 7)



Iona Tributary 2 West Looking Upstream  
South of Dove Valley Road Alignment (Location 8)



Iona Tributary 2 West Channel  
South of Dove Valley Road Alignment (Location 8)



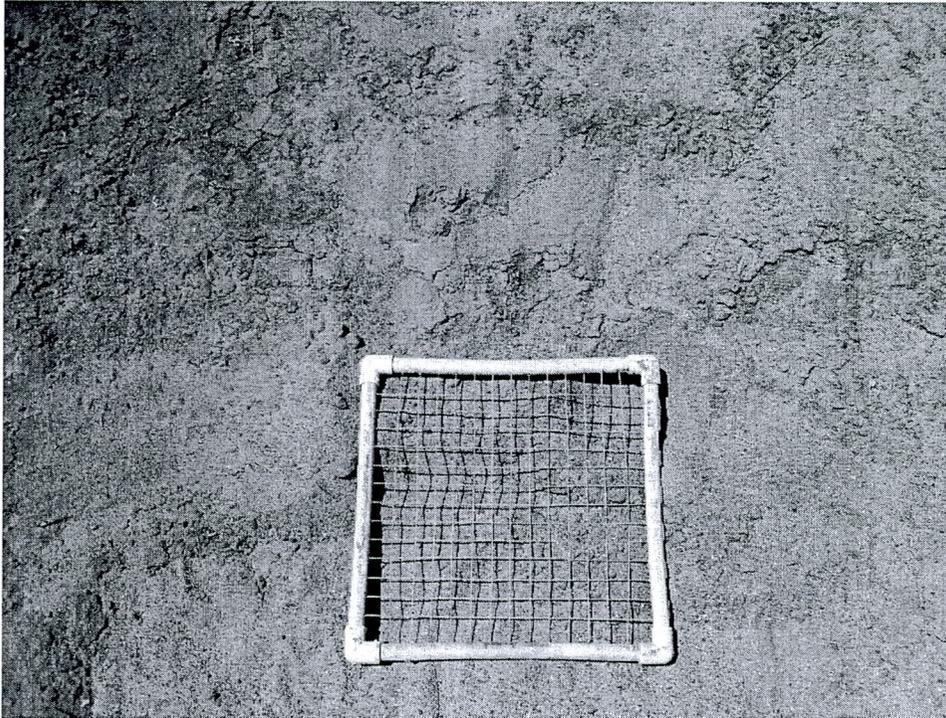
Iona Tributary 2 West Looking Upstream at Left Bank  
South of Dove Valley Road Alignment (Location 8)



Iona Tributary 2 West Looking Upstream at Right Bank  
South of Dove Valley Road Alignment (Location 8)



Iona Tributary 2 West Looking Upstream  
East of 259<sup>th</sup> Avenue Alignment Near Limit of Study (Location 9)



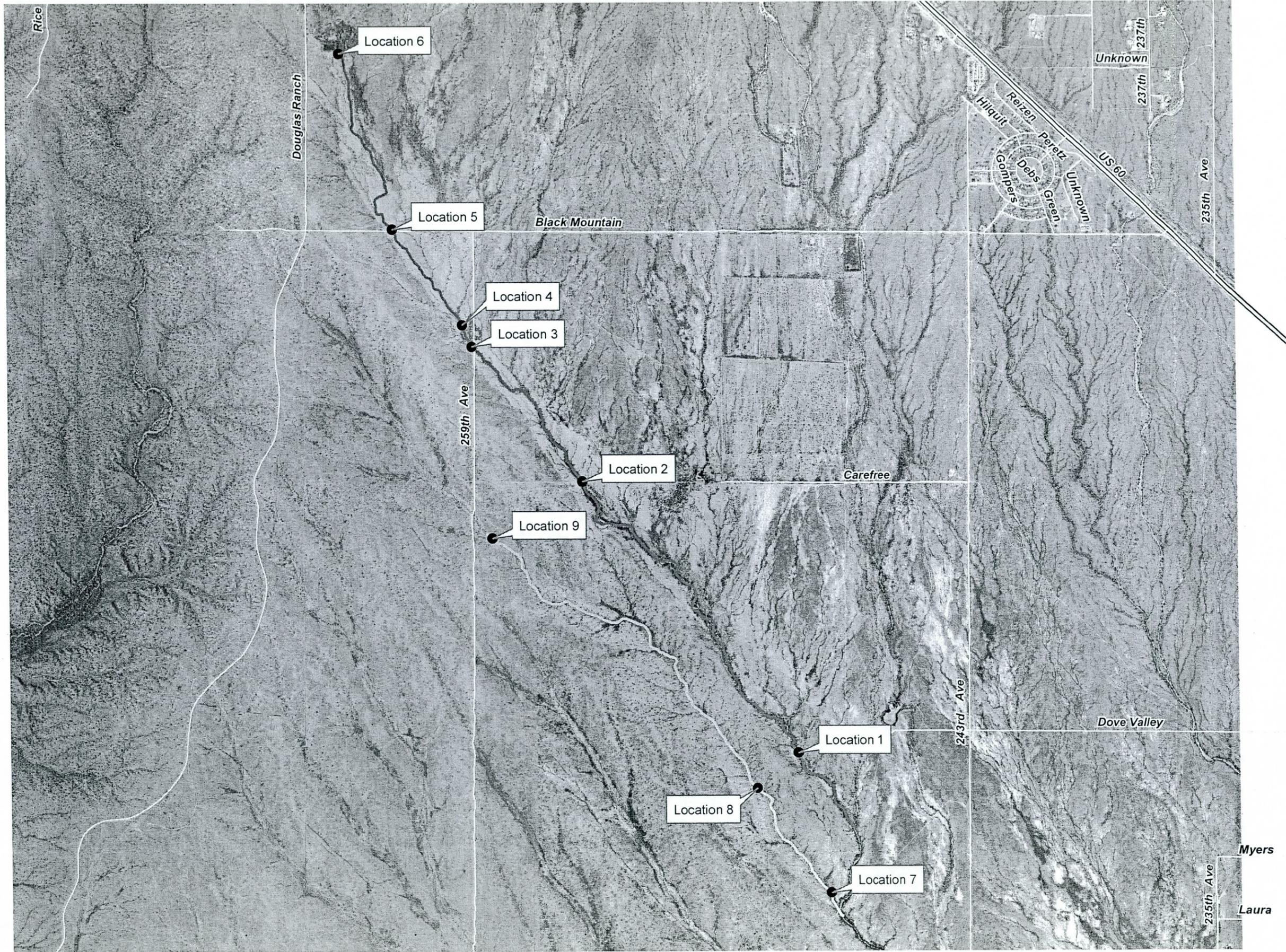
Iona Tributary 2 West Channel  
East of 259<sup>th</sup> Avenue Alignment Near Limit of Study (Location 9)



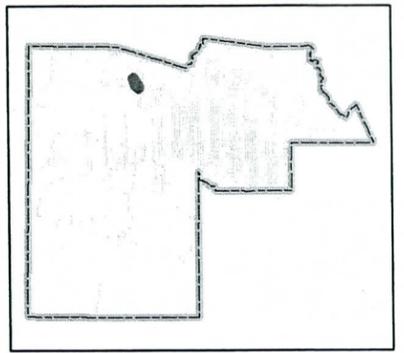
Iona Tributary 2 West Looking Upstream at Left Bank  
East of 259<sup>th</sup> Avenue Alignment Near Limit of Study (Location 9)



Iona Tributary 2 West Looking Upstream at Right Bank  
East of 259<sup>th</sup> Avenue Alignment Near Limit of Study (Location 9)



Flood Control District of Maricopa County



Wittmann Phase IV Floodplain Delineation Study

Legend

- Iona Tributary 1 West
- Iona Tributary 1 West Extension
- Iona Tributary 2 West
- Streets

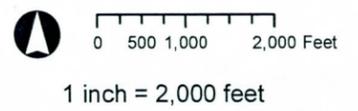


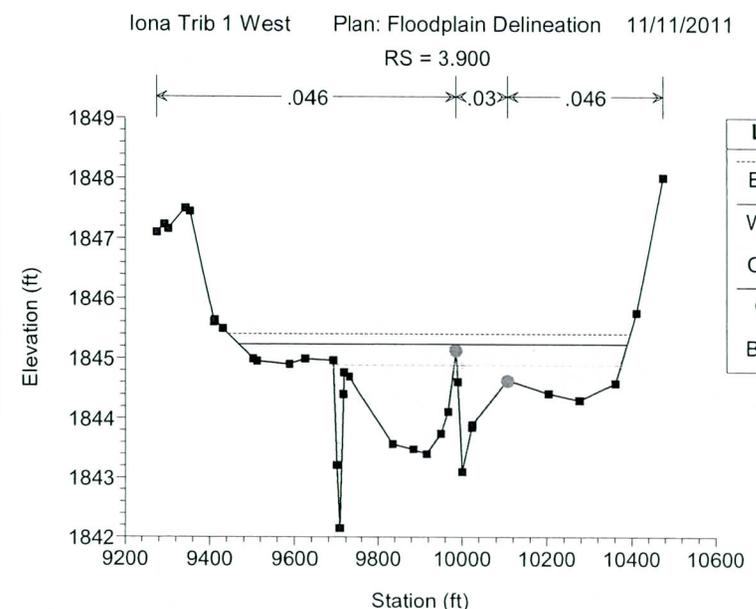
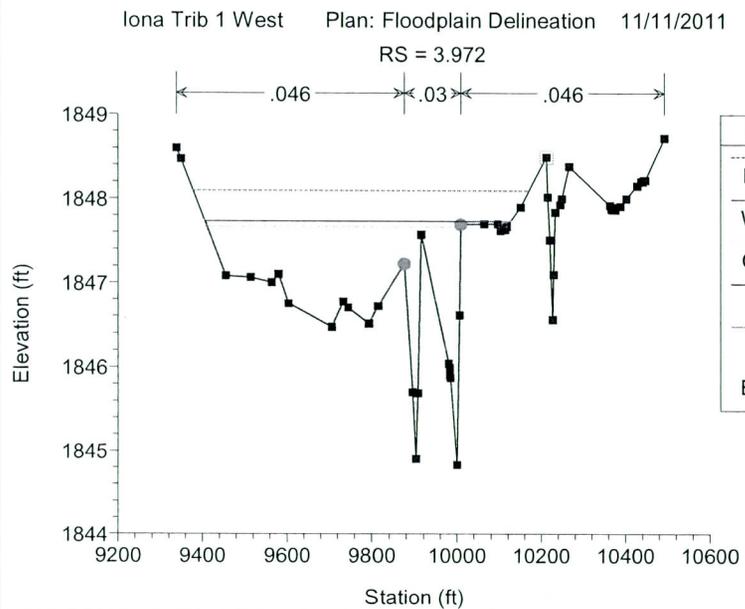
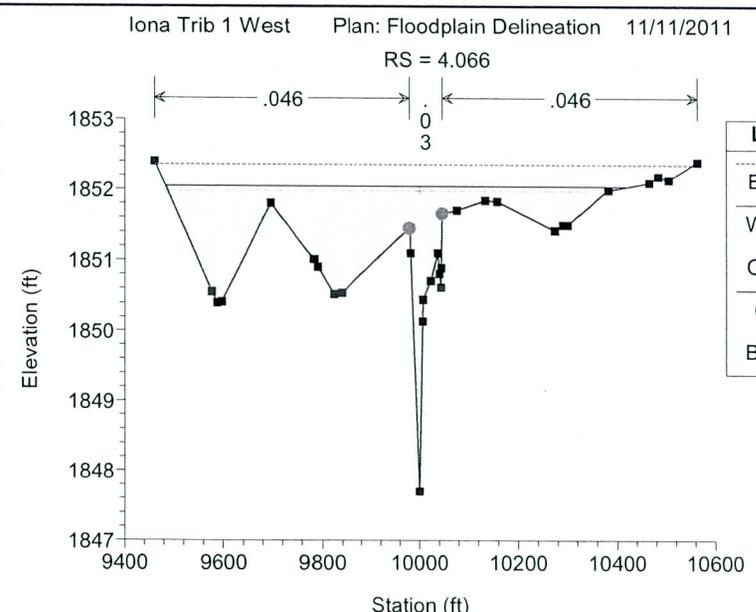
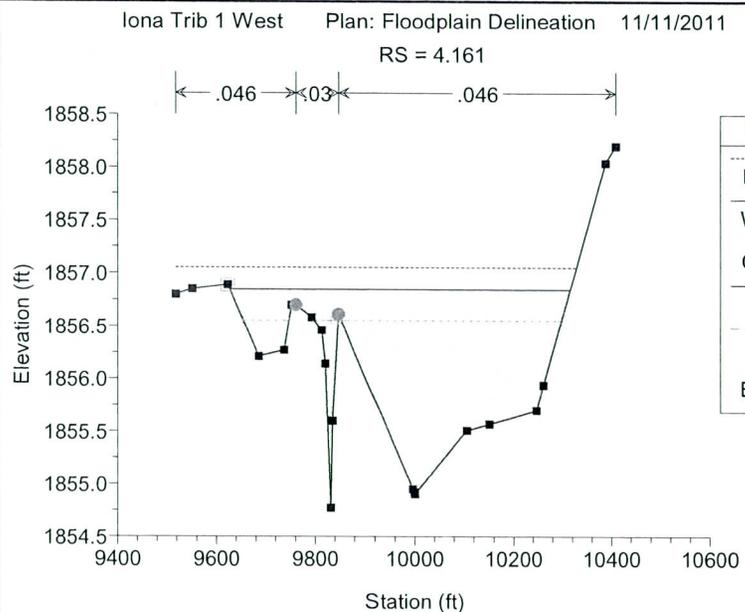
Photo Locations n Value Determination

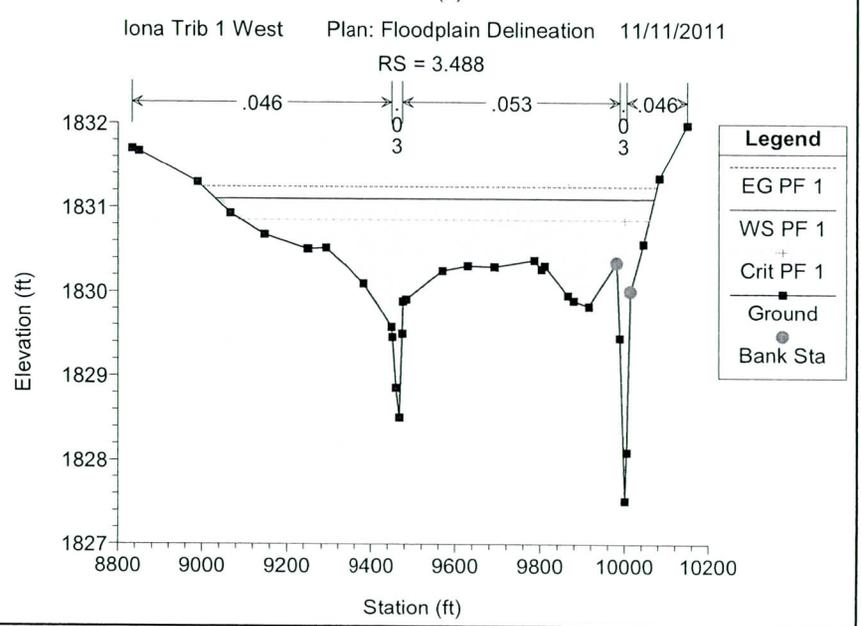
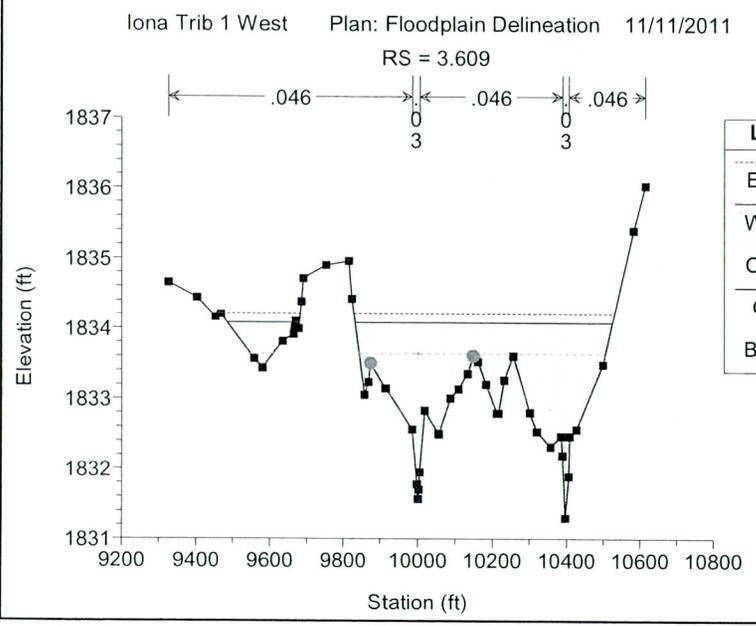
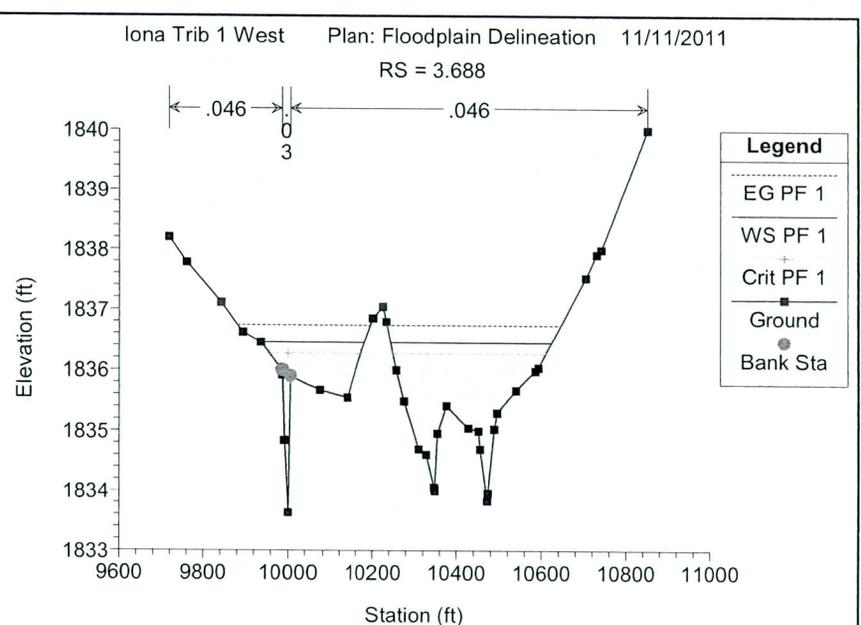
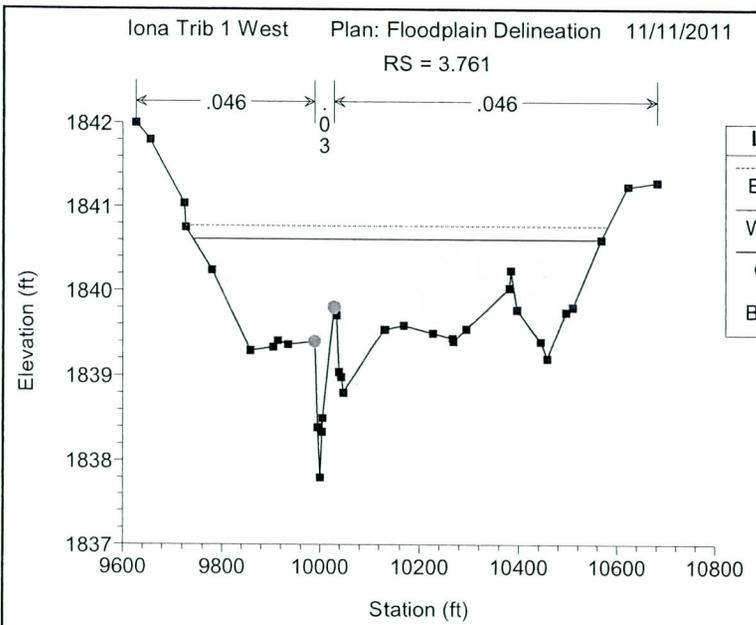
**RBF** CONSULTING PLANNING ■ DESIGN ■ CONSTRUCTION  
 16605 NORTH 28th AVENUE, SUITE 100  
 PHOENIX, ARIZONA 85053-7550  
 602-467-2200 FAX 602-467-2201 www.RBF.com

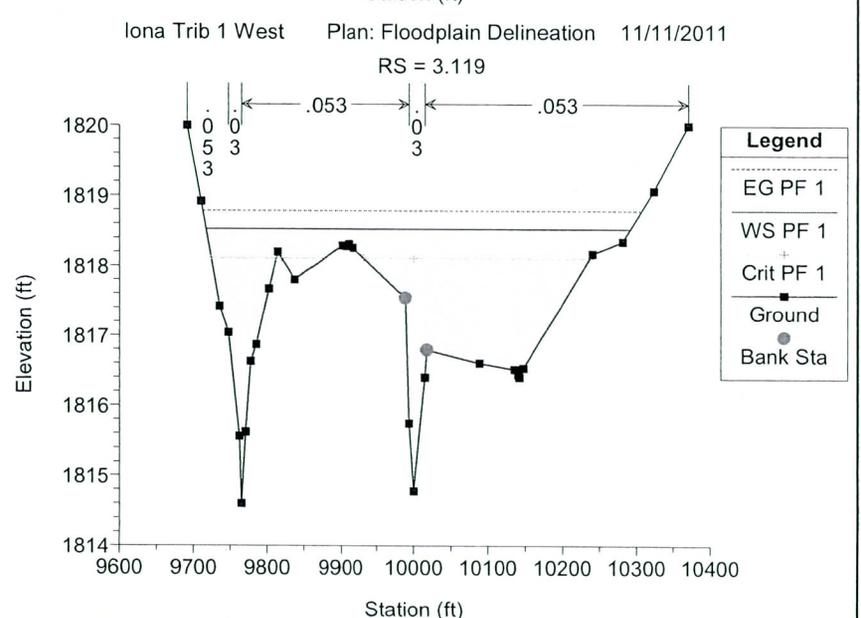
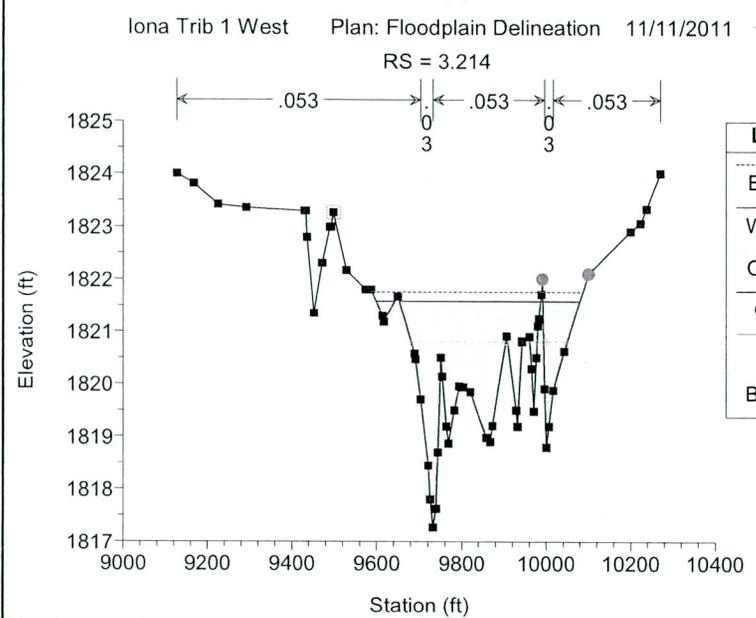
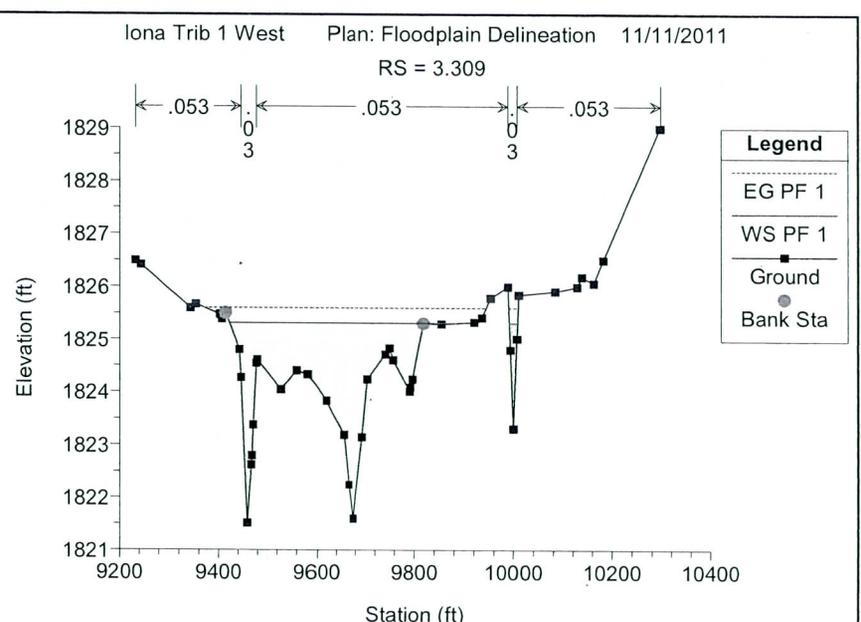
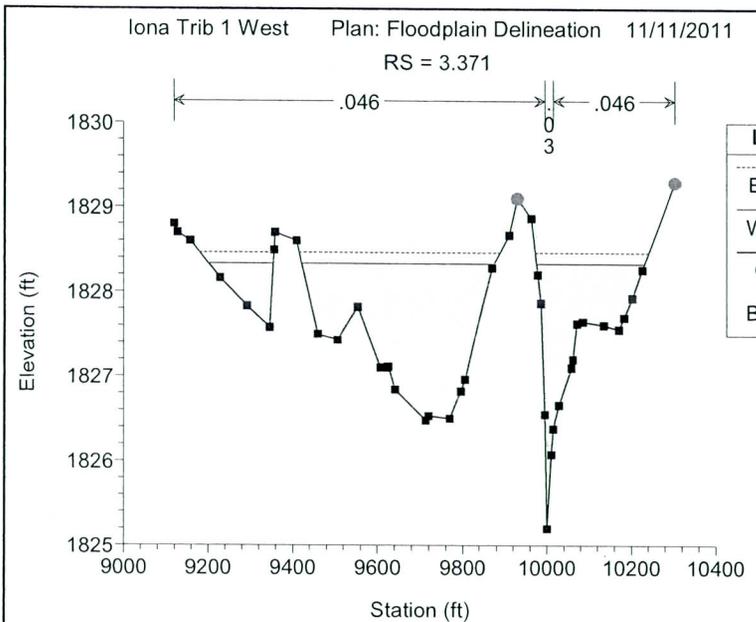
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DATE <b>9.08.11</b>			



**E.2 Cross Section Plots**

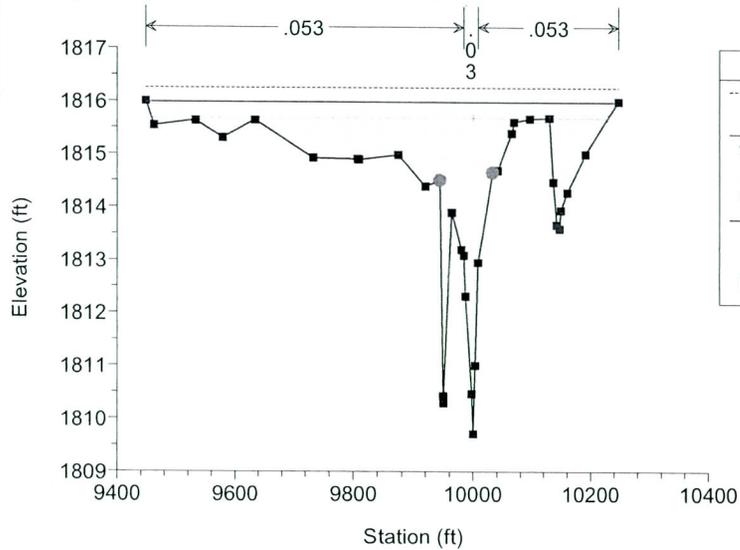






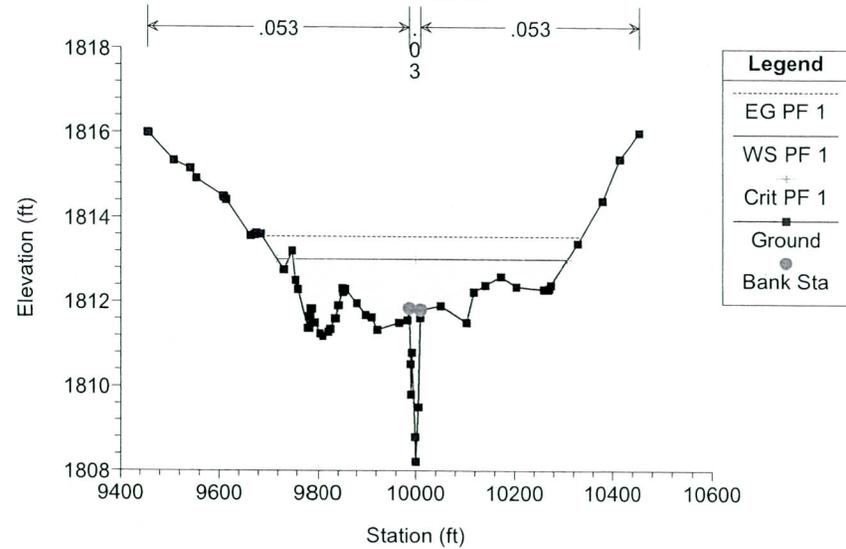
Iona Trib 1 West Plan: Floodplain Delineation 11/11/2011

RS = 3.025



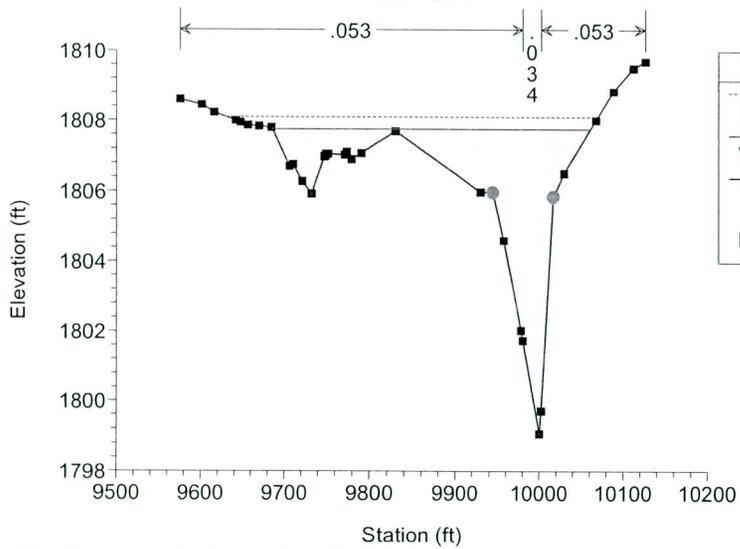
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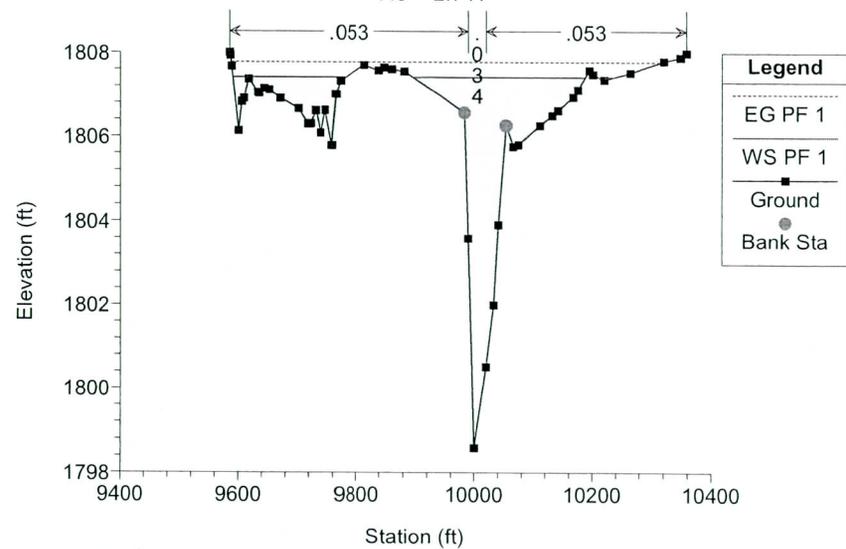
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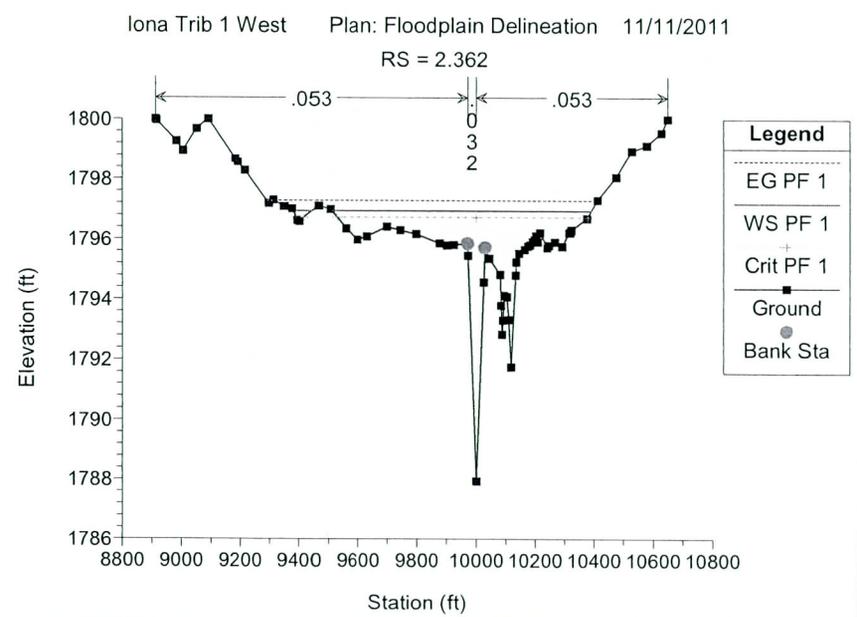
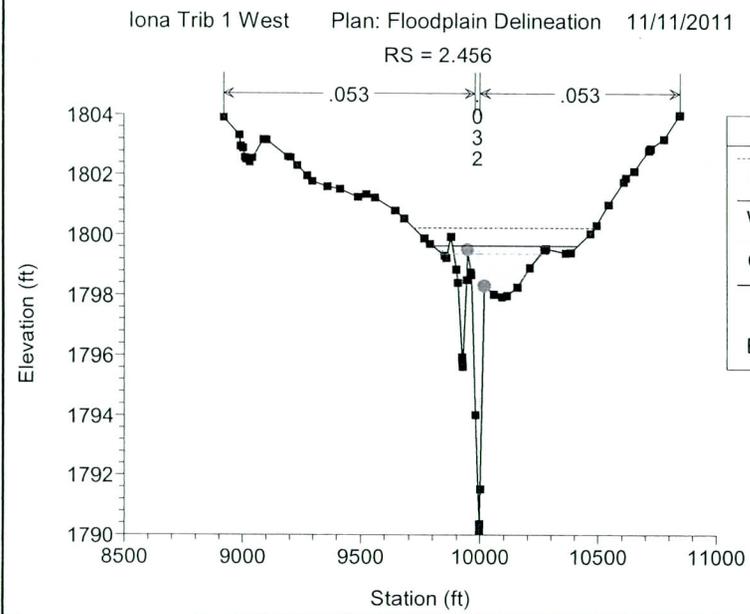
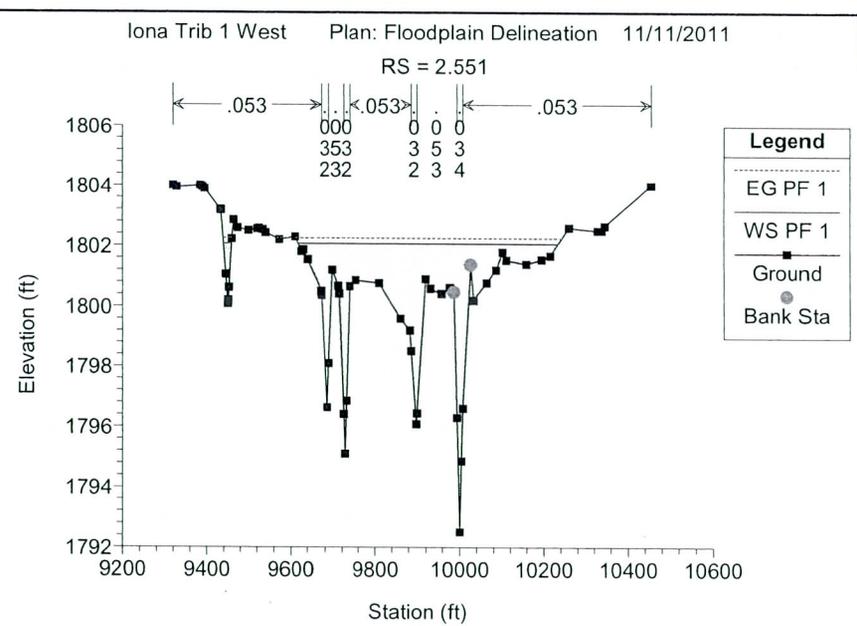
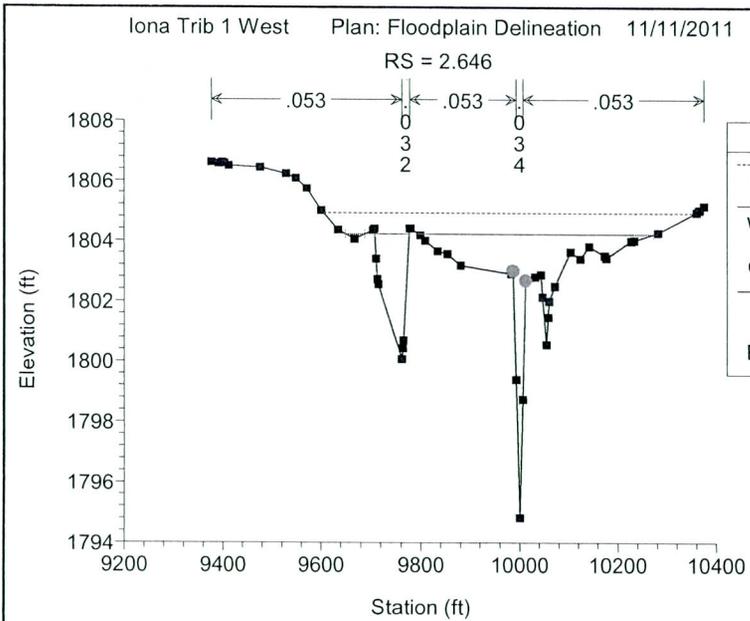
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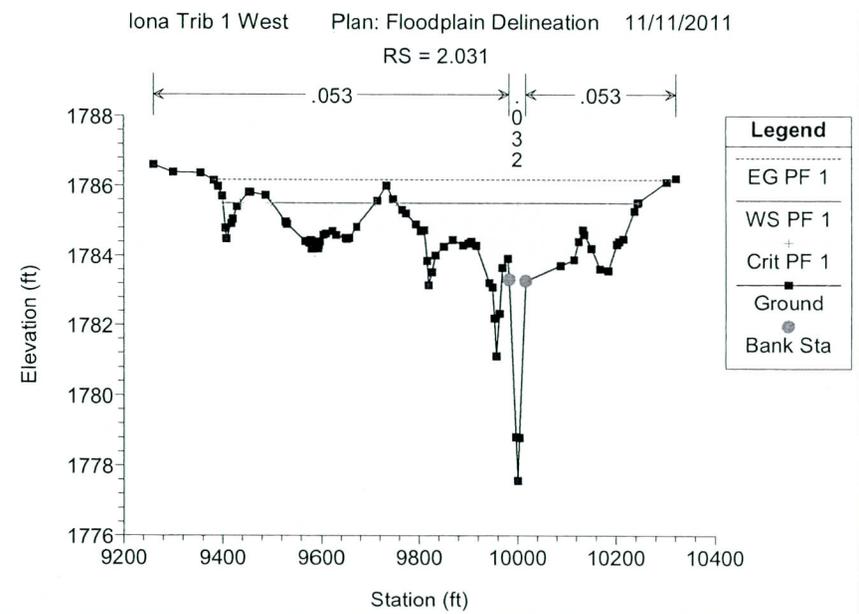
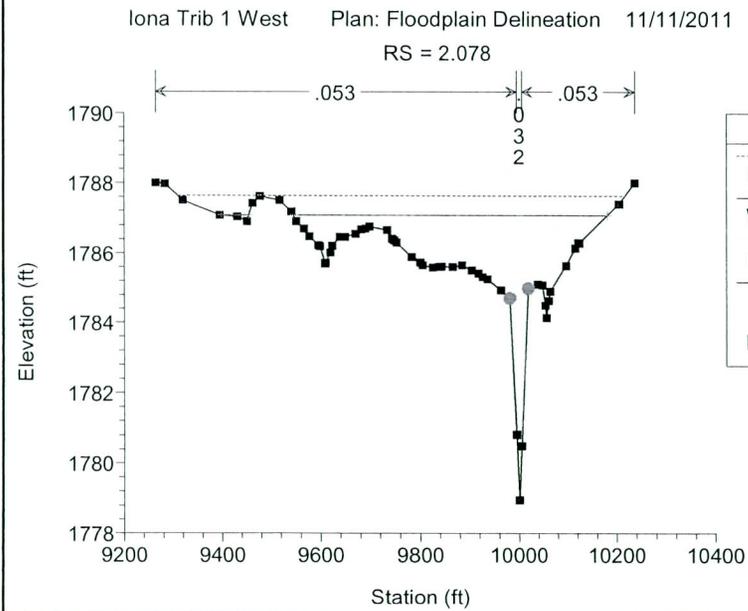
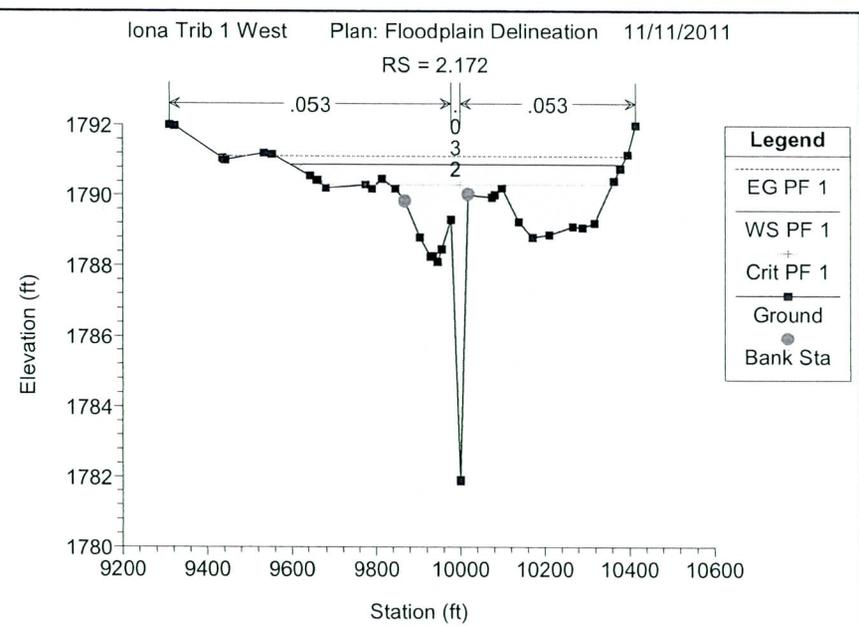
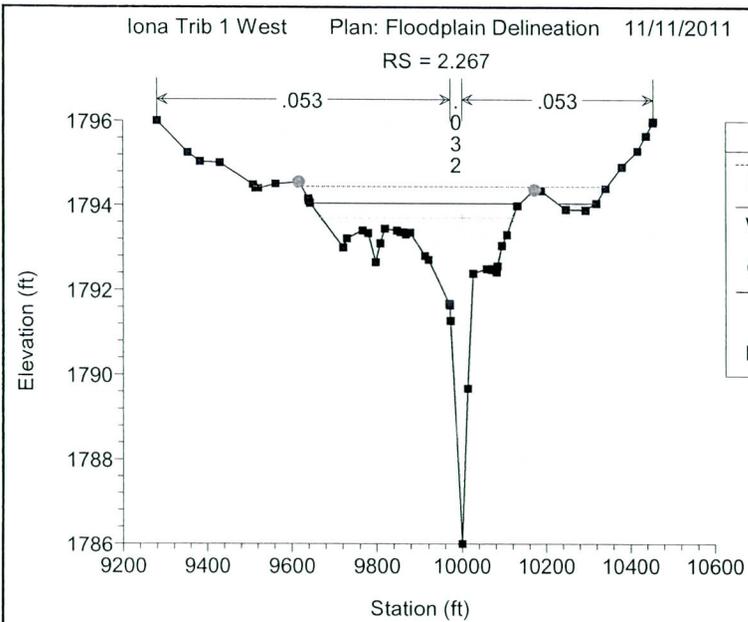


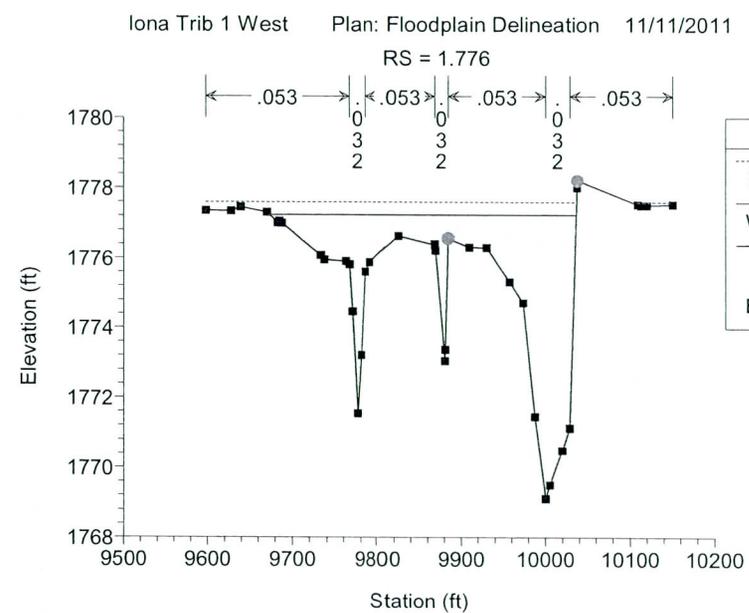
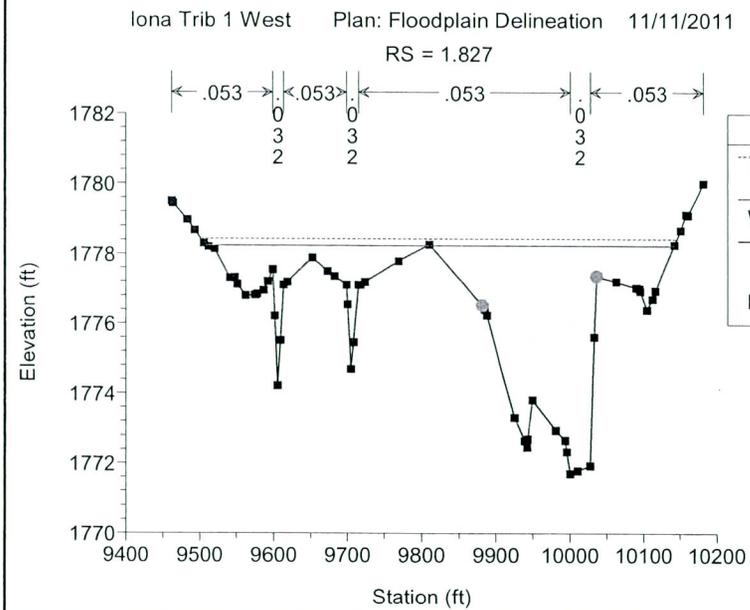
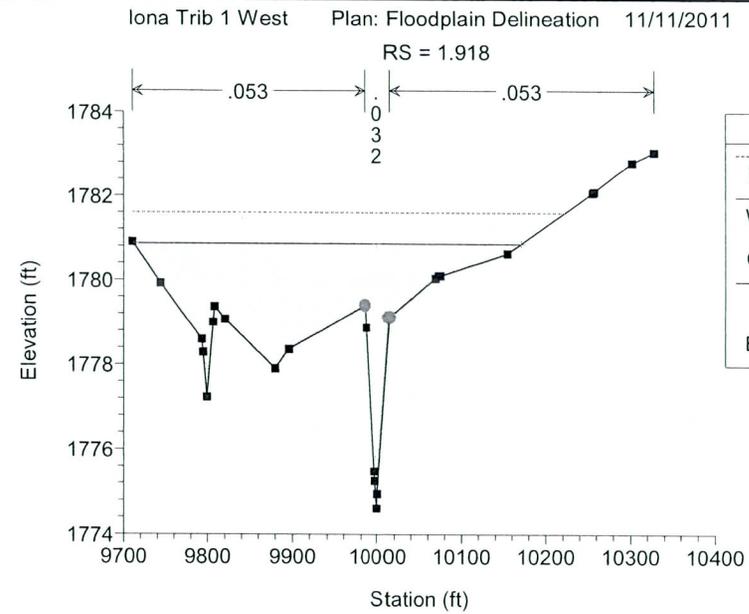
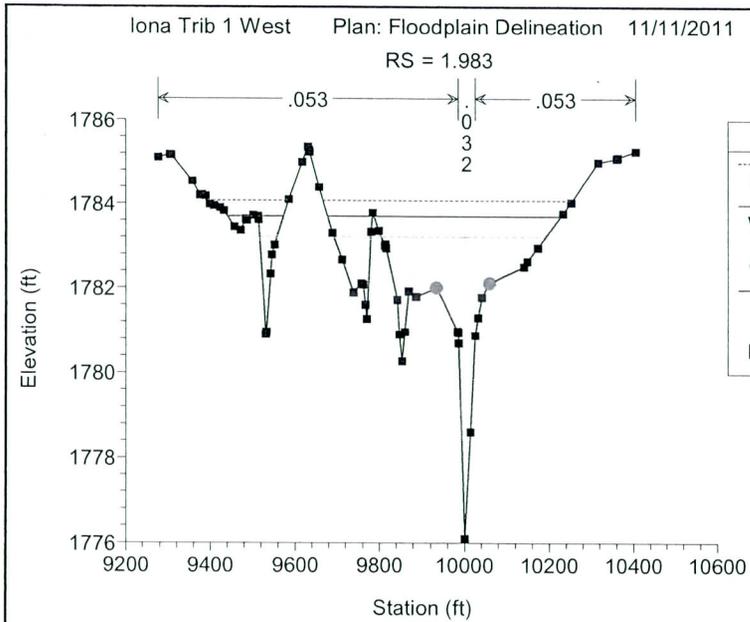
Iona Trib 1 West Plan: Floodplain Delineation 11/11/2011

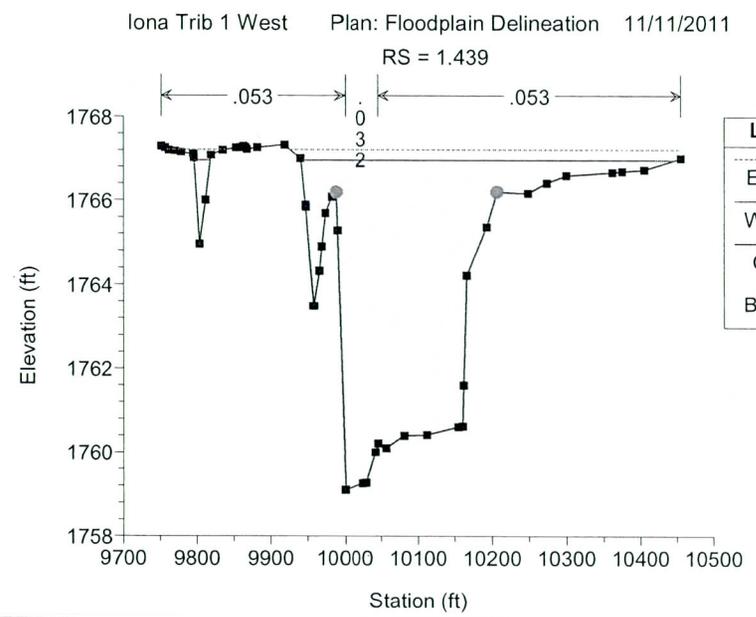
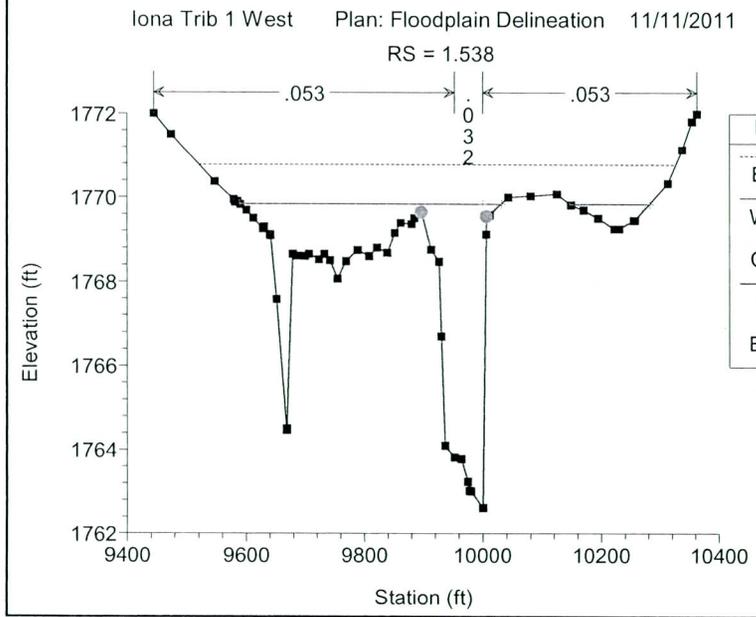
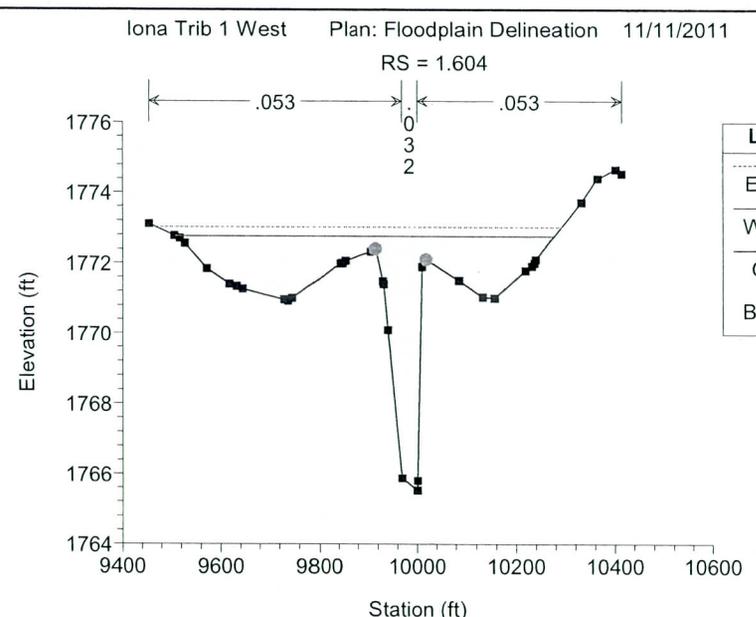
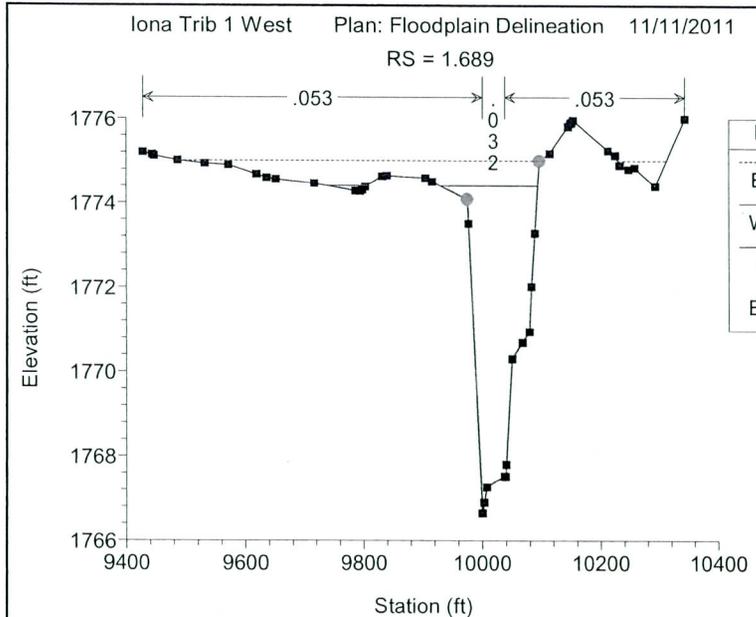
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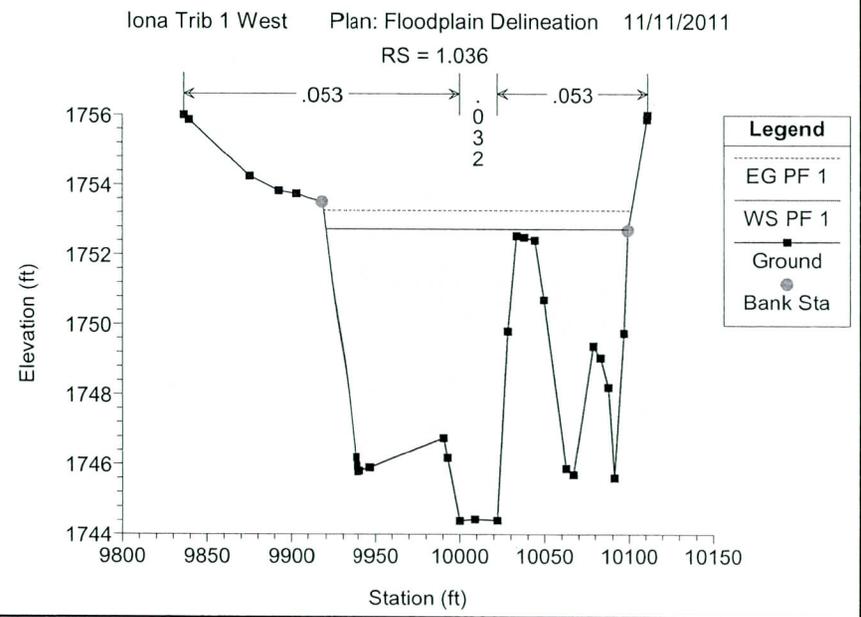
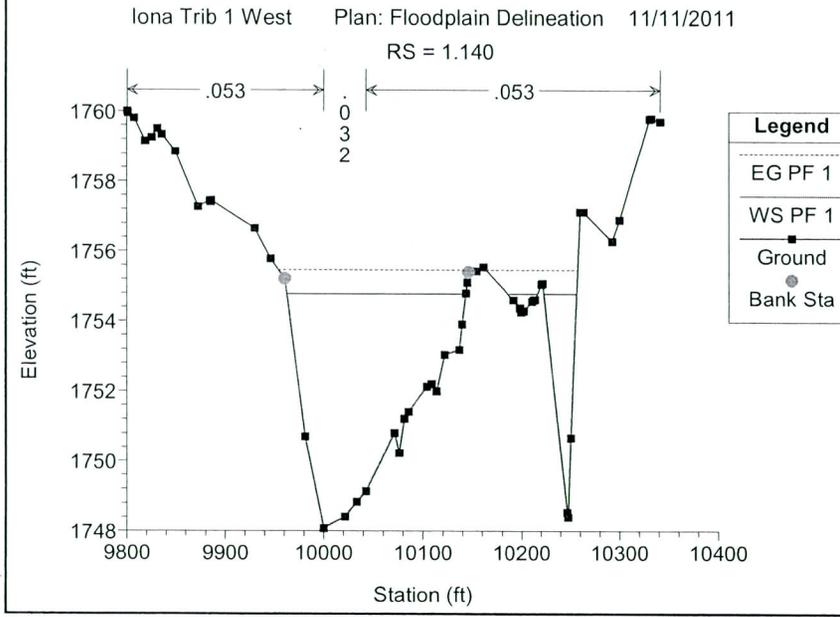
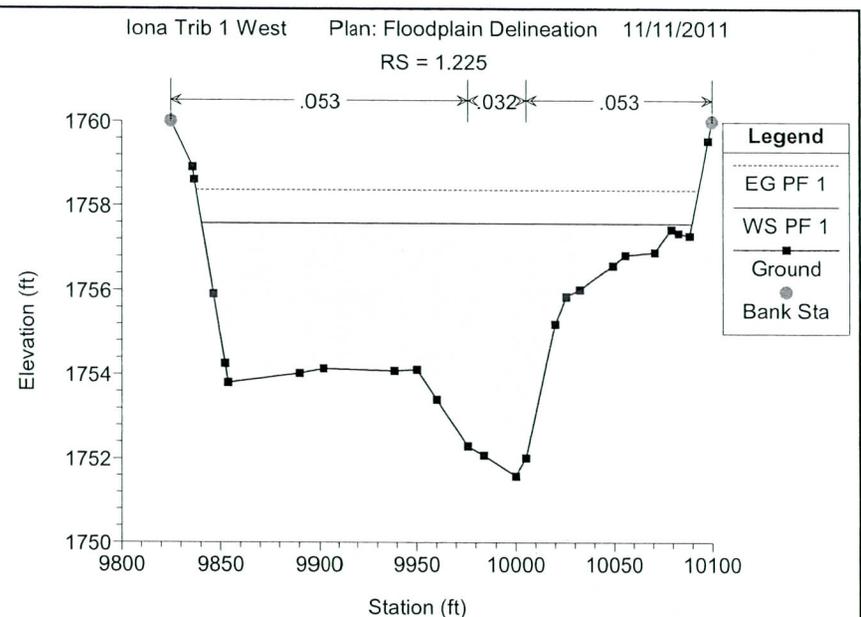
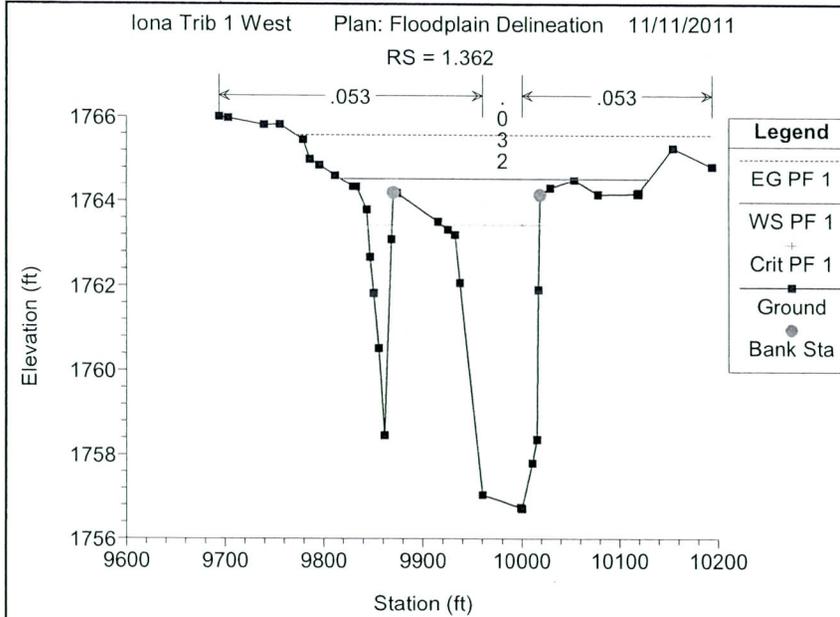


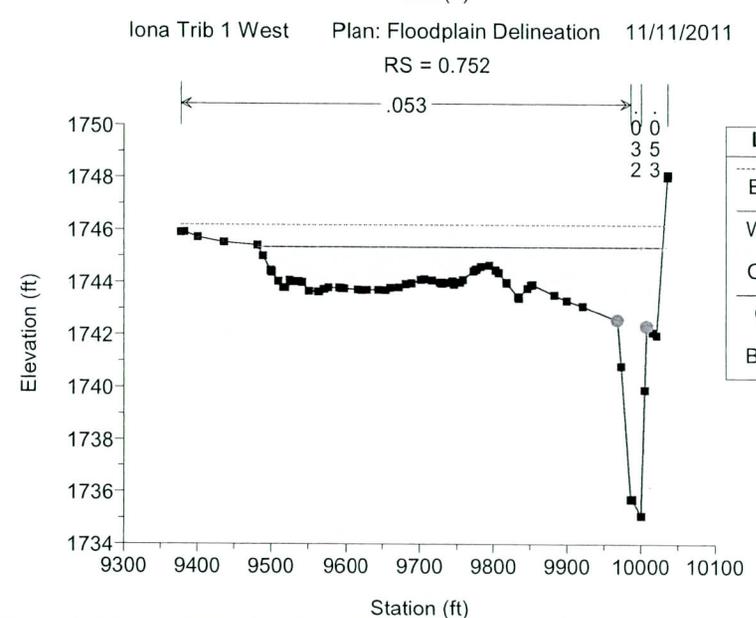
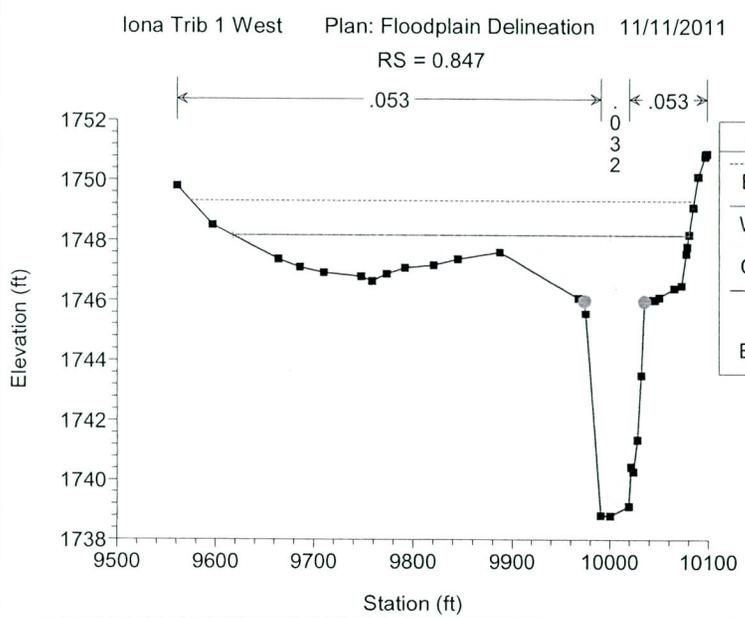
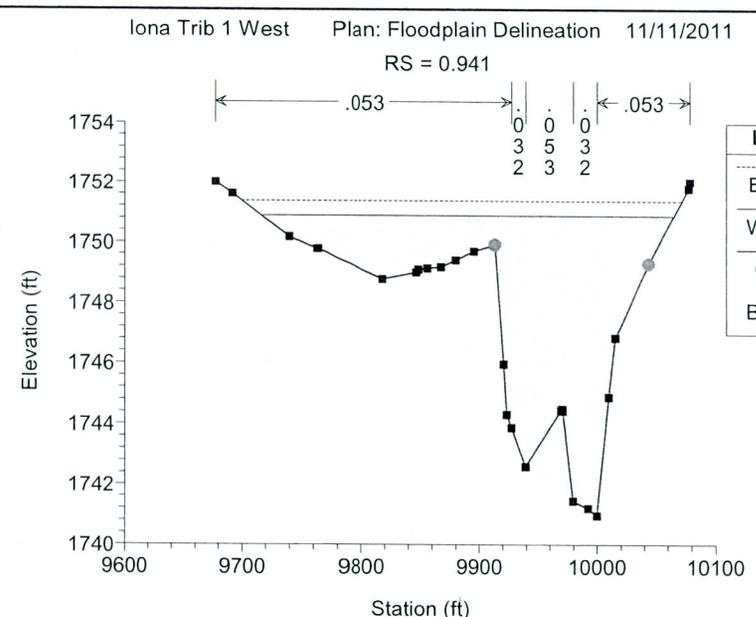
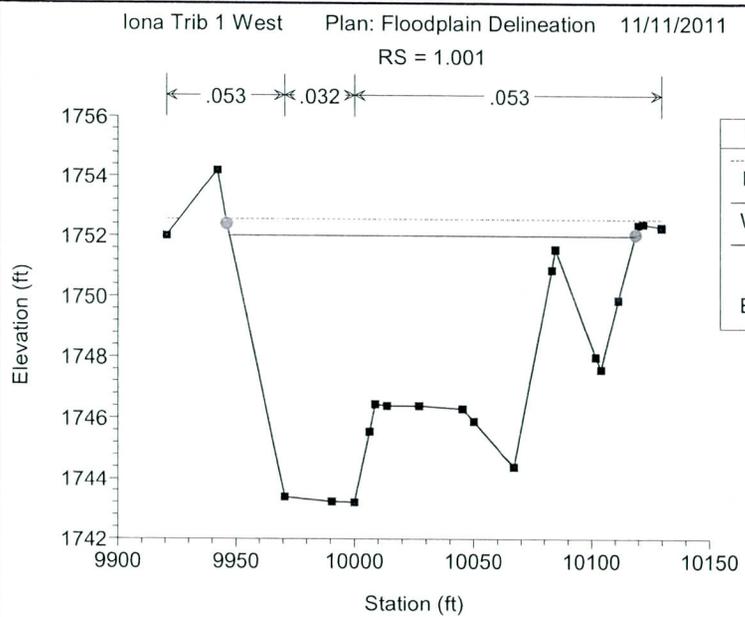


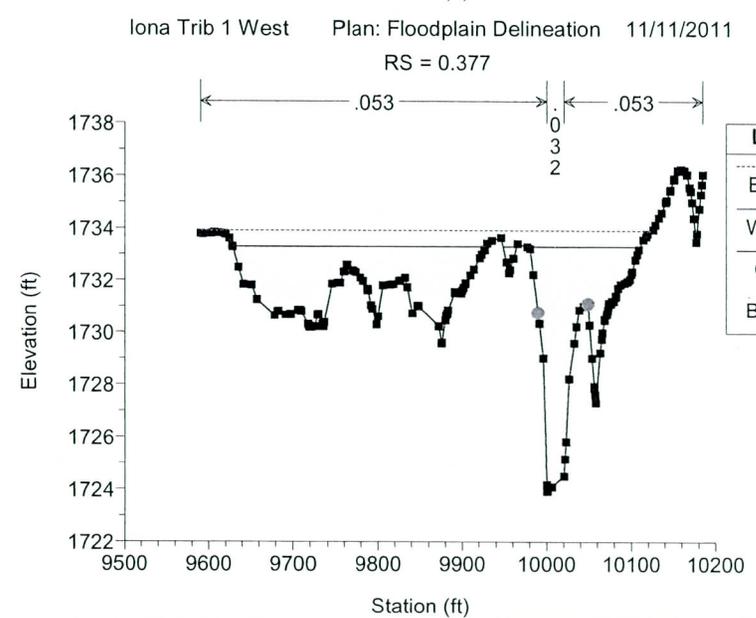
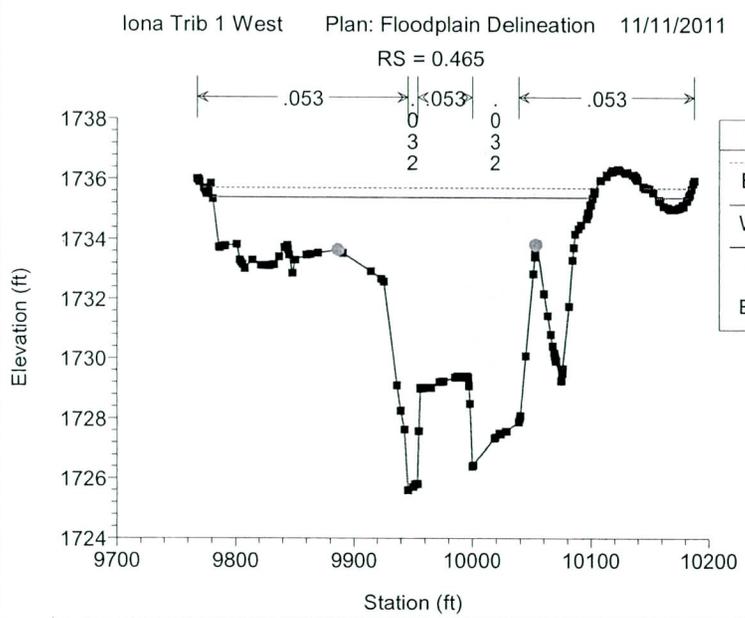
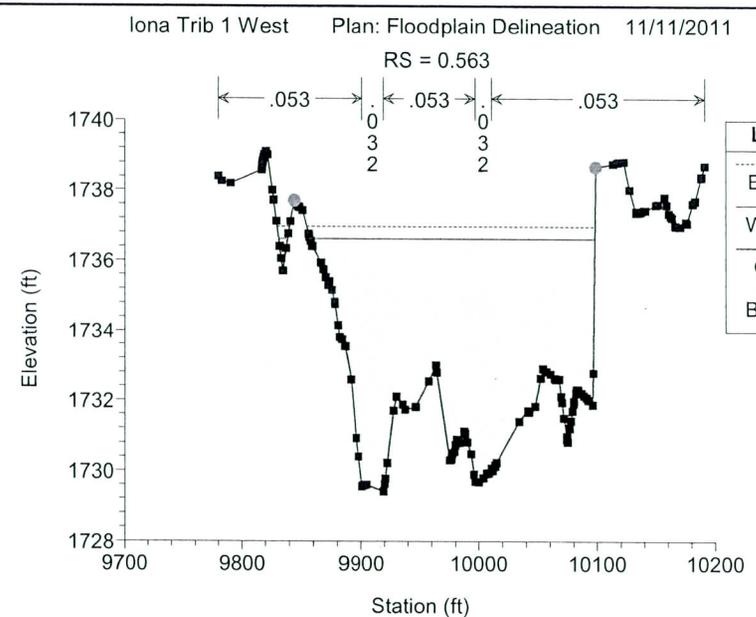
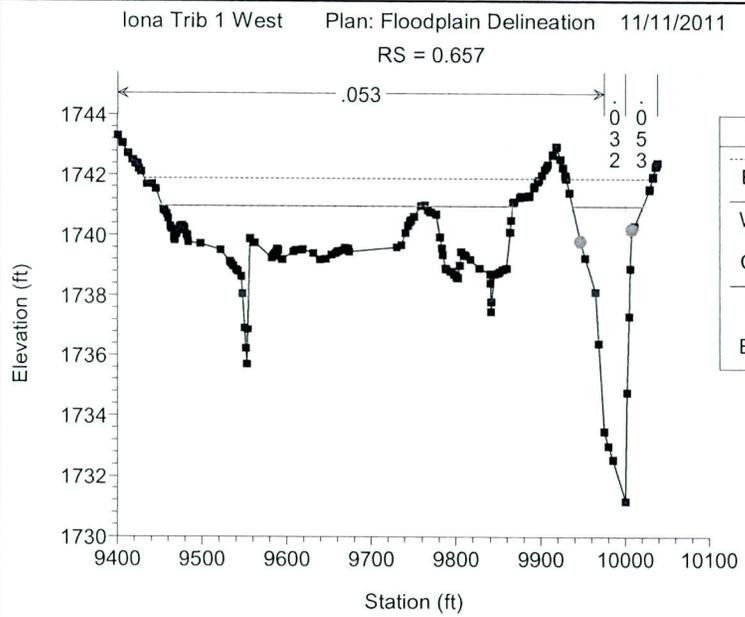


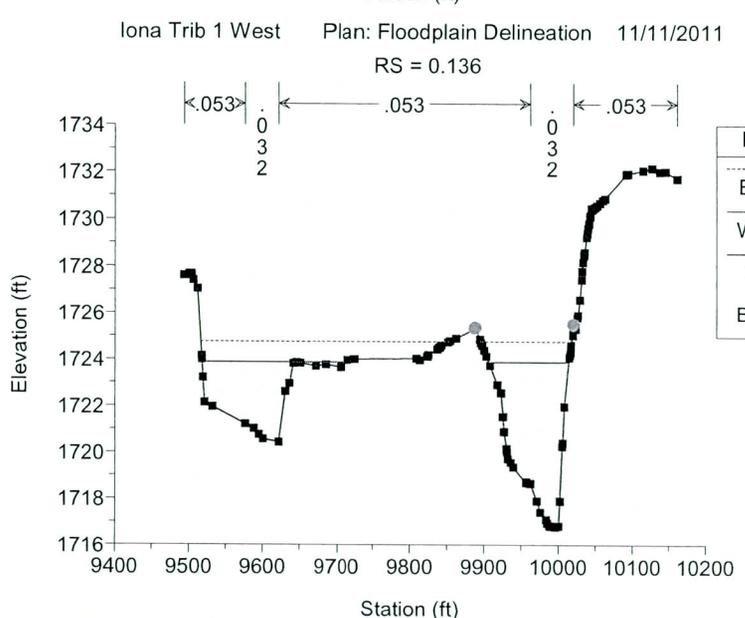
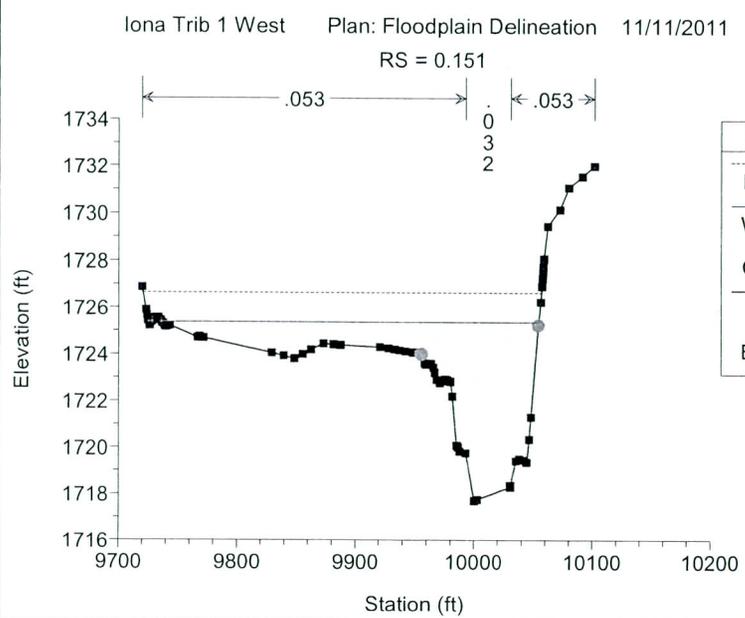
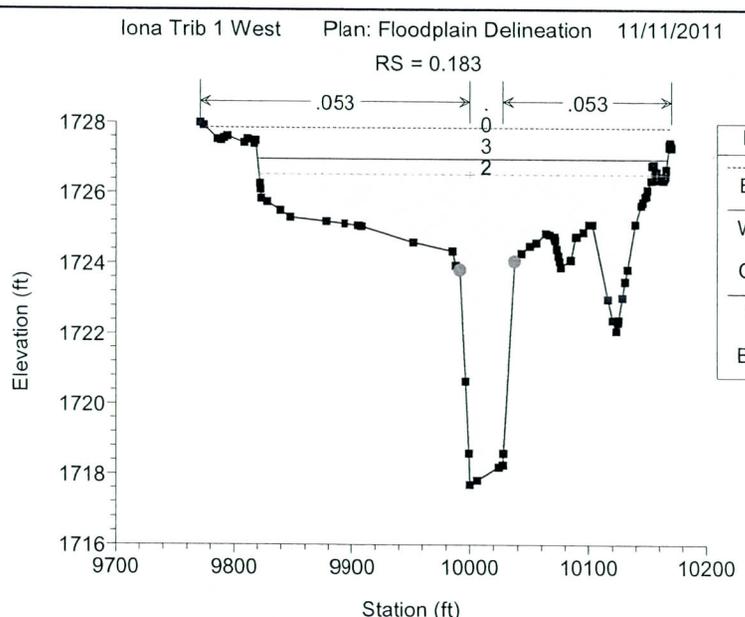
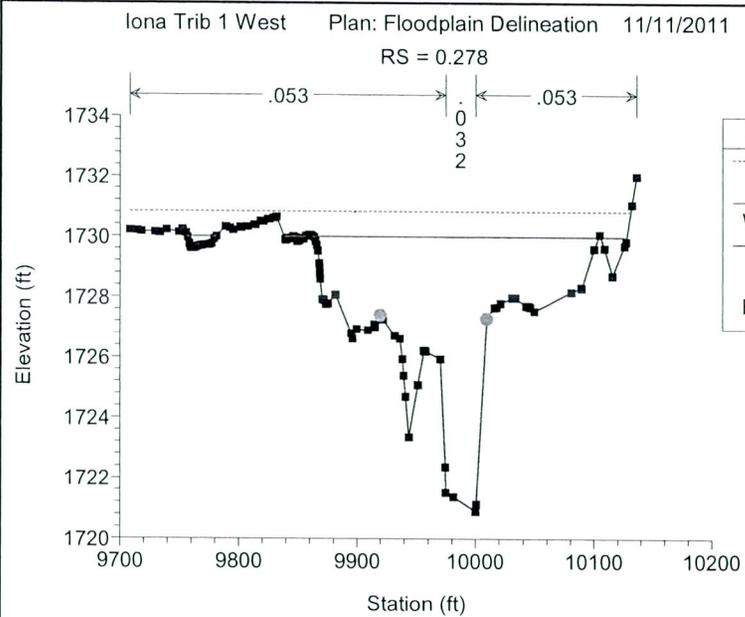






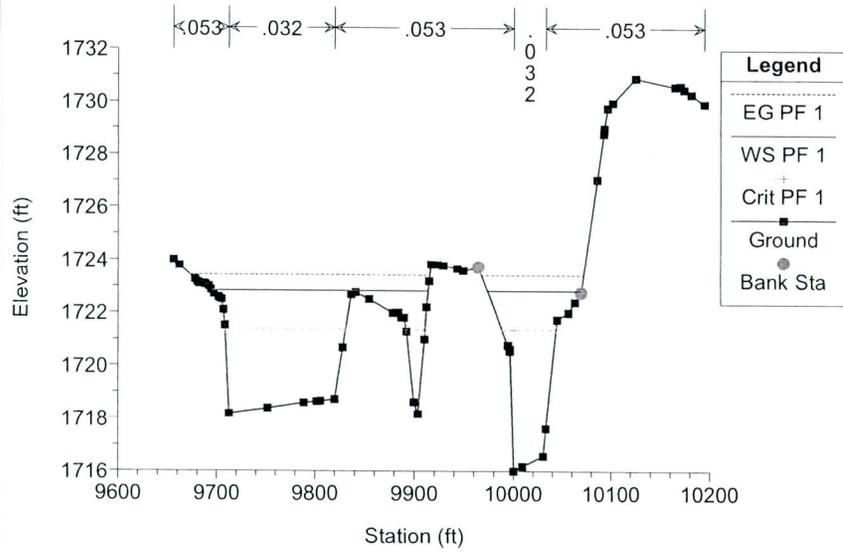


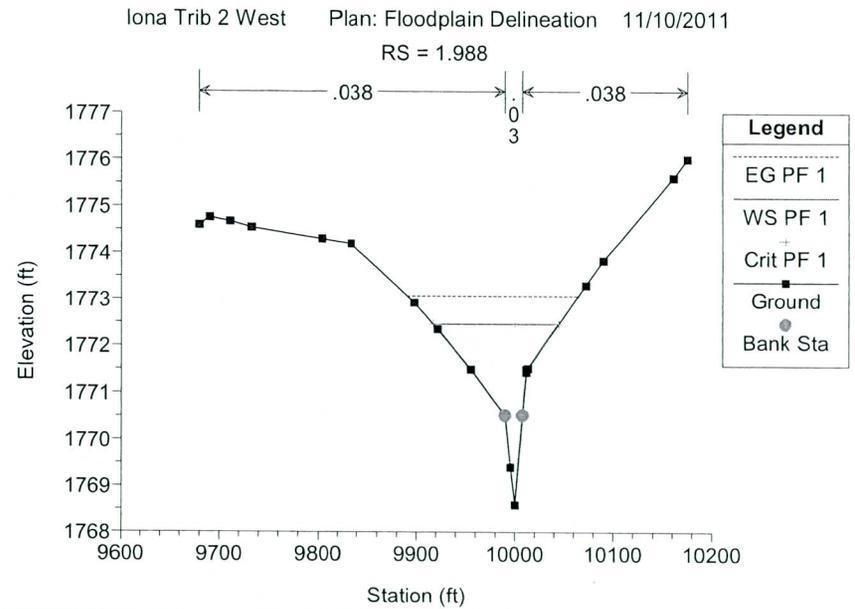
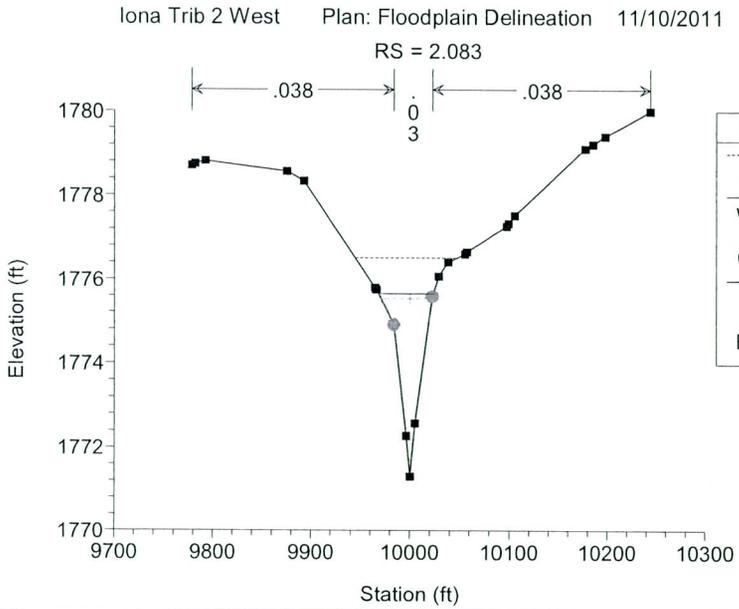
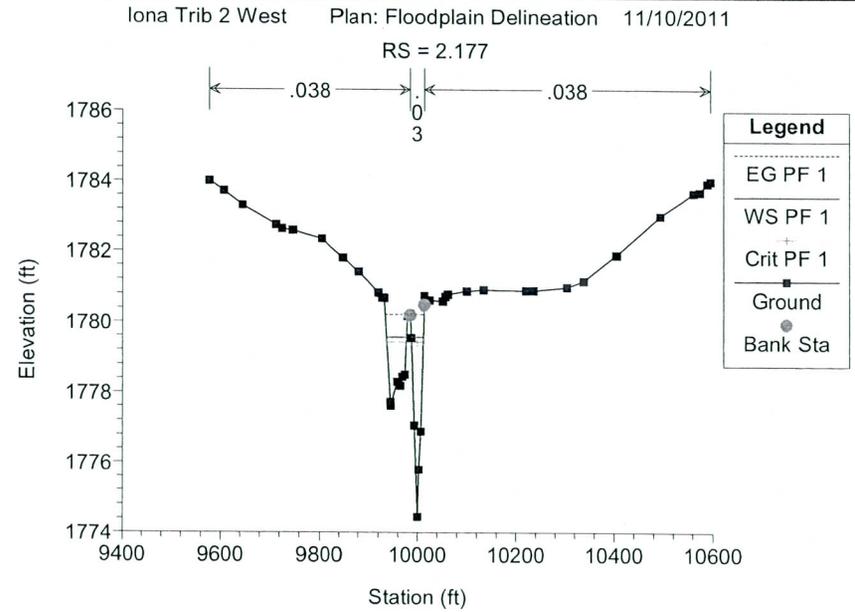
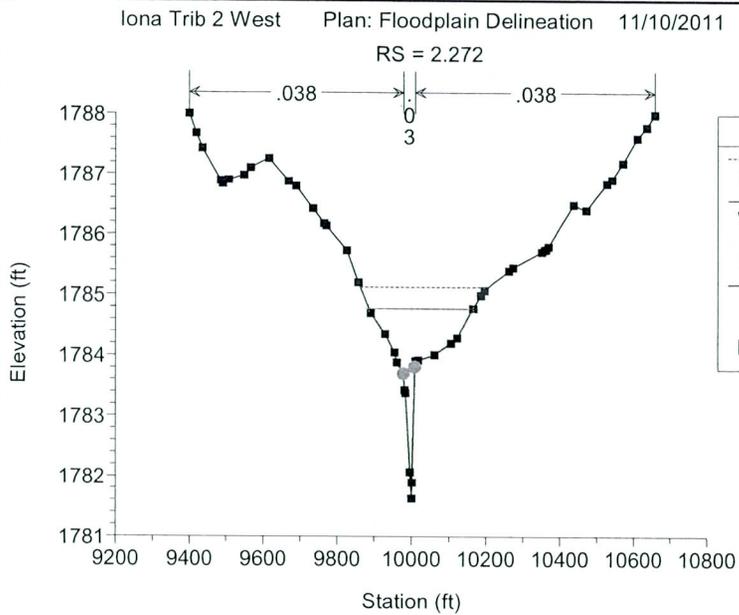


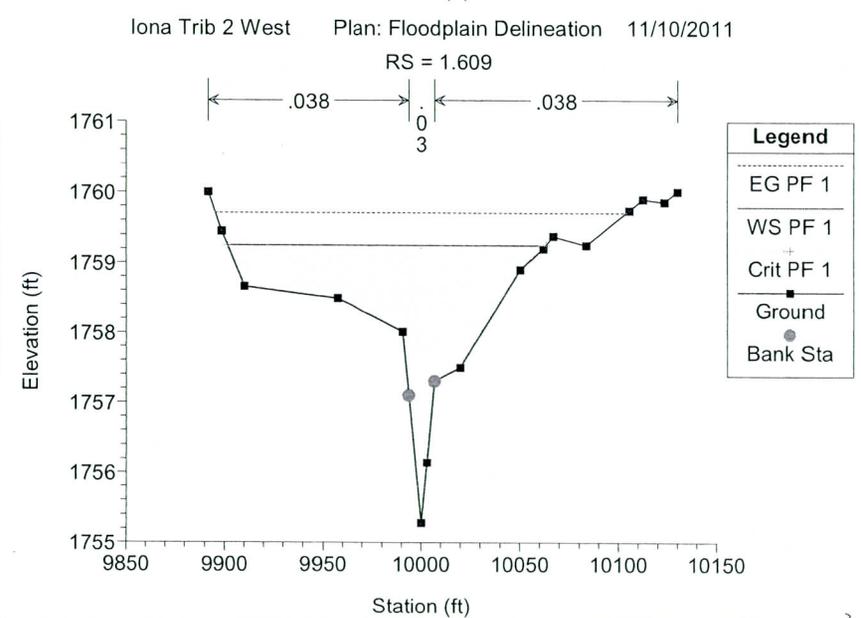
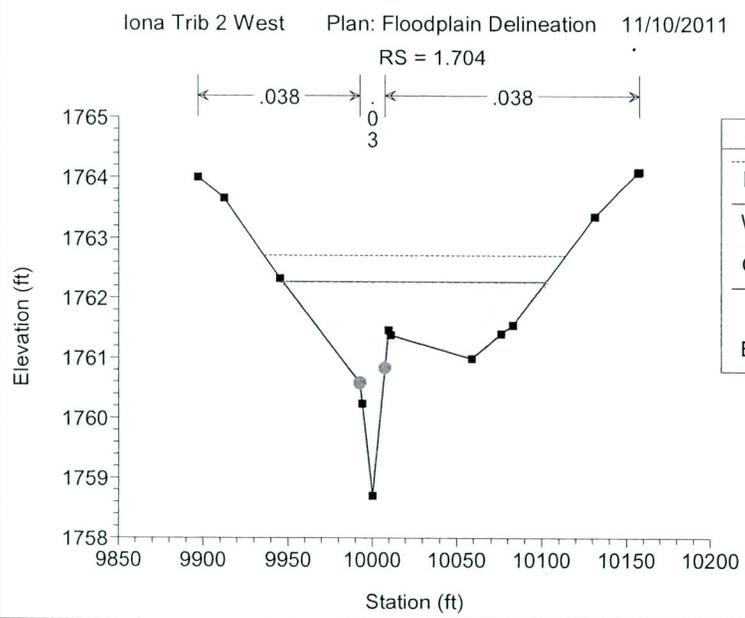
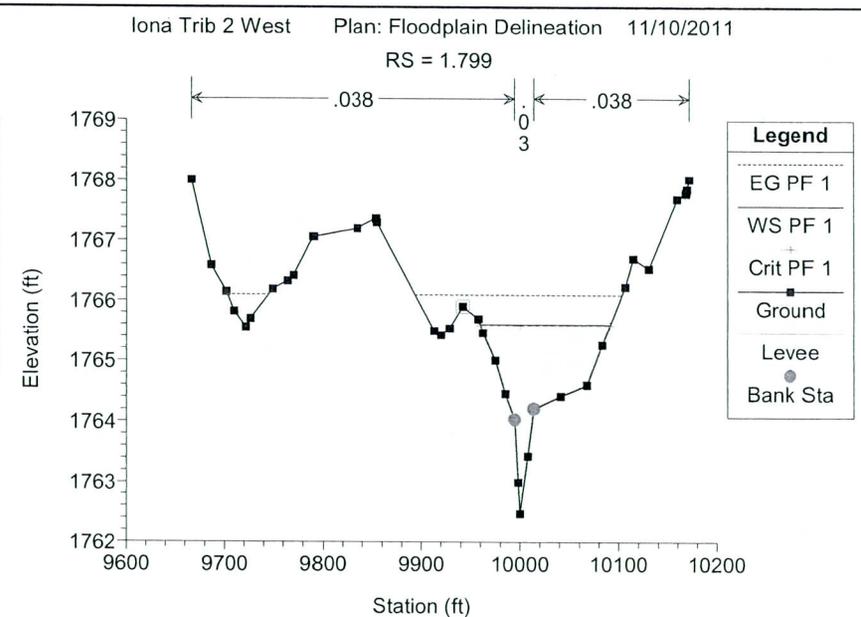
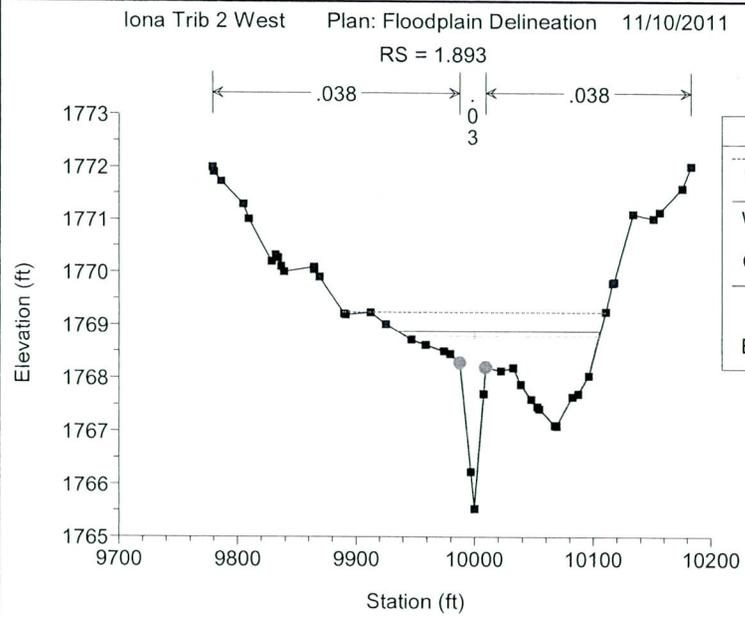


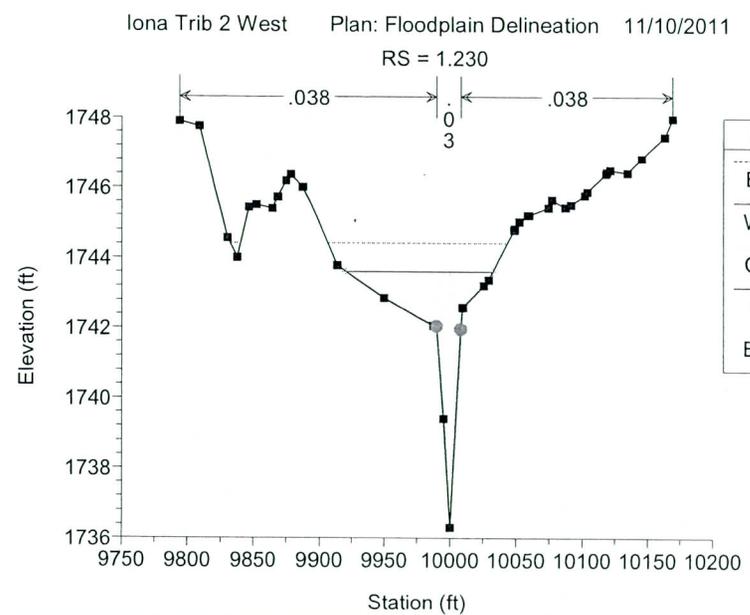
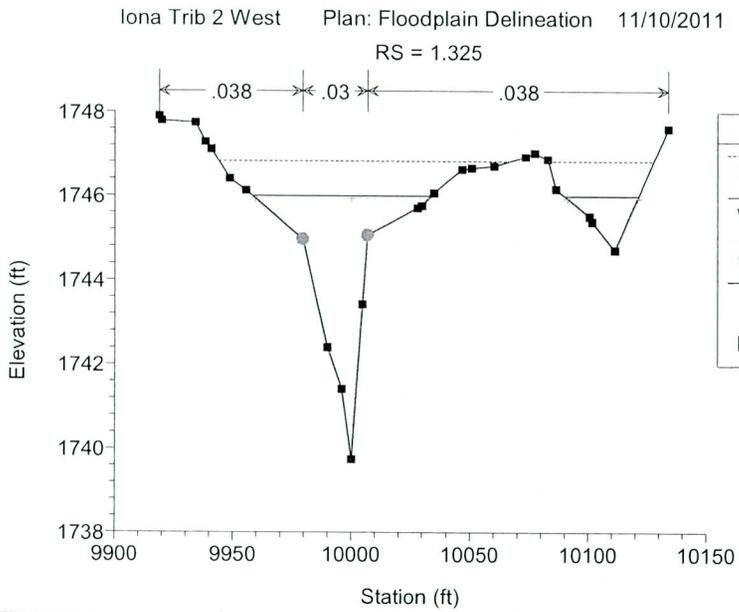
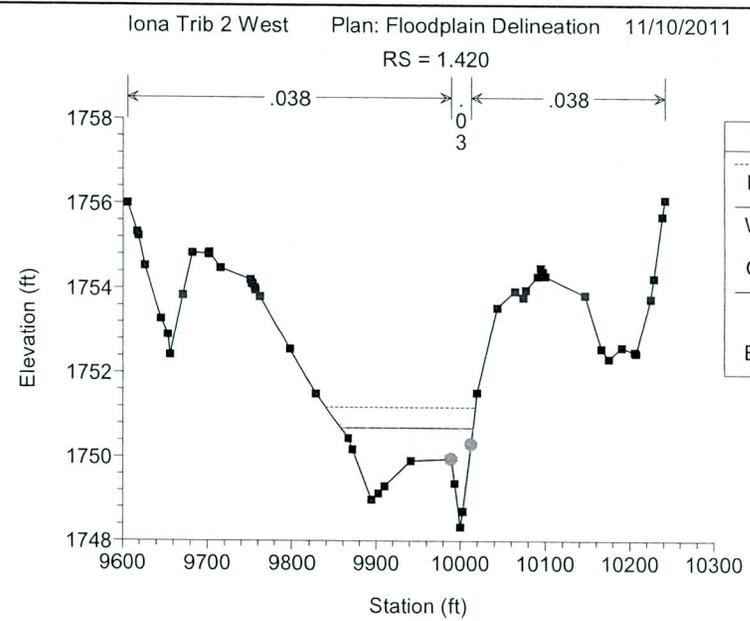
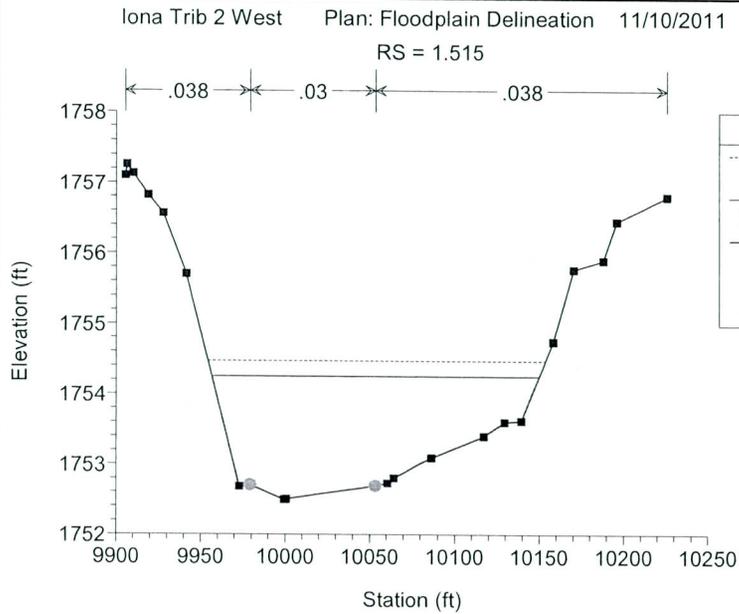
Iona Trib 1 West Plan: Floodplain Delineation 11/11/2011

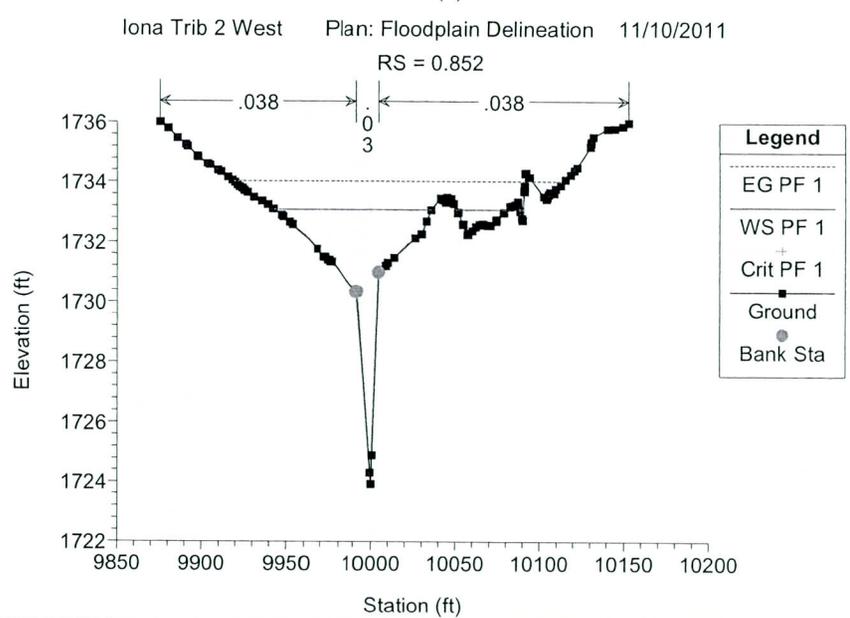
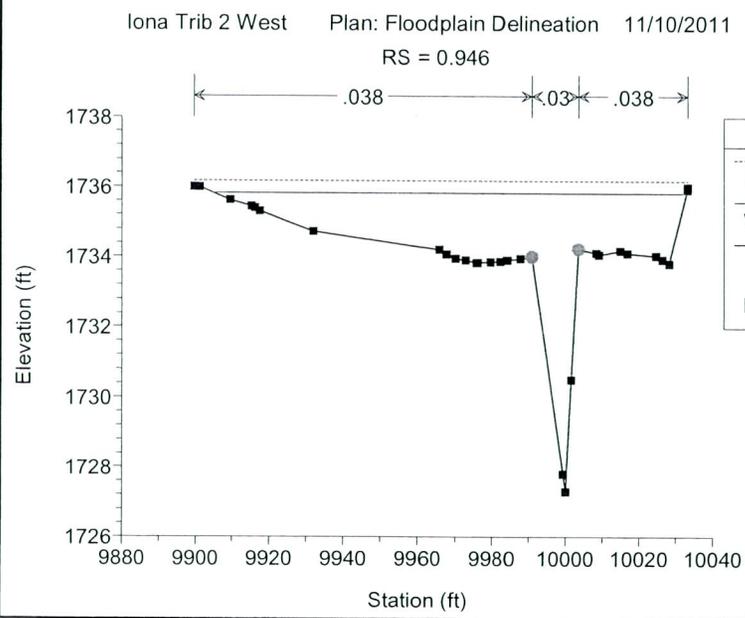
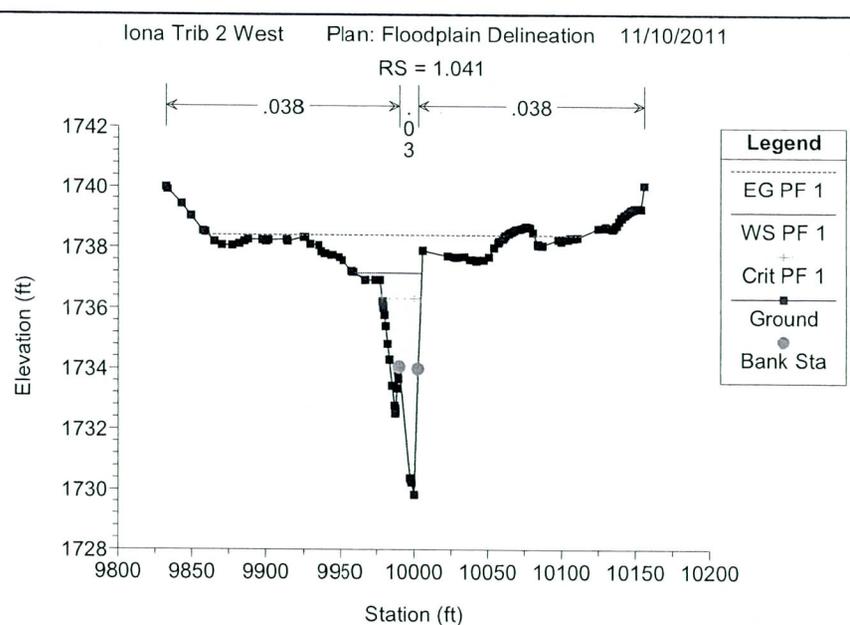
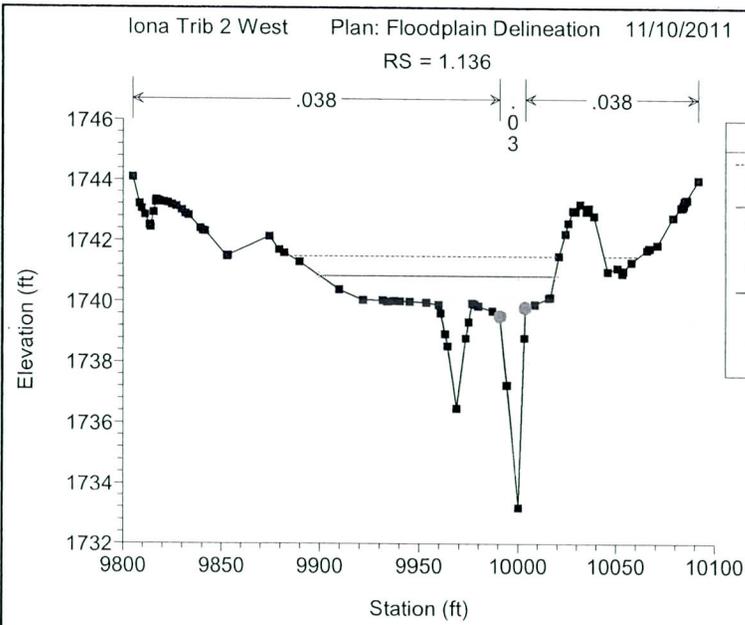
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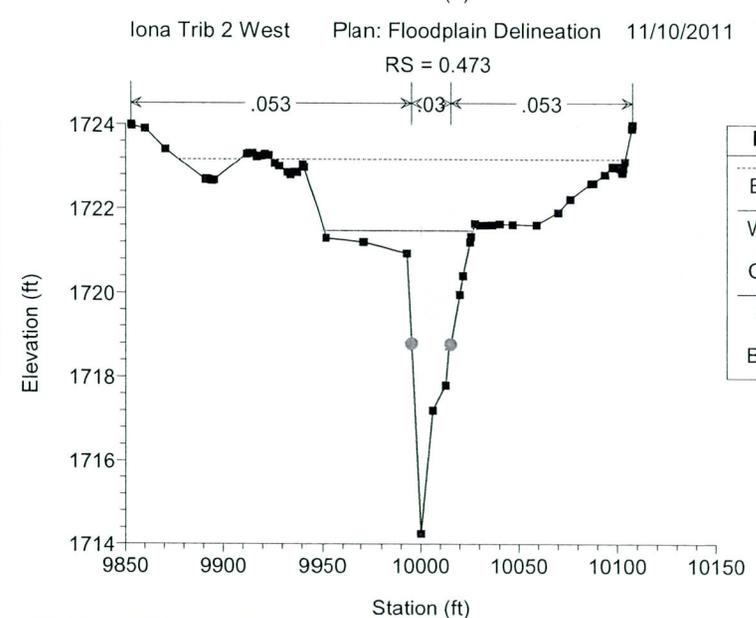
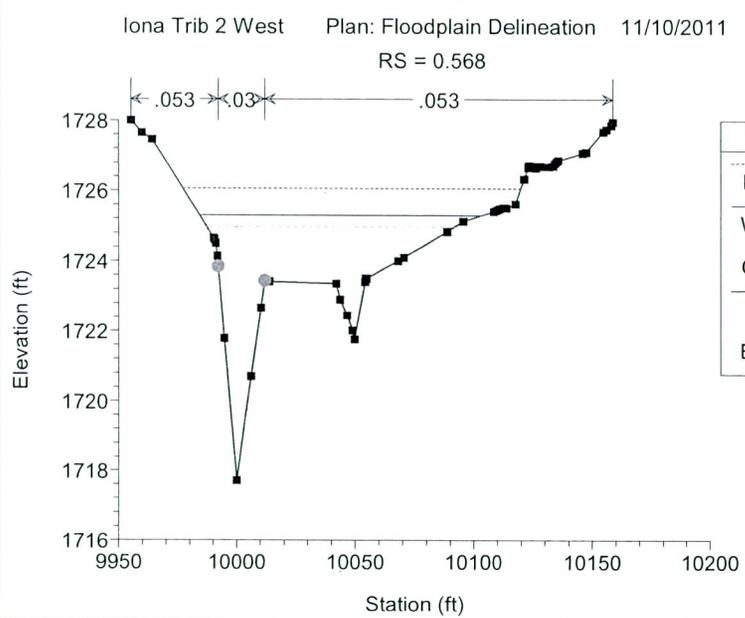
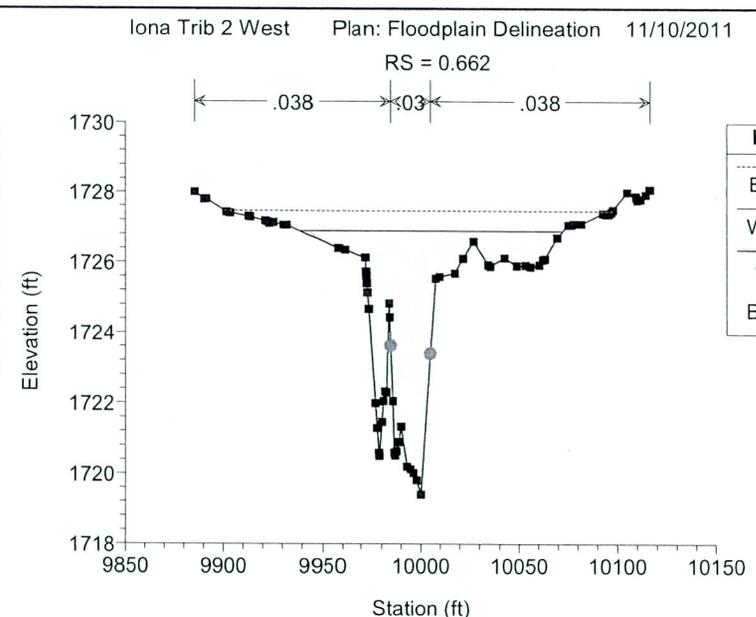
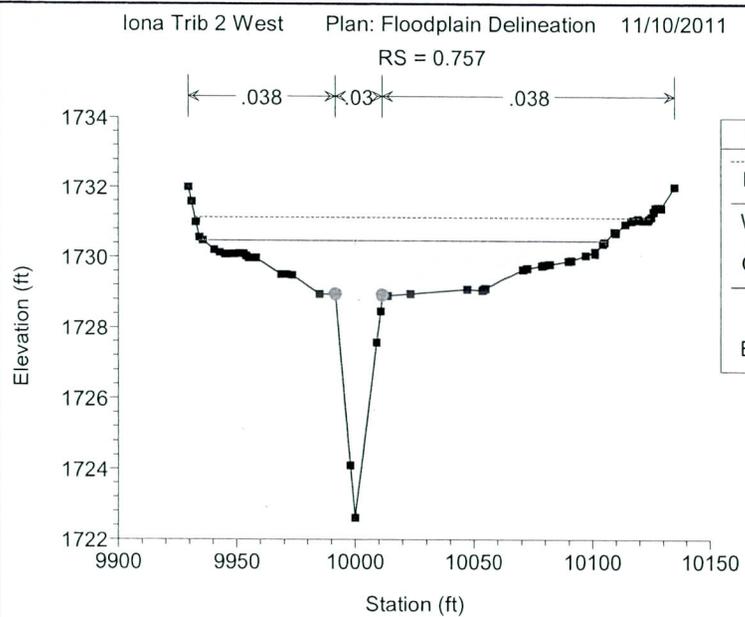


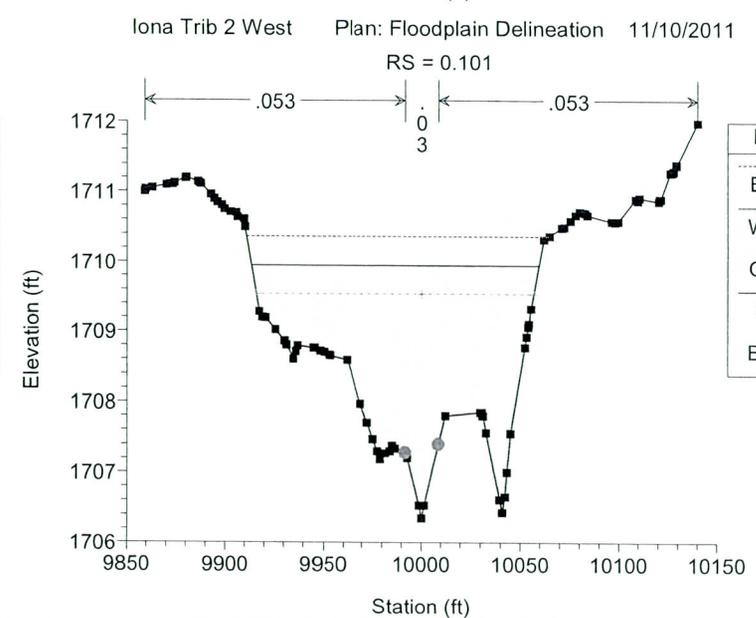
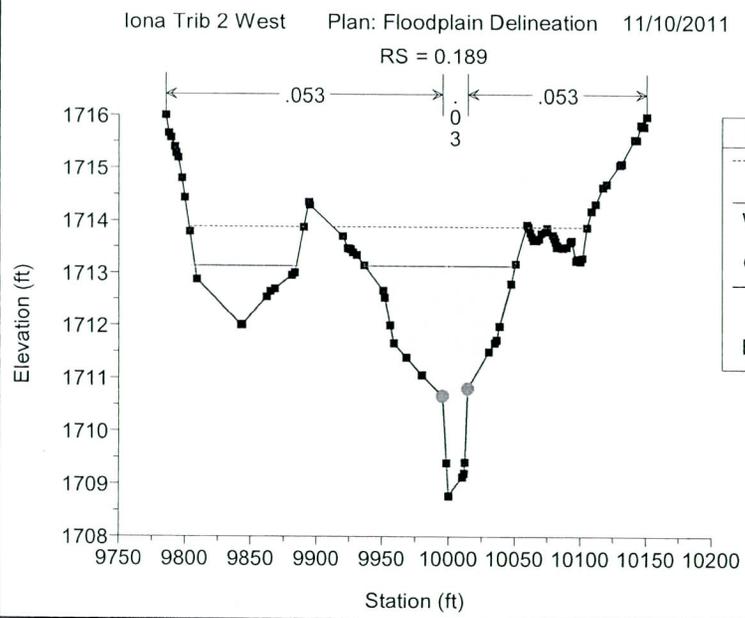
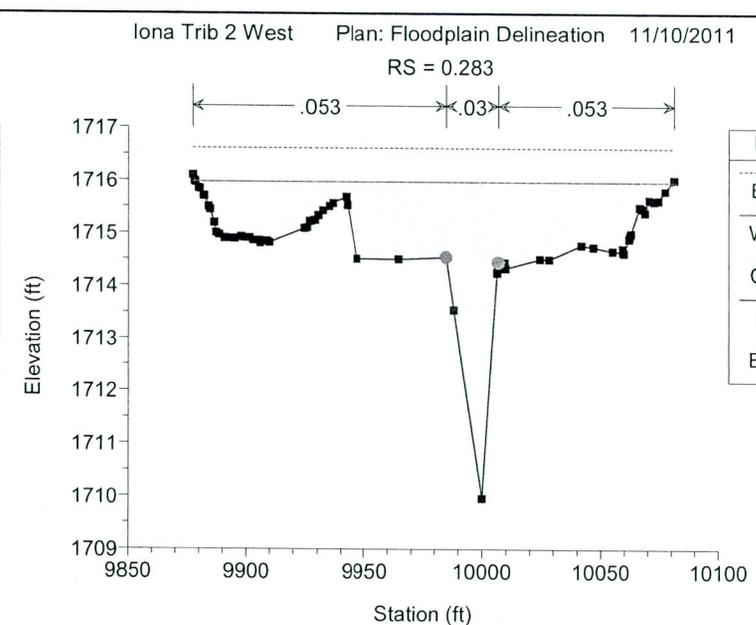
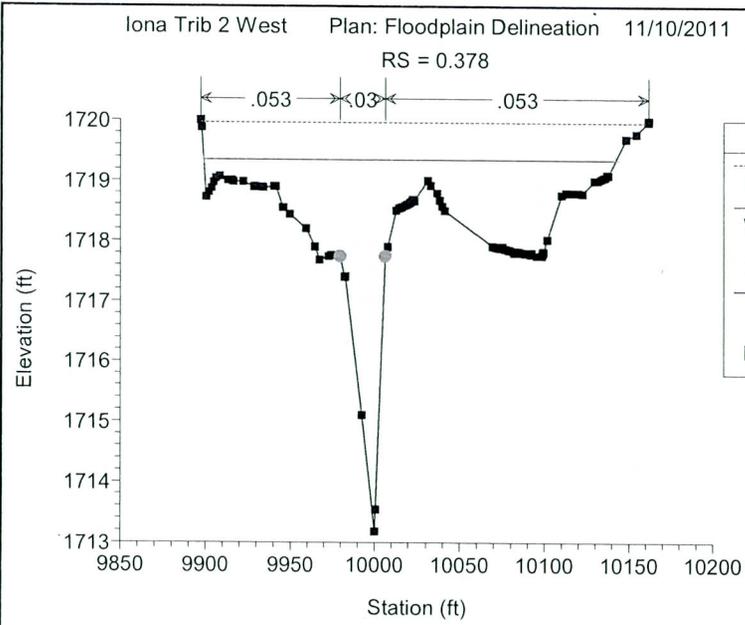














**E.3 Expansion and Contraction Coefficient**



**E.4 Analysis of Structures**

(Not Applicable to This Study)



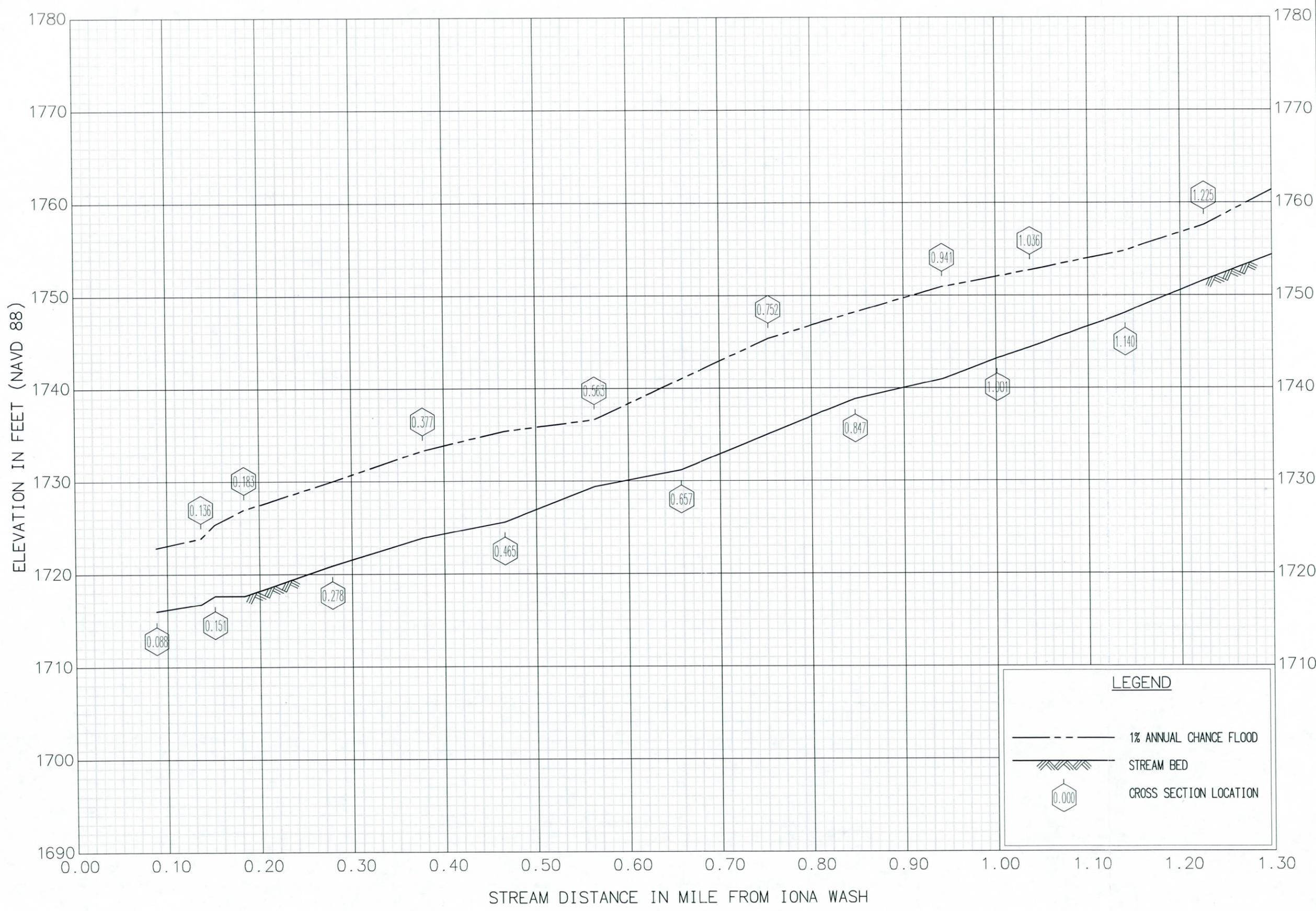
## **E.5 Hydraulic Calculations**

HEC-RAS Plan: FP River: T5N-R3W-S17-1W Reach: 1 West Profile: PF 1

Reach	River Sta	Profile	Q Total (cfs)	Min Ch El (ft)	W.S. Elev (ft)	Crit W.S. (ft)	E.G. Elev (ft)	E.G. Slope (ft/ft)	Vel Chnl (ft/s)	Flow Area (sq ft)	Top Width (ft)	Froude # Chl
1 West	4.161	PF 1	2334.00	1854.77	1856.85	1856.55	1857.05	0.010530	3.43	660.28	688.75	0.81
1 West	4.066	PF 1	2334.00	1847.70	1852.04	1851.97	1852.36	0.008346	6.65	716.89	938.69	0.87
1 West	3.972	PF 1	2334.00	1844.83	1847.73	1847.67	1848.09	0.010258	6.18	592.87	720.89	0.93
1 West	3.900	PF 1	2334.00	1843.10	1845.23	1844.88	1845.40	0.007805	4.76	800.81	920.04	0.79
1 West	3.761	PF 1	2334.00	1837.80	1840.61		1840.77	0.005853	5.50	857.25	826.35	0.73
1 West	3.688	PF 1	2334.00	1833.63	1836.46	1836.27	1836.74	0.012016	7.66	607.22	625.60	1.03
1 West	3.609	PF 1	2334.00	1831.57	1834.08	1833.63	1834.21	0.005230	2.95	855.69	886.27	0.47
1 West	3.488	PF 1	2334.00	1827.52	1831.10	1830.85	1831.24	0.006845	5.12	887.78	1039.43	0.61
1 West	3.371	PF 1	2334.00	1825.20	1828.33		1828.47	0.006687	3.36	807.16	864.04	0.61
1 West	3.309	PF 1	2334.00	1821.51	1825.32		1825.61	0.010853	4.27	543.87	479.35	0.65
1 West	3.214	PF 1	2334.00	1818.80	1821.58	1820.80	1821.76	0.005199	2.44	695.26	467.17	0.40
1 West	3.119	PF 1	2334.00	1814.78	1818.53	1818.11	1818.79	0.007270	6.62	688.20	574.83	0.71
1 West	3.025	PF 1	2334.00	1809.71	1815.98	1815.68	1816.25	0.003572	5.11	870.14	797.39	0.50
1 West	2.930	PF 1	2334.00	1808.22	1813.02	1813.02	1813.57	0.009819	9.88	628.62	579.96	0.99
1 West	2.761	PF 1	2334.00	1799.06	1807.75		1808.10	0.003193	5.17	645.47	375.26	0.40
1 West	2.741	PF 1	2334.00	1798.58	1807.41		1807.77	0.002756	5.19	672.71	509.61	0.39
1 West	2.646	PF 1	3423.00	1794.82	1804.22	1804.22	1804.92	0.010527	9.63	712.94	583.31	0.73
1 West	2.551	PF 1	3423.00	1792.51	1802.07		1802.25	0.002620	4.37	1107.48	635.41	0.35
1 West	2.456	PF 1	3423.00	1790.13	1799.63	1799.36	1800.23	0.008465	7.41	753.26	599.76	0.63
1 West	2.362	PF 1	3561.00	1787.93	1796.93	1796.71	1797.28	0.004090	6.28	1178.28	943.45	0.49
1 West	2.267	PF 1	3561.00	1786.00	1794.05	1793.71	1794.44	0.007958	5.04	716.39	585.68	0.74
1 West	2.172	PF 1	3561.00	1781.88	1790.89	1790.30	1791.11	0.005654	4.52	1101.83	788.31	0.47
1 West	2.078	PF 1	3561.00	1778.95	1787.08	1787.08	1787.64	0.008514	8.35	881.59	691.74	0.65
1 West	2.031	PF 1	3561.00	1777.57	1785.51	1785.51	1786.16	0.004929	9.28	1010.91	738.62	0.73
1 West	1.983	PF 1	3561.00	1776.10	1783.70	1783.20	1784.08	0.003847	5.92	998.45	663.40	0.59
1 West	1.918	PF 1	3561.00	1774.60	1780.86	1780.86	1781.60	0.008959	10.53	748.96	459.26	0.94
1 West	1.827	PF 1	3561.00	1771.70	1778.24		1778.43	0.002112	3.87	1182.72	628.04	0.31
1 West	1.776	PF 1	3561.00	1769.10	1777.23		1777.59	0.006108	5.04	747.44	363.08	0.50
1 West	1.689	PF 1	3561.00	1766.65	1774.41		1775.01	0.004770	6.19	587.13	238.96	0.50
1 West	1.604	PF 1	3561.00	1765.52	1772.76		1773.02	0.003774	4.99	1180.67	769.45	0.43
1 West	1.538	PF 1	5467.00	1762.61	1769.85	1769.85	1770.78	0.009465	8.63	911.98	577.75	0.71
1 West	1.439	PF 1	5467.00	1759.10	1766.95		1767.21	0.001920	4.20	1446.31	526.98	0.31
1 West	1.362	PF 1	5467.00	1756.71	1764.54	1763.44	1765.57	0.011922	8.40	721.76	313.95	0.74
1 West	1.225	PF 1	5467.00	1751.58	1757.58		1758.36	0.008239	7.08	771.70	249.42	0.71
1 West	1.140	PF 1	5467.00	1748.07	1754.78		1755.47	0.004915	6.85	849.26	244.74	0.60
1 West	1.036	PF 1	5467.00	1744.39	1752.73		1753.26	0.003315	5.83	937.55	179.44	0.45
1 West	1.001	PF 1	5467.00	1743.20	1752.01		1752.56	0.004334	5.92	922.78	171.52	0.45
1 West	0.941	PF 1	5467.00	1740.96	1750.90		1751.38	0.003094	5.84	1132.29	349.46	0.40

HEC-RAS Plan: FP River: T5N-R3W-S17-2W Reach: Reach 1 Profile: PF 1

Reach	River Sta	Profile	Q Total (cfs)	Min Ch El (ft)	W.S. Elev (ft)	Crit W.S. (ft)	E.G. Elev (ft)	E.G. Slope (ft/ft)	Vel Chnl (ft/s)	Flow Area (sq ft)	Top Width (ft)	Froude # Chl
Reach 1	2.272	PF 1	652.00	1781.63	1784.78	1784.78	1785.12	0.005754	5.99	201.20	279.36	0.74
Reach 1	2.177	PF 1	652.00	1774.44	1779.58	1779.43	1780.22	0.006772	7.29	114.55	65.15	0.80
Reach 1	2.083	PF 1	652.00	1771.30	1775.66	1775.54	1776.51	0.007980	7.46	91.84	54.87	0.88
Reach 1	1.988	PF 1	652.00	1768.59	1772.46	1772.46	1773.07	0.005812	7.61	144.63	127.62	0.78
Reach 1	1.893	PF 1	652.00	1765.53	1768.88	1768.78	1769.25	0.006757	6.34	163.11	170.64	0.79
Reach 1	1.799	PF 1	652.00	1762.47	1765.61	1765.59	1766.10	0.007185	7.25	148.02	131.79	0.84
Reach 1	1.704	PF 1	652.00	1758.70	1762.27	1762.25	1762.71	0.006544	7.35	164.29	155.77	0.81
Reach 1	1.609	PF 1	652.00	1755.28	1759.25	1759.25	1759.71	0.005555	7.50	170.75	161.61	0.76
Reach 1	1.515	PF 1	811.00	1752.49	1754.25		1754.47	0.003860	4.30	240.18	193.19	0.59
Reach 1	1.420	PF 1	811.00	1748.33	1750.70	1750.70	1751.18	0.013805	7.49	159.69	158.19	1.09
Reach 1	1.325	PF 1	811.00	1739.74	1745.99	1745.94	1746.82	0.005122	7.66	138.77	106.74	0.72
Reach 1	1.230	PF 1	811.00	1736.30	1743.61	1743.61	1744.42	0.004587	8.01	154.32	112.57	0.68
Reach 1	1.136	PF 1	811.00	1733.19	1740.85	1740.85	1741.50	0.006115	8.12	168.31	119.06	0.68
Reach 1	1.041	PF 1	811.00	1729.83	1737.16	1736.33	1738.40	0.005319	9.65	112.07	44.98	0.73
Reach 1	0.946	PF 1	811.00	1727.29	1735.82		1736.17	0.002991	6.13	221.52	127.93	0.48
Reach 1	0.852	PF 1	1074.00	1723.92	1733.08	1733.08	1734.03	0.005710	9.36	189.90	124.40	0.68
Reach 1	0.757	PF 1	1074.00	1722.61	1730.47	1730.47	1731.13	0.003853	7.73	238.80	170.01	0.63
Reach 1	0.662	PF 1	1074.00	1719.40	1726.90		1727.46	0.002052	6.75	249.99	134.08	0.48
Reach 1	0.568	PF 1	1074.00	1717.71	1725.30	1724.96	1726.05	0.004388	8.20	229.50	119.54	0.67
Reach 1	0.473	PF 1	1074.00	1714.25	1721.47	1721.47	1723.18	0.007253	10.77	125.20	75.96	0.88
Reach 1	0.378	PF 1	1074.00	1713.18	1719.35	1719.35	1719.96	0.004343	7.48	292.38	243.15	0.68
Reach 1	0.283	PF 1	1074.00	1709.96	1715.98	1715.98	1716.61	0.005078	7.99	279.56	201.98	0.73
Reach 1	0.189	PF 1	1074.00	1708.77	1713.14	1713.14	1713.89	0.005466	8.75	257.19	190.45	0.79
Reach 1	0.101	PF 1	1074.00	1706.35	1709.95	1709.54	1710.37	0.005203	7.55	280.04	146.12	0.76

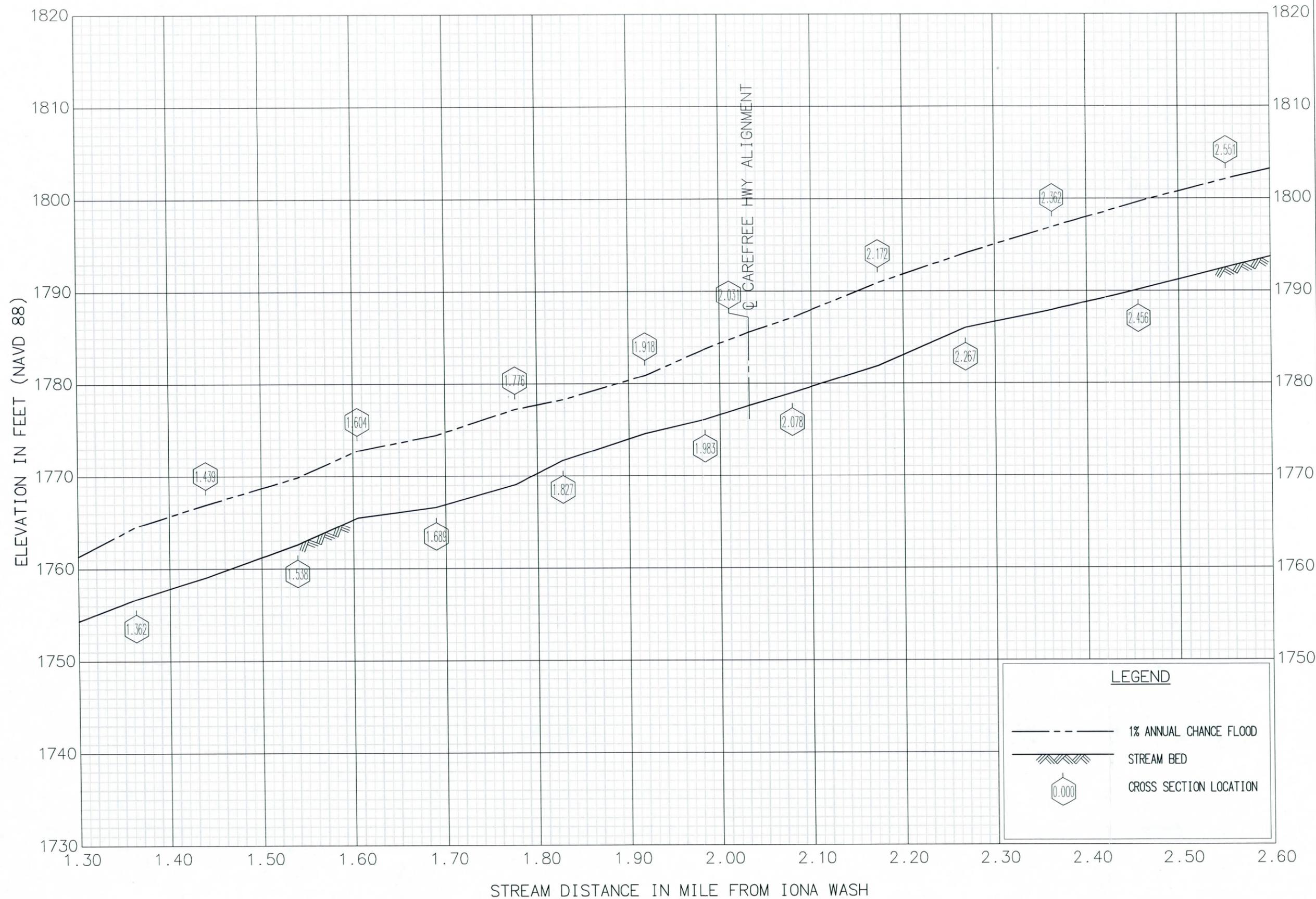


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- CROSS SECTION LOCATION

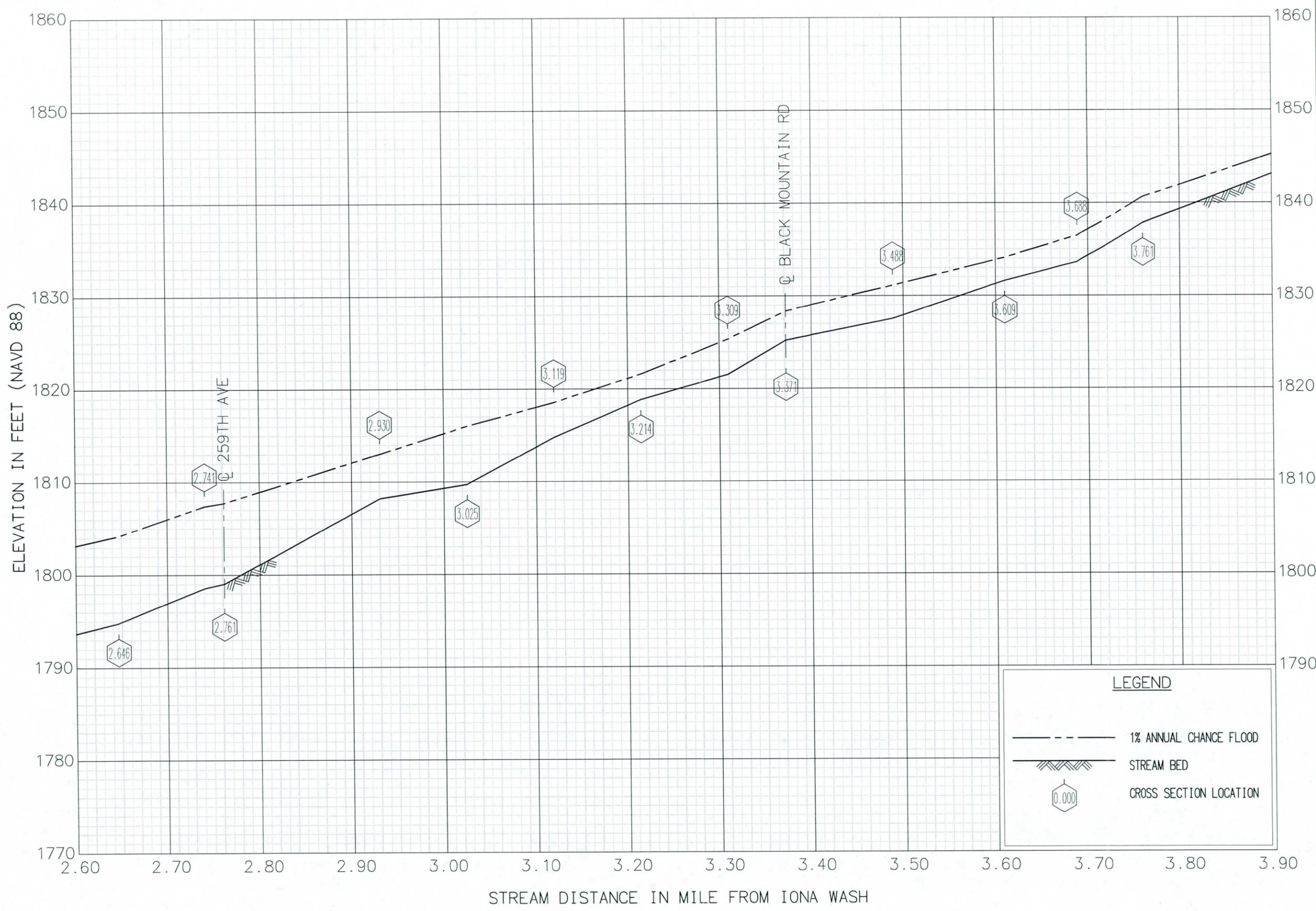
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**MARICOPA COUNTY, AZ**  
 MARICOPA

FLOOD PROFILES  
**IONA TRIBUTARY 1 WEST (T5NR3WS17-1W)**



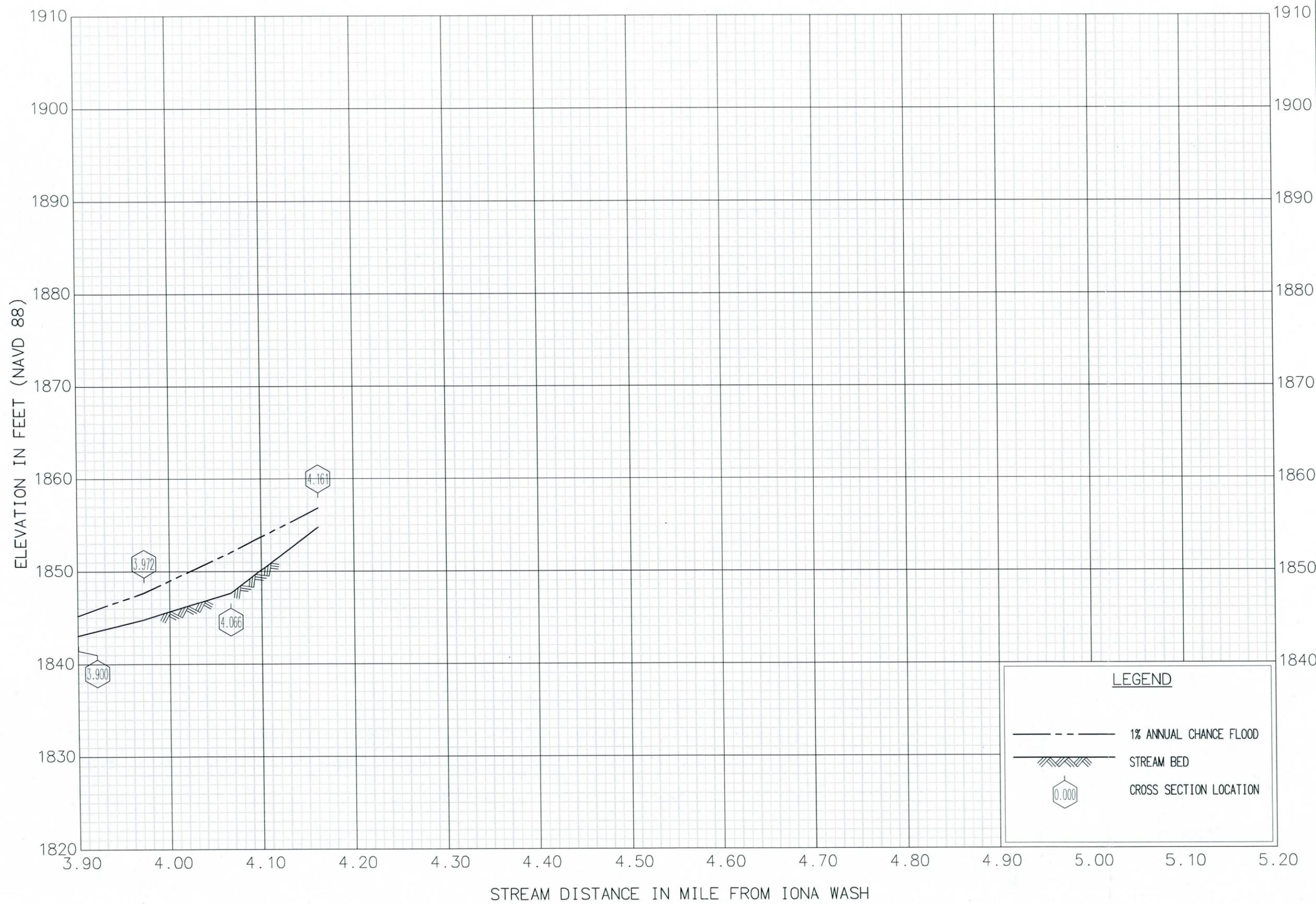
FEDERAL EMERGENCY MANAGEMENT AGENCY  
**MARICOPA COUNTY, AZ**  
 MARICOPA

FLOOD PROFILES  
 IONA TRIBUTARY 1 WEST (T5NR3WS17-1W)



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 MARICOPA

FLOOD PROFILES  
 IONA TRIBUTARY 1 WEST (T5NR3WS17-1W)

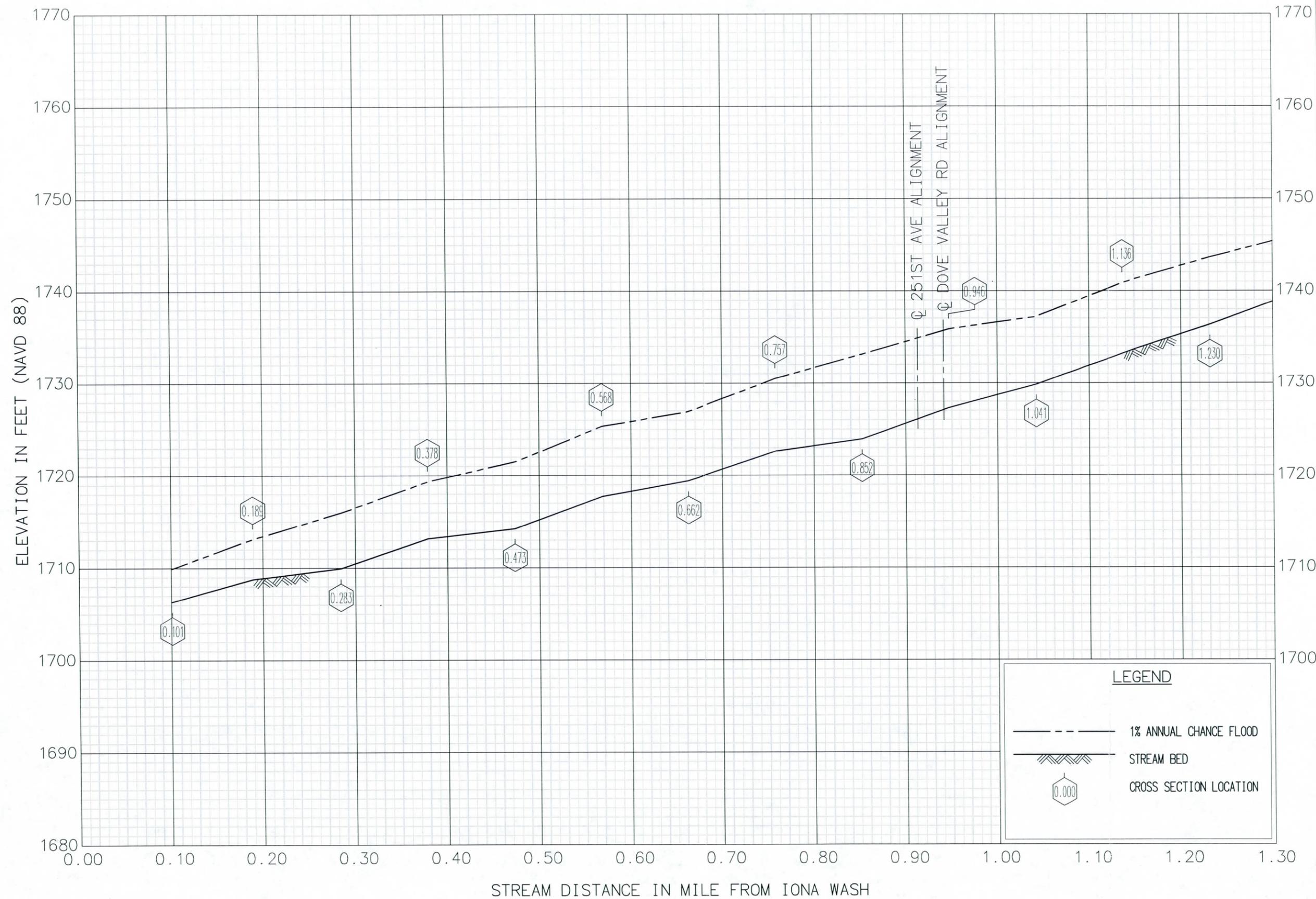


LEGEND

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 MARICOPA

FLOOD PROFILES  
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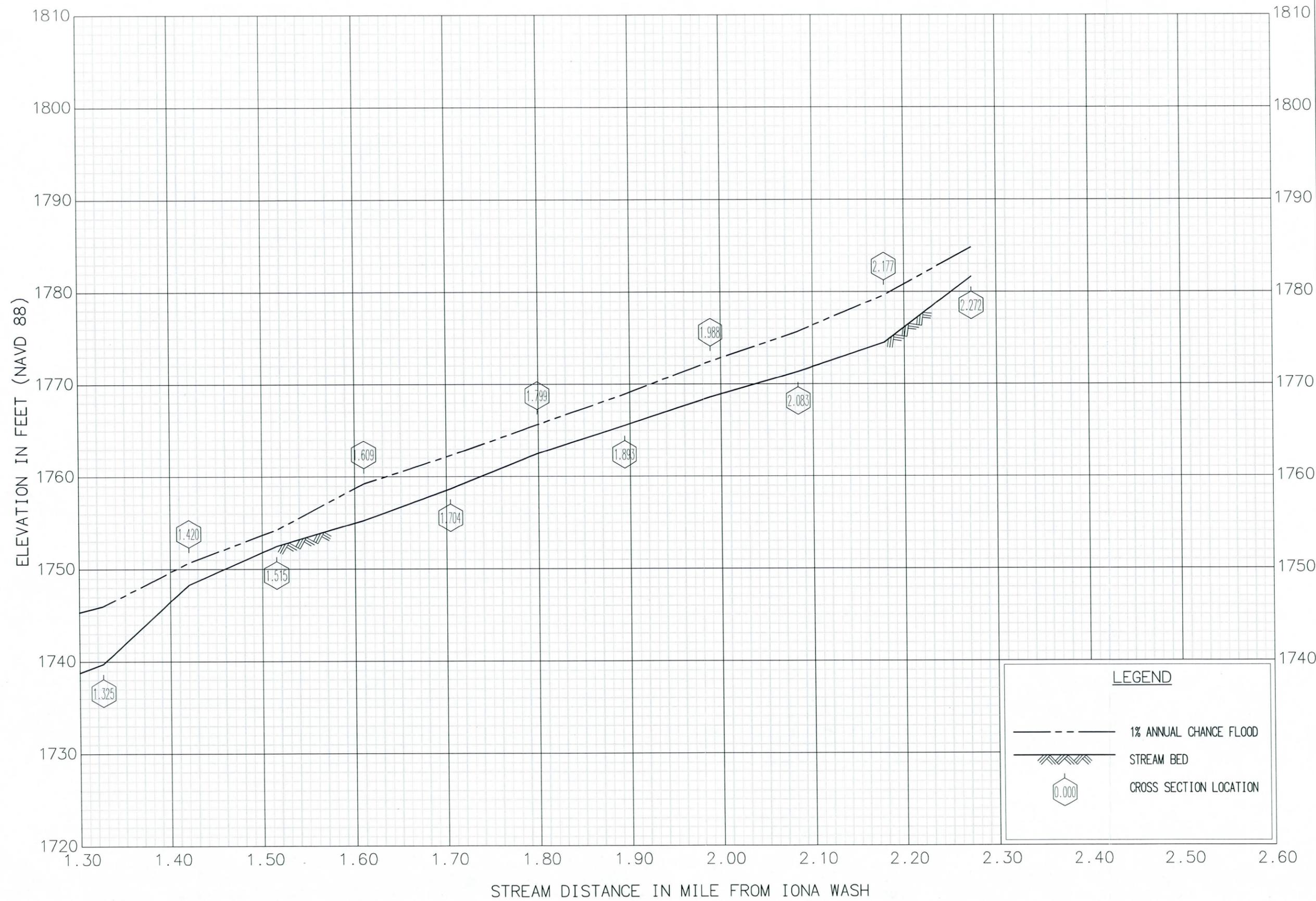


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FEDERAL EMERGENCY MANAGEMENT AGENCY  
**MARICOPA COUNTY, AZ**  
 MARICOPA

FLOOD PROFILES  
**IONA TRIBUTARY 2 WEST (T5NR3WS17-2W)**



FEDERAL EMERGENCY MANAGEMENT AGENCY  
**MARICOPA COUNTY, AZ**  
 MARICOPA

FLOOD PROFILES  
**IONA TRIBUTARY 2 WEST (T5NR3WS17-2W)**



# Flood Control District of Maricopa County

INTEROFFICE MEMORANDUM

**Date:** November 28, 2011  
**To:** Timothy S. Phillips, P.E., Chief Engineer and General Manager  
**From:** Jonathan Lesperance, Hydrology and Hydraulics Branch  
**Subject:** Wittmann Phase IV Floodplain Delineation Study TDN, Contract FCD 2011C003

The floodplain study for the Wittmann Phase IV FDS is ready for use as the best available technical information. The study documentation will be sent to FEMA for review and incorporation into the County's FIRM panels. Please find attached an MT-2 form requesting your signature.

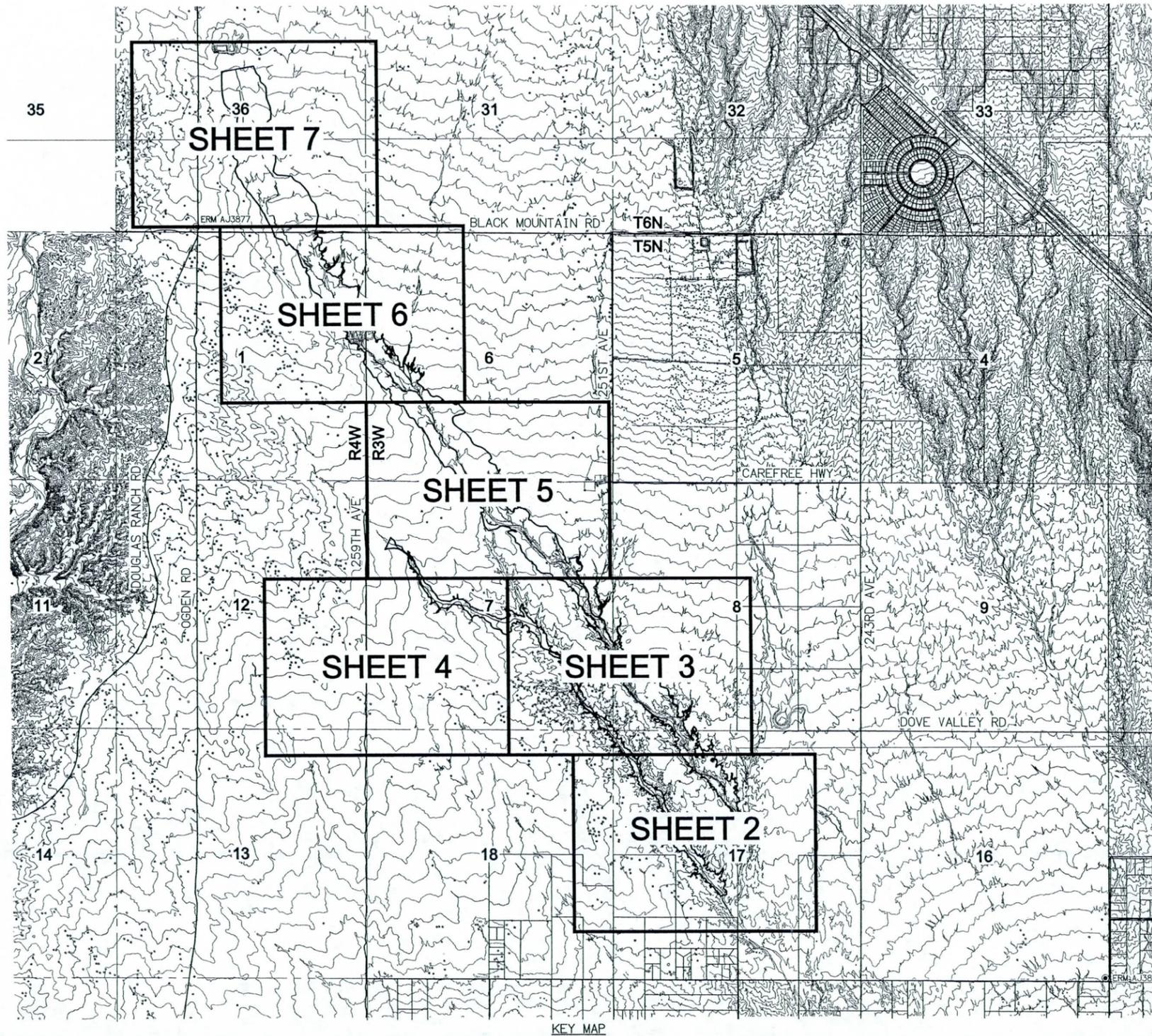
The background for the study includes the following:

The study delineates approximately 6.5 linear miles of limited detail Zone AE floodplain (no floodway). The topographic basis for the study is 2 and 4-foot contour interval mapping in NAVD88 vertical datum by Landata Airborne Systems, Inc. The photography for topographic mapping of the study area was flown April 23, 2002. The study Consultant was RBF Consulting. The project manager for the Consultant was Nathan Ford, P.E. The project manager for the District was Jonathan Lesperance.

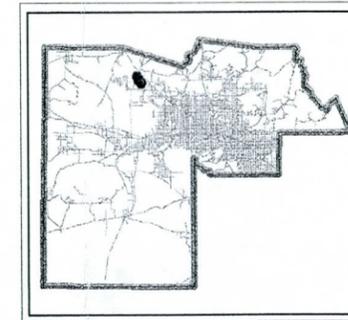
Please concur and authorize below the use of this new study.

Date: 11/28/11 Jonathan Lesperance Project Manager	Date: [Signature] 12/7/11 Timothy S. Phillips, P.E. Chief Engineer and General Manager
Date: 12/5/11 Amir Motamedi, P.E. Hydrology/Hydraulics Branch Manager	Date: [Signature] 12/5/2011 Ed Raleigh, P.E. Engineering Division Manager
Date: 12/6/2011 Kelli Seftich Floodplain Management & Services Division Manager	Date: _____
File Copies: 1. _____ 2. _____	<input checked="" type="checkbox"/> YES <input type="checkbox"/> GIS Posted (Pending Floodplain Only)      Date: _____ <input checked="" type="checkbox"/> NO GIS In Progress

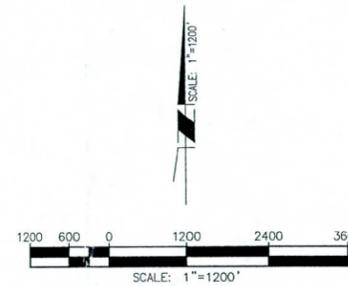
FLOODPLAIN WORK MAP  
FOR  
**WITTMANN PHASE IV**  
**FLOODPLAIN DELINEATION STUDY**  
CONTRACT FCD 2011C003



FLOOD CONTROL DISTRICT  
OF MARICOPA COUNTY



WITTMANN PHASE IV  
FLOODPLAIN DELINEATION STUDY



NOTE: WITTMANN PHASE IV FLOODPLAIN DELINEATION STUDY  
INCLUDES IONA TRIBUTARY 1 WEST (T5NR3WS17-1W), IONA  
TRIBUTARY 1 WEST EXTENSION (T5NR3WS17-1WE) & IONA  
TRIBUTARY 2 WEST (T5NR3WS17-2W).

**SHEET INDEX**

- 1 COVER SHEET
- 2-7 FLOODPLAIN WORK MAP

**ENGINEER**

RBF CONSULTING  
NATHAN E. FORD, P.E.  
16605 N. 28TH AVENUE, STE 100  
PHOENIX, ARIZONA 85053  
PHONE (602) 467-2200  
FAX (602) 467-2201

**DATUM**

HORIZONTAL: NORTH AMERICAN  
DATUM 1983  
VERTICAL: NORTH AMERICAN  
VERTICAL DATUM 1988 (NAVD88)

**ELEVATION REFERENCE MARKS**

- ERM AJ3877 (1844.7 FT)
- ERM AJ3873 (1679.9 FT)

NOTE: DATA FOR ELEVATION REFERENCE  
MARKS (ERM) OBTAINED FROM THE NATIONAL  
GEODETIC SURVEY DATA SHEETS AVAILABLE  
AT WWW.NGS.NOAA.GOV

NOTE:  
THIS MAP WAS PREPARED USING 2 AND 4 FT CONTOUR DATA  
PROVIDED BY THE FLOOD CONTROL DISTRICT OF MARICOPA  
COUNTY FROM AERIAL PHOTOGRAPHS FLOWN ON APRIL 2002  
BY LANDATA AIRBORNE SYSTEMS, INC.

**RBF CONSULTING**  
16605 NORTH 28TH AVENUE, SUITE 100  
PHOENIX, ARIZONA 85053-7550  
PHONE (602) 467-2200 • FAX (602) 467-2201 • WWW.RBF.COM

PROJECT NO. FCD 2011C003  
PROJECT NAME WITTMANN PHASE IV  
PLAN TYPE FLOODPLAIN WORK MAP  
SCALE 1" = 1200'

DESIGNED BY NATHAN E. FORD, P.E.  
CHECKED BY NATHAN E. FORD, P.E.  
DRAWN BY NATHAN E. FORD, P.E.  
DATE 11/21/11

NO.	BY	DESCRIPTION	APP'D	DATE

ENGINEER/CONSULTOR  
**FLOOD CONTROL DISTRICT OF MARICOPA COUNTY**

PROJECT NAME  
**WITTMANN PHASE IV**

PLAN TYPE  
**FLOODPLAIN WORK MAP**

SEALED

40064  
NATHAN E. FORD

Expires 12/31/12

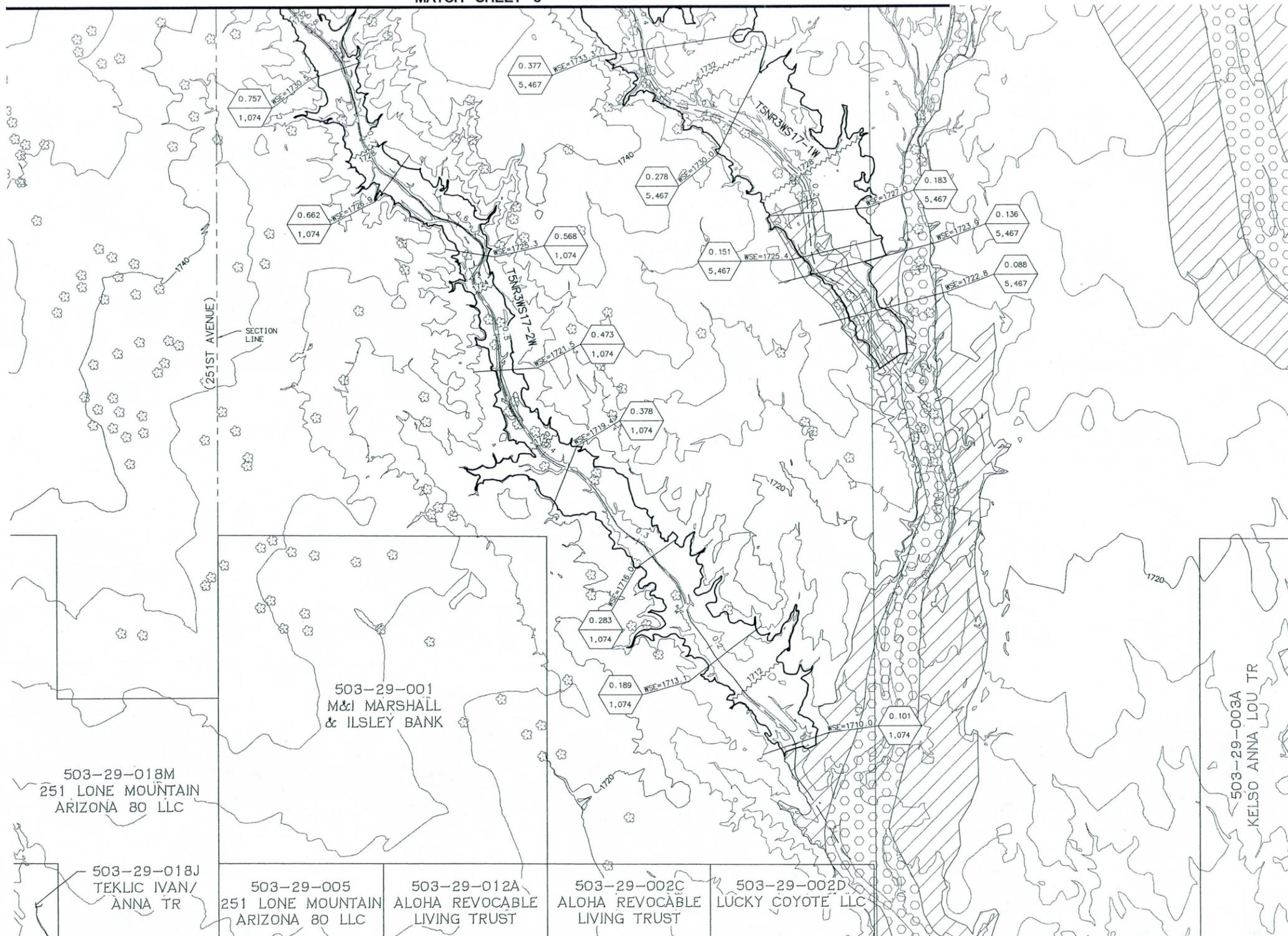
ORIGINAL PLAN DATE  
**NOV/2011**

LATEST PLAN REVISION  
-

SHEET NUMBER  
**1 OF 7**

PROJECT NUMBER  
**45104608**

MATCH SHEET 3



503-29-018M  
251 LONE MOUNTAIN  
ARIZONA 80 LLC

503-29-001  
M&I MARSHALL  
& ILSLEY BANK

503-29-018J  
TEKLIĆ IVAN/  
ANNA TR

503-29-005  
251 LONE MOUNTAIN  
ARIZONA 80 LLC

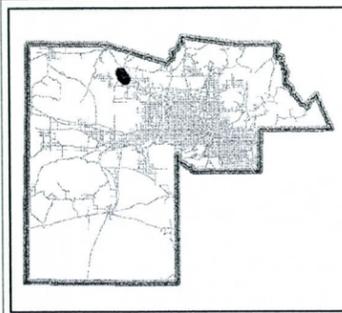
503-29-012A  
ALOHA REVOCABLE  
LIVING TRUST

503-29-002C  
ALOHA REVOCABLE  
LIVING TRUST

503-29-002D  
LUCKY COYOTE LLC

503-29-003A  
KELSO ANNA LOU TR

FLOOD CONTROL DISTRICT  
OF MARICOPA COUNTY



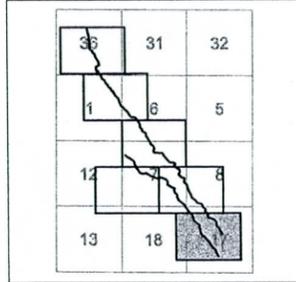
WITTMANN PHASE IV  
FLOODPLAIN DELINEATION STUDY



SCALE: 1"=200'

LEGEND

- EFFECTIVE ZONE AE
- EFFECTIVE FLOODWAY
- HYDRAULIC BASE LINE
- T4N-R2W-S15N WASH I.D.
- STATION RIVER MILE
- CROSS SECTION Q100 (CFS)
- CROSS SECTIONS
- ZONE AE FLOODPLAIN
- EXISTING CONTOUR
- BASE FLOOD ELEVATION



NOTE:  
THIS MAP WAS PREPARED USING 4 FT CONTOUR DATA PROVIDED BY THE FLOOD CONTROL DISTRICT OF MARICOPA COUNTY FROM AERIAL PHOTOGRAPHS FLOWN ON APRIL 2002 BY LANDATA AIRBORNE SYSTEMS, INC.

WITTMANN PHASE IV  
FLOODPLAIN WORK MAP

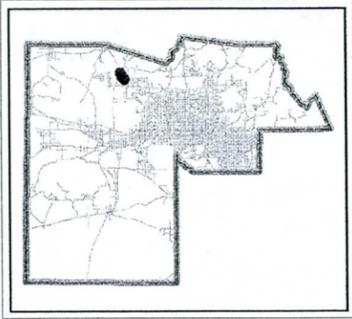
RBF CONSULTING PLANNING ■ DESIGN ■ CONSTRUCTION  
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LAYOUT	DRAFTED	CHECKED
NEF	DLP	NEF
DRAWING SCALE(S)		
1" = 200'		
PLAN DATE	SHEET NO.	
NOV/2011	2	
PROJECT NUMBER	OF 7 SHEETS	
45104608		

H:\PROJECTS\45104608\45104608\DRAWING\WITTMANN\_PHASE\_IV\_4608-SW-002.DWG DPEARCE 11/17/11 11:57 am

FLOOD CONTROL DISTRICT  
OF MARICOPA COUNTY



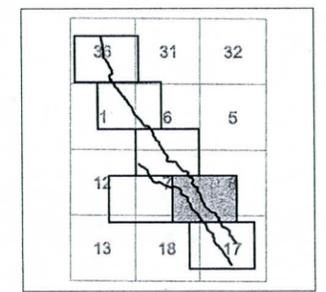
WITTMANN PHASE IV  
FLOODPLAIN DELINEATION STUDY



SCALE: 1"=200'

LEGEND

- EFFECTIVE ZONE AE
- EFFECTIVE FLOODWAY
- HYDRAULIC BASE LINE
- T4N-R2W-S15N WASH I.D.
- STATION RIVER MILE
- CROSS SECTION Q100 (CFS)
- CROSS SECTIONS
- ZONE AE FLOODPLAIN
- 1700 EXISTING CONTOUR
- 1700 BASE FLOOD ELEVATION



NOTE:  
THIS MAP WAS PREPARED USING 4 FT CONTOUR DATA PROVIDED BY THE FLOOD CONTROL DISTRICT OF MARICOPA COUNTY FROM AERIAL PHOTOGRAPHS FLOWN ON APRIL 2002 BY LANDATA AIRBORNE SYSTEMS, INC.

WITTMANN PHASE IV  
FLOODPLAIN WORK MAP

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	LAYOUT	DRAFTED	CHECKED
	NEF	DLP	NEF
DRAWING SCALE(S)			SHEET NO. <b>3</b> OF 7 SHEETS
1" = 200'			
PLAN DATE	NOV/2011		
PROJECT NUMBER	45104608		

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MATCH SHEET 5

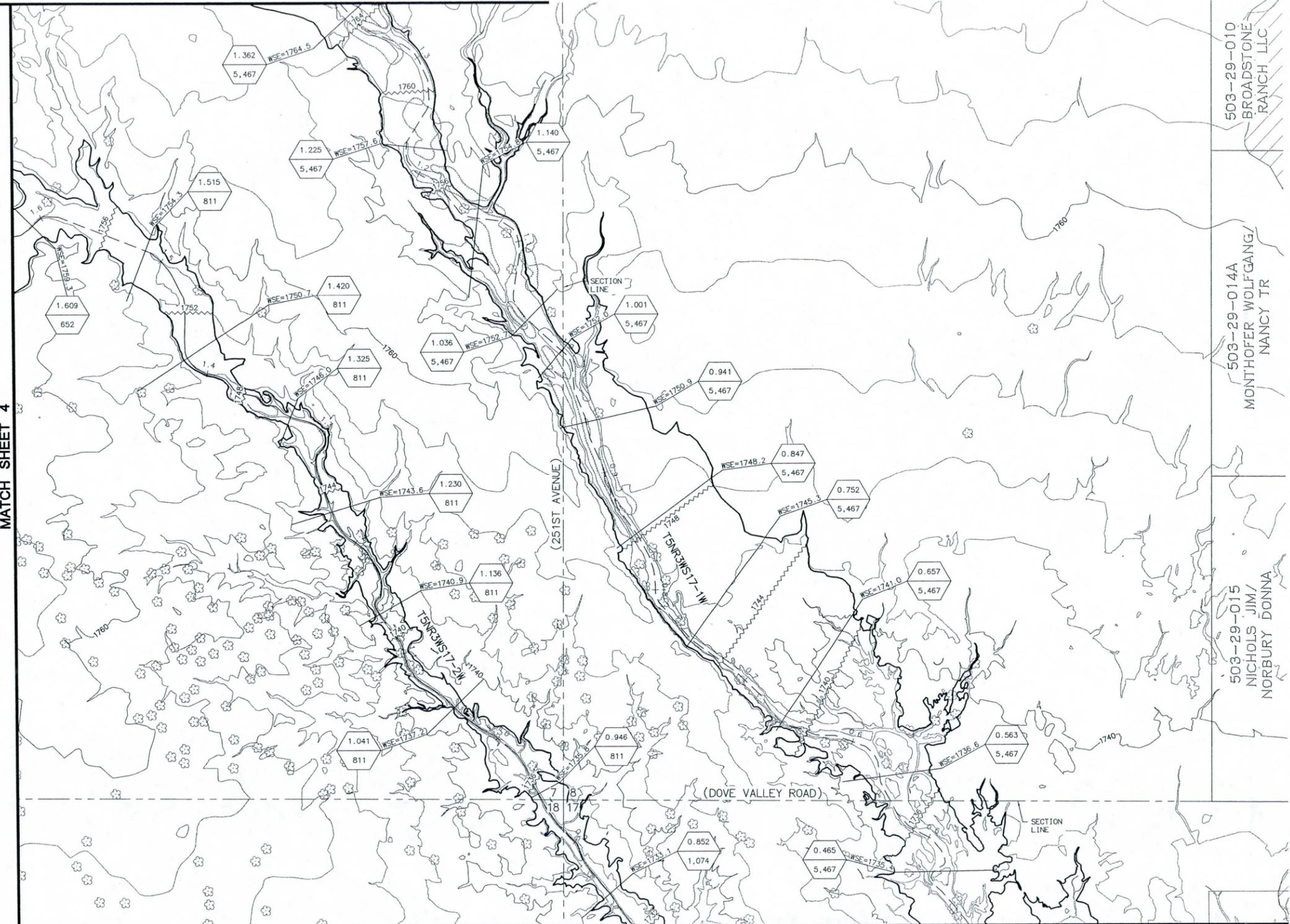
MATCH SHEET 4

MATCH SHEET 2

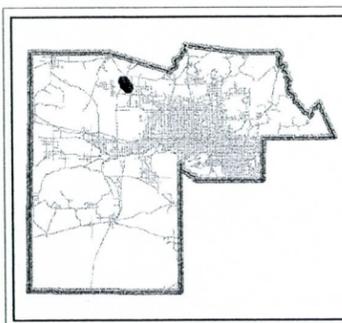
503-29-010  
BROADSTONE  
RANCH LLC

503-29-014A  
MONTHOFFER WOLFGANG/  
NANCY TR

503-29-015  
NICHOLS JIM/  
NORBURY DONNA



FLOOD CONTROL DISTRICT  
OF MARICOPA COUNTY



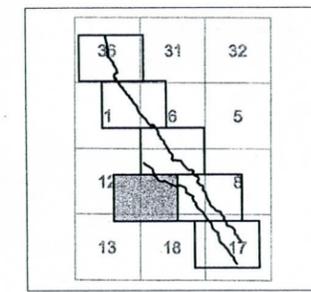
WITTMANN PHASE IV  
FLOODPLAIN DELINEATION STUDY



SCALE: 1"=200'

LEGEND

- EFFECTIVE ZONE AE
- EFFECTIVE FLOODWAY
- HYDRAULIC BASE LINE
- T4N-R2W-S15N WASH I.D.
- STATION RIVER MILE
- CROSS SECTION Q100 (CFS)
- CROSS SECTIONS
- ZONE AE FLOODPLAIN
- EXISTING CONTOUR
- 1700 BASE FLOOD ELEVATION



NOTE:  
THIS MAP WAS PREPARED USING 4 FT CONTOUR DATA PROVIDED BY THE FLOOD CONTROL DISTRICT OF MARICOPA COUNTY FROM AERIAL PHOTOGRAPHS FLOWN ON APRIL 2002 BY LANDATA AIRBORNE SYSTEMS, INC.

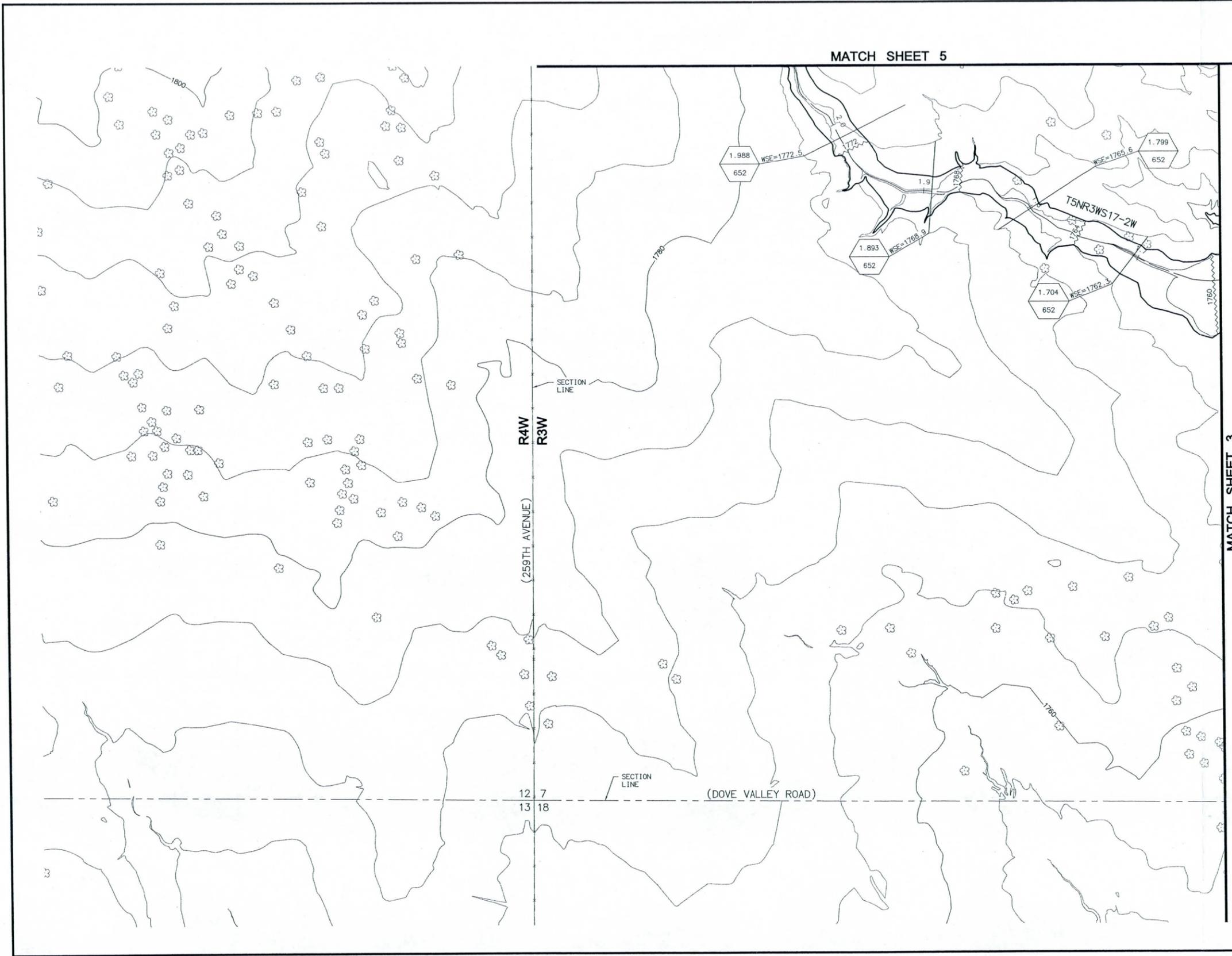
WITTMANN PHASE IV  
FLOODPLAIN WORK MAP

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	LAYOUT	DRAFTED	CHECKED
	NEF	DLP	NEF
DRAWING SCALE(S) 1" = 200'			
PLAN DATE NOV/2011			SHEET NO. 4
PROJECT NUMBER 45104608			OF 7 SHEETS

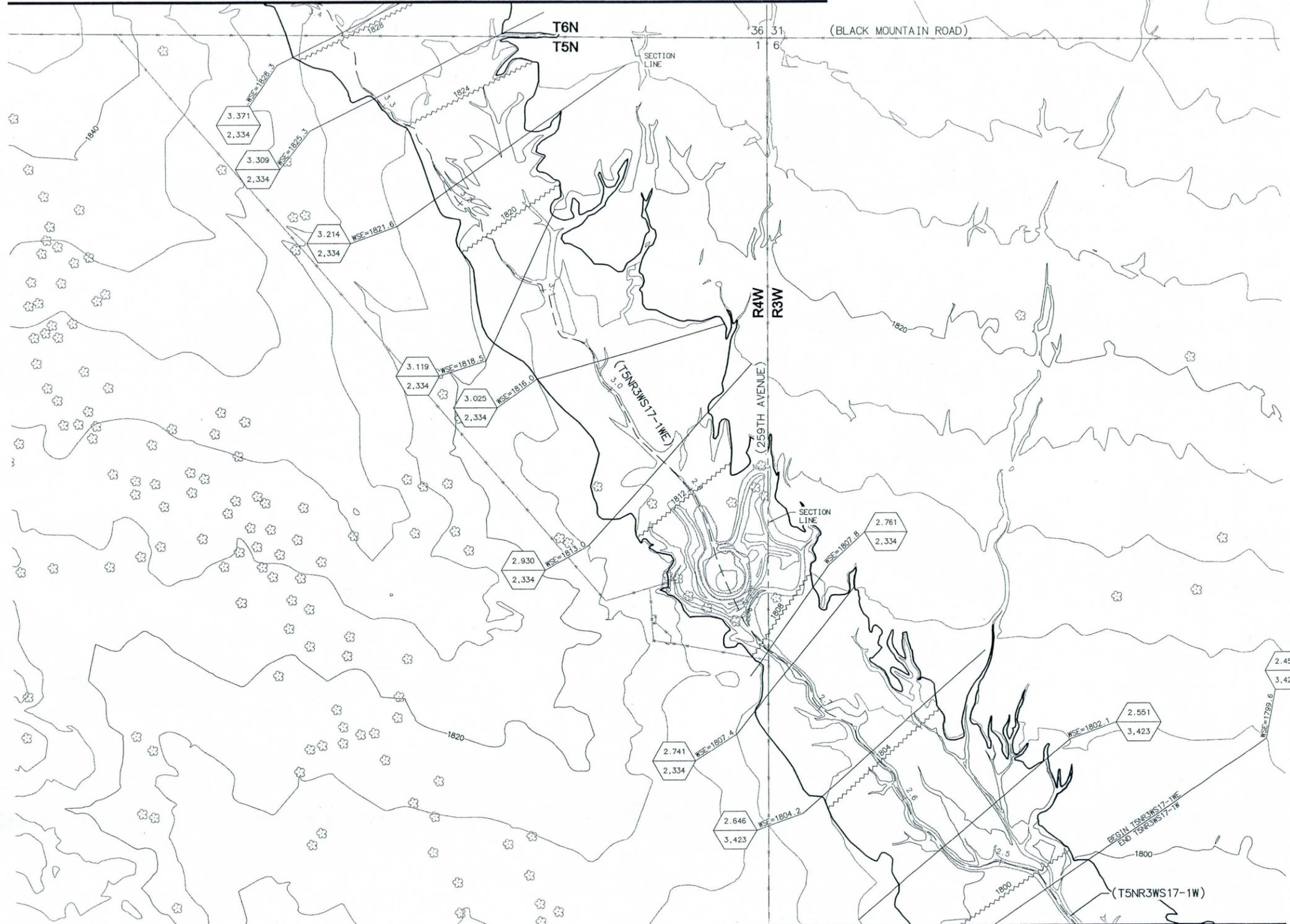
MATCH SHEET 5

MATCH SHEET 3



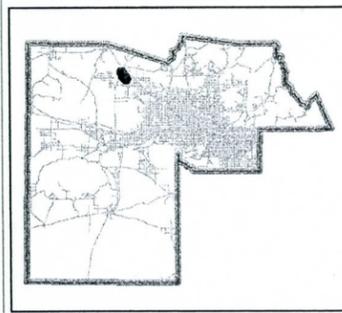


MATCH SHEET 7



MATCH SHEET 5

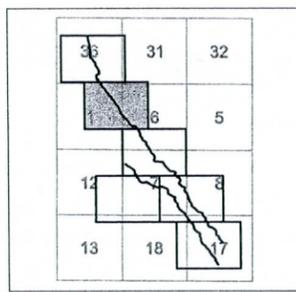
FLOOD CONTROL DISTRICT OF MARICOPA COUNTY



WITTMANN PHASE IV FLOODPLAIN DELINEATION STUDY



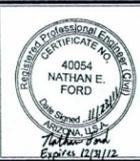
- LEGEND**
- EFFECTIVE ZONE AE
  - EFFECTIVE FLOODWAY
  - HYDRAULIC BASE LINE
  - T4N-R2W-S15N WASH I.D.
  - 0.189 STATION RIVER MILE
  - 1156 CROSS SECTION Q100 (CFS)
  - CROSS SECTIONS
  - ZONE AE FLOODPLAIN
  - 1700 EXISTING CONTOUR
  - 1700 BASE FLOOD ELEVATION



NOTE: THIS MAP WAS PREPARED USING 4 FT CONTOUR DATA PROVIDED BY THE FLOOD CONTROL DISTRICT OF MARICOPA COUNTY FROM AERIAL PHOTOGRAPHS FLOWN ON APRIL 2002 BY LANDATA AIRBORNE SYSTEMS, INC.

WITTMANN PHASE IV FLOODPLAIN WORK MAP

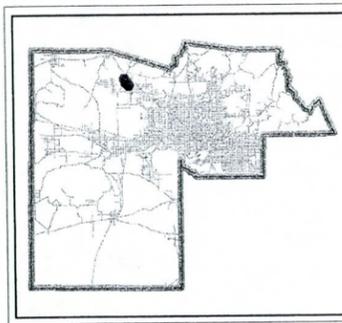
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LAYOUT	NEF	DRAFTED	DLP	CHECKED	NEF
DRAWING SCALE(S) 1" = 200'					
PLAN DATE NOV/2011					SHEET NO. 6
PROJECT NUMBER 45104608					OF 7 SHEETS

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FLOOD CONTROL DISTRICT OF MARICOPA COUNTY



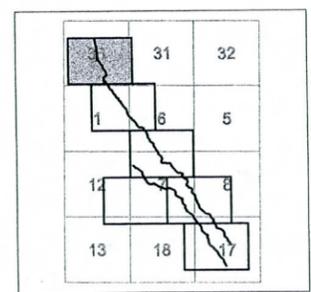
WITTMANN PHASE IV FLOODPLAIN DELINEATION STUDY



SCALE: 1"=200'

LEGEND

- EFFECTIVE ZONE AE
- EFFECTIVE FLOODWAY
- HYDRAULIC BASE LINE
- T4N-R2W-S15N** WASH I.D.
- STATION RIVER MILE CROSS SECTION
- Q100 (CFS)
- CROSS SECTIONS
- ZONE AE FLOODPLAIN
- EXISTING CONTOUR
- BASE FLOOD ELEVATION

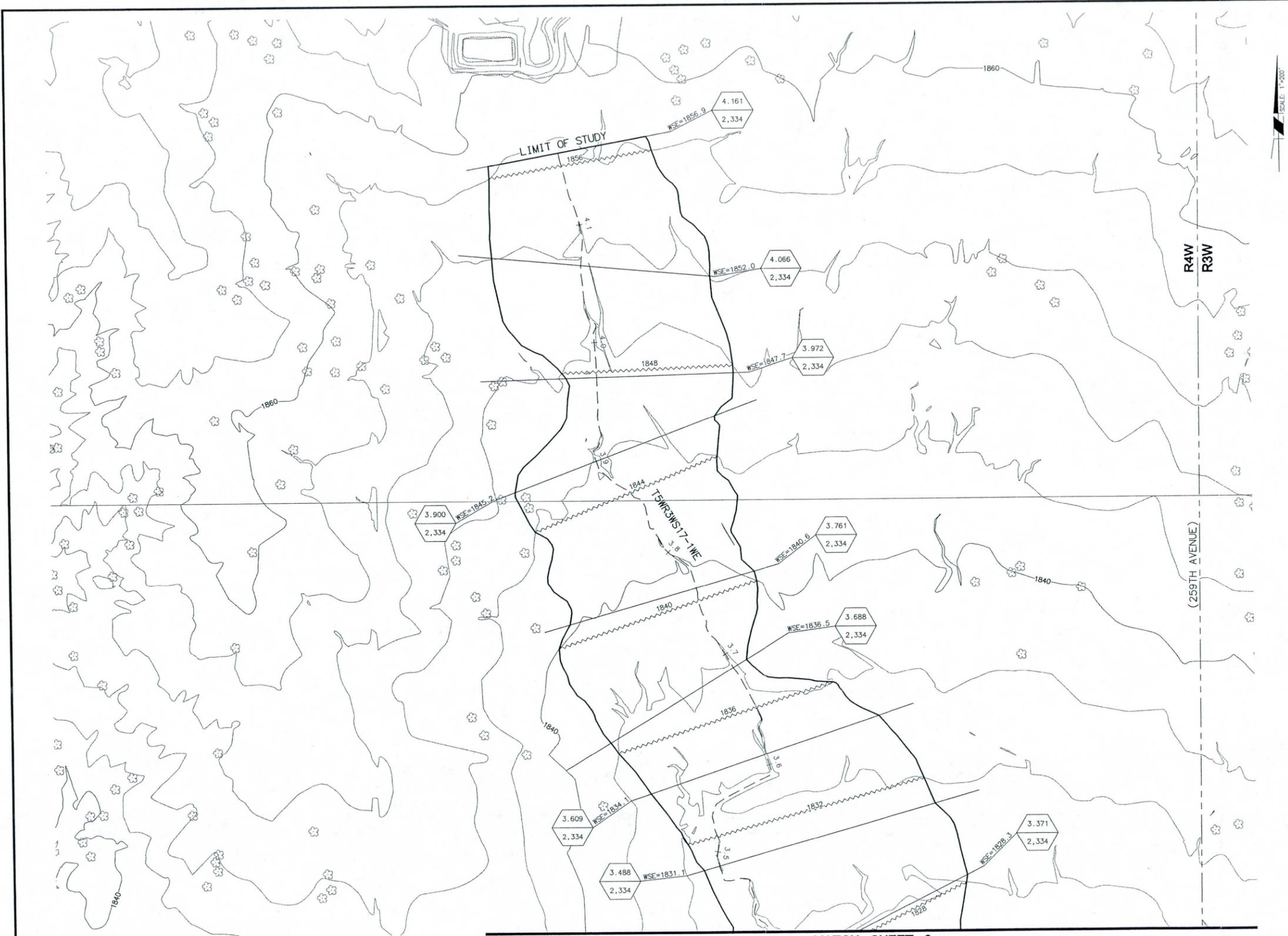


NOTE: THIS MAP WAS PREPARED USING 4 FT CONTOUR DATA PROVIDED BY THE FLOOD CONTROL DISTRICT OF MARICOPA COUNTY FROM AERIAL PHOTOGRAPHS FLOWN ON APRIL 2002 BY LANDATA AIRBORNE SYSTEMS, INC.

WITTMANN PHASE IV FLOODPLAIN WORK MAP

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	LAYOUT	DRAFTED	CHECKED
	NEF	DLP	NEF
DRAWING SCALE(S)		1" = 200'	
PLAN DATE		NOV/2011	
PROJECT NUMBER		45104608	
SHEET NO.			<b>7</b>
			OF 7 SHEETS



MATCH SHEET 6

H:\DATA\45104608\CADD\STRMATER\DWG\WITTMANN\_PHASE\_IV\4608-SW-007.DWG - PDATE: 10/25/11 5:00 pm