

**PRE-DESIGN CONCEPT STUDY REPORT
FOR
SIPHON DRAW DRAINAGE IMPROVEMENTS
(ADDITIONAL MESA ALTERNATIVES)**

CONTRACT FCD 2003 C019
ASSIGNMENT NO. 4

July 2005
WP #031902.04

Prepared for:

Flood Control District of Maricopa County
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July 19, 2005

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Re: **Siphon Draw Drainage Improvements**
Pre-Design Concept Study
Contract FCD 2003 C 019
Assignment No. 4
WP# 031902.04

Dear Ms. Terry:

Wood, Patel & Associates, Inc. (Wood/Patel) is pleased to submit two copies of this final report for the *Pre-Design Concept Study for Siphon Draw Drainage Improvements*.

Our contract and scope of services required a concept type pre-design study and letter report. Due to the complex nature of the preferred alternative and associated hydraulic elements, it was felt that by expanding the report content with additional text and exhibits it would be more valuable to the District.

Please note that the preferred alternative consists of 11 different large to medium size retention areas, two major split structures, complex flow spillover structures, diversions, and conveyance system. Due to the scope limitations and agreed upon work effort with the District, hydraulic analysis was performed using a simplified approach. In addition, work efforts towards Section 404 requirements and landscaping evaluation were excluded from the scope.

To address some land requirement issues raised by Sunland Springs Village development, the preferred alternative includes two channel options along the north side of the SRP transmission line corridor. These options are earthen and concrete lined channels.

We have enjoyed this complex and challenging assignment and truly believe our knowledge can benefit the District for the next phase of work, including the final design efforts.



Ms. Felicia Terry
Flood Control District of Maricopa County
Siphon Draw Drainage Improvements
Pre-Design Concept Study

July 19, 2005
Page 2 of 2

Please feel free to call if we can be of any further assistance.

Sincerely,

WOOD, PATEL & ASSOCIATES, INC.


Ashok C. Patel, P.E., R.L.S., CFM
Principal

ACP/ac

cc: Keith Nath, P.E., City of Mesa (one copy of report)

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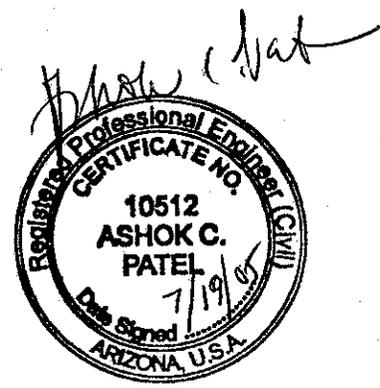
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1.0 INTRODUCTION

Project History

The East Mesa Area Drainage Master Plan (ADMP) (FCD #95-32) was completed by Dibble & Associates in July 1998 for the Flood Control District of Maricopa County (FCDMC). Siphon Draw Detention Basin, Sunland Springs Channel (Meridian Road Collection System), and Elliot Road Channel were among the recommended improvements with high priorities. Since the completion of the ADMP, development has occurred which impacts the facilities that were recommended. In 2003, Wood, Patel & Associates, Inc. (Wood/Patel) was contracted to provide pre-design plans for the Siphon Draw Drainage Improvements which included Siphon Draw Detention Basin, Meridian Road Collection System, and Elliot Road Channel Phase 2 (from 104th St. to Meridian Road).

The objectives of the pre-design study were to investigate alternatives for providing flood protection west of Meridian Road along the project corridor. Specifically, the pre-design study modified the ADMP hydrologic model to include the changes in land use into the existing drainage systems; revised the model hydrologic conditions in Pinal County; evaluated the ADMP-recommended improvements of Siphon Draw Drainage System; identified potential detention basin locations and sizes; determined appropriate collection and conveyance system locations, alignments, cross sections, and Right-Of-Way (ROW) requirements east of the 104th Street alignment; and developed probable cost estimates for each of the alternatives.

The pre-design study also investigated various conveyance cross-sections for each segment of the conveyance system. Design plans were prepared for the preferred alternative to the 20 percent level. All potential detention basins were located within Pinal County as consistent with the ADMP. For details of the study, please see the draft report titled *Siphon Draw Drainage Improvements Pre-Design Study Report* by Wood/Patel (August 2004).

Subsequent to the completion of alternatives evaluation, it was learned that neither City of Apache Junction nor Pinal County showed any support in pursuing any of the food control alternatives within the Pinal County area. FCDMC, in association with the City of Mesa, expressed an interest to evaluate possible flood attenuation and conveyance alternatives within the City of Mesa. Plate 1 shows the project location and vicinity map.

Sunland Springs Village (SSV) is located on the northwest corner of Meridian Road and Paloma Avenue alignment (Salt River Project (SRP) power line corridor). Its zoning was approved based upon the assumption that a channel and a basin will be built along Meridian Road per the East Mesa ADMP. Since that alternative is not possible, SSV cannot proceed in completing its south portion of development as approved by the City of Mesa. Therefore, it is mutually beneficial that SSV, FCDMC, and the City of Mesa work together to find a feasible flood mitigation alternative.

Pre-Design Concept Study

Wood/Patel has been requested to provide engineering services to support the evaluation of additional alternatives within the City of Mesa to manage the storm water near the boundary of Pinal and Maricopa Counties. This portion of the work effort is limited to providing a final letter report which addresses a brief history of the project, describes new alternatives, and discusses modifications to the alternatives to address SRP and City of Mesa issues. The Pre-Design Concept Study provides a description of the preferred alternative, concept layout (plan view) of detention basins and pertinent hydraulic elements. An estimate is included for a probable cost of construction.

Scope of Work

In accordance with the Scope of Work, the following sub-tasks have been performed for the Pre-Design Concept Study:

1. Coordinating and meeting with FCDMC, City of Mesa, Apache Junction, SSV and their consultants;
2. Collecting data;
3. Evaluating a channel and a detention basin corridor parallel to the SRP power line corridor to convey all off-site flow, which enters at the southeast corner of SSV and Meridian Road, and possibly some of the SSV onsite flows. Preliminary hydraulic analyses, cost estimates, and ROW requirements were developed for the possible improvements;
4. Evaluating a basin at the northwest corner of Meridian Road and the SRP power line corridor including hydrologic and hydraulic analyses. Cost estimates and ROW requirements were developed for the possible improvements;

5. Evaluating a basin at the southwest corner of the SRP power line corridor and the west boundary of Meridian Pointe subdivision within the City of Mesa parcel including hydrologic and hydraulic analyses. Cost estimates and ROW requirements were developed for the possible improvements;
6. Evaluating a basin at the southwest corner of Siphon Draw Wash Corridor and the west boundary of Meridian Pointe subdivision including hydrologic and hydraulic analyses. Cost estimates and ROW requirements were developed for the possible improvements;
7. Exploring the possibility of using the SRP Transmission Line Easement for detention;
8. Making modifications to the existing pipe structures near 104th Street and Elliot Road;
9. Preparing a letter summary report including exhibits and probable opinion of cost.

It should be noted that the scope required a concept type study by the FCDMC. The anticipated effort is significantly less than a typical pre-design study.

The study has only used readily available data such as topographic maps, utilities, boundary surveys, aerial photographs, and unit prices. No field work, including topographic survey or potholing, was performed.

2.0 DEVELOPMENT OF ALTERNATIVES

Since Elliot Road detention basins and outfall channel (storm drain) have been constructed at the downstream boundary of the Pre-Design Concept Study (104th Street), one of the major constraints to the alternatives development is that the design flow for the system at this point should be less than the 78-inch pipe capacity of 508 cfs as established by the East Mesa ADMP based on the ultimate conditions of the watershed. Potential detention basins must be located within Maricopa County as discussed in the previous section.

A total of three alternatives were evaluated for management of storm water from Pinal County. Three detention basin sites were identified and evaluated: Basin A is located on the southeast corner of the SSV; Basin B is located on the west of Mountain Road and the south of Siphon Draw Wash; and Basin C is located on the south of Paloma Avenue alignment (SRP power line corridor) and the west of Mountain Road. The design parameters for the three basins are shown on Plates 2 to 4.

The **first alternative** (see Plate 2) considered two offline detention basins: Basin A and Basin B. For the 100-year, 24-hour design storm, an estimated peak discharge of 1,681 cfs from areas east of Meridian Road will concentrate at the southeast corner of SSV. A base flow of 350 cfs will bypass Basin A through a 78-inch storm drain, then, it will combine with the bleed-off flow from Basin A. The combined flow will continue westerly through the 78-inch pipe, which is then directed south at Mountain Road to Siphon Draw Wash through a proposed open channel. The flow rate discharged into the Siphon Draw Wash is estimated to be 415 cfs, and that flow will continue westerly to 104th Street through Siphon Draw Wash.

Siphon Draw Wash and its tributaries collect flows from areas east of Meridian Road and the flow concentrates east of Meridian Road within Siphon Draw Wash Corridor (686 cfs) through Meridian Pointe subdivision. This flow crosses Meridian Road through three culverts into Siphon Draw Wash. At Mountain Road, a base flow of 80 cfs bypasses Basin B and flows south to the Elliot Road storm drain through a 54-inch pipe. The remainder flow from Siphon Draw Wash will be discharged into Basin B. Bleed-off flow from Basin B will combine with flow in the Elliot Road storm drain and the combined flow (122 cfs) will continue west to 104th Street through a 54-inch pipe.

The **second alternative** (see Plate 3) considered three offline detention basins: Basin A, Basin B, and Basin C. For the 100-year, 24-hour design storm, an estimated peak discharge of 1,681 cfs from areas east of Meridian Road will concentrate on the southeast corner of the SSV. A base flow of 750 cfs will bypass Basin A through an open channel and the remainder flow will be discharged into Basin A, then the base flow will combine with the bleed-off flow from Basin A. The combined flow will continue westerly in an open channel to Basin C. A base flow of 350 cfs will bypass Basin C through an open channel, and the remainder flow will be discharged into Basin C. The base flow then will combine with the bleed-off flow from Basin C. The combined flow will continue to south along Mountain Road to Siphon Draw Wash through a proposed open channel. The flow rate discharged into the Siphon Draw Wash is estimated to be about 400 cfs.

Flow into Siphon Draw Wash from areas east of Meridian Road and its operations are the same as alternative one.

The **third alternative** (see Plate 4) has offline detention basins B and C plus a series of small offline detention basins within SRP power line corridor along the south boundary of the SSV. Basins B and C are sub-divided by the proposed pads for SRP power line poles and towers. For the 100-year, 24-hour design storm, an estimated peak discharge of 1,681 cfs from areas east of Meridian Road will concentrate on the southeast corner of the SSV in a sediment basin A. An open channel conveys this flow westerly to Mountain Road, collects runoff from north areas of SSV, and at the same time diverts flow above the channel capacity to the basins within SRP power line corridor on both the east and west sides of Mountain Road. A base flow of 300 cfs will bypass Basins C through an open channel and the remainder will be discharged into Basin C. The base flow will continue south into Siphon Draw Wash along Mountain Road. The bleed-off flow from Basin C will directly flow into storm drain F along Mountain Road.

Siphon Draw Wash and its tributaries collect flows from areas east of Meridian Road and the flow concentrates east of Meridian Road within Siphon Draw Wash Corridor (686 cfs) through Meridian Pointe subdivision. This flow crosses Meridian Road through three culverts into Siphon Draw Wash. At Mountain Road, the combined base flow of 80 cfs from Siphon Draw Wash and bleed-off flow from Basins C bypasses Basins B and flows south to the Elliot Road storm drain through a 54-inch pipe. At the junction of Siphon Draw Wash and open channel E, a base flow of 200 cfs will divert into Siphon Draw Wash and continue westerly to 104th Street. The remaining flow from Siphon Draw Wash will be discharged into Basins B. Bleed-off flow

from Basin B will combine with flow in the Elliot Road storm drain and the combined flow (149 cfs) will continue west to 104th Street through a 54-inch pipe.

For illustration purposes, a base alternative (see Plate 5) was also evaluated. This base alternative assumed that no further action will be taken by the FCDMC and the City of Mesa, and that the developer will be obligated to convey the design storm water from areas east of Meridian Road to downstream washes as per natural and historical conditions. The existing channel is located along the north side of the SRP power line corridor from Meridian Road to the west of Signal Butte Road. The design flow for the existing channel is established to be 1,750 cfs (per report by Clouse Engineering, June 1996).

3.0 EVALUATION OF ALTERNATIVES

The probable cost to implement each alternative was estimated to assist with alternative selection. Among all the three alternatives evaluated, Alternative 3 is identified to be the preferred alternative by the City of Mesa, the FCDMC, and the SSV. Plate 6 illustrates the preferred alternative concept plan. The estimate of probable cost can be found in Table 2 and Table 3, and the cost estimation sheets for all alternatives are included in Appendix C.

Note that the cost estimates were prepared for alternative selection purposes. All common items were excluded from the total cost. Therefore, the cost estimates are not the total construction cost of any alternatives.

4.0 HYDROLOGIC MODELING

The hydrologic analyses were performed using the HEC-1 software. Three HEC-1 hydrologic models were developed for the three post project condition alternatives. Existing condition land uses were assumed within Pinal County, and future condition land uses were used within Maricopa County. For the purpose of this Pre-Design Concept Study, the FCDMC agreed that it would be acceptable to make no physical divisions to any of the HEC-1 sub-basins. Instead, flow diversions were used for all sub-basin split flow evaluations. The HEC-1 hydrologic input and output files for the three alternatives are included on the floppy disk. The HEC-1 model output file for the preferred alternative is included in Appendix A. The peak flows at the key concentration locations are summarized in Table 1 for all of the models, and are shown on Plate 6 for the preferred alternative.

Hydraulics

Due to the scope limitations and agreed upon work effort with the District, detailed hydraulic analysis using StormCad, HEC-RAS or other software were not performed. The hydraulic elements were sized based upon simplified hydraulic calculations (see Appendix D), such as channel and pipe normal depths, culvert hydraulics and weir flow equations. For complex split structures and drop structures, element sizes were estimated based upon judgment.

Storage Volume Requirements

All basins depicted for the preferred alternative concept plan are preliminarily sized based on the off-line basin concept. Accordingly, the following volumes are required:

Basins	{	A	= 4 Ac-Ft (sedimentation purpose)
		T ₁ , T ₂ , T ₃	= 33 Ac-Ft (off-line, flood storage)
		T ₄ , T ₅ , T ₆ , T ₇	= 18 Ac-Ft (off-line, flood storage)
		C ₁ , C ₂	= 90 Ac-Ft (off-line, flood storage)
		B ₁ , B ₂	= 75 Ac-Ft (off-line, flood storage)

5.0 PREFERRED ALTERNATIVE

Subsequent to the selection of the preferred alternative (alternative 3), Wood/Patel refined this alternative with more site specific details. Some modifications to alternative 3 were also needed to account for the input received from SRP. The refined alternative number 3 includes sediment basin A, offline detention basins B and C plus a series of small offline detention basins within SRP power line corridor along the south boundary of the SSV. Basin B consists of B1 and B2, and is divided by the Western Area Power Administration (WAPA) easement. Basin C consists of basin C1 and C2, and it is divided by the future SRP power line corridor.

For the 100-year, 24-hour design storm, an estimated peak discharge of 1,681 cfs from areas east of Meridian Road will concentrate on the southeast corner of the SSV in sediment basin A. An open channel conveys this flow westerly to Mountain Road, collects runoff from north areas of SSV, and at the same time diverts flow above the channel capacity to the basins within SRP power line corridor on both east and west sides of Mountain Road. Where a base flow of 300 cfs will bypass Basins C1 and C2 through an open channel E and the remainder flow will be discharged into Basins C1 and C2. The base flow will continue south into Siphon Draw Wash along Mountain Road. The bleed-off flow from Basins C1 and C2 will directly flow into storm drain F along Mountain Road (extension).

Siphon Draw Wash and its tributaries collect flows from areas east of Meridian Road and the flow concentrates east of Meridian Road within Siphon Draw Wash Corridor (686 cfs), which are conveyed through the Meridian Pointe subdivision. This flow crosses Meridian Road through three culverts into Siphon Draw Wash corridor. At Mountain Road (extension), the combined flow consisting of 80 cfs from Siphon Draw Wash plus bleed-off flow from Basins C1 and C2 bypasses Basins B1 and B2 and flows south to the Elliot Road storm drain through a 54-inch storm drain F. At the junction of Siphon Draw Wash and open channel E, a flow of 200 cfs (Section 404) will be diverted into Siphon Draw Wash and will continue westerly to 104th Street. The remaining flow from Siphon Draw Wash will be discharged into Basins B1 and B2. Bleed-off flow from Basins B1 and B2 will combine with flow in the Elliot Road storm drain and the combined flow (149 cfs) will continue west to 104th Street through a 54-inch storm drain F.

The preliminary opinion of probable cost for the preferred alternative with earthen channel D is \$11,833,500 and with concrete channel D is \$12,329,100. These costs include all hydraulic

elements associated with the preferred alternative concept plan including contingencies (construction, design, field, and allowances for change orders). The land acquisition costs are excluded from the estimate. Exhibit A presents preliminary concept plans. To clarify the hydraulic function of various elements, the following summary is provided:

Sediment Basin A

A channel along Meridian Road is constructed by SSV to collect all offsite flows (1681 cfs) from the upper watershed. The channel flow will be directed into sediment Basin A to intercept sediment from the more frequent storms. Therefore, the sediment basin will require frequent periodic cleaning. The basin discharges into channel D.

Channel D

For channel D, two options have been considered: earthen and concrete.

The channel takes the outflow (1681 cfs) from sediment Basin A and directs it to the 3- 10'x5' box culvert and collects runoff from the SSV contributing area to the north (210 cfs). There are three spillover structures proposed along the south bank of the channel; it allows 516 cfs (about 33 Ac-Ft storage volume) to spillover into the detention basins (T1, T2, and T3) located within the SRP corridor.

At the inlet of the box structure, a side weir is proposed to divert 170 cfs peak flow to west into detention basins T4, T5, T6, and T7 (about 18 Ac-Ft storage volume). The detained flow is released by a bleed-off pipe into the existing wash near Signal Butte Road. As a result, the peak discharge delivered at the 3-10'x5' box structure is reduced from 1681 cfs to 1220 cfs.

Splitter Structure - ST1

At the south end of the 3-10'x5' box culvert a splitter structure (ST1) diverts the total flow of 1,050 cfs in two directions. The first 300 cfs from the 3-10'x5' box culvert is directed by the splitter structure ST1 into the proposed earthen channel E. The earthen channel outfalls to splitter structure 2 (ST2) at Siphon Draw Wash along the east line of the Meridian Pointe subdivision.

The flow in excess of 300 cfs overtops the west wall of the concrete channel and weirs into an inlet structure to a 10'x 6' box culvert. The box outlet takes flow into detention basins C1 and C2 (estimated 90 Ac-Ft). These basins are connected by an equalizer structure consisting of 1-10'x

4' box culvert. These basins are evacuated by a storm drain located at the southeast corner of Basin C2.

Splitter Structure - ST2

A second splitter structure (ST2) is located across Siphon Draw Wash at the west end of Meridian Pointe subdivision and collects flows from Siphon Draw Wash (686 cfs), excess flow from the Meridian Pointe subdivision as well as from channel E (300 cfs). The combined peak flow of 1,051 cfs at the splitter structure ST2 diverted in three directions as listed below:

- Initial flow of 200 cfs is directed into the existing Siphon Draw Wash. This flow will help in maintaining the natural vegetation (Section 404) in the wash corridor.
- An inlet structure to 54-inch storm drain F is located at the south side of splitter structure ST2. The surface inlet structure will intercept about 30 cfs of the surface flow.
- The remainder flow of 771 cfs will overtop the weir structure located on the southwest corner of the splitter structure and will be discharged into detention basins B1 and B2 (estimated 75 Ac-Ft). These basins are connected by 1-10'x 4' equalizer box culvert. The basins drain via a 48-inch storm drain located at the southwest corner of Basin B1 into the Elliot Road storm drain G.

Existing Flood Control Facility at 104th Street and Elliot Road

As part of Phase I Improvements of Elliot Road Basins and Outfall Channel (FCD Contract 98-44, PCN No. 4420431), several flood control improvements have been constructed near the intersection of 104th Street and Elliot Road.

1. 78-inch SD (concrete pipe) on the south side of Elliot Road's south right-of-way with a conveyance capacity of 508 cfs.
2. 78-inch SD (concrete pipe) on the north side of Elliot Road's north right-of-way with a conveyance capacity of 385 cfs.
3. Interceptor channel along the east side of 104th Street.
4. Splitter structure on the northeast corner of 104th Street and Elliot Road intersection.

Due to the design concept changes resulting from this Pre-Design Concept Study, design modifications will be required for the existing splitter structure noted above in item 4. Please refer to Appendix B for the details on the design change required.

6.0 SRP COORDINATION

Several elements of the proposed detention basins/channels from the Pre-Design Concept Study are impacted by existing and proposed SRP/WAPA corridors. To understand SRP's needs, a coordination meeting was arranged.

Wood/Patel met with SRP on May 5, 2005 to get input on their transmission corridor constraints and requirements. Those requirements are listed below based on their input:

250 ft. wide SRP Corridor

1. The east-west 250 ft. SRP corridor along the south side of the SSV is going to change with the newly planned transmission facility. New poles/towers will be placed and the old ones may be removed. These towers are typically spread about 800 ft. to 1,000 ft. apart. Wood/Patel used 900 ft. spacing as shown on the concept drawing. Based on SRP's recommendations a 110 ft. x 200 ft. flat pad is located at the future pole/tower locations.
2. The side slope for the depressed areas in between the pads could be as steep as 1:1. The concept drawing shows that slope to be 4:1.
3. A 35 ft. wide access path is required along the south edge of the SRP corridor to accommodate maintenance access.
4. A 25 ft. wide space is required along the north edge of the SRP corridor for the maintenance road.
5. To allow the storm drainage to spill over into the corridor depressions (retention basins), a hard lined spill over weir will be placed as shown on the concept drawing. The weir will be designed such that it will flood no more than 1 ft. in a 100-year type storm event.
6. The depressions will be drained by gravity via 12-inch bleed-off pipe.
7. SRP maintenance ramps at a slope maximum of 10% will be provided at both the east and west end of the depressions.

North-South Planned SRP Corridor east of Browning Substation

1. A 150 ft. x 200 ft. flat pad is required near the SEC of the substation as depicted on the concept drawing.
2. A 20 ft. wide maintenance road will be required along the west edge of the planned SRP corridor as depicted on the concept drawing.
3. A portion of the corridor outside of the pad and maintenance road will be used for the detention area. The side slope of the detention area should be no steeper than 1:1. The concept drawing depicts a side slope of 4:1 with maintenance ramps on a 10% slope.

Western Area Power Administration (WAPA) Corridor

It is Wood/Patel's understanding that the WAPA corridor constraints and requirements are similar to those of SRP. The concept plan depicts the detention basin layout based on that understanding.

1. A 20 ft. wide maintenance road will be required along the west edge of the future transmission line corridor as depicted on the concept drawing.
2. A portion of the corridor outside of the pad and maintenance road will be used for the detention area. The side slope of the detention area should be no steeper than 1:1. The concept drawing depicts a side slope of 4:1 with maintenance ramps on a 10% slope.

Subsequent to the SRP submittal, Wood/Patel received their comments on June 24, 2005 regarding the corridor constraints.

Due to the timing issues for the final submittal (June 30th), SRP comments have not been incorporated into this Pre-Design Concept Study. These comments are identified below:

1. The 250 ft. corridor will become a 300 ft. corridor after the new 50 ft. right-of-way acquisition from SSV by SRP.

2. A 20 ft. wide easement may be required by SRP along the west edge of Meridian Pointe from the SRP corridor to Siphon Draw Wash. This easement is to be negotiated with and obtained from the City of Mesa.

7.0 CONCLUSIONS AND RECOMMENDATIONS

The recommended drainage system is Alternative 3, which is the combination of sediment basin A, offline detention Basins B1, B2, C1, and C2, storm drain elements F and G, and open channel elements D and E. Additional elements include several detention basins within SRP power line corridor along south boundary of the SSV.

The detention basin footprints include 4:1 side slopes with access and maintenance roads. Based upon the input from FCDMC and City of Mesa, landscape/aesthetics were not evaluated as part of this study. This decision was dictated primarily because of the land constraints.

This report includes only RGRCP as a choice for the pipe material. There are a number of locations where alternative material types can be explored to reduce the improvement costs.

Ongoing maintenance of the designed or recommended drainage systems is required to preserve the design integrity and purpose of the drainage system. Failure to provide maintenance can prevent the drainage system from performing to its intended design purpose and can result in reduced performance. As part of the final design process, a regular maintenance program needs to be implemented to have the drainage system perform to the level of protection as presented in this report.

TABLE 1

Peak Flow Summary Table

Table 1
Peak Flow (cfs) Summary from HEC-1 Models

Major concentration points/Locations	Hydrograph Name	HEC-1 Model Names					
		FINAL8.DAT	EMADMP98	S60EM_BP.DAT	S60EMOP1.DAT	S60EMOP2.DAT	S60EMOP3.DAT
Model number		1	2	3	4	5	6
					Alternative 1	Alternative 2	Alternative 3
CAP overchute A	CAP1A	217	217	710	710	710	710
CAP overchute B	CAP1B	217	217	710	710	710	710
Subbasin 65A	65A	2503	2503	1235	1235	1235	1235
Total flow to Basin A at Meridian Rd.	B1FLOW	N/A	N/A	N/A	1681	1681	1681
Flow at Mountain Rd. before splits	CBASI/CMOUNT	N/A	N/A	N/A	415	805	1736
Flow at D/S of Basin C to SDW	CBAS3	N/A	N/A	N/A	414	400	355
Total flow into SDW at Meridian Rd.	SD TOT	N/A	N/A	N/A	686	686	686
Total flow at D/S of Basin B into Pipe	CBAS2	N/A	N/A	N/A	122	122	149
Basin EA by-pass flow to Elliot Rd.	DI65B	30	N/A	30	370	370	370
EA by-pass & storm drain at 104th St	CP65A	538	509	N/A	491	491	493
Release of EA & storm drain	CP65	548	851	1097	504	502	500
Crismon Rd by-pass & storm drain	CP66C	956	991	1427	908	908	823
W. side of WB in storm drain	RC-WA	973	991	1386	921	921	830
Release of WB & storm drain	CP66D	986	1074	1386	921	921	830
E. side of Ellsworth Rd in storm drain	66T66D	986	1074	1385	920	920	830
At Ellsworth Rd.	C66D	1101	1162	1502	1056	1055	957
Box culvert at Ellsworth Rd.	CULVT	1100	1162	1581	1044	1044	947
Santan Freeway Channel at Outlet	OUTLET	N/A	1162	1588	1167	1167	1064
<i>2/3 mile S. of Elliot</i>	C70A	1213	1346	1609	1191	1191	1099
<i>Santan Freeway Channel</i>	C76A	1365	1346	1615	1278	1278	1235

Model Explanation

- 1 FINAL8.DAT = HEC-1 model used for Elliot Basin and Outfall Channel design;
- 2 EMADMP98 = Peak flows found in the ADMP report for East Mesa, 1998;
- 3 S60EM_BP.DAT = HEC-1 model for the pre-project condition - Siphon Draw Basin project;
- 4 S60EMOP1.DAT = HEC-1 model for Alternative 1;
- 5 S60EMOP2.DAT = HEC-1 model for Alternative 2;
- 6 S60EMOP3.DAT = HEC-1 model for Alternative 3 (Preferred Alternative).

TABLES 2 & 3
Cost Summary Tables

Siphon Draw Drainage Improvements - Additional Alternatives

June 30, 2005

Flood Control District of Maricopa County

W/P # 031902.04

FCD 2003 C019

TABLE 2 COST SUMMARY**(Estimates of Probable Cost Based on Concept Analysis**)**

Alternative	MAJOR ELEMENTS	LAND REQUIREMENT (acres)	COMMENTS
1	\$7,311,000	34	Deeper Basins "A"&"B"
2	\$7,530,000	53	Deeper Basin "B"
3*	\$7,966,000	46	Deeper Basins "B"&"C"
3a*	\$8,818,000	40	Concrete channel "D" for Alt. 3

* -- Basins for these options may not have enough land for K & G purpose.

** -- Note that the cost estimates were performed for selected drainage elements only for alternative selection purposes. All common items were excluded from these costs. Therefore, the cost estimates shown are not the true total construction cost of any alternatives.

TABLE 3 COST SUMMARY (a)
(Interim Alternative -No Action by FCDMC/Mesa)

Alternative	MAJOR ELEMENTS	LAND REQUIREMENT	COMMENTS
Interim Alternative	\$2,347,000	2	Earth channel "D" & "H" only
Interim Alternative-a	\$4,330,000	12	Concrete channel "D" & "H" only

(a) No action by FCDMC/Mesa conditions. Developer may have to safely convey flood water to D/S washes prior to building new homes for the southern portion of the Sunland Springs Village. This option should be considered as an interim alternative since it is not a complete solution. It does not address the peak flow attenuation and the floodplains D/S of Signal Butte will remain the same as they are.

TABLE 4

Probable Cost for Preferred Alternative

<u>Siphon Draw Drainage Improvements Alternatives</u>					
Flood Control District of Maricopa County					W/P # 031902.04
FCD 2003 C019					
Table 4a Preliminary Opinion of Probable Cost for Preferred Alternative (Concrete Channel "D")					
MAJOR SYSTEM ELEMENTS					
ITEM	DESCRIPTION	UNIT PRICE	UNIT	QUANTITY	AMOUNT
1	Excavation (Channel "D")	\$6	CY	27,400	\$164,400
2	Concrete Lining (Channel "D")	\$500	CY	2,327	\$1,163,500
3	Excavation (Basin "A")	\$6	CY	15,000	\$90,000
4	Excavation (Basin "T1")	\$6	CY	40,000	\$240,000
5	Excavation (Basin "T2")	\$6	CY	32,500	\$195,000
6	Excavation (Basin "T3")	\$6	CY	16,200	\$97,200
7	Excavation (Basin "T4")	\$6	CY	13,000	\$78,000
8	Excavation (Basin "T5")	\$6	CY	14,500	\$87,000
9	Excavation (Basin "T6")	\$6	CY	22,400	\$134,400
10	Excavation (Basin "T7")	\$6	CY	6,500	\$39,000
11	Excavation (Basin "C1")	\$6	CY	60,000	\$360,000
12	Excavation (Basin "C2")	\$6	CY	130,400	\$782,400
13	Excavation (Basin "B1")	\$6	CY	75,700	\$454,200
14	Excavation (Basin "B2")	\$6	CY	84,000	\$504,000
15	3 - 10' x 5' Box Culvert ("Box 1")	\$500	CY	200	\$100,000
16	1 - 10' x 4' Box Culvert ("Box 2")	\$500	CY	90	\$45,000
17	1 - 10' x 6' Box Culvert ("Box 3")	\$500	CY	185	\$92,500
18	2-10'x4' Culverts ("C1" to "C2"&"B1" to "B2")	\$500	CY	200	\$100,000
19	60" RGRCP Pipe to Basin "T4"	\$180	LF	70	\$12,600
20	54" RGRCP Pipe "F" and "G"	\$150	LF	6,500	\$975,000
21	48" RGRCP Pipes "T4" to "T5" & "B2" to "G"	\$135	LF	500	\$67,500
22	42" RGRCP Pipe "T5" to "T6"	\$120	LF	200	\$24,000
23	24" RGRCP Pipe "T6" to "T7"	\$55	LF	200	\$11,000
24	18" RGRCP Pipe for Basins T1,T2,T3,T7	\$45	LF	850	\$38,250
25	12" RGRCP Pipe for Basin A	\$35	LF	200	\$7,000
26	Concrete Weirs "T1", "T2", and "T3"	\$500	CY	540	\$270,000
27	Concrete Splitter Structure ST1 ("D3" to "C2")	\$500	CY	340	\$170,000
28	Concrete Splitter Structure ST2 (SDW to "B2")	\$500	CY	340	\$170,000
29	Gunite Channel to from "D3" to Basin "T4"	\$500	CY	400	\$200,000
30	Gunite Channel from "ST2" to SDW	\$500	CY	200	\$100,000
31	Manholes	\$6,000.00	EA	17	\$102,000
32	Inlet to Pipe "F"	\$5,000.00	EA	1	\$5,000
33	Flap Gate	\$3,000.00	EA	3	\$9,000
34	Loose Riprap	\$60	CY	1,500	\$90,000
35	Landscaping	\$0.50	SF	2,482,920	\$1,241,460
SUBTOTAL MAJOR SYSTEM ELEMENTS					\$8,219,410
CONTINGENCIES:					
Construction				20%	\$1,643,882
Design & Field Engineering				18%	\$1,775,393
Change Orders				7%	\$690,430
TOTAL MAJOR SYSTEM ELEMENTS					\$12,329,100
1. Construction Contingencies @ 25% of the Total Construction Cost					
2. Design and Field Engineering Costs @ 18% of the sum of Total Construction Cost and Construction Contingencies					
3. Change Orders @ 7% of the sum of Total Construction Cost and Construction Contingencies					
LAND REQUIREMENT:					
ITEM	DESCRIPTION		UNIT	QUANTITY	
1	City of Mesa Plant Site		AC	36.0	
2	SRP Corridor (basins area)		AC	23.4	
3	Sunland Springs Village		AC	8.0	

APPENDIX A

**HEC-1 Model Input and Output Files
for Preferred Alternative**

```

129 ID *****
130 ID FILENAME: SDIBB.DAT
131 ID
132 ID THIS MODEL REPRESENTS THE FUTURE CONDITION OF THE WATERSHED.
133 ID TOTAL DRAINAGE AREA IS APPROXIMATELY 213 SQ. MI.
134 ID THIS MODEL USES A Kn VALUE OF 0.09 FOR DESERT LAND USE DUE TO SHEET FLOW
135 ID CONDITIONS.
136 ID
137 ID 100-YEAR 24-HOUR FREQUENCY
138 ID AREAL REDUCTIONS FROM FCD HYDROLOGY MANUAL
139 ID THIS MODEL INCLUDES INFLOW FROM NORTH OF THE SUPERSTITION FREEWAY
140 ID AND EAST OF THE CAP
141 ID
142 ID DATA FROM THE QUEEN CREEK ADMS HAS BEEN ADDED TO CALCULATE FLOWS INTO THE
143 ID EMF. MUSKINGUM ROUTING NSTEPS WERE ADJUSTED TO BE WITHIN THE SUGGESTED
144 ID RANGE.
145 ID
146 ID METHODOLOGY
147 ID THE US CORPS OF ENGINEERS FLOOD HYDROLOGY MODEL HEC-1 DATED SEP1990 VER 4.0
148 ID SCS TYPE II RAINFALL DISTRIBUTION
149 ID S-GRAPH HYDROGRAPH
150 ID GREEN AND AMPT INFILTRATION EQUATION USED FOR CALCULATING LOSSES
151 ID NORMAL DEPTH STORAGE CHANNEL ROUTING
152 ID APPROXIMATE DIRECTION, LOCATION, AND LENGTH OF THE WASHES HAVE BEEN
153 ID EVALUATED BASED ON FIELD INVESTIGATION, USGS MAPS, LANDIS AERIAL SURVEYS
154 ID DATED 1994
155 ID THE NOAA TECHNICAL MEMORANDUM NOAA ATLAS 2 DEPTH AREA RATIOS
156 ID
157 ID ORIGINAL STUDY PERFORMED BY LISA C. YOUNG AND AFSHIN AHOUREIYAN, UPDATED BY
158 ID DAVID DEGERNESS (OCT-DEC, 1996). REVIEWED BY VALERIE A. SWICK
159 ID AND AMIR MOTAMEDI OF THE FLOOD CONTROL DISTRICT
160 ID HYDROLOGY BRANCH ENGINEERING DIVISION, FLOOD CONTROL
161 ID DISTRICT OF MARICOPA COUNTY, DECEMBER - JULY 1995.
162 ID
163 ID ASSUMED VELOCITY OF 1 FT/SEC FOR SHEET FLOW, 2-3 FT/SEC FOR WASH/NATURAL
164 ID CHANNEL, 3 FT/SEC FOR ROAD AND GRASS CHANNEL, 10FT/SEC FOR CONCRETE CHANNEL
165 ID

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HEC-1 INPUT

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LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10
166 ID VELOCITIES FOR ADMP IMPROVEMENT CHANNELS FROM DIBBLE AND ASSOCIATES
167 ID SUGGESTED ALTERNATIVES (JULY 1, 1997)
168 ID
169 ID *****
170 ID **** THE FOLLOWING NOTE WAS ADDED BY PRIMATECH ENGINEERS ON 06-12-2001 ****
171 ID *****
172 ID NOTE: MUST USE NEBUILD.DSS AS THE DSS FILE TO IMPORT FLOWS ACROSS THE
173 ID SUPERSTITION FREEWAY.
174 ID *****
175 ID
176 ID
177 ID NOTE: MUST USE NDIBF.DSS AS THE DSS FILE TO IMPORT FLOWS ACROSS THE
178 ID SUPERSTITION FREEWAY.
179 ID
180 ID DDM MCUHP2 SE MESA ADMP - SOUTH OF SUPERSTITION FWY, FUTURE CONDITIONS
    *DIAGRAM
181 IT      5  1APR97  0000  1000
182 IO      5
183 IN     15
184 JD    3.60  0.01
185 PC    .000  .002  .005  .008  .011  .014  .017  .020  .023  .026
186 PC    .029  .032  .035  .038  .041  .044  .048  .052  .056  .060
187 PC    .064  .068  .072  .076  .080  .085  .090  .095  .100  .105
188 PC    .110  .115  .120  .126  .133  .140  .147  .155  .163  .172
189 PC    .181  .191  .203  .218  .236  .257  .283  .387  .663  .707
190 PC    .735  .758  .776  .791  .804  .815  .825  .834  .842  .849
191 PC    .856  .863  .869  .875  .881  .887  .893  .898  .903  .908
192 PC    .913  .918  .922  .926  .930  .934  .938  .942  .946  .950
193 PC    .953  .956  .959  .962  .965  .968  .971  .974  .977  .980
194 PC    .983  .986  .989  .992  .995  .998  1.000
195 JD    3.38  10.0
196 JD    3.24  30.0
197 JD    3.10  60.0
198 JD    3.05  90.0
199 JD    3.00  120.0
200 JD    2.97  150.0
    *
201 KK    SOSS
202 KM    INFLOW FROM SOSSAMAN BASIN VIA SOSSAMAN CHANNEL
203 KM    QI CARDS ARE BASED ON THE PEAK OF 1800CFS TO SOSSAMAN CHANNEL
204 BA    12.50
205 ZR    =QI A=SOSSAMAN DRAIN B=AT SUPERSTITION C=FLOW E=5MIN F=100YR
    *
    * DDM ***** Preserved *****

```

206 KK RSOSS
 207 KM ROUTE FLOWS VIA SOSSAMAN CHANNEL TO BASELINE ROAD
 208 RS 1 FLOW -1
 209 RC .030 .025 .030 3500 .005
 210 RX 0 5 10 35 75 110 115 120
 211 RY 10 10 10 4 4 10 10 10

* DDM ***** Updated *****

HEC-1 INPUT

PAGE 5

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

212 KK 59A
 213 KM BASIN 59A
 214 KM THE FOLLOWING PARAMETERS WERE PROVIDED FOR THIS BASIN
 215 KM L= .9 Lca= .3 S= 34.9 Kn= .070 LAG= 29.7
 216 KM PHOENIX VALLEY S-GRAPH WAS USED FOR THIS BASIN
 217 BA .26
 218 LG .23 .25 4.55 .42 33.00
 219 UI 30. 77. 144. 186. 246. 364. 293. 226. 172. 123.
 220 UI 64. 48. 30. 15. 9. 9. 9. 0. 0. 0.
 221 UI 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.

222 KK R59A
 223 KM RETAIN THE 100 YEAR 2 HOUR RUNOFF VOLUME
 224 DT D59A 2
 225 DI 0 10000
 226 DQ 0 10000

227 KK C59A
 228 KM SOSSAMAN DRAIN AT BASELINE ROAD
 229 HC 2

230 KK 59A59B
 231 KM ROUTE S59A TO 59B VIA SOSSAMAN CHANNEL
 232 KM BLOCK WALL ON LEFT BANK, SOSSAMAN ROAD ON RIGHT BANK
 233 RS 4 FLOW -1
 234 RC .025 .018 .013 6500 .0015
 235 RX 0 3 13 38 78 103 128 203
 236 RY 16 10 10 0 0 10 8 10

* DDM ***** Updated *****

237 KK 59B
 238 KM BASIN 59B
 239 KM THE FOLLOWING PARAMETERS WERE PROVIDED FOR THIS BASIN
 240 KM L= 1.2 Lca= .7 S= 33.9 Kn= .087 LAG= 58.3
 241 KM PHOENIX VALLEY S-GRAPH WAS USED FOR THIS BASIN
 242 BA .94
 243 LG .22 .24 4.65 .41 24.00
 244 UI 54. 54. 93. 193. 244. 284. 318. 361. 415. 501.
 245 UI 653. 666. 546. 473. 422. 364. 319. 273. 233. 172.
 246 UI 110. 94. 89. 68. 54. 54. 19. 17. 17. 17.
 247 UI 17. 17. 17. 17. 0. 0. 0. 0. 0. 0.
 248 UI 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.

* DDM ***** Preserved *****

HEC-1 INPUT

PAGE 6

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

249 KK R59B
 250 KM RETAIN 100 YR 2 HR RUNOFF VOLUME
 251 DT D59B 63
 252 DI 0 10000
 253 DQ 0 10000

* DDM ***** Preserved *****

254 KK C59B
 255 KM SOSSAMAN CHANNEL AT GUADALUPE ROAD
 256 HC 2

* DDM ***** Preserved *****

257 KK 59BT60
 258 KM ROUTE 59B TO 60 GUADALUPE CHANNEL. Assumed v=5ft/sec for NSTP calculation
 259 RS 4 FLOW -1
 260 RC .02 .013 .02 5500 .0005
 261 RX 0 518 522 522 560 560 580 2580
 262 RY 8.5 8.5 8.5 0 0 8 7 6

* DDM ***** Updated *****

263 KK 60
 264 KM BASIN 60
 265 KM THE FOLLOWING PARAMETERS WERE PROVIDED FOR THIS BASIN
 266 KM L= 2.4 Lca= 1.4 S= 31.8 Kn= .087 LAG= 102.0
 267 KM PHOENIX VALLEY S-GRAPH WAS USED FOR THIS BASIN
 268 BA 2.30
 269 LG .18 .24 4.65 .43 35.00
 270 UI 76. 76. 76. 76. 130. 250. 286. 330. 364. 395.
 271 UI 422. 452. 484. 522. 571. 607. 689. 820. 915. 1008.
 272 UI 885. 793. 723. 669. 626. 591. 536. 496. 459. 424.
 273 UI 387. 362. 324. 276. 219. 169. 134. 134. 126. 125.
 274 UI 121. 76. 76. 76. 76. 56. 23. 23. 23. 23.
 275 UI 23. 23. 23. 23. 23. 23. 23. 23. 23. 23.
 276 UI 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.
 277 UI 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.

* DDM ***** Preserved *****

278 KK R60
 279 KM RETAIN 100 YR 2 HR RUNOFF VOLUME
 280 DT D60 170
 281 DI 0 10000
 282 DQ 0 10000

* DDM ***** Preserved *****

HEC-1 INPUT

PAGE 7

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

283 KK EMFGUA
 284 KM COMBINE S59 AND S60 AT EMF, GUADALUPE ROAD
 * KO 21
 285 HC 2

* DDM ***** Preserved *****

286 KK GUATEL
 287 KM ROUTE EMF FLOW FROM GUADALUPE ROAD TO ELLIOT ROAD
 288 RS 3 FLOW -1
 289 RC .03 .022 .03 6000 .0003
 290 RX 0 500 520 553 693 726 740 742
 291 RY 14 12 11 0 0 11 11 12

* DDM ***** Updated *****

292 KK 64
 293 KM BASIN 64
 294 KM THE FOLLOWING PARAMETERS WERE PROVIDED FOR THIS BASIN
 295 KM L= 1.2 Lca= .6 S= 25.4 Kn= .051 LAG= 34.4
 296 KM PHOENIX VALLEY S-GRAPH WAS USED FOR THIS BASIN
 297 BA .81
 298 LG .18 .25 4.70 .41 54.00
 299 UI 79. 155. 338. 438. 543. 709. 988. 778. 624. 493.
 300 UI 388. 253. 139. 120. 79. 45. 24. 24. 24. 24.
 301 UI 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.
 302 UI 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.

* DDM ***** Preserved *****

303 KK R64
 304 KM R64 IS WHAT REMAINS AFTER THE DIVERSION OF FLOW UP TO 67 AC-FT. THIS IS SENT
 305 KM TO TAPE 21 FOR RECALL INTO FCD'S EMF MODELS. KK BLOCK THERE MUST BE UPDATED
 306 KM TO REFLECT THE CHANGE OF WHAT GETS SENT TO THE TAPE 21.
 307 KM RETAIN 100 YR 2 HR RUNOFF VOLUME

* KO 21

308 DT D64 67
 309 DI 0 10000
 310 DQ 0 10000

* DDM ***** Preserved *****

311 KK EMFELL
 312 KM COMBINE EMF FLOW WITH FLOW FROM SUBBASIN 64 AT ELLIOT ROAD
 313 HC 2

* DDM ***** Preserved *****

314 KK ELTWAR
 315 KM ROUTE EMF FLOW AT ELLIOT ROAD TO WARNER ROAD VIA THE EMF
 316 RS 2 FLOW -1
 317 RC .03 .022 .03 5500 .0003
 318 RX 0 500 520 553 693 726 740 742
 319 RY 14 12 11 0 0 11 11 12

*

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

320 KK 62B
 321 KM BASIN 62B
 322 KM THE FOLLOWING PARAMETERS WERE PROVIDED FOR THIS BASIN
 323 KM L= .6 Lca= .3 S= 47.5 Kn= .021 LAG= 8.0
 324 KM PHOENIX VALLEY S-GRAPH WAS USED FOR THIS BASIN
 325 BA .23
 326 LG .11 .25 4.65 .46 78.00
 327 UI 334. 940. 431. 83. 0. 0. 0. 0. 0.
 328 UI 0. 0. 0. 0. 0. 0. 0. 0. 0.
 *

329 KK R62B
 330 KM RETAIN 100 YR 2 HR RUNOFF VOLUME
 331 DT D62B 19
 332 DI 0 10000
 333 DQ 0 10000
 *

* DDM ***** Preserved *****

334 KK 62BTD
 335 KM ROUTE 62B TO 62D VIA HAWES ROAD
 336 RS 6 FLOW -1
 337 RC .045 .04 .045 5280 .0041
 338 RX 0 100 125 127 177 179 224 324
 339 RY 3 2 1.50 0 0 1.5 2 3
 *

* DDM ***** Updated *****

340 KK 62D
 341 KM BASIN 62D
 342 KM THE FOLLOWING PARAMETERS WERE PROVIDED FOR THIS BASIN
 343 KM L= .9 Lca= .3 S= 30.7 Kn= .045 LAG= 21.3
 344 KM PHOENIX MOUNTAIN S-GRAPH WAS USED FOR THIS BASIN
 345 BA .46
 346 LG .23 .25 4.65 .40 50.00
 347 UI 76. 300. 519. 753. 475. 369. 286. 203. 163. 111.
 348 UI 85. 63. 47. 36. 26. 14. 14. 14. 14. 0.
 349 UI 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.
 350 UI 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.
 *

* DDM ***** Preserved *****

351 KK R62D
 352 KM RETAIN 100 YR 2 HR RUNOFF VOLUME
 353 DT D62D 35
 354 DI 0 10000
 355 DQ 0 10000
 *

* DDM ***** Preserved *****

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

356 KK CP62D
 357 KM COMBINE FLOWS FROM SUBBASINS 62B AND 62D
 358 HC 2
 *

* DDM ***** Preserved *****

359 KK 62DTF
 360 KM ROUTE 62D TO 62F VIA HAWES ROAD
 361 RS 4 FLOW -1
 362 RC .045 .024 .045 3600 .0033
 363 RX 0 500 750 753 793 796 1046 1546
 364 RY 3 1.5 1.25 0 0 1.25 1.5 3
 *

* DDM ***** Updated *****

365 KK 62F
 366 KM BASIN 62F
 367 KM THE FOLLOWING PARAMETERS WERE PROVIDED FOR THIS BASIN
 368 KM L= .6 Lca= .4 S= 31.9 Kn= .042 LAG= 18.1
 369 KM PHOENIX VALLEY S-GRAPH WAS USED FOR THIS BASIN
 370 BA .26
 371 LG .21 .25 4.65 .41 54.00
 372 UI 66. 224. 350. 546. 371. 235. 98. 56. 18. 15.
 373 UI 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.
 374 UI 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.
 *

* DDM ***** Preserved *****

375 KK R62F

376 KM RETAIN 100 YR 2 HR RUNOFF VOLUME
 377 DT D62F 18
 378 DI 0 10000
 379 DQ 0 10000
 *
 * DDM ***** Preserved *****

380 KK CP62F
 381 KM COMBINE FLOWS FROM 62D AND 62F
 382 HC 2
 *
 * DDM ***** Preserved *****

383 KK 62T63
 384 KM ROUTE CP62F TO SUBBASIN 63 VIA WASH.
 385 KM WASH CROSSES HAWES, NORTH OF ELLIOT
 386 RS 7 FLOW -1
 387 RC .045 .04 .045 6000 0.0055
 388 RX 0 500 750 770 780 800 1050 1550
 389 RY 5 4 3 0 0 3 4 5
 *
 * DDM ***** Inserted *****

HEC-1 INPUT

PAGE 10

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

390 KK 63
 391 KM BASIN 63
 392 KM THE FOLLOWING PARAMETERS WERE PROVIDED FOR THIS BASIN
 393 KM L= 1.4 Lca= .7 S= 28.2 Kn= .035 LAG= 26.8
 394 KM PHOENIX VALLEY S-GRAPH WAS USED FOR THIS BASIN
 395 BA .91
 396 LG .18 .25 4.65 .43 55.00
 397 UI 114. 346. 595. 780. 1159. 1291. 930. 689. 485. 241.
 398 UI 170. 113. 35. 35. 35. 35. 0. 0. 0. 0.
 399 UI 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.
 *
 * DDM ***** Preserved *****

400 KK R63
 401 KM RETAIN 100 YR 2 HR RUNOFF VOLUME
 402 DT D63 71
 403 DI 0 10000
 404 DQ 0 10000
 *
 * DDM ***** Preserved *****

405 KK CP63
 406 KM COMBINE FLOWS FROM SUBBASIN 63 AND CP62F
 407 HC 2
 *
 * DDM ***** Preserved *****

408 KK 63T71
 409 KM ROUTE CP63 TO S71 VIA SHEET FLOW
 410 KM SOSSAMAN SOUTH OF ELLIOT
 411 RS 6 FLOW -1
 412 RC .055 .045 .055 5280 .0005
 413 RX 0 1000 1005 1010 1013 1043 1543 2043
 414 RY 6 5 0 0 3 5 2 5
 *
 * DDM ***** Updated *****

415 KK 68B
 416 KM BASIN 68B
 417 KM THE FOLLOWING PARAMETERS WERE PROVIDED FOR THIS BASIN
 418 KM L= .6 Lca= .3 S= 32.3 Kn= .020 LAG= 7.7
 419 KM PHOENIX VALLEY S-GRAPH WAS USED FOR THIS BASIN
 420 BA .25
 421 LG .10 .25 5.20 .36 80.00
 422 UI 377. 1018. 412. 72. 0. 0. 0. 0. 0. 0.
 423 UI 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.
 *

HEC-1 INPUT

PAGE 11

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

424 KK R68
 425 KM RETAIN 100 YR 2 HR RUNOFF VOLUME
 426 DT D68B 24
 427 DI 0 10000
 428 DQ 0 10000
 *

429 KK 68BT69
 430 KM ROUTE S68B TO S69 VIA WASH CROSSING HAWES
 431 RS 3 FLOW -1
 432 RC .045 .04 .045 2750 .0036
 433 RX 0 500 950 1003 1007 1057 1511 2011
 434 RY 4 3.5 3 0 0 2 2.5 3
 *

435 KK 69
 436 KM BASIN 69
 437 KM THE FOLLOWING PARAMETERS WERE PROVIDED FOR THIS BASIN
 438 KM L= .7 Lca= .3 S= 22.4 Kn= .020 LAG= 9.0
 439 KM PHOENIX VALLEY S-GRAPH WAS USED FOR THIS BASIN
 440 BA .09
 441 LG .10 .25 4.70 .45 80.00
 442 UI 104. 320. 213. 54. 11. 0. 0. 0. 0. 0.
 443 UI 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.
 *

* DDM ***** Preserved *****

444 KK R69
 445 KM RETAIN 100 YR 2 HR RUNOFF VOLUME
 446 DT D69 9
 447 DI 0 10000
 448 DQ 0 10000
 *

* DDM ***** Preserved *****

449 KK C69
 450 KM COMBINE FLOWS FROM SUBBASIN 68B AND 69
 451 HC 2
 *

* DDM ***** Preserved *****

452 KK 69T71
 453 KM ROUTE S69 TO S71 VIA WASH AND SHEET FLOW, INCREASE OVERBANK N VALUES
 454 RS 7 FLOW -1
 455 RC .055 .045 .055 6000 .0033
 456 RX 0 500 1000 1001 1002 1500 2000 2500
 457 RY 4 3 2 0 0 2 3 4
 *

* DDM ***** Inserted *****

HEC-1 INPUT

PAGE 12

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

458 KK 71
 459 KM BASIN 71
 460 KM THE FOLLOWING PARAMETERS WERE PROVIDED FOR THIS BASIN
 461 KM L= 1.6 Lca= .8 S= 26.4 Kn= .020 LAG= 16.8
 462 KM PHOENIX VALLEY S-GRAPH WAS USED FOR THIS BASIN
 463 BA 1.09
 464 LG .10 .25 4.65 .47 80.00
 465 UI 331. 1085. 1805. 2349. 1459. 780. 329. 144. 67. 67.
 466 UI 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.
 467 UI 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.
 *

468 KK R71
 469 KM RETAIN 100 YR 2 HR RUNOFF VOLUME
 470 DT D71 106
 471 DI 0 10000
 472 DQ 0 10000
 *

* DDM ***** Preserved *****

473 KK C71
 474 KM original wording "COMBINE S71 WITH S69 AND S68B"
 475 KM was revised by Dibble to be "COMBINE FLOWS FROM S63, S68B, S69 AND S71,
 476 KM WHICH COMBINES HYDROGRAPHS 63T71, 69T71 AND R71"
 477 KM CONCENTRATION POINT IS ALONG SOSSAMAN AT THE MESQUITE ST ALIGNMENT
 478 HC 3
 *

479 KK 71T72
 480 KM ROUTE C71 TO S72 VIA DIKE
 481 KM WASH WEST OF INTERSECTION OF SOSSAMAN & WARNER
 482 RS 4 FLOW -1
 483 RC .055 .045 .055 3750 .0037
 484 RX 0 500 1000 1007 1017 1025 1530 2030
 485 RY 9 8.5 8 0 0 8 8.5 9
 *

* DDM ***** Inserted *****

486 KK 72
 487 KM BASIN 72

488 KM THE FOLLOWING PARAMETERS WERE PROVIDED FOR THIS BASIN
 489 KM L= 1.6 Lca= .9 S= 13.1 Kn= .020 LAG= 20.3
 490 KM PHOENIX VALLEY S-GRAPH WAS USED FOR THIS BASIN
 491 BA .84
 492 LG .10 .25 5.40 .33 80.00
 493 UI 161. 600. 906. 1496. 1347. 912. 566. 247. 153. 50.
 494 UI 43. 43. 0. 0. 0. 0. 0. 0. 0. 0.
 495 UI 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.
 *

* DDM ***** Preserved *****

HEC-1 INPUT

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LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

496 KK R72
 497 KM RETAIN 100 YR 2 HR RUNOFF VOLUME
 498 DT D72 83
 499 DI 0 10000
 500 DQ 0 10000
 *
 * CONCENTRATION POINT ADDED PRIOR TO EMF COMBINE SO THAT FLOWS CAN BE SENT TO
 * TAP21.

501 KK CPWAR
 * KO 21
 502 HC 2
 *
 * DDM ***** Preserved *****

503 KK EMFWAR
 504 KM COMBINE ROUTED FLOW FROM 71 WITH 72 WITH EMF (HYDROGRAPH ELTWAR)
 505 HC 2
 * DDM ***** Preserved *****

506 KK WARTKN
 507 KM ROUTE EMF WARNER ROAD FLOW TO KNOX ROAD
 508 RS 1 FLOW -1
 509 RC .03 .022 .03 2500 .0003
 510 RX 0 500 520 553 693 726 740 742
 511 RY 14 12 11 0 0 11 11 12
 *

512 KK 70B
 513 KM BASIN 70B
 514 KM THE FOLLOWING PARAMETERS WERE PROVIDED FOR THIS BASIN
 515 KM L= 1.6 Lca= 1.1 S= 29.9 Kn= .022 LAG= 20.7
 516 KM PHOENIX VALLEY S-GRAPH WAS USED FOR THIS BASIN
 517 BA .38
 518 LG .11 .15 8.00 .12 76.00
 519 UI 68. 259. 390. 636. 615. 418. 267. 119. 73. 30.
 520 UI 19. 19. 0. 0. 0. 0. 0. 0. 0. 0.
 521 UI 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.
 *
 * DDM ***** Preserved *****

522 KK R70B
 523 KM RETAIN 100 YR 2 HR RUNOFF VOLUME
 524 DT D70B 38
 525 DI 0 10000
 526 DQ 0 10000
 *

527 KK 70BT76
 528 KM ROUTE 70B TO 76B VIA WASH CROSSING SOSSAMAN, SOUTH OF WARNER ROAD
 529 RS 6 FLOW -1
 530 RC .045 .04 .045 5500 .0041
 531 RX 0 500 1000 1003 1007 1011 1511 2011
 532 RY 4 3.5 3 0 0 2 2.5 3
 *

HEC-1 INPUT

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LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

533 KK 76B
 534 KM BASIN 76B
 535 KM THE FOLLOWING PARAMETERS WERE PROVIDED FOR THIS BASIN
 536 KM L= 1.8 Lca= .9 S= 27.4 Kn= .021 LAG= 18.9
 537 KM PHOENIX VALLEY S-GRAPH WAS USED FOR THIS BASIN
 538 BA .64
 539 LG .10 .15 8.80 .09 78.00
 540 UI 148. 515. 789. 1294. 957. 629. 303. 157. 70. 35.
 541 UI 35. 0. 0. 0. 0. 0. 0. 0. 0. 0.
 542 UI 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.
 *
 * DDM ***** Preserved *****

543 KK R76B
544 KM RETAIN 100 YR 2 HR RUNOFF VOLUME
545 DT D76B 66
546 DI 0 10000
547 DQ 0 10000
*

548 KK KNOX
549 KM COMBINE FLOWS AT KNOX ROAD
* KO 21
550 HC 2
*

551 KK EMFKNX
552 KM COMBINE FLOWS INTO THE EMF AT KNOX ROAD
553 KM THIS COMBINES HYDROGRAPHS WARTKN, 70BT76 and R76B.
554 HC 2
*

555 KK 65A
556 KM BASIN 65A
557 KM THE FOLLOWING PARAMETERS WERE PROVIDED FOR THIS BASIN
558 KM L= 1.60 Lca= .90 S= 51.2 Kn= .089 LAG= 69.7
559 KM PHOENIX VALLEY S-GRAPH WAS USED FOR THIS BASIN
560 BA 2.535
561 LG .35 .36 5.10 .26 1.00
562 UI 122. 122. 122. 345. 457. 572. 635. 703. 775. 875.
563 UI 972. 1166. 1460. 1573. 1316. 1148. 1032. 937. 826. 739.
564 UI 654. 583. 493. 362. 244. 217. 201. 198. 122. 122.
565 UI 122. 52. 38. 38. 38. 38. 38. 38. 38. 38.
566 UI 38. 0. 0. 0. 0. 0. 0. 0. 0. 0.
567 UI 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.
*

568 KK REMA1
569 KM DIVERT 37% OF FLOW TO SUBBASIN 65A1
570 DT 65A1
571 DI 0 100 200 500 1000 2000 4000
572 DQ 0 37 74 185 370 740 1480
*

HEC-1 INPUT

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1
LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

573 KK REMA2
574 KM DIVERT 28.6% OF REMA1 FLOW (18% of 65A) TO SUBBASIN 65A2a
575 DT 65A2a
576 DI 0 100 200 500 1000 2000 4000
577 DQ 0 28.6 57.2 143 286 572 1144
*

578 KK REMA3
579 KM DIVERT 15.6% OF REMA2 FLOW (7% of 65A) TO SUBBASIN 65A2b
580 DT 65A2b
581 DI 0 100 200 500 1000 2000 4000
582 DQ 0 15.6 31.2 78 156 312 624
*

583 KK 65A3
584 KM DIVERT 13.2% OF REMA3 FLOW (5% of 65A) TO SUBBASIN 65A2c
585 DT 65A2c
586 DI 0 100 200 500 1000 2000 4000
587 DQ 0 13.2 26.4 66 132 264 528
*

588 KK CAP1A
589 KM INFLOW FROM EAST OF THE CAP THROUGH 2 - 72" PIPE OVERCHUTES
590 KM STATION #131+90 AND 158+00 SALT-GILA AQUEDUCT REACH 2
591 BA 0.965
592 ZR =QI A=CAP1A B=OVERCHUTE C=FLOW E=5MIN F=100YEAR
*

593 KK RCAP1A
594 KM ROUTE FLOW FROM CAP OVERCHUTE TO A POINT ON THE MARICOPA/PINAL COUNTY LINE
595 KM 2000 FEET NORTH OF THE GUADALUPE ROAD COUNTY LINE INTERSECTION. ROUTING WILL
596 KM BE BY A NATURAL CHANNEL. THIS IS THEN ROUTED FOR 1200 FT
597 KM IN A CHANNEL (DIBBLE ID MN3) TO THE POINT WHERE THE ROUTED CAP1B FLOW
598 KM INTERCEPTS THE CHANNEL. ORIGINAL SLOPE =.01
599 RS 13 FLOW -1
600 RC .045 .04 .045 4900 .010
601 RX 0 500 1000 1006 1026 1032 1511 2011
602 RY 4 3.5 3 0 0 3 3.5 4
*

603 KK RRCPIA

604 KM REACH MN-5 AND CULVERT MNC-1
 605 KM ROUTE FLOW FROM WHERE RCAP1A FLOWS INTO THE NEW CHANNEL ALONG MERIDIAN ROAD
 606 KM USES REVISED ROUTING PARAMETERS, CHANNEL MN-5 SHAPE
 607 RS 1 FLOW -1
 608 RC 0.025 0.015 0.025 2350 .0017
 609 RX 0 8 16 27 43 53 61 69
 610 RY 5.1 5.2 5.3 0 0 5.3 5.2 5.1
 *

HEC-1 INPUT

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

611 KK C65A1
 612 KM COMBINE FLOWS FROM SUBBASIN 65A3 and CAP1A
 * KO 1
 613 HC 2 1.81
 *
 614 KK RCHL1
 615 KM ROUTE FLOW FROM GUADALUPE BASIN TO D/S of 65A2c
 616 RS 2 FLOW -1
 617 RC 0.025 0.015 0.025 1500 .0015
 618 RX 0.0 8.0 16.0 34 56 74 82 90
 619 RY 8.8 8.9 9.0 0 0 9.0 8.9 8.8
 *

620 KK D65A2c
 621 KM RETURN FLOW FROM SUBBASIN 65A2c
 622 DR 65A2c
 *

623 KK CHAN1
 * KO 1
 624 KM TOTAL FLOW AT MERIDIAN ROAD CHANNEL (U/P Stream)
 625 HC 2 1.94
 *

626 KK RCHL2
 627 KM ROUTE FLOW FROM 65A2c TO 65A2b
 628 RS 2 FLOW -1
 629 RC 0.025 0.015 0.025 1300 .0015
 630 RX 0.0 8.0 16.0 34 56 74 82 90
 631 RY 8.8 8.9 9.0 0 0 9.0 8.9 8.8
 *

632 KK CAP1B
 633 KM INFLOW FROM EAST OF THE CAP THROUGH 2 - 72" PIPE OVERCHUTES
 634 KM STATION #131+90 AND 158+00 SALT-GILA AQUEDUCT REACH 2
 635 BA 0.965
 636 ZR =QI A=CAP1B B=OVERCHUTE C=FLOW E=5MIN F=100YEAR
 *

637 KK RCAP1B
 638 KM ROUTE FLOW FROM CAP1B OVERCHUTE TO A POINT ALONG THE MARICOPA/PINAL COUNTY
 639 KM LINE 1000 FEET NORTH OF THE INTERSECTION OF GUADALUPE ROAD AND THE COUNTY
 640 KM LINE. ROUTING WILL BE BY A NATURAL CHANNEL. ORIGINAL SLOPE=.01
 641 RS 14 FLOW -1
 642 RC .045 .04 .045 4900 .010
 643 RX 0 500 1000 1006 1026 1032 1511 2011
 644 RY 4 3.5 3 0 0 3 3.5 4
 *

HEC-1 INPUT

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

645 KK CHAN2
 * KO 1
 646 KM TOTAL FLOW AT MERIDIAN ROAD CHANNEL (Middle Reach)
 647 HC 2 2.91
 *

648 KK D65A2b
 649 KM RETURN FLOW FROM SUBBASIN 65A2b
 650 DR 65A2b
 *

651 KK B1FLOW
 652 KM TOTAL FLOW AT MERIDIAN ROAD FOR BASIN 1
 653 HC 2 3.09
 *

654 KK RCHL3
 655 KM ROUTE FLOW FROM BASIN1 TO MOUNTAIN ROAD BASIN 3
 656 RS 3 FLOW -1
 657 RC 0.040 0.035 0.040 2500 .0050

658	RX	0.0	8.0	16.0	34	56	74	82	90
659	RY	8.8	8.9	9.0	0	0	9.0	8.9	8.8
	*								
660	KK	65B							
661	KM	BASIN 65B							
662	KM	THE FOLLOWING PARAMETERS WERE PROVIDED FOR THIS BASIN							
663	KM	L=	2.0	Lca=	1.2	S=	37.5	Kn=	.036
664	KM	LAG=	36.6						
665	BA	PHOENIX VALLEY S-GRAPH WAS USED FOR THIS BASIN							
666	BA	1.37							
667	LG	.18	.25	6.00	.24	53.00			
668	UI	126.	218.	506.	669.	809.	1014.	1468.	1422.
669	UI	720.	562.	337.	218.	182.	126.	71.	39.
670	UI	39.	0.	0.	0.	0.	0.	0.	0.
671	UI	0.	0.	0.	0.	0.	0.	0.	0.
	*								
671	KK	R65B							
672	KM	RETAIN 100 YR 2 HR RUNOFF VOLUME minus the volume within SRP (19 Ac-Ft)							
673	DT	D65B	101						
674	DI	0	10000						
675	DQ	0	10000						
	*								
676	KK	NW65B							
677	KM	Divert 18% flow to Meridian Pointe subdivision							
678	DT	MPOINT							
679	DI	0	10000						
680	DQ	0	1800						
	*								

HEC-1 INPUT

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

681	KK	N65B							
682	KM	Divert 74% to southwest (79% of 65B), 26% (21% of 65B) to the SRP Corridor							
683	DT	DW65B							
684	DI	0	10000						
685	DQ	0	7400						
	*								
686	KK	CMOUNT							
687	KM	COMBINE FLOWS FROM CHANNEL AND SUBBASIN "F" of Sunland Spring							
688	* KO	1							
688	HC	2	3.38						
	*								
689	KK	DMNT1							
690	KM	Divert about 33 Ac-Ft to East Mountain Road Basins within SRP Corridor							
691	* KO	1							
691	DT	DSRP1							
692	DI	0	50	150	350.0	500	750.0	800	1220
693	DQ	0	0	0	0.0	0.0	0.0	0.0	280
	*								
694	KK	DMNT2							
695	KM	Divert about 18 Ac-Ft to West Mountain Road Basins within SRP Corridor							
696	DT	DSRP2							
697	DI	0	50	350.0	500	800	1000	1050	1500
698	DQ	0	0	0.0	0.0	0.0	0.0	0.0	450
	*								
699	KK	SRP1IN							
700	KM	RETURN FLOW TO SRP BASIN 1							
701	DR	DSRP1							
	*								
702	KK	SRP1							
703	KM	3 Basins with SRP Corridor East Mountain Rd, Total Vol = 33 Ac-Ft							
704	KM	OFF-LINE DETENTION BASIN							
705	KM	WITH 12" OUTLET PIPE AND 100' WEIR, Simplified E-V Relationship							
706	RS	1	STOR	0					
707	SV	0	1.50	3.5	6.5	9.5	12.5	16	24
708	SE	82.0	83.0	84.0	85	86	87	88	90
709	SL	82.5	0.785	.62	.5				
710	SS	92.0	100	2.5	1.5				
	*								
711	KK	CSRP1							
712	KM	COMBINE FLOWS FROM BASIN SRP1 OUTLET AND BYPASS							
713	HC	2							
	*								

HEC-1 INPUT

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

714 KK BPAS3
 715 KM By-pass 300 cfs to channel, and Divert Remaining to Basin 3
 716 DT D_BAS3
 717 DI 0 50 150 300.0 500 700.0 800 1000 1500 2000.
 718 DQ 0 0 0 0.0 200 400.0 500 700 1200 1700.
 *

719 KK RCHL E
 720 KM ROUTE FLOW through Channel E from SRP Corridor to Siphon Draw Wash
 721 RS 1 FLOW -1
 722 RC 0.040 0.035 0.040 1600 .0050
 723 RX 0.0 8.0 16.0 34 56 74 82 90
 724 RY 9.0 9.0 9.0 0 0 9.0 9.0 9.0
 *

725 KK BAS3IN
 726 KM RETURN FLOW TO BASIN 3
 727 DR D_BAS3
 *

728 KK BASN3
 729 KM WEST Mountain Road, North of Siphon Draw Wash
 730 KM OFF-LINE DETENTION BASIN
 731 KM WITH 30" OUTLET PIPE AND 100' WEIR
 732 RS 1 STOR 0
 733 SV 0 7 13 27 41 56 72 87 100 120
 734 SE 70.0 70.5 71.0 72 73 74 75 76 77 78
 735 SL 70.5 4.909 .62 .5
 736 SS 77.0 100 2.5 1.5
 *

737 KK CBAS3
 738 KM COMBINE FLOWS FROM BASIN OUTLET AND BYPASS
 739 HC 2
 *

740 KK D65A2a
 741 KM RETURN FLOW FROM SUBBASIN 65A2a
 742 DR 65A2a
 *

743 KK DB65A1
 744 KM RETURN FLOW FROM SUBBASIN 65A1
 745 DR 65A1
 *

746 KK SD_TOT
 747 KM TOTAL FLOW INTO SIPHON DRAW WASH
 748 HC 2 1.39
 *

HEC-1 INPUT

PAGE 20

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

749 KK RSDW1
 750 KM ROUTE FLOW FROM MERIDIAN ROAD TO MOUNTAIN ROAD THROUGH DRAINAGE CORRIDOR
 751 RS 2 FLOW -1
 752 RC .045 .035 .045 3000 .0070
 753 RX 0.0 30.0 88.0 100 150 162 220 250
 754 RY 9.0 8.0 4.5 0 0 4.5 8.0 9.0
 *

755 KK B65B
 756 KM RETURN FLOW FROM SUBBASIN 65B Meridian Pointe area
 757 DR MPOINT
 *

758 KK SDWASH
 759 KM TOTAL FLOW AT SIPHON DRAW WASH AND MOUNTAIN ROAD
 760 HC 3
 *

761 KK BPSDW
 762 KM By-pass 200 cfs to SIPHON DRAW WASH, and Remaining to Basin B & pipe
 763 DT DSOUTH
 764 DI 0 50 80 200.0 400 600.0 800 1000 1500 2000.
 765 DQ 0 0 0 0.0 200 400.0 600 800 1300 1800.
 *

766 KK RCHL4
 767 KM ROUTE FLOW FROM BASIN3 TO 104TH STREET
 768 RS 3 FLOW -1
 769 RC 0.040 0.035 0.040 5500 .0050
 770 RX 0.0 8.0 16.0 34 56 74 82 90
 771 RY 8.8 8.9 9.0 0 0 9.0 8.9 8.8

```

*
772 KK 65AW
773 KM BASIN 65AW
774 KM THE FOLLOWING PARAMETERS WERE PROVIDED FOR THIS BASIN
775 KM L= .9 Lca= .6 S= 54.7 Kn= .049 LAG= 26.1
776 KM PHOENIX VALLEY S-GRAPH WAS USED FOR THIS BASIN
777 BA .43
778 LG .24 .25 5.30 .29 32.00
779 UI 56. 176. 295. 391. 603. 594. 432. 316. 210. 101.
780 UI 73. 43. 17. 17. 17. 0. 0. 0. 0. 0.
781 UI 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.
*

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```

782 KK R65AW
783 KM RETAIN 100 YR 2 HR RUNOFF VOLUME
784 DT D65AW 31
785 DI 0 10000
786 DQ 0 10000
*

```

HEC-1 INPUT

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

```

787 KK 65AT65
788 KM ROUTE C65A TO BASIN 65B VIA A WASH, (THIS WASH IS NORTH OF SIPHON DRAW)
789 KM THIS IS THE PART OF 65A WHICH IS WEST OF THE MERIDIAN RD ALIGNMENT
790 RS 11 FLOW -1
791 RC .045 .04 .045 9500 .007
792 RX 0 500 1000 1003 1053 1056 1511 2011
793 RY 4 3.5 3 0 0 2 2.5 3
*

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```

794 KK W65B
795 KM RETURN FLOW FROM SUBBASIN E65B
796 DR DW65B
*

```

```

797 KK CP65B
798 KM COMBINE FLOW FROM SUBBASIN 65AW (WEST OF MERIDIAN RD) WITH FLOW FROM
799 KM SUBBASIN 65B, AND FLOW FROM BASIN1
800 HC 3 4.60
*

```

```

801 KK DI65B
802 KM DIVERSION STRUCTURE TO ROUTE PEAK FLOW TO ELLIOT BASIN (EAST)
803 KM By-pass 370 cfs to Elliot Storm Drain, and Divert Remaining to E Basin
804 DT DIRS65
805 DI 0 100 200 370.0 500 600.0 800 1000 1300 1800.
806 DQ 0 0 0 0.0 130 230.0 430 630 930 1430.
*

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```

807 KK S_TOT
808 KM RETURN FLOW TO Siphon Draw Wash Basin B and Pipe
809 DR DSOUTH
*

```

```

810 KK BPAS2
811 KM By-pass 80 cfs to Pipe, and Remaining to Basin B
812 DT D_BAS2
813 DI 0 50 80 200.0 400 600.0 800 1000 1500 2000.
814 DQ 0 0 0 120.0 320 520.0 720 920 1420 1920.
*

```

```

815 KK PIPE_F
816 KM ROUTE FLOW FROM SIPHON DRAW WASH TO ELLIOT ROAD THROUGH PIPE
817 RK 1400 0.0050 0.012 CIRC 5
*

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818 KK BAS2IN
819 KM RETURN FLOW TO Siphon Draw Wash Basin
820 DR D_BAS2
*

```

HEC-1 INPUT

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

```

821 KK SD_BAS
822 KM SIPHON DRAW WASH BASIN
823 KM OFF-LINE DETENTION BASIN LOCATED AT W. MOUNTAIN ROAD & S. SIHON DRAW WASH
824 KM WITH 30" OUTLET PIPE AND 100' WEIR
825 RS 1 STOR 0
826 SV 0 3.00 6 13 20 36 53 72 82 92
827 SE 65.0 65.5 66 67 68 70 72 74 75 76
828 SL 65.5 4.909 .62 .5

```

829 SS 75.0 100 2.5 1.5
*

830 KK CBAS2
831 KM COMBINE FLOWS FROM BASIN OUTLET AND BYPASS
832 HC 2
*

833 KK RSDW2
834 KM ROUTE FLOW FROM MOUNTAIN ROAD TO 104TH STREET THROUGH PIPE ALONG ELLIOT ROAD
835 RD 6500 0.0060 0.012 CIRC 4.5
*

836 KK CP65A
837 KM COMBINE FLOW FROM CP65B AND FLOW FROM ELLIOT ROAD PIPE
838 HC 2
*

839 KK 65AT-3
840 KM ROUTE FROM 104th ST TO E. BASIN OUTLET
841 RD 1500 0.0065 0.012 CIRC 7.0
*

842 KK DR65B
843 KM RETURN DIVERT TO EAST DETENTION BASIN
844 DR DIRS65
*

845 KK RS65A
846 KM ELLIOT BASIN, EAST
847 RS 1 STOR 0
848 SV 0 5.40 9.30 13.90 18.80 24.00 29.50 35.30 41.40 48.00
849 SE 1429.0 1433.0 1434.0 1435.0 1436.0 1437.0 1438.0 1439.0 1440.0 1441.0
850 SL 1430.0 0.7854 .62 .5
851 SS 1439.0 200 2.5 1.5
*

852 KK CP65
853 KM COMBINE FLOWS FROM EAST ELLIOT BASIN AND ELLIOT STORM DRAIN
854 KM BEFORE COMBINING WITH FLOWS FROM THE BYPASS CRISMON CHANNEL
855 HC 2
*

HEC-1 INPUT

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

856 KK 65T66
857 KM ROUTE FROM E. BASIN OUTLET TO CRISMON ROAD.
858 RD 1200 0.0065 0.012 CIRC 7.5
*

859 KK ADOT-E
860 KM INFLOW FROM NORTH OF THE SUPERSTITION FREEWAY ENTERING 67A
861 KM FROM EAST ADOT DETENTION BASIN 4105.
862 BA 0.01
863 ZR =QI A=ADOT EAST BASIN B=AT SUPERSTITION C=FLOW E=5MIN F=100YR
*

864 KK AET67A
865 KM ROUTE SUPERSTITION FLOW THROUGH 67A TO BASELINE ROAD
866 IN 15
867 RS 6 FLOW -1
868 RC .045 .040 .045 5500 .010
869 RX 0 100 110 120 130 140 150 250
870 RY 5 4 3 1 1 3 4 5
*

871 KK 67A
872 KM BASIN 67A
873 KM THE FOLLOWING PARAMETERS WERE PROVIDED FOR THIS BASIN
874 KM L= 1.0 Lca= .7 S= 42.9 Kn= .042 LAG= 25.7
875 KM PHOENIX VALLEY S-GRAFH WAS USED FOR THIS BASIN
876 BA .30
877 LG .21 .25 4.70 .39 43.00
878 UI 39. 126. 208. 277. 433. 400. 292. 213. 134. 67.
879 UI 47. 25. 12. 12. 12. 0. 0. 0. 0. 0.
880 UI 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.
*

881 KK R67A
882 KM RETAIN 100 YR 2. HR RUNOFF VOLUME
883 DT D67A 21
884 DI 0 10000
885 DQ 0 10000
*

886 KK C67A
 887 KM COMBINE FLOWS FROM ADOT-E AND SUBBASIN 67A
 888 HC 2
 *

889 KK 67ATC
 890 KM ROUTE 67A TO 67C VIA WASH CROSSING BASELINE
 891 RS 7 FLOW -1
 892 RC .055 .045 .055 6300 .0071
 893 RX 0 500 980 1003 1007 1031 1511 2011
 894 RY 4 3.5 3 0 0 3 3.5 4
 *

HEC-1 INPUT

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

895 KK SUP2
 896 KM INFLOW FROM NORTH OF SUPERSTITION FREEWAY, DISCHARGING INTO 67B
 * KO 1
 897 BA 0.01
 898 ZR =QI A=ADOT WEST BASIN B=AT SUPERSTITION C=FLOW E=5MIN F=100YR
 *

899 KK RSUP2
 900 KM ROUTE SUP2 THROUGH SUBBASIN 67B
 * KO 2
 901 IN 15
 902 RS 5 FLOW -1
 903 RC .045 .045 .045 4500 .0056
 904 RX 0 500 1000 1003 1007 1011 1511 2011
 905 RY 4 3.5 3 0 0 2 2.5 3
 *

906 KK 67B
 907 KM BASIN 67B
 908 KM THE FOLLOWING PARAMETERS WERE PROVIDED FOR THIS BASIN
 909 KM L= 1.2 Lca= .9 S= 28.0 Kn= .034 LAG= 26.4
 910 KM PHOENIX VALLEY S-GRAPH WAS USED FOR THIS BASIN
 911 BA .53
 912 LG .17 .25 4.90 .38 56.00
 913 UI 68. 210. 356. 470. 713. 741. 536. 395. 269. 131.
 914 UI 94. 59. 21. 21. 21. 0. 0. 0. 0. 0.
 915 UI 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.
 *

916 KK R67B
 917 KM RETAIN 100 YR 2 HR RUNOFF VOLUME
 918 DT D67B 41
 919 DI 0 10000
 920 DQ 0 10000
 *

921 KK C67B
 922 KM COMBINE FLOWS FROM SUP2 AND SUBBASIN 67B
 * KO 2
 923 HC 2
 *

924 KK 67BTC
 925 KM REACH CN-4, CN-5 plus culvert CNC-4.
 926 KM ROUTE FLOW IN THE CRISMON CHANNEL FROM BASELINE ROAD (C67B) TO
 927 KM GUADALUPE ROAD (C67C)
 928 RS 2 FLOW -1
 929 RC .025 .015 .025 5180 .0019
 930 RX 0 8 16 24.4 36.4 44.8 52.8 60.8
 931 RY 4.0 4.1 4.2 0 0 4.2 4.1 4.0
 *

HEC-1 INPUT

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

932 KK 67C
 933 KM BASIN 67C
 934 KM THE FOLLOWING PARAMETERS WERE PROVIDED FOR THIS BASIN
 935 KM L= 1.2 Lca= .7 S= 40.2 Kn= .049 LAG= 32.3
 936 KM PHOENIX VALLEY S-GRAPH WAS USED FOR THIS BASIN
 937 BA .93
 938 LG .25 .25 5.10 .32 31.00
 939 UI 96. 213. 432. 557. 702. 1006. 1133. 842. 667. 518.
 940 UI 365. 193. 157. 96. 59. 30. 30. 30. 30. 0.
 941 UI 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.
 942 UI 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.
 *

943 KK R67C

944 KM RETAIN 100 YR 2 HR RUNOFF VOLUME
 945 DT D67C 67
 946 DI 0 10000
 947 DQ 0 10000
 *

948 KK C67C
 949 KM COMBINE SUBBASINS 67C AND 67A AND 67B
 950 HC 3
 *

951 KK 67CT67
 952 KM REACH CN-3 plus culvert CNC-3
 953 KM ROUTE FLOW IN THE CRISMON CHANNEL FROM C67C (@ GUADALUPE ROAD & CRISMON ROAD)
 954 KM TO C67D (AT APPROX. 1/2 MILE SOUTH OF GUADALUPE ROAD).
 * Sta. 39+00 to Guadalupe Rd.
 955 RS 1 FLOW -1
 956 RC .025 .015 .025 2420 .0018
 957 RX 0 6 12 24 64 76 82 88
 958 RY 4 3 2 0 0 2 3 4
 *

959 KK 67D
 960 KM BASIN 67D
 961 KM THE FOLLOWING PARAMETERS WERE PROVIDED FOR THIS BASIN
 962 KM L= .6 Lca= .4 S= 34.7 Kn= .050 LAG= 20.5
 963 KM PHOENIX VALLEY S-GRAPH WAS USED FOR THIS BASIN
 964 BA .13
 965 LG .25 .25 5.20 .30 30.00
 966 UI 23. 87. 132. 216. 202. 137. 86. 38. 23. 9.
 967 UI .6. 6. 0. 0. 0. 0. 0. 0. 0. 0.
 968 UI 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.
 *

969 KK R67D
 970 KM RETAIN 100 YR 2 HR RUNOFF VOLUME
 971 DT D67D 9
 972 DI 0 10000
 973 DQ 0 10000
 *

HEC-1 INPUT

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

974 KK C67D
 975 KM COMBINE HYDROGRAPHS AT CP67D
 976 HC 2
 *

977 KK 67DT66
 978 KM REACH CN-2 plus culvert CNC-2
 979 KM ROUTE FLOW IN THE CRISMON CHANNEL FROM APPROX. 1/2 MILE SOUTH
 980 KM OF GUADALUPE ROAD TO THE INFLOW SPILLWAY FOR THE ELLIOT DETENTION BASIN.
 * Sta. 20+00 to Sta. 39+00
 981 RS 1 FLOW -1
 982 RC .032 .032 .032 1900 0.0035
 983 RX 0 6 12 24 64 76 82 88
 984 RY 4 3 2 0 0 2 3 4
 *

985 KK 66A
 986 KM BASIN 66A
 987 KM THE FOLLOWING PARAMETERS WERE PROVIDED FOR THIS BASIN
 988 KM L= .7 Lca= .3 S= 55.9 Kn= .047 LAG= 17.1
 989 KM PHOENIX VALLEY S-GRAPH WAS USED FOR THIS BASIN
 990 BA .26
 991 LG .24 .25 6.00 .22 35.00
 992 UI 78. 256. 417. 576. 363. 205. 84. 39. 16. 16.
 993 UI 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.
 994 UI 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.
 *

* DDM ***** Preserved *****

995 KK R66A
 996 KM RETAIN 100 YR 2 HR RUNOFF VOLUME
 997 DT D66A 21
 998 DI 0 10000
 999 DQ 0 10000
 * DDM ***** Preserved *****

1000 KK 66ATB
 1001 KM ROUTE S66A TO 66B VIA WASH CROSSING BASELINE
 1002 RS 7 FLOW -1
 1003 RC .045 .04 .045 7500 .0077
 1004 RX 0 500 980 1003 1007 1031 1511 2011
 1005 RY 4 3.5 3 0 0 3 3.5 4

*
* DDM ***** Updated *****

1006 KK 66B
 1007 KM BASIN 66B
 1008 KM THE FOLLOWING PARAMETERS WERE PROVIDED FOR THIS BASIN
 1009 KM L= 1.6 Lca= 1.0 S= 43.3 Kn= .050 LAG= 42.8
 1010 KM PHOENIX VALLEY S-GRAPH WAS USED FOR THIS BASIN
 1011 BA .67
 1012 LG .25 .25 5.00 .33 30.00
 1013 UI 53. 56. 185. 248. 297. 352. 426. 590. 636. 496.
 1014 UI 419. 346. 286. 232. 152. 93. 86. 58. 53. 21.
 HEC-1 INPUT

PAGE 27

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10
 1015 UI 16. 16. 16. 16. 16. 0. 0. 0. 0. 0.
 1016 UI 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.
 *

* DDM ***** Preserved *****

1017 KK R66B
 1018 KM RETAIN 100 YR 2 HR RUNOFF VOLUME
 1019 DT D66B 48
 1020 DI 0 10000
 1021 DQ 0 10000

* DDM ***** Preserved *****

1022 KK CP66B
 1023 KM COMBINE S66A AND S66B
 1024 HC 2
 *

* DDM ***** Preserved *****

1025 KK 66BTC
 1026 KM ROUTE 66B TO 66C VIA WASH
 1027 RS 7 FLOW -1
 1028 RC .045 .04 .045 6000 .0150
 1029 RX 0 500 995 1003 1007 1016 1511 2011
 1030 RY 4 3.5 3 0 0 3 3.5 4
 *

* DDM ***** Updated *****

1031 KK 66C
 1032 KM BASIN 66C
 1033 KM THE FOLLOWING PARAMETERS WERE PROVIDED FOR THIS BASIN
 1034 KM L= 1.1 Lca= .7 S= 46.5 Kn= .039 LAG= 24.3
 1035 KM PHOENIX VALLEY S-GRAPH WAS USED FOR THIS BASIN
 1036 BA .50
 1037 LG .19 .25 5.40 .29 48.00
 1038 UI 69. 243. 385. 528. 817. 635. 463. 325. 159. 103.
 1039 UI 63. 21. 21. 21. 0. 0. 0. 0. 0. 0.
 1040 UI 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.
 *

* DDM ***** Preserved *****

1041 KK R66C
 1042 KM RETAIN 100 YR 2 HR RUNOFF VOLUME
 1043 DT D66C 42
 1044 DI 0 10000
 1045 DQ 0 10000
 *

* DDM ***** Preserved *****

1046 KK CP66C1
 1047 KM Split up hydrograph combination in order to separate flows.
 1048 KM Combine Hydrographs 66BTC (from Sub. 66A) and R66C (from Sub. 66C)
 * KO 2 2
 1049 HC 2
 *

HEC-1 INPUT

PAGE 28

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

1050 KK CP66C2
 1051 KM Combine Hydrograph CP66C1 (from Subbasins 66A, 66B and 66C)
 1052 KM plus hydrograph 67DT66 (C67D)
 * KO 1
 1053 HC 2
 *
 1054 KK DI66
 1055 KM DIVERT FLOW TO DETENTION BASIN WA
 1056 KM By-pass Flow Reduced to 410 cfs from 458, SZ, 5-17-99
 * KO 1 2
 1057 DT DB66

1106 KK RS66D2
 * KO 1
 1107 KM ELLIOT BASIN, WEST B
 1108 KM TWO PONDS OPERATING IN SERIES.
 1109 KM Bottom Elevation Lowered to 1413.5 ft from 1414, and 36" Bleed-off
 1110 KM Pipe Reduced to 18" from WB to Elliot Channel
 * Since the bleed-off pipe length is short, no routing is provided.
 * Existing SS = 1420.5 80 2.5 1.5, SZ, 5-18-99
 1111 RS 1 STOR 0
 1112 SV 0 4.40 8.80 14.50 21.00 28.00 35.30 42.90 50.90 59.20
 1113 SE 1412.0 1415 1416 1417 1418 1419 1420 1421 1422 1423
 1114 SL 1413.0 1.7672 .62 .5
 1115 SS 1422.6 50 2.5 1.5
 *

HEC-1 INPUT

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

1116 KK CP66D
 1117 KM COMBINE FLOWS FROM WEST ELLIOT BASIN AND ELLIOT CHANNEL
 1118 KM AT THE OUTLET PIPE.
 * KO 1
 1119 HC 2
 *
 1120 KK 66T66D
 * KM REACH ET-6
 1121 KM ROUTE FROM DETENTION BASIN WB OUTLET TO ELLSWORTH RD
 1122 KM 2350 -> 3200, SZ, 5-17-99
 * First portion
 * RD card used for routing (Sta. 36+44 to 48+80)
 1123 RD 1236 0.0052 0.012 CIRC 9.5
 *
 1124 KK 66-66D
 * KM REACH ET-6
 1125 KM ROUTE FROM DETENTION BASIN WB OUTLET TO ELLSWORTH RD
 1126 KM 2350 -> 3200, SZ, 5-17-99
 * Second portion
 * RD card used for routing (Sta. 12+46 to Sta. 36+44)
 1127 RD 2398 0.0040 0.012 CIRC 9.5
 *
 1128 KK 66D
 1129 KM BASIN 66D
 1130 KM THE FOLLOWING PARAMETERS WERE PROVIDED FOR THIS BASIN
 1131 KM L= 1.0 Lca= .7 S= 28.6 Kn= .020 LAG= 13.2
 1132 KM PHOENIX VALLEY S-GRAPH WAS USED FOR THIS BASIN
 1133 BA .31
 1134 LG .10 .17 6.80 .19 80.00
 1135 UI 162. 480. 845. 540. 232. 89. 24. 0. 0. 0.
 1136 UI 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.
 *

* DDM ***** Preserved *****

1137 KK R66D
 1138 KM RETENTION REDUCED BY 77% FROM 31 TO 7 AC-FT
 1139 KM DUE TO DEVELOPMENT USING DETENTION BASIN
 * The developer does not participate in the basin so the retention volume
 * increased to 31 A-F
 1140 DT D66D 31
 1141 DI 0 10000
 1142 DQ 0 10000
 * DDM ***** Updated *****

1143 KK 61A
 1144 KM BASIN 61A
 1145 KM THE FOLLOWING PARAMETERS WERE PROVIDED FOR THIS BASIN
 1146 KM L= .9 Lca= .4 S= 36.8 Kn= .037 LAG= 19.1
 1147 KM PHOENIX VALLEY S-GRAPH WAS USED FOR THIS BASIN
 1148 BA .52
 1149 LG .19 .25 4.20 .56 52.00

HEC-1 INPUT

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

1150 UI 117. 412. 628. 1037. 786. 517. 261. 132. 62. 28.
 1151 UI 28. 0. 0. 0. 0. 0. 0. 0. 0. 0.
 1152 UI 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.
 *

* DDM ***** Preserved *****

1153 KK R61A
 1154 KM RETAIN 100 YR 2 HR RUNOFF VOLUME
 1155 DT D61A 42
 1156 DI 0 10000

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1157      DQ      0 10000
          * DDM ***** Preserved *****

1158      KK      61ATB
1159      KM      ROUTING 61A TO 61B VIA ELLSWORTH ROAD
1160      RS      6 FLOW -1
1161      RC      .035 .024 .035 5280 .005
1162      RX      0 500 750 752 802 852 1102 1602
1163      RY      3 2 1.5 1.2 1.2 1.5 2 3
          * DDM ***** Updated *****

1164      KK      61B
1165      KM      BASIN 61B
1166      KM      THE FOLLOWING PARAMETERS WERE PROVIDED FOR THIS BASIN
1167      KM      L= 1.4 Lca= .7 S= 39.7 Kn= .047 LAG= 33.6
1168      KM      PHOENIX VALLEY S-GRAPH WAS USED FOR THIS BASIN
1169      BA      1.09
1170      LG      .24 .25 4.80 .37 35.00
1171      UI      109. 223. 475. 615. 765. 1049. 1335. 1025. 822. 643.
1172      UI      495. 288. 187. 143. 109. 36. 34. 34. 34. 34.
1173      UI      0. 0. 0. 0. 0. 0. 0. 0. 0. 0.
1174      UI      0. 0. 0. 0. 0. 0. 0. 0. 0. 0.
          *
          * DDM ***** Preserved *****

1175      KK      R61B
1176      KM      RETAIN 100 YR 2 HR RUNOFF VOLUME
1177      DT      D61B 81
1178      DI      0 10000
1179      DQ      0 10000
          * DDM ***** Preserved *****

1180      KK      CP61B
1181      KM      COMBINE FLOWS FROM S61A AND S61B
1182      HC      2
          * DDM ***** Preserved *****

1183      KK      61T66D
1184      KM      ROUTE CP61B TO SUBBASIN 66D ALONG ELLSWORTH ROAD. ROUTING WILL BE
1185      KM      THE SAME AS WAS GIVEN FOR SUBBASIN 61A
1186      RS      6 FLOW -1
1187      RC      .035 .024 .035 5280 .008
1188      RX      0 500 750 752 802 852 1102 1602
1189      RY      3 2 1.5 1.2 1.2 1.5 2 3
          *
          * DDM ***** Updated *****

                                     HEC-1 INPUT
                                     PAGE 32

LINE      ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

1190      KK      67E
1191      KM      BASIN 67E
1192      KM      THE FOLLOWING PARAMETERS WERE PROVIDED FOR THIS BASIN
1193      KM      L= 1.2 Lca= .7 S= 32.3 Kn= .038 LAG= 26.9
1194      KM      PHOENIX VALLEY S-GRAPH WAS USED FOR THIS BASIN
1195      BA      .58
1196      LG      .19 .25 5.40 .30 50.00
1197      UI      73. 219. 378. 496. 732. 830. 597. 443. 315. 157.
1198      UI      110. 73. 24. 22. 22. 22. 0. 0. 0. 0.
1199      UI      0. 0. 0. 0. 0. 0. 0. 0. 0. 0.
          *
          * DDM ***** Preserved *****

1200      KK      R67E
1201      KM      RETAIN 100 YR 2 HR RUNOFF VOLUME
1202      KM      DUE TO DEVELOPMENT USING DETENTION BASIN
1203      DT      D67E 50
1204      DI      0 10000
1205      DQ      0 10000
          * DDM ***** Preserved *****

1206      KK      C67E
1207      KM      COMBINE FLOWS FROM ELLSWORTH ROAD JUST NORTH OF ELLIOT ROAD
1208      HC      2
          *

1209      KK      C66D
1210      KM      COMBINE ELLIOT CHANNEL FLOW WITH HYDROGRAPH C67E @ ELLIOT RD & ELLSWORTH RD.
1211      HC      3
          *

1212      KK      66T70A
1213      KM      Pipe Routing, Reach ET-5
          * SE corner curve of Elliot Rd. & Ellsworth Rd.
          * RD card used for routing (Junction Structure to Sta. 12+46), L = 253
1214      RD      400 0.0004 0.012 CIRC 9.5

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*
1215 KK 66T70B
1216 KM Pipe Routing, Reach ET-5
* Elliot Rd. to Culvert along Ellsworth Rd. 2-102" pipe = 144" pipe
* RD card used for routing (Sta. 85+65 to Sta. 97+51)
1217 RD 1186 0.0015 0.012 CIRC 12
*
1218 KK CULVT
1219 KM Pipe Routing, Culvert
* 2-102" pipe culvert crossing Ellsworth Rd.
* RD card used for routing l = 196 ft
1220 RD 400 0.0002 0.012 CIRC 12
*

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HEC-1 INPUT

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

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1221 KK 66T70C
1222 KM REACH ET-4, ET-5( COMPRISED OF ET-5A AND ET-5B) .
1223 KM ROUTE FROM ELLSWORTH Culvert TO SANTAN FREEWAY.
1224 RS 2 FLOW -1
1225 RC .032 .032 .032 2490 .0008
1226 RX 0 10 20 56 76 112 120 130
1227 RY 6.5 6.1 6.0 0 0 6.0 6.1 6.5
*

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1228 KK 62A
1229 KM BASIN 62A
1230 KM THE FOLLOWING PARAMETERS WERE PROVIDED FOR THIS BASIN
1231 KM L= .8 Lca= .5 S= 30.0 Kn= .020 LAG= 10.2
1232 KM PHOENIX VALLEY S-GRAPH WAS USED FOR THIS BASIN
1233 BA .38
1234 LG .10 .25 4.50 .52 80.00
1235 UI 335. 1057. 1010. 367. 93. 38. 0. 0. 0. 0.
1236 UI 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.
*

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1237 KK R62A
1238 KM RETAIN 100 YR 2 HR RUNOFF VOLUME
1239 DT D62A 33
1240 DI 0 10000
1241 DQ 0 10000
*

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1242 KK 62ATC
1243 KM ROUTE 62A TO 62C BY A CHANNEL ALONG SANTAN FWY
1244 KM Concrete Channel, Parameters are approximate
1245 RS 3 FLOW -1
1246 RC .016 .016 .016 5280 .0033
1247 RX 100 110 120 130 138 148 158 168
1248 RY 10 10 5 0 0 5 10 10
*

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```

1249 KK 62C
1250 KM BASIN 62C
1251 KM THE FOLLOWING PARAMETERS WERE PROVIDED FOR THIS BASIN
1252 KM L= .6 Lca= .3 S= 24.2 Kn= .049 LAG= 19.8
1253 KM PHOENIX VALLEY S-GRAPH WAS USED FOR THIS BASIN
1254 BA .55
1255 LG .23 .25 4.65 .40 47.00
1256 UI 112. 406. 615. 1024. 853. 571. 330. 154. 83. 28.
1257 UI 28. 0. 0. 0. 0. 0. 0. 0. 0. 0.
1258 UI 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.
*

```

* DDM ***** Preserved *****

HEC-1 INPUT

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

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1259 KK R62C
1260 KM RETAIN 100 YR 2 HR RUNOFF VOLUME
1261 DT D62C 31
1262 DI 0 10000
1263 DQ 0 10000
*

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```

1264 KK C62C
1265 KM COMBINE FLOW FROM BASIN 62A AND 62C
1266 HC 2
*

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1267 KK 62CTE
1268 KM ROUTE BASIN 62C TO BASIN 62E BY CHANNEL ON EAST SIDE OF PROPOSED SANTAN

```

1269 KM FREEWAY ALIGNMENT
 1270 RS 2 FLOW -1
 1271 RC .030 .030 .030 2000 .0003
 1272 RX 0 5 10 25 45 55 60 65
 1273 RY 9 7 6.5 0 0 6.5 7 9
 * DDM ***** Updated *****

1274 KK 62E
 1275 KM BASIN 62E
 1276 KM THE FOLLOWING PARAMETERS WERE PROVIDED FOR THIS BASIN
 1277 KM L= .6 Lca= .3 S= 31.9 Kn= .050 LAG= 20.4
 1278 KM PHOENIX VALLEY S-GRAPH WAS USED FOR THIS BASIN
 1279 BA .15
 1280 LG .25 .25 4.65 .39 45.00
 1281 UI 29. 108. 163. 268. 246. 167. 104. 46. 28. 10.
 1282 UI 8. 8. 0. 0. 0. 0. 0. 0. 0. 0.
 1283 UI 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.
 *
 * DDM ***** Preserved *****

1284 KK R62E
 1285 KM RETAIN 100 YR 2 HR RUNOFF VOLUME
 1286 DT D62E 12
 1287 DI 0 10000
 1288 DQ 0 10000
 *
 * DDM ***** Preserved *****

1289 KK CP62E
 1290 KM COMBINE FLOWS FROM SUBBASIN 62C AND SUBBASIN 62E
 1291 HC 2
 *
 * DDM ***** Preserved *****

1292 KK 62T68A
 1293 KM ROUTE FLOW FROM CP62E TO SUBBASIN 68A BY CHANNEL ALONG PROPOSED ALIGNMENT
 1294 KM OF THE SANTAN FREEWAY
 * ZW A=62T68A B=NORTH OF ELLIOT C=FLOW F=100YR FUTURE
 1295 RS 2 FLOW -1
 1296 RC .030 .030 .030 3280 .00015
 HEC-1 INPUT

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10
 1297 RX 0 5 10 20 30 40 45 50
 1298 RY 20 15 15 0 0 15 15 20
 *
 * DDM ***** Updated *****

1299 KK 68A
 1300 KM BASIN 68A
 1301 KM THE FOLLOWING PARAMETERS WERE PROVIDED FOR THIS BASIN
 1302 KM L= .7 Lca= .4 S= 37.7 Kn= .032 LAG= 13.7
 1303 KM PHOENIX VALLEY S-GRAPH WAS USED FOR THIS BASIN
 1304 BA .35
 1305 LG .16 .25 5.70 .27 66.00
 1306 UI 168. 506. 914. 635. 301. 114. 34. 26. 0. 0.
 1307 UI 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.
 *

1308 KK R68A
 1309 KM RETAIN 100 YR 2 HR RUNOFF VOLUME
 1310 DT D68A 31
 1311 DI 0 10000
 1312 DQ 0 10000
 *
 * DDM ***** Preserved *****

1313 KK CP68A
 1314 KM COMBINE FLOW FROM SUBBASIN 68A WITH THE ROUTED FLOW FROM C62E
 * KO 3 2
 * ZW A=COMBINED FLOW, CP68A B=FROM BASIN AND ROUTE C=FLOW F=100YR FUTURE
 1315 HC 2
 *
 * DDM ***** Preserved *****

1316 KK 68T70A
 1317 KM ROUTE FLOW FROM CP68A AT ELLIOT AND SANTAN FREEWAY ALIGNMENT TO SUBBASIN
 1318 KM 70A, AT THE POINT WHERE SIPHON DRAW INTERSECTS THE FREEWAY ALIGNMENT
 1319 KM CHANNEL IS NATURAL AND ONLY APPROXIMATE IN ROUTING PARAMETERS
 1320 RS 2 FLOW -1
 1321 RC .030 .030 .030 3960 .0006
 1322 RX 0 5 10 20 30 40 45 50
 1323 RY 10 5 4 0 0 4 5 10
 *
 * This combining operation is added for Elliot Basin Outlet
 *

1324 KK OUTLET
 1325 KM COMBINE FLOWS FROM 68T70A and Elliot Basin Outfall Channel
 1326 HC 2
 *

1327 KK R-OUT
 1328 KM ROUTE FLOW FROM OUTLET SOUTH ELLIOT AND SANTAN FREEWAY ALIGNMENT TO SUBBASIN
 1329 KM 70A, AT THE POINT WHERE HAWES ROAD INTERSECTS THE FREEWAY ALIGNMENT
 1330 KM Concrete Channel, Parameters are approximate
 1331 RS 1 FLOW -1
 1332 RC .016 .016 .016 2640 .0030
 HEC-1 INPUT

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10
 1333 RX 100 110 120 130 146 156 166 176
 1334 RY 10 10 5 0 0 5 10 10

* DDM ***** Inserted *****

1335 KK 70A
 1336 KM BASIN 70A
 1337 KM THE FOLLOWING PARAMETERS WERE PROVIDED FOR THIS BASIN
 1338 KM L= .9 Lca= .4 S= 23.5 Kn= .025 LAG= 12.7
 1339 KM PHOENIX VALLEY S-GRAPH WAS USED FOR THIS BASIN
 1340 BA .54
 1341 LG .13 .15 7.60 .13 67.00
 1342 UI 306. 899. 1524. 903. 346. 120. 44. 0. 0. 0.
 1343 UI 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.

* DDM ***** Preserved *****

1344 KK R70A
 1345 KM RETAIN 100 YR 2 HR RUNOFF VOLUME
 1346 DT D70A 52
 1347 DI 0 10000
 1348 DQ 0 10000
 *

* DDM ***** Preserved *****

1349 KK C70A
 1350 KM COMBINE FLOWS FROM R_OUT AND SUBBASIN 70A
 1351 HC 2
 *

1352 KK 70T76A
 1353 KM DIBBLE DRAINAGE FACILITY
 1354 KM ROUTE FLOW ALONG NEW SANTAN FREEWAY ALIGNMENT
 1355 KM REACH ET-3A, ET-3B
 1356 RS 2 FLOW -1
 1357 RC .025 .025 .025 4500 0.0005
 1358 RX 0 8 16 59 91 134 142 150
 1359 RY 7.4 7.3 7.2 0 0 7.2 7.1 7.0
 *

1360 KK 76A
 1361 KM BASIN 76A
 1362 KM THE FOLLOWING PARAMETERS WERE PROVIDED FOR THIS BASIN
 1363 KM L= 2.9 Lca= 1.7 S= 24.1 Kn= .030 LAG= 42.9
 1364 KM PHOENIX VALLEY S-GRAPH WAS USED FOR THIS BASIN
 1365 BA 1.91
 1366 LG .15 .15 8.80 .08 56.00
 1367 UI 150. 159. 528. 707. 847. 1004. 1213. 1673. 1826. 1424.
 1368 UI 1201. 992. 822. 667. 444. 266. 247. 171. 150. 65.
 1369 UI 46. 46. 46. 46. 46. 0. 0. 0. 0. 0.
 1370 UI 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.

* DDM ***** Preserved *****

HEC-1 INPUT

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

1371 KK R76A
 1372 KM RETAIN 100 YR 2 HR RUNOFF VOLUME
 1373 DT D76A 185
 1374 DI 0 10000
 1375 DQ 0 10000
 *

1376 KK C76A
 1377 KM COMBINE HYDROGRAPHS 70T76A (SANTAN FREEWAY CHANNEL FLOWS) WITH SUBBASIN 76A
 1378 HC 2
 *

* KK*DBSAN
 * KM DIVERT FROM SANTAN CHANNEL INTO THE RAY DETENTION BASIN

* KM ADDED BY CPE IN JUNE 2000.
 * KM USES A REALISTIC SIDE-WEIR EQUATION TO FORM POWER CURVE
 * KM WEIR CREST = 4.5 FT; WEIR LENGTH = 200FT; 4.0 FT DIV STRUCTURE.
 * KO 3
 * DT SANDB 1537
 * DI 0 750 772 819 892 999 1356 8138
 * DQ 0 0 7 31 78 154 439 6509
 *

1379 KK 76ATPR
 1380 KM DIBBLE DRAINAGE FACILITY
 1381 KM ROUTE FLOW ALONG NEW SANTAN FREEWAY ALIGNMENT TO NEW POWERLINE FLOODWAY ALGN.
 1382 KM REACH ET-2A, ET-2B
 1383 RS 2 FLOW -1
 1384 RC .025 .025 .025 5750 0.0005
 1385 RX 0 8 16 61 93 138 146 154
 1386 RY 7.7 7.6 7.5 0 0 7.5 7.4 7.3
 *

1387 KK EMFSTN
 1388 KM COMBINE HYDROGRAPHS 76ATPR (SANTAN FREEWAY CHANNEL FLOWS) WITH flow in EMF
 1389 HC 2
 *

1390 KK KXTRY
 1391 KM ROUTE EMF KNOX ROAD FLOW TO RAY ROAD
 1392 RS 1 FLOW -1
 1393 RC .03 .022 .03 3000 .0003
 1394 RX 0 500 520 553 693 726 740 742
 1395 RY 14 12 11 0 0 11 11 12
 *

1396 KK 73A
 1397 KM BASIN 73A
 1398 KM THE FOLLOWING PARAMETERS WERE PROVIDED FOR THIS BASIN
 1399 KM L= 2.3 Lca= 1.0 S= 34.9 Km= .093 LAG= 94.5
 1400 KM PHOENIX VALLEY S-GRAPH WAS USED FOR THIS BASIN
 1401 BA .95
 1402 LG .35 .36 5.00 .27 .00
 1403 UI 34. 34. 34. 34. 84. 117. 134. 158. 171. 185.
 1404 UI 197. 214. 232. 254. 274. 317. 381. 429. 424. 369.
 HEC-1 INPUT

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LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10
 1405 UI 332. 303. 282. 263. 240. 220. 202. 185. 169. 157.
 1406 UI 134. 107. 90. 60. 60. 57. 55. 54. 34. 34.
 1407 UI 34. 34. 16. 10. 10. 10. 10. 10. 10. 10.
 1408 UI 10. 10. 10. 10. 10. 10. 0. 0. 0. 0.
 1409 UI 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.
 *

1410 KK 73ATB
 1411 KM ROUTE 73A TO 73B VIA WASH RUNNING DIAGONALLY ACROSS 73B
 1412 RS 3 FLOW -1
 1413 RC .06 .05 .06 2700 .0074
 1414 RX 0 500 1000 1003 1007 1011 1511 2011
 1415 RY 4 3.5 3 0 0 2 2.5 3
 *

1416 KK 73B
 1417 KM BASIN 73B
 1418 KM THE FOLLOWING PARAMETERS WERE PROVIDED FOR THIS BASIN
 1419 KM L= .6 Lca= .4 S= 26.3 Km= .050 LAG= 21.6
 1420 KM PHOENIX VALLEY S-GRAPH WAS USED FOR THIS BASIN
 1421 BA .42
 1422 LG .27 .25 5.30 .27 21.00
 1423 UI 67. 268. 402. 621. 719. 486. 332. 162. 94. 50.
 1424 UI 20. 20. 0. 0. 0. 0. 0. 0. 0. 0.
 1425 UI 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.
 *

* DDM ***** Preserved *****

1426 KK R73B
 1427 KM RETAIN 100 YR 2 HR RUNOFF VOLUME
 1428 DT D73B 29
 1429 DI 0 10000
 1430 DQ 0 10000
 *

* DDM ***** Preserved *****

1431 KK CP73B
 1432 KM COMBINE S73A AND S73B
 1433 HC 2
 *

1434 KK 73BTC

1435 KM ROUTE 73B TO 73C VIA WASH CROSSING MOUNTAIN ROAD
 1436 KM THEN ROUTED SOUTH ALONG GENERAL MOTORS BERM TO SW CORNER OF 73C
 1437 RS 6 FLOW -1
 1438 RC .035 .022 .035 5000 .0036
 1439 RX 0 500 700 710 720 730 930 1430
 1440 RY 8 7.5 5 0 0 5 7.5 8
 *

HEC-1 INPUT

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

1441 KK 73C
 1442 KM BASIN 73C
 1443 KM THE FOLLOWING PARAMETERS WERE PROVIDED FOR THIS BASIN
 1444 KM L= .6 Lca= .3 S= 43.7 Kn= .020 LAG= 7.0
 1445 KM PHOENIX VALLEY S-GRAPH WAS USED FOR THIS BASIN
 1446 BA .58
 1447 LG .10 .25 5.30 .34 80.00
 1448 UI 1073. 2551. 742. 111. 0. 0. 0. 0. 0.
 1449 UI 0. 0. 0. 0. 0. 0. 0. 0. 0.
 *

1450 KK R73C
 1451 KM RETAIN 100 YR 2 HR RUNOFF VOLUME
 1452 DT D73C 58
 1453 DI 0 10000
 1454 DQ 0 10000
 *

* DDM ***** Preserved *****

1455 KK CP73C
 1456 KM COMBINE 73C AND 73B
 1457 HC 2
 *

* DDM ***** Preserved *****

1458 KK 73T74C
 1459 KM ROUTE 73C TO 74C VIA GM BERM, WEST EDGE OF 74C
 1460 RS 4 FLOW -1
 1461 RC .035 .022 .035 3500 .0034
 1462 RX 0 500 1000 1001 1016 1516 2016 2516
 1463 RY 6 5.5 5 2 2 3.5 4.5 5.5
 *

* DDM ***** Updated *****

1464 KK 74A
 1465 KM BASIN 74A
 1466 KM THE FOLLOWING PARAMETERS WERE PROVIDED FOR THIS BASIN
 1467 KM L= 2.4 Lca= 1.0 S= 42.2 Kn= .095 LAG= 92.9
 1468 KM PHOENIX VALLEY S-GRAPH WAS USED FOR THIS BASIN
 * KO 2 2
 1469 BA .75
 1470 LG .35 .36 5.00 .27 .00
 1471 UI 27. 27. 27. 27. 73. 96. 111. 129. 140. 151.
 1472 UI 163. 175. 193. 208. 228. 268. 317. 362. 327. 287.
 1473 UI 260. 239. 222. 206. 187. 171. 160. 142. 132. 118.
 1474 UI 99. 79. 56. 48. 47. 45. 45. 32. 27. 27.
 1475 UI 27. 19. 8. 8. 8. 8. 8. 8. 8. 8.
 1476 UI 8. 8. 8. 8. 8. 0. 0. 0. 0. 0.
 1477 UI 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.
 *

HEC-1 INPUT

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

1478 KK DB74A
 1479 KM NEW BASIN REGRADED ON 18 DEC 97, by Dibble & Associates
 1480 KM revised to be on-line detention basin
 1481 KM DETENTION BASIN LOCATED EAST OF MERIDIAN ROAD & north of Powerline Floodway
 1482 KM WITH 24-INCH OUTFALL & 350-FT WEIR.
 * KO 2 2

1483 RS 1 STOR 0
 1484 SA 0 1.43 5.38 11.14 18.97
 1485 SE 76.0 78 80 82 84
 1486 SL 77.0 3.14 .62 .5
 1487 SS 82.20 350 2.5 1.5
 *

1488 KK 74ATB
 1489 KM ROUTE 74A TO 74B VIA WASH CROSSING COUNTY LINE
 1490 RS 4 FLOW -1
 1491 RC .045 .04 .045 3500 .0054
 1492 RX 0 500 1000 1003 1007 1011 1511 2011
 1493 RY 4 3.5 3 0 0 2 2.5 3
 *

1494 KK 74B
 1495 KM BASIN 74B
 1496 KM THE FOLLOWING PARAMETERS WERE PROVIDED FOR THIS BASIN
 1497 KM L= .6 Lca= .5 S= 32.1 Kn= .050 LAG= 23.3
 1498 KM PHOENIX VALLEY S-GRAPH WAS USED FOR THIS BASIN
 1499 BA .33
 1500 LG .30 .25 6.00 .18 5.00
 1501 UI 48. 177. 273. 388. 563. 406. 289. 188. 87. 58.
 1502 UI 28. 15. 15. 0. 0. 0. 0. 0. 0. 0.
 1503 UI 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.

*
 * DDM ***** Preserved *****

1504 KK R74B
 1505 KM RETAIN 100 YR 2 HR RUNOFF VOLUME
 1506 DT D74B 22
 1507 DI 0 10000
 1508 DQ 0 10000

*
 * DDM ***** Preserved *****

1509 KK CP74B
 1510 KM COMBINE S74A AND S74B
 1511 HC 2

*
 * DDM ***** Preserved *****

1512 KK 74BTC
 1513 KM ROUTE 74B TO 74C VIA WASH CROSSING MOUNTAIN ROAD
 1514 RS 4 FLOW -1
 1515 RC .045 .04 .045 3500 .0051
 1516 RX 0 500 980 1003 1007 1031 1511 2011
 1517 RY 4 3.5 3 0 0 3 3.5 4

*
 * DDM ***** Updated *****

HEC-1 INPUT

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1 LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

1518 KK 74C
 1519 KM BASIN 74C
 1520 KM THE FOLLOWING PARAMETERS WERE PROVIDED FOR THIS BASIN
 1521 KM L= .7 Lca= .5 S= 25.4 Kn= .020 LAG= 10.3
 1522 KM PHOENIX VALLEY S-GRAPH WAS USED FOR THIS BASIN
 1523 BA .34
 1524 LG .10 .15 7.00 .17 80.00
 1525 UI 303. 950. 940. 348. 92. 35. 0. 0. 0. 0.
 1526 UI 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.

*
 * DDM ***** Preserved *****

1527 KK R74C
 1528 KM RETAIN 100 YR 2 HR RUNOFF VOLUME
 1529 DT D74C 35
 1530 DI 0 10000
 1531 DQ 0 10000

*
 * DDM ***** Preserved *****

1532 KK CP74C
 1533 KM COMBINE 74C AND 74B AND 73C
 1534 HC 3

*
 * DDM ***** Preserved *****

1535 KK 74CT75
 1536 KM ROUTE CP74C TO S75 VIA POWERLINE FLOODWAY TO ELLSWORTH RD & RAY ROAD.
 1537 KM Vel of 10 ft/sec for NSTP calc.
 1538 RS 3 FLOW -1
 1539 RC .03 .013 .03 10560 .0049
 1540 RX 0 1005 1023 1030.5 1036.5 1044 1062 2067
 1541 RY 6 5 5 0 0 5 5 6

*
 * DDM ***** Updated *****

1542 KK 75
 1543 KM BASIN 75 , WHICH IS THE GENERAL MOTORS DESERT PROVING GROUNDS
 1544 KM THE FOLLOWING PARAMETERS WERE PROVIDED FOR THIS BASIN
 1545 KM L= 4.0 Lca= 3.0 S= 20.0 Kn= .087 LAG= 182.0
 1546 KM PHOENIX VALLEY S-GRAPH WAS USED FOR THIS BASIN
 1547 BA 4.01 0.25
 1548 KM THE 0.25 FACTOR IS PER FCDMC, AMIR MOTAMEDI, WHO HAS TOURED THE
 1549 KM PROVING GROUNDS WITH GM PERSONNEL AND FIELD VERIFIED THIS FACTOR.
 1550 LG .34 .35 6.80 .13 3.00
 1551 UI 74. 74. 74. 74. 74. 74. 74. 74. 180. 243.

1552	UI	247.	284.	284.	335.	342.	363.	375.	396.	412.	422.
1553	UI	437.	458.	475.	495.	519.	550.	567.	585.	622.	672.
1554	UI	737.	819.	865.	935.	1002.	935.	870.	813.	764.	726.
1555	UI	693.	664.	637.	614.	593.	574.	547.	519.	496.	473.
1556	UI	452.	437.	425.	387.	377.	363.	347.	341.	289.	284.
1557	UI	233.	213.	213.	147.	131.	131.	131.	130.	122.	122.
1558	UI	122.	122.	122.	75.	74.	74.	74.	74.	74.	74.
1559	UI	74.	43.	23.	23.	23.	23.	23.	23.	23.	23.

HEC-1 INPUT

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LINE	ID	1	2	3	4	5	6	7	8	9	10
1560	UI	23.	23.	23.	23.	23.	23.	23.	23.	23.	23.
1561	UI	23.	23.	23.	23.	23.	23.	23.	23.	0.	0.
1562	UI	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.

1563 KK C75
 1564 KM COMBINE FLOWS FROM C74C AND SUBBASIN 75
 1565 HC 2

* DDM ***** Preserved *****

1566	KK	75TPC
1567	KM	ROUTE 75 THROUGH POWERLINE FLOODWAY TO AIR FORCE CHANNEL
1568	RS	2 FLOW -1
1569	RC	.03 .013 .03 3900 .0041
1570	RX	0 1005 1023 1030.5 1036.5 1044 1062 2067
1571	RY	6 5 5 0 0 5 5 6

* DDM ***** Updated *****

1572	KK	77A
1573	KM	BASIN 77A
1574	KM	THE FOLLOWING PARAMETERS WERE PROVIDED FOR THIS BASIN
1575	KM	L= 2.9 Lca= 1.5 S= 31.1 Kn= .092 LAG= 119.0
1576	KM	PHOENIX VALLEY S-GRAPH WAS USED FOR THIS BASIN
1577	BA	1.74
1578	LG	.35 .36 5.00 .27 .00
1579	UI	49. 49. 49. 49. 49. 108. 162. 185. 205. 230.
1580	UI	244. 264. 278. 293. 311. 333. 358. 380. 406. 462.
1581	UI	537. 584. 659. 601. 541. 496. 461. 430. 407. 385.
1582	UI	362. 334. 311. 293. 273. 252. 238. 226. 189. 161.
1583	UI	141. 104. 87. 87. 83. 81. 73. 49. 49.
1584	UI	49. 49. 49. 22. 15. 15. 15. 15. 15. 15.
1585	UI	15. 15. 15. 15. 15. 15. 15. 15. 15. 15.
1586	UI	15. 0. 0. 0. 0. 0. 0. 0. 0. 0.
1587	UI	0. 0. 0. 0. 0. 0. 0. 0. 0. 0.

1588 KK 77ATB
 1589 KM ROUTE 77A TO 77B VIA WASH CROSSING COUNTY LINE
 1590 RS 3 FLOW -1
 1591 RC .045 .04 .045 3000 .006
 1592 RX 0 500 980 1003 1007 1031 1511 2011
 1593 RY 4 3.5 3 0 0 3 3.5 4

* DDM ***** Updated *****

1594	KK	77B
1595	KM	BASIN 77B
1596	KM	THE FOLLOWING PARAMETERS WERE PROVIDED FOR THIS BASIN
1597	KM	L= .6 Lca= .3 S= 26.3 Kn= .050 LAG= 19.3
1598	KM	PHOENIX VALLEY S-GRAPH WAS USED FOR THIS BASIN
1599	BA	.36
1600	LG	.30 .25 5.30 .24 5.00
1601	UI	78. 277. 421. 700. 544. 360. 189. 93. 46. 19.

HEC-1 INPUT

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LINE	ID	1	2	3	4	5	6	7	8	9	10
1602	UI	19.	0.	0.	0.	0.	0.	0.	0.	0.	0.
1603	UI	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.

* DDM ***** Preserved *****

1604 KK R77B
 1605 KM RETAIN 100 YR 2 HR RUNOFF VOLUME
 1606 DT D77B 16
 1607 DI 0 10000
 1608 DQ 0 10000

* DDM ***** Preserved *****

1609 KK CP77B
 1610 KM COMBINE S77A AND S77B
 1611 HC 2

*
* DDM ***** Preserved *****

1612 KK 77BTC
 1613 KM ROUTE 77B TO 77C VIA WASH CROSSING MOUNTAIN ROAD, THEN SOUTH ALONG
 1614 KM WESTERN EDGE OF 77C
 1615 RS 5 FLOW -1
 1616 RC .045 .04 .045 4750 .0042
 1617 RX 0 500 950 1003 1007 1061 1511 2011
 1618 RY 4 3.5 3 0 0 3 3.5 4

*
* DDM ***** Updated *****

1619 KK 77C
 1620 KM BASIN 77C
 1621 KM THE FOLLOWING PARAMETERS WERE PROVIDED FOR THIS BASIN
 1622 KM L= .5 Lca= .3 S= 32.3 Kn= .020 LAG= 7.2
 1623 KM PHOENIX VALLEY S-GRAPH WAS USED FOR THIS BASIN
 1624 BA .28
 1625 LG .10 .25 5.80 .27 79.00
 1626 UI 492. 1219. 393. 60. 0. 0. 0. 0. 0. 0.
 1627 UI 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.

*
* DDM ***** Preserved *****

1628 KK R77C
 1629 KM RETAIN 100 YR 2 HR RUNOFF VOLUME
 1630 DT D77C 28
 1631 DI 0 10000
 1632 DQ 0 10000

*
* DDM ***** Preserved *****

1633 KK C77C
 1634 KM COMBINE FLOWS FROM C77B AND SUBBASIN 77C
 1635 HC 2

*
* DDM ***** Preserved *****

HEC-1 INPUT

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

1636 KK 77CT78
 1637 RS 3 FLOW -1
 1638 RC 0.035 0.022 0.035 2400 0.002
 1639 RX 0 100 110 115 120 125 130 135
 1640 RY 4 3 2.5 0 0 2.5 8 9

*
* DDM ***** Updated *****

1641 KK 78A
 1642 KM BASIN 78A
 1643 KM THE FOLLOWING PARAMETERS WERE PROVIDED FOR THIS BASIN
 1644 KM L= 3.3 Lca= 1.3 S= 30.2 Kn= .090 LAG= 118.0
 1645 KM PHOENIX VALLEY S-GRAPH WAS USED FOR THIS BASIN
 1646 BA 1.88
 1647 LG .35 .36 5.00 .27 .00
 1648 UI 54. 54. 54. 54. 54. 124. 176. 203. 227. 252.
 1649 UI 268. 290. 305. 322. 342. 366. 396. 417. 451. 515.
 1650 UI 612. 641. 716. 643. 579. 531. 494. 464. 437. 417.
 1651 UI 385. 356. 334. 315. 290. 270. 255. 233. 206. 159.
 1652 UI 153. 95. 95. 95. 88. 88. 88. 65. 54. 54.
 1653 UI 54. 54. 45. 16. 16. 16. 16. 16. 16. 16.
 1654 UI 16. 16. 16. 16. 16. 16. 16. 16. 16. 16.
 1655 UI 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.
 1656 UI 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.

1657 KK 78ATB
 1658 KM ROUTE FLOW FROM 78A TO 78B VIA WASH CROSSING COUNTY LINE
 1659 RS 4 FLOW -1
 1660 RC .045 .04 .045 3500 .0042
 1661 RX 0 500 980 1003 1007 1031 1511 2011
 1662 RY 4.5 3.5 3 0 0 3 3.5 4.5

*
* DDM ***** Updated *****

1663 KK 78B
 1664 KM BASIN 78B
 1665 KM THE FOLLOWING PARAMETERS WERE PROVIDED FOR THIS BASIN
 1666 KM L= .6 Lca= .4 S= 31.6 Kn= .050 LAG= 20.9
 1667 KM PHOENIX VALLEY S-GRAPH WAS USED FOR THIS BASIN
 1668 KM FCDMC REPORTS BASIN ALREADY BUILT-OUT, WITH RURAL 1 TO 2 ACRE LOTS
 1669 KM AND THERE IS NO RETENTION FOR THIS BASIN.
 1670 BA .39
 1671 LG .30 .15 8.40 .07 5.00
 1672 UI 68. 262. 393. 635. 638. 433. 283. 126. 78. 34.

1673	UI	19.	19.	0.	0.	0.	0.	0.	0.	0.
1674	UI	0.	0.	0.	0.	0.	0.	0.	0.	0.

1675 KK C78B
 1676 KM COMBINE FLOW FROM SUBBASIN 78B AND SUBBASIN 78A
 1677 HC 2
 *

HEC-1 INPUT

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

1678 KK 78BTC
 1679 KM ROUTE 78B TO 78C VIA WASH CROSSING MOUNTAIN ROAD, THEN SOUTH ALONG
 1680 KM WESTERN EDGE OF 78C.
 1681 RS 3 FLOW -1
 1682 RC .035 .022 .035 4500 .0033
 1683 RX 0 100 110 115 120 125 130 135
 1684 RY 5 4 3.5 0 0 3.5 8 9
 *

1685 KK 78C
 1686 KM BASIN 78C
 1687 KM THE FOLLOWING PARAMETERS WERE PROVIDED FOR THIS BASIN
 1688 KM L= .5 Lca= .3 S= 31.7 Kn= .026 LAG= 9.0
 1689 KM PHOENIX VALLEY S-GRAPH WAS USED FOR THIS BASIN
 1690 BA .28
 1691 LG .13 .10 11.20 .03 64.00
 1692 UI 319. 980. 652. 164. 34. 0. 0. 0. 0. 0.
 1693 UI 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.
 *

1694 KK R78C
 1695 KM RETAIN 100 YR 2 HR RUNOFF VOLUME
 1696 DT D78C 24
 1697 DI 0 10000
 1698 DQ 0 10000
 *

1699 KK C78C
 1700 KM COMBINE FLOWS FROM 78B, 78C & 77C @ WILLIAMS FIELD ROAD & SIGNAL BUTTE ROAD.
 1701 HC 3
 *

1702 KK 78CT79
 1703 KM ROUTE 78C TO 79A AT ELLSWORTH VIA GM CHANNEL TO
 1704 KM WILLIAMS FIELD RD & ELLSWORTH ROAD.
 1705 RS 4 FLOW -1
 1706 RC .035 .022 .035 10560 .0044
 1707 RX 0 500 800 805 820 825 1125 1625
 1708 RY 7 6 5 0 0 5 6 7
 *

1709 KK 79A
 1710 KM BASIN 79A
 1711 KM THE FOLLOWING PARAMETERS WERE PROVIDED FOR THIS BASIN
 1712 KM L= 2.3 Lca= 1.2 S= 23.2 Kn= .090 LAG= 106.0
 1713 KM PHOENIX VALLEY S-GRAPH WAS USED FOR THIS BASIN.
 1714 BA 2.01
 1715 LG .35 .30 8.00 .08 .00
 1716 UI 64. 64. 64. 64. 82. 209. 232. 262. 299. 321.
 1717 UI 347. 367. 390. 421. 458. 489. 527. 610. 726. 792.
 1718 UI 825. 727. 655. 600. 557. 523. 495. 451. 417. 390.
 1719 UI 361. 328. 308. 285. 244. 190. 169. 113. 113. 109.
 1720 UI 105. 105. 85. 64. 64. 64. 64. 37. 20. 20.
 1721 UI 20. 20. 20. 20. 20. 20. 20. 20. 20. 20.
 *

HEC-1 INPUT

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

1722	UI	20.	20.	20.	0.	0.	0.	0.	0.	0.
1723	UI	0.	0.	0.	0.	0.	0.	0.	0.	0.

1724 KK C79A1
 1725 KM COMBINE FLOWS FROM 78C AND 79A @ WILLIAMS FIELD ROAD & ELLSWORTH ROAD.
 1726 HC 2
 *

1727 KK 78F
 1728 KM BASIN 78F
 1729 KM THE FOLLOWING PARAMETERS WERE PROVIDED FOR THIS BASIN
 1730 KM L= 3.7 Lca= 2.1 S= 29.8 Kn= .090 LAG= 147.0
 1731 KM PHOENIX VALLEY S-GRAPH WAS USED FOR THIS BASIN
 1732 BA 4.19
 1733 LG .35 .36 5.00 .27 .00

1734	UI	96.	96.	96.	96.	96.	96.	148.	315.	325.	368.
1735	UI	399.	442.	468.	495.	521.	541.	566.	595.	626.	663.
1736	UI	706.	737.	774.	845.	936.	1085.	1119.	1276.	1239.	1127.
1737	UI	1033.	968.	907.	862.	820.	782.	751.	712.	667.	629.
1738	UI	597.	570.	541.	502.	472.	454.	436.	368.	339.	276.
1739	UI	276.	174.	170.	170.	168.	158.	158.	158.	156.	96.
1740	UI	96.	96.	96.	96.	96.	79.	29.	29.	29.	29.
1741	UI	29.	29.	29.	29.	29.	29.	29.	29.	29.	29.
1742	UI	29.	29.	29.	29.	29.	29.	29.	0.	0.	0.
1743	UI	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.

*

1744	KK	82A1									
1745	KM	BASIN 82A1									
1746	KM	THE FOLLOWING PARAMETERS WERE PROVIDED FOR THIS BASIN									
1747	KM	L= 3.6 Lca= .9 S= 33.9 Kn= .090 LAG= 103.0									
1748	KM	PHOENIX VALLEY S-GRAPH WAS USED FOR THIS BASIN									
1749	BA	3.12									
1750	LG	.35	.36	5.00	.27	.00					
1751	UI	102.	102.	102.	102.	163.	335.	381.	436.	485.	526.
1752	UI	564.	599.	644.	693.	757.	801.	901.	1048.	1222.	1356.
1753	UI	1223.	1084.	987.	913.	852.	800.	742.	675.	629.	589.
1754	UI	529.	494.	459.	391.	304.	266.	180.	180.	173.	167.
1755	UI	167.	123.	102.	102.	102.	102.	36.	31.	31.	31.
1756	UI	31.	31.	31.	31.	31.	31.	31.	31.	31.	31.
1757	UI	31.	0.	0.	0.	0.	0.	0.	0.	0.	0.
1758	UI	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.

*

1759	KK	C82A1									
1760	KM	COMBINE FLOWS FROM SUBBASINS 78F AND 82A1 NORTH OF PECOS ROAD AT									
1761	KM	NEW DETENTION BASIN									
1762	HC	2									

*

HEC-1 INPUT

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

1763	KK	DB82A1									
1764	KM	PECOS NORTH BASIN									
1765	KM	NEW DETENTION BASIN LOCATED EAST OF MERIDIAN ROAD & 660' NORTH OF PECOS RD.									
1766	KM	WITH 1-42" RCP OUTLET & 86' SPILLWAY AT ELEV=44									
	* KO	2	2								
1767	RS	1	STOR	0							
1768	SV	0	8	34	73	113	153	195	237	280	346
1769	SE	36	37	38	39	40	41	42	43	44	46.1
1770	SL	33.5	9.6	.62	.5						
1771	SS	44	195	3	1.5						

*

1772	KK	PS-9									
1773	KM	REACH PS-9									
1774	KM	OUTFLOW CHANNEL FROM NEW DETENTION BASIN 82A TO MAIN CHANNEL @ PECOS									
1775	RS	1	FLOW	-1							
1776	RC	.025	.025	.025	500	.0005					
1777	RX	0	8	16	42	46	72	80	88		
1778	RY	5.5	4.2	4.3	0	0	4.3	4.2	4.5		

*

1779	KK	CAP2									
1780	KM	INFLOW FROM EAST OF THE CAP THROUGH 1 - 36" PIPE OVERCHUTE									
1781	KM	STATION #536+00 SALT-GILA AQUEDUCT REACH 2									
1782	KM	QI CARDS BASED ON OVERCHUTE CAPACITY OF 64 CFS									
1783	IN	60									
1784	BA	.01									
1785	QI	0	20	64	64	64	64	64	64	64	64
1786	QI	64	64	64	64	64	64	64	64	64	64
1787	QI	64	64	64	64	64					

*

1788	KK	RCAP2									
1789	KM	ROUTE CAP2 THROUGH 82A2 VIA WASH TO SUBBASIN 82A2									
1790	IN	15									
1791	RS	27	FLOW	-1							
1792	RC	.045	.04	.045	24000	.05					
1793	RX	0	500	1000	1010	1020	1030	1530	2030		
1794	RY	8	5	3	0	0	3	5	8		

*

1795	KK	82A2									
1796	KM	BASIN 82A2									
1797	KM	THE FOLLOWING PARAMETERS WERE PROVIDED FOR THIS BASIN									
1798	KM	L= 4.6 Lca= 2.9 S= 27.2 Kn= .089 LAG= 183.0									
1799	KM	PHOENIX VALLEY S-GRAPH WAS USED FOR THIS BASIN									
1800	BA	4.13									
1801	LG	.35	.36	5.00	.27	1.00					

1802	UI	76.	76.	76.	76.	76.	76.	76.	177.	249.
1803	UI	250.	291.	291.	339.	349.	371.	381.	406.	430.
1804	UI	448.	466.	483.	503.	529.	561.	578.	596.	680.
1805	UI	743.	805.	911.	923.	1027.	979.	901.	845.	751.
1806	UI	717.	686.	659.	635.	613.	594.	570.	540.	487.
1807	UI	470.	455.	437.	407.	393.	372.	362.	349.	291.

HEC-1 INPUT

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LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

1808	UI	263.	218.	218.	179.	134.	134.	134.	134.	128.	125.
1809	UI	125.	125.	125.	96.	76.	76.	76.	76.	76.	76.
1810	UI	76.	67.	23.	23.	23.	23.	23.	23.	23.	23.
1811	UI	23.	23.	23.	23.	23.	23.	23.	23.	23.	23.
1812	UI	23.	23.	23.	23.	23.	23.	23.	23.	23.	0.
1813	UI	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.
1814	UI	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.

*

1815 KK CP82A2
 1816 KM COMBINE FLOW FROM ROUTED CAP2 AND SUBBASIN 82A2
 1817 HC 2
 *

1818 KK 82A4
 1819 KM BASIN 82A4
 1820 KM THE FOLLOWING PARAMETERS WERE PROVIDED FOR THIS BASIN
 1821 KM L= 3.5 Lca= 1.5 S= 29.1 Kn= .090 LAG= 128.0
 1822 KM PHOENIX VALLEY S-GRAPH WAS USED FOR THIS BASIN
 1823 BA 2.13

1824	LG	.35	.36	5.00	.27	.00					
1825	UI	56.	56.	56.	56.	56.	70.	184.	193.	214.	248.
1826	UI	265.	281.	302.	316.	332.	352.	374.	401.	424.	447.
1827	UI	494.	557.	650.	691.	742.	667.	606.	560.	523.	490.
1828	UI	466.	443.	419.	389.	364.	343.	325.	299.	281.	267.
1829	UI	251.	214.	182.	161.	126.	99.	99.	97.	92.	92.
1830	UI	92.	69.	56.	56.	56.	56.	56.	27.	17.	17.
1831	UI	17.	17.	17.	17.	17.	17.	17.	17.	17.	17.
1832	UI	17.	17.	17.	17.	17.	17.	0.	0.	0.	0.
1833	UI	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.

*

1834 KK 82A4T3
 1835 KM REACH MN-2
 1836 KM ROUTE FLOW FROM SUBBASIN 82A4 TO DETENTION BASIN 82A3
 1837 RS 2 FLOW -1
 1838 RC .025 .025 .025 1050 .0005
 1839 RX 0 8 16 47 107 138 146 154
 1840 RY 5.0 5.1 5.2 0 0 5.2 5.1 5.0
 *

1841 KK 82A3
 1842 KM BASIN 82A3
 1843 KM THE FOLLOWING PARAMETERS WERE PROVIDED FOR THIS BASIN
 1844 KM L= 3.6 Lca= 2.0 S= 28.3 Kn= .090 LAG= 145.0
 1845 KM PHOENIX VALLEY S-GRAPH WAS USED FOR THIS BASIN
 1846 BA 2.02

1847	LG	.35	.36	5.00	.27	.00					
1848	UI	47.	47.	47.	47.	47.	47.	82.	154.	162.	180.
1849	UI	200.	218.	230.	246.	257.	268.	280.	294.	310.	330.
1850	UI	351.	365.	387.	425.	477.	553.	566.	633.	583.	529.
1851	UI	492.	460.	434.	410.	392.	376.	361.	336.	316.	299.
1852	UI	284.	271.	250.	237.	228.	216.	190.	180.	136.	135.
1853	UI	101.	83.	83.	83.	78.	77.	77.	77.	52.	47.
1854	UI	47.	47.	47.	47.	42.	14.	14.	14.	14.	14.

HEC-1 INPUT

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LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

1855	UI	14.	14.	14.	14.	14.	14.	14.	14.	14.	14.
1856	UI	14.	14.	14.	14.	14.	0.	0.	0.	0.	0.
1857	UI	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.

*

1858 KK CP82A3
 1859 KM COMBINE FLOW FROM SUBBASIN 82A4 AND SUBBASIN 82A3 BEFORE DETENTION BASIN
 1860 HC 2
 *

1861 KK CP82A5
 1862 KM COMBINE FLOWS FROM CAP OVERCHUTE AND SUBBASIN 82A
 1863 HC 2
 *

1864 KK DB82B
 1865 KM PECOS SOUTH BASIN
 1866 KM NEW DETENTION BASIN LOCATED EAST OF MERIDIAN ROAD & 660' SOUTH OF PECOS RD.

1867 KM WITH 1-66" RCP OUTLET & 80' SPILLWAY AT ELEV 41
 * KO 2 2
 1868 RS 1 STOR 0
 1869 SV 0 3.5 9.6 25.7 62.7 110.5 158.7 207.5 257.3 320
 1870 SE 31.5 33 34 35 36 37 38 39 40 42.1
 1871 SL 33 23.7 .62 .5
 1872 SS 41 80 3 1.5
 *

1873 KK MN-1
 1874 KM REACH MN-1 plus culvert PSC-7
 1875 KM ROUTE FLOW FROM NEW DETENTION BASIN 82B TO MAIN LINE CHANNEL @ PECOS
 1876 RS 1 FLOW -1
 1877 RC .025 .025 .025 1030 .0005
 1878 RX 0 8 16 47 55 87 95 103
 1879 RY 5.1 5.2 5.3 0 0 5.3 5.2 5.1
 *

1880 KK CP82A6
 1881 KM COMBINE FLOWS AFTER DETENTION BASINS.
 1882 HC 2
 *

1883 KK 82TBOX
 1884 KM REACH PS-8
 1885 KM ROUTE FLOW FROM DETENTION BASIN DB82B TO 1000' FOOT LONG BOX CULVERT (PSC-6).
 1886 RS 1 FLOW -1
 1887 RC .025 .025 .025 750 .0005
 1888 RX 0 8 16 47.2 67 98 106 114
 1889 RY 5.0 5.1 5.2 0 0 5.2 5.1 5.0
 *

HEC-1 INPUT

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

1890 KK BOXCLV
 1891 KM REACH PSC-6
 1892 KM ROUTE FLOW THROUGH BOX CULVERT
 1893 RS 1 FLOW -1
 1894 RC .015 .012 .015 1000 .0020
 1895 RX 0 8 16 16.01 28.01 28.02 36 44
 1896 RY 5.5 5.2 5 0 0 5 5.2 5.5
 *

1897 KK BOXT78
 1898 KM REACH PS-5, PS-6, PS-7 plus culverts PSC-5 & PSC-4
 1899 KM ROUTE FLOW FROM 1000' BOX CULVERT TO C78D (SIGNAL BUTTE ROAD)
 1900 RS 1 FLOW -1
 1901 RC .025 .025 .025 3400 .0005
 1902 RX 0 8 16 47 67 98 106 114
 1903 RY 5.4 5.3 5.2 0 0 5.2 5.3 5.4
 *

1904 KK 78D
 1905 KM BASIN 78D
 1906 KM THE FOLLOWING PARAMETERS WERE PROVIDED FOR THIS BASIN
 1907 KM L= 1.2 Lca= .5 S= 21.7 Km= .030 LAG= 19.5
 1908 KM PHOENIX VALLEY S-GRAPH WAS USED FOR THIS BASIN
 1909 BA .89
 1910 LG .15 .15 8.00 .11 55.00
 1911 UI 189. 678. 1029. 1713. 1367. 909. 496. 240. 123. 47.
 1912 UI 47. 0. 0. 0. 0. 0. 0. 0. 0. 0.
 1913 UI 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.
 *

1914 KK R78D
 1915 KM RETAIN 100 YR 2 HR RUNOFF VOLUME
 1916 DT D78D 84
 1917 DI 0 10000
 1918 DQ 0 10000
 *

* DDM ***** Updated *****

1919 KK 82B
 1920 KM BASIN 82B
 1921 KM THE FOLLOWING PARAMETERS WERE PROVIDED FOR THIS BASIN
 1922 KM L= .9 Lca= .4 S= 21.2 Km= .030 LAG= 17.2
 1923 KM PHOENIX VALLEY S-GRAPH WAS USED FOR THIS BASIN
 1924 BA .92
 1925 LG .15 .25 5.00 .36 55.00
 1926 UI 266. 879. 1420. 2004. 1268. 727. 294. 142. 55. 55.
 1927 UI 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.
 1928 UI 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.
 *

HEC-1 INPUT

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

1929 KK R82
 1930 KM RETAIN 100 YR 2 HR RUNOFF VOLUME
 1931 DT D82 1
 1932 DI 0 10000
 1933 DQ 0 10000
 *

1934 KK DTTRW
 1935 KM DIVERTING 110.7 ACRE-FEET DUE TO ON-SITE RETENTION
 1936 KM VOLUMES WERE DERIVED FROM DRAINAGE REPORT - REFERENCE 7.
 1937 DT TRW 110.7
 1938 DI 0 10000
 1939 DQ 0 10000
 *
 * DDM ***** Preserved *****

1940 KK C78D
 1941 KM COMBINE FLOWS FROM 78D, 82B AND ROUTED FLOW 82T78D
 1942 KM @ PECOS ROAD AND SIGNAL BUTTE ROAD.
 1943 HC 3
 *
 * DDM ***** Preserved *****

1944 KK 78DTE
 1945 KM REACH PS-2, PS-3, PS-4 plus culverts PSC-3 AND PSC-2.
 1946 KM ROUTE FLOWS FROM 78D (PECOS RD AND SIGNAL BUTTE RD) TO 78E (PECOS AND CRISMON
 1947 RS 2 FLOW -1
 1948 RC .025 0.025 0.025 5100 .0005
 1949 RX 0 8 16 53.2 93.2 130.4 138.4 146.4
 1950 RY 6.0 6.1 6.2 0 0 6.3 6.1 6.0
 *

1951 KK 78E
 1952 KM BASIN 78E
 1953 KM THE FOLLOWING PARAMETERS WERE PROVIDED FOR THIS BASIN
 1954 KM L= 1.1 Lca= .5 S= 17.4 Kn= .087 LAG= 57.4
 1955 KM PHOENIX VALLEY S-GRAPH WAS USED FOR THIS BASIN
 1956 BA 1.01
 1957 LG .35 .26 8.80 .06 1.00
 1958 UI 59. 59. 108. 212. 271. 313. 351. 402. 460. 565.
 1959 UI 741. 698. 577. 502. 447. 382. 335. 289. 241. 166.
 1960 UI 104. 100. 97. 60. 59. 45. 18. 18. 18. 18.
 1961 UI 18. 18. 18. 18. 0. 0. 0. 0. 0. 0.
 1962 UI 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.
 *

1963 KK 83
 1964 KM BASIN 83
 1965 KM THE FOLLOWING PARAMETERS WERE PROVIDED FOR THIS BASIN
 1966 KM L= 2.0 Lca= .5 S= 15.0 Kn= .030 LAG= 25.8
 1967 KM PHOENIX VALLEY S-GRAPH WAS USED FOR THIS BASIN
 1968 BA 1.01
 1969 LG .15 .25 5.00 .36 55.00
 1970 UI 131. 423. 700. 931. 1454. 1362. 993. 726. 464. 226.
 HEC-1 INPUT

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

1971 UI 161. 89. 40. 40. 40. 0. 0. 0. 0. 0.
 1972 UI 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.
 *

1973 KK R83
 1974 KM RETAIN 100 YR 2 HR RUNOFF VOLUME
 1975 DT D83 83
 1976 DI 0 10000
 1977 DQ 0 10000
 *

1978 KK C78E
 1979 KM COMBINE FLOWS FROM 78D AND 78E (CRISMON ROAD)
 1980 HC 3
 *

1981 KK 78ET84
 1982 KM REACH PS-1
 1983 KM ROUTE FLOWS WEST ALONG PECOS IN A PROPOSED CHANNEL
 1984 KM ROUTE FLOWS FROM CRISMON ROAD TO ELLSWORTH ROAD.
 1985 RS 2 FLOW -1
 1986 RC .025 0.025 0.025 4840 .0005
 1987 RX 0 8 16 53.2 93.2 130.4 138.4 146.4
 1988 RY 6.0 6.1 6.2 0 0 6.2 6.1 6.0
 *

1989 KK 84
 1990 KM BASIN 84
 1991 KM THE FOLLOWING PARAMETERS WERE PROVIDED FOR THIS BASIN
 1992 KM L= 2.0 Lca= .5 S= 12.5 Kn= .030 LAG= 26.7
 1993 KM PHOENIX VALLEY S-GRAPH WAS USED FOR THIS BASIN
 1994 BA .99
 1995 LG .15 .25 4.70 .40 55.00
 1996 UI 125. 380. 651. 855. 1278. 1399. 1008. 747. 520. 258.
 1997 UI 182. 120. 38. 38. 38. 0. 0. 0. 0. 0.
 1998 UI 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.

*
 1999 KK R84
 2000 KM RETAIN 100 YR 2 HR RUNOFF VOLUME
 2001 DT D84 85
 2002 DI 0 10000
 2003 DQ 0 10000

*
 2004 KK C84
 2005 KM COMBINE FLOWS FROM 78E AND 84 AT ELLSWORTH AND PECOS ROAD
 2006 KM CHANNEL EAST SIDE OF GATEWAY WILLIAMS FLOWING TO THE NORTH
 2007 HC 2

* DDM ***** Preserved *****

HEC-1 INPUT

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LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

2008 KK 84T79B
 2009 KM REACH EH-3B
 2010 KM ROUTE FLOWS FROM THE CORNER OF PECOS AND ELLSWORTH ROADS TO
 2011 KM THE SOUTH OF WILLIAMS FIELD ROAD AND ELLSWORTH ROAD
 2012 KM (THIS IS THE COMBINE POINT FROM BASIN 79B)
 2013 RS 1 FLOW -1
 2014 RC .015 .015 .015 3383 .0010
 2015 RX 0 8 16 30 55 69 77 85
 2016 RY 6.7 6.8 6.9 0 0 6.9 6.8 6.7

*
 2017 KK 79B
 2018 KM BASIN 79B
 2019 KM THE FOLLOWING PARAMETERS WERE PROVIDED FOR THIS BASIN
 2020 KM L= 1.4 Lca= .6 S= 9.0 Kn= .090 LAG= 77.7
 2021 KM PHOENIX VALLEY S-GRAPH WAS USED FOR THIS BASIN
 2022 BA 1.00
 2023 LG .35 .25 9.70 .05 .00
 2024 UI 43. 43. 43. 85. 150. 179. 207. 232. 250. 275.
 2025 UI 306. 335. 388. 479. 557. 515. 444. 396. 360. 331.
 2026 UI 295. 267. 240. 216. 192. 155. 123. 76. 76. 71.
 2027 UI 71. 48. 43. 43. 36. 13. 13. 13. 13. 13.
 2028 UI 13. 13. 13. 13. 13. 13. 0. 0. 0. 0.
 2029 UI 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.

*
 2030 KK C79B1
 2031 KM FLOWS FROM SOUTH CHANNEL ALONG ELLSWORTH ROAD.
 2032 HC 2

*
 2033 KK 79BTB2
 2034 KM REACH EH-3A
 2035 KM ROUTE FLOWS FROM THE COMBINE POINT OF SUB-BASIN 79B TO
 2036 KM WILLIAMS FIELD ROAD AND ELLSWORTH ROAD
 2037 RS 2 FLOW -1
 2038 RC .025 .015 .025 5000 .0010
 2039 RX 0 8 16 30 55 69 77 85
 2040 RY 6.7 6.8 6.9 0 0 6.9 6.8 6.7

*
 2041 KK C79B2
 2042 KM COMBINE 79A AND ROUTED 79B (WHICH IS HYDROGRAPH C79B1)
 2043 HC 2

*
 2044 KK 79TPC2
 2045 KM REACH EH-1, EH-2, plus culvert EHC-1
 2046 KM ROUTE FLOWS THROUGH WILLIAMS-GATEWAY (SUBBASIN 80A) BY WAY OF NEW NORTH
 2047 KM PERIMETER CHANNEL ABOUT 1/2 MILE WEST OF ELLSWORTH ROAD
 2048 RS 1 FLOW -1
 2049 RC .025 .015 .025 4760 .0014
 2050 RX 0 8 16 33 61 78 86 94
 2051 RY 8.4 8.5 8.4 0 0 8.4 8.5 8.4

HEC-1 INPUT

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LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

2052 KK CPPWR
2053 KM COMBINE FLOWS FROM 75 AND 79 IN THE POWERLINE FLOODWAY ALONG RAY ROAD
2054 KM AT ABOUT 1/2 MILE WEST OF ELLSWORTH ROAD
2055 HC 2

*
* KK*DBPWR
* KM DIVERT FROM POWERLINE CHANNEL INTO THE RAY DETENTION BASIN
* KM USES A REALISTIC SIDE-WEIR EQUATION TO FORM POWER CURVE
* KM WEIR CREST = 3.25FT; WEIR LENGTH = 750; 4.0FT DIV STRUCTURE.
* KO 3
* DT PWRDB 1537
* DI 0 528 544 576 628 704 955 5730
* DQ 0 0 11 40 89 162 410 5174
*

2056 KK PWR80
2057 KM REACH PR-3, PR-4, plus culvert PRC-2
2058 KM ROUTE FLOWS FROM PLF COMBINE TO CATCH POINT AT 80A VIA PLF IMPROVEMENT
2059 RS 1 FLOW -1
2060 RC .025 .015 .025 3680 .0014
2061 RX 0 8 16 34 62 79 87 95
2062 RY 8.5 8.6 8.7 0 0 8.7 8.6 8.5
*

2063 KK 80A
2064 KM BASIN 80A
2065 KM THE FOLLOWING PARAMETERS WERE PROVIDED FOR THIS BASIN
2066 KM L= 3.8 Lca= 2.2 S= 14.2 Kn= .030 LAG= 58.2
2067 KM PHOENIX VALLEY S-GRAPH WAS USED FOR THIS BASIN
2068 BA 2.64
2069 LG .15 .15 9.70 .06 55.00
2070 UI 153. 153. 265. 544. 690. 802. 899. 1020. 1171. 1421.
2071 UI 1851. 1871. 1534. 1330. 1185. 1024. 896. 770. 653. 479.
2072 UI 305. 265. 251. 187. 153. 153. 50. 47. 47. 47.
2073 UI 47. 47. 47. 47. 0. 0. 0. 0. 0. 0.
2074 UI 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.
*

2075 KK R80A
2076 KM RETAIN 100 YR 2 HR RUNOFF VOLUME
2077 DT D80A 33
2078 DI 0 10000
2079 DQ 0 10000
*

* THE SECTION BELOW COMBINES THE ROUTED FLOW FROM SANTAN AND POWER,
* THEN ROUTES THRU THE RAY DB
* KKDRSNDB
* KM RETURNS THE DIVERSION FROM THE SANTAN CHANNEL
* DR SANDB
*
* KKDRPWDB
* KM RETURNS THE DIVERSION FROM THE POWERLINE CHANNEL
* DR PWRDB
*
* KK CPRAY
* KM COMBINES THE TWO ROUTED FLOWS IN THE RAY DETENTION BASIN
* HC 2 1
*
* KK*RTRAY
* KM ROUTES THE HYDROGRAPH OUT OF THE RAY BASIN
* KM Currently incorporates a dummy set of outflow data
* KO 1
* RS 1 STOR -1
* SV 0 244 493 747 1005 1269 1537
* SE 0 1.54 3.08 4.63 6.17 7.71 9.25
* SQ 0 1 2 3 4 5 6
*

HEC-1 INPUT

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LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

2080 KK CP80A
2081 KM COMBINE FLOW IN THE POWERLINE FLOODWAY WITH FLOW COMING FROM SUBBASIN 80A
2082 KM THE LOCATION FOR THIS COMBINATION AT THE NW CORNER OF SUBBASIN 80A
2083 HC 2
* HC 3
*

2084 KK PWRSAN
2085 KM REACH PR-1, PR-2 plus culvert PRC-1
2086 KM ROUTE FLOWS FROM COMBINE POINT AT 80A VIA PLF RE-ALIGNMENT.
2087 RS 1 FLOW -1
2088 RC .015 .015 .015 3500 .0014

2089 RX 0 8 16 74 154 212 220 228
 2090 RY 9.5 9.6 9.7 0 0 9.7 9.6 9.5
 *

2091 KK EMFPW
 2092 KM COMBINE FLOW FROM THE POWERLINE FLOODWAY WITH FLOW IN THE EMF
 2093 HC 2
 *

2094 KK POWTWI
 2095 KM ROUTE EMF FLOW TO WILLIAMS FIELD ROAD VIA THE EMF
 2096 KM THIS SECTION IS CONCRETE LINED TO PAST POWER ROAD BRIDGE
 2097 RS 2 FLOW -1
 2098 RC .03 .012 .03 4750 .0003
 2099 RX 0 500 520 553 693 726 740 742
 2100 RY 14 12 11 0 0 11 11 12
 *

2101 KK 80B
 2102 KM BASIN 80B
 2103 KM THE FOLLOWING PARAMETERS WERE PROVIDED FOR THIS BASIN
 2104 KM L= 1.5 Lca= .9 S= 18.4 Kn= .044 LAG= 41.9
 2105 KM PHOENIX VALLEY S-GRAPH WAS USED FOR THIS BASIN

* KO 21
 2106 BA 1.12
 2107 LG .13 .17 6.80 .18 48.00
 2108 UI 90. 105. 319. 433. 515. 614. 754. 1063. 1032. 814.
 2109 UI 687. 563. 464. 359. 213. 155. 136. 90. 73. 28.
 2110 UI 28. 28. 28. 28. 28. 0. 0. 0. 0. 0.
 2111 UI 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.
 *

2112 KK R80B
 2113 KM RETAIN 100 YR 2 HR RUNOFF VOLUME
 * KO 21

2114 DT D80B 4
 2115 DI 0 10000
 2116 DQ 0 10000
 *

* Subbasin 81B routed to EMFWIL per discussions with the FCDMC as part of the
 * Chandler Heights/Rittenhouse Basin Design Project. QAZ
 *

HEC-1 INPUT

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

2117 KK 81B
 2118 KM BASIN 81B
 2119 KM THE FOLLOWING PARAMETERS WERE PROVIDED FOR THIS BASIN
 2120 KM L= 1.1 Lca= .4 S= 6.9 Kn= .033 LAG= 24.7
 2121 KM PHOENIX VALLEY S-GRAPH WAS USED FOR THIS BASIN
 2122 BA .84
 2123 LG .10 .25 4.70 .45 67.00
 2124 UI 115. 393. 631. 857. 1343. 1088. 796. 566. 296. 185.
 2125 UI 115. 39. 35. 35. 0. 0. 0. 0. 0. 0.
 2126 UI 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.
 *

2127 KK R81B
 2128 KM RETAIN 100 YR 2 HR RUNOFF VOLUME
 * KO 3 21
 2129 DT D81B 35
 2130 DI 0 10000
 2131 DQ 0 10000
 *

* 81B to be combined with 80B and exported to EMF routing model by 80B81B
 * EMFWIL to combine 80B81B with flow from POWTWI for this model
 * qaz
 *

2132 KK 80B81B
 2133 KM COMBINE FLOWS FROM 80B & 81B AND EXPORT TO ROUTING MODEL
 * KO 21
 2134 HC 2
 *

2135 KK EMFWIL
 2136 KM COMBINE FLOWS INTO THE EMF WEST OF WILLIAMS AFB FROM 80B, 81B, EMF POWERLINE
 2137 HC 2
 *

2138 KK WILTSP
 2139 KM ROUTE EMF FLOW FROM WILLIAMS FIELD ROAD TO THE SOUTHERN PACIFIC RAILROAD
 2140 KM (AT RITTENHOUSE ROAD)
 2141 RS 3 FLOW -1

2142 RC .03 .022 .03 5000 .0003
 2143 RX 0 500 520 553 693 726 740 742
 2144 RY 14 12 11 0 0 11 11 12

*
 * DDM ***** Updated *****
 * KKEMFRT1 Hydrograph name changed by Dibble & Associates to avoid two
 * KM different hydrographs with the same name.
 * KM COMBINE 81A & 81B AND RITTENHOUSE (HYDROGRAPH WILTSP, FROM EMFWIL)
 * HC 2

* THE NEXT KK BLOCKS COME FROM THE QUEEN CREEK ADMS

* ***** UPDATED TO GREEN-AMPT *****
 HEC-1 INPUT

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

2145 KK SUB258
 2146 KM PINAL COUNTY BASIN. PARAMETERS BASED ON EXISTING LAND-USE
 2147 KM TO MODEL PINAL COUNTY'S PRE .VS. POST DEVELOPMENT DRAINAGE CRITERIA
 2148 KM BASIN 258
 2149 KM THE FOLLOWING PARAMETERS WERE PROVIDED FOR THIS BASIN
 2150 KM L= 4.6 Lca= 2.5 S= 24.8 Kn= .062 LAG= 122.0
 2151 KM AGRICULTURAL S-GRAPH WAS USED FOR THIS BASIN
 2152 BA 3.65
 2153 LG .34 .17 4.70 .42 18.00
 2154 UI 110. 110. 110. 110. 110. 217. 232. 299. 386. 574.
 2155 UI 583. 717. 579. 644. 772. 918. 802. 927. 927. 927.
 2156 UI 927. 927. 927. 927. 834. 772. 772. 772. 866. 865.
 2157 UI 583. 579. 583. 650. 605. 458. 421. 421. 381. 331.
 2158 UI 315. 309. 352. 309. 211. 211. 190. 178. 178. 159.
 2159 UI 136. 136. 136. 125. 101. 101. 101. 101. 101. 74.
 2160 UI 63. 63. 63. 63. 63. 63. 63. 63. 29. 14.
 2161 UI 14. 14. 14. 14. 14. 14. 14. 14. 14. 14.
 2162 UI 14. 14. 14. 14. 14. 14. 14. 14. 14. 14.
 2163 UI 14. 14. 14. 14. 14. 14. 14. 14. 14. 14.
 2164 UI 14. 14. 14. 14. 14. 14. 14. 14. 14. 0.
 2165 UI 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.
 2166 UI 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.

2167 KK RO259
 2168 KM ROUTE SUB258 TO CO262
 2169 RM 8 1.57 0.20

* ***** UPDATED TO GREEN-AMPT *****

2170 KK SUB260
 2171 KM MARICOPA COUNTY BASIN. PARAMETERS BASED ON FUTURE LAND-USE
 2172 KM BASIN 260
 2173 KM THE FOLLOWING PARAMETERS WERE PROVIDED FOR THIS BASIN
 2174 KM L= 1.0 Lca= .5 S= 23.2 Kn= .045 LAG= 27.3
 2175 KM PHOENIX VALLEY S-GRAPH WAS USED FOR THIS BASIN
 2176 BA .98
 2177 LG .27 .25 4.80 .36 24.00
 2178 UI 121. 355. 623. 813. 1175. 1413. 1018. 764. 555. 284.
 2179 UI 199. 121. 55. 37. 37. 37. 0. 0. 0. 0.
 2180 UI 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.

2181 KK RETAIN
 2182 KM 100-YR, 2HR RETENTION VOLUME FOR SUBBASIN LOCATED IN MARICOPA COUNTY
 2183 DT RETDIV 69
 2184 DI 0 10000
 2185 DQ 0 10000

* DDM ***** Preserved *****

HEC-1 INPUT

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

2186 KK CO262
 2187 KM COMBINE SUB260 AND RO259
 2188 HC 2

* DDM ***** Preserved *****

2189 KK RO263
 2190 KM ROUTE CO262 TO CO266
 2191 RM 8 1.56 0.20

* ***** UPDATED TO GREEN-AMPT *****

2192 KK SUB264

2193 KM MARICOPA COUNTY BASIN. PARAMETERS BASED ON FUTURE LAND-USE
 2194 KM BASIN 264
 2195 KM THE FOLLOWING PARAMETERS WERE PROVIDED FOR THIS BASIN
 2196 KM L= 1.0 Lca= .6 S= 20.0 Kn= .050 LAG= 32.9
 2197 KM PHOENIX VALLEY S-GRAPH WAS USED FOR THIS BASIN
 2198 BA 1.00
 2199 LG .25 .25 4.70 .38 31.00
 2200 UI 102. 217. 450. 584. 730. 1015. 1235. 921. 734. 572.
 2201 UI 426. 228. 171. 115. 82. 31. 31. 31. 31. 0.
 2202 UI 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.
 2203 UI 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.
 *

2204 KK RETAIN
 2205 KM 100-YR, 2HR RETENTION VOLUME FOR SUBBASIN LOCATED IN MARICOPA COUNTY
 2206 DT RETDIV 73
 2207 DI 0 10000
 2208 DQ 0 10000

* DDM ***** Preserved *****

2209 KK CO266
 2210 KM COMBINE SUB264 AND RO263
 2211 HC 2

* DDM ***** Preserved *****

2212 KK RO267
 2213 KM ROUTE CO266 TO CO270
 2214 RM 16 3.31 0.20

* ***** UPDATED TO GREEN-AMPT *****

2215 KK SUB268
 2216 KM MARICOPA COUNTY BASIN. PARAMETERS BASED ON FUTURE LAND-USE
 2217 KM BASIN 268
 2218 KM THE FOLLOWING PARAMETERS WERE PROVIDED FOR THIS BASIN
 2219 KM L= 2.0 Lca= 1.1 S= 13.4 Kn= .046 LAG= 55.1
 2220 KM PHOENIX VALLEY S-GRAPH WAS USED FOR THIS BASIN
 2221 BA .97
 2222 LG .23 .25 4.65 .39 34.00
 2223 UI 59. 59. 122. 219. 281. 324. 365. 422. 489. 637.
 HEC-1 INPUT

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10
 2224 UI 762. 634. 536. 474. 408. 355. 301. 255. 185. 113.
 2225 UI 102. 97. 63. 59. 45. 18. 18. 18. 18. 18.
 2226 UI 18. 18. 0. 0. 0. 0. 0. 0. 0. 0.
 2227 UI 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.
 *

2228 KK RETAIN
 2229 KM 100-YR, 2HR RETENTION VOLUME FOR SUBBASIN LOCATED IN MARICOPA COUNTY
 2230 DT RETDIV 68
 2231 DI 0 10000
 2232 DQ 0 10000

* DDM ***** Preserved *****

2233 KK CO270
 2234 KM COMBINE RUNOFF FROM RO267 AND SUB268
 2235 HC 2

* DDM ***** Preserved *****

2236 KK RO283
 2237 KM ROUTE CO282 TO CONCENTRATION POINT AT QUEEN CREEK ROAD
 2238 RM 14 2.78 0.20

* THIS IS THE END OF THE QUEEN CREEK ADMS INSERT

* DDM ***** Updated *****

2239 KK 88A
 2240 KM BASIN 88A
 2241 KM THE FOLLOWING PARAMETERS WERE PROVIDED FOR THIS BASIN
 2242 KM L= .8 Lca= .2 S= 13.2 Kn= .020 LAG= 9.2
 2243 KM PHOENIX VALLEY S-GRAPH WAS USED FOR THIS BASIN
 2244 BA .50
 2245 LG .10 .25 5.00 .40 80.00
 2246 UI 549. 1709. 1208. 323. 71. 0. 0. 0. 0. 0.
 2247 UI 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.
 *

* DDM ***** Preserved *****

2248 KK R88A

2249 KM RETAIN 100 YR 2 HR RUNOFF VOLUME
 2250 DT D88A 50
 2251 DI 0 10000
 2252 DQ 0 10000
 *

2253 KK 88AT89
 2254 KM REACH RH-2b,RH-2a,RH-1,EXISTING CHANNEL (FCD 97-34), plus culvert RHC-1
 2255 KM ROUTE 88A TO 89A VIA THE PROPOSED CHANNEL ALONG QUEEN CREEK ROAD
 2256 KM FROM CRISMON ROAD TO ELLSWORTH ROAD
 2257 RS 1 FLOW -1
 2258 RC .025 .025 .025 5135 .0010
 2259 RX 0 8 16 45 55 85 93 101
 2260 RY 4.7 4.8 4.9 0 0 4.9 4.8 4.7
 *

HEC-1 INPUT

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

2261 KK 89A
 2262 KM BASIN 89A
 2263 KM THE FOLLOWING PARAMETERS WERE PROVIDED FOR THIS BASIN
 2264 KM L= 1.0 Lca= .6 S= 19.0 Kn= .020 LAG= 13.5
 2265 KM PHOENIX VALLEY S-GRAPH WAS USED FOR THIS BASIN
 2266 BA .50
 2267 LG .10 .25 4.65 .47 80.00
 2268 UI 247. 742. 1328. 891. 408. 158. 41. 38. 0. 0.
 2269 UI 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.
 *

2270 KK R89A
 2271 KM RETAIN 100 YR 2 HR RUNOFF VOLUME
 2272 DT D89A 50
 2273 DI 0 10000
 2274 DQ 0 10000
 *

2275 KK C89A
 2276 KM COMBINE FLOWS FROM 88A AND 89A AT QUEEN CREEK ROAD AND ELLSWORTH ROAD
 2277 HC 2
 *

2278 KK 89ATRI
 2279 KM ROUTE 89A TO RITTENHOUSE ROAD VIA THE PROPOSED CHANNEL ALONG QUEEN CREEK ROAD
 2280 KM FROM ELLSWORTH ROAD TO RITTENHOUSE ROAD
 2281 KM REVISED 02.24.98 WITH RITTENHOUSE CHANNEL DESIGN.
 2282 RS 1 FLOW -1
 2283 RC .025 .025 .025 3145 .0005
 2284 RX 0 10 26 39 49 72 78 100
 2285 RY 12.2 9.7 5.7 0 0 5.7 9.7 12.2
 *

2286 KK C283
 2287 KM COMBINE FLOWS FROM QUEEN CREEK ADMS AND EC ADMP AT QUEEN CREEK ROAD AND
 2288 KM RITTENHOUSE ROAD.
 * KO 2
 2289 HC 2
 *

2290 KK 283T90
 2291 KM ROUTE FLOWS FROM CONCENTRATION POINT 283 AT QUEEN CREEK ROAD NORTH IN
 2292 KM RITTENHOUSE CHANNEL TO THE HALF MILE STREET BETWEEN QUEEN CREEK ROAD AND
 2293 KM GERMANN ROAD (RYAN STREET)
 2294 KM
 2295 KM REVISED 02.24.98 WITH RITTENHOUSE CHANNEL DESIGN (TYPICAL SECTION #5).
 2296 KM
 * KO 2
 2297 RS 1 FLOW -1
 2298 RC .025 .025 .025 4400 .0005
 2299 RX 0 22 28 51 61 83 90 108
 2300 RY 11.2 9.7 5.7 0 0 5.7 9.7 11.2
 *

HEC-1 INPUT

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

2301 KK 90A
 2302 KM BASIN 90A
 2303 KM THE FOLLOWING PARAMETERS WERE PROVIDED FOR THIS BASIN
 2304 KM L= .6 Lca= .2 S= 24.2 Kn= .038 LAG= 12.8
 2305 KM PHOENIX VALLEY S-GRAPH WAS USED FOR THIS BASIN
 2306 BA .48
 2307 LG .10 .25 4.60 .49 62.00
 2308 UI 269. 789. 1351. 812. 319. 113. 39. 0. 0. 0.
 2309 UI 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.

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*
2310 KK R90A
2311 KM RETAIN 100 YR 2 HR RUNOFF VOLUME
2312 DT D90A 42
2313 DI 0 10000
2314 DQ 0 10000
*
2315 KK C90A
2316 KM COMBINE FLOWS FLOW C283 AND SUBBASIN 90 AT RYAN STREET ALIGNMENT
* KO 2
2317 HC 2
*
2318 KK 90ATB
2319 KM ROUTE FLOWS FROM SUBBASIN 90A TO 90B VIA CHANNEL
2320 KM REVISED 02.24.98 WITH RITTENHOUSE CHANNEL DESIGN (TYPICAL SECTION #4).
2321 KM
* KO 2
2322 RS 1 FLOW -1
2323 RC .025 .025 .025 4400 .0005
2324 RX 0 22 28 51 61 83 90 108
2325 RY 11.2 9.7 5.7 0 0 5.7 9.7 11.2
*
2326 KK 87A
2327 KM BASIN 87A
2328 KM THE FOLLOWING PARAMETERS WERE PROVIDED FOR THIS BASIN
2329 KM L= 1.0 Lca= .5 S= 24.8 Kn= .020 LAG= 11.7
2330 KM PHOENIX VALLEY S-GRAPH WAS USED FOR THIS BASIN
2331 BA .49
2332 LG .10 .25 5.00 .40 80.00
2333 UI 333. 979. 1448. 720. 221. 66. 0. 0. 0. 0.
2334 UI 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.
*
2335 KK R87A
2336 KM RETAIN 100 YR 2 HR RUNOFF VOLUME
2337 DT D87A 49
2338 DI 0 10000
2339 DQ 0 10000
*

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HEC-1 INPUT

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LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10
2340 KK 87ATE
2341 KM ROUTE 87A TO 87B VIA SHEET FLOW
2342 RS 3 FLOW -1
2343 RC .040 .040 .040 2640 .0056
2344 RX 0 500 1000 1005 1006 1011 1511 2011
2345 RY 1 .5 0 0 0 .5 1 1.5
*
2346 KK 87B
2347 KM BASIN 87B
2348 KM THE FOLLOWING PARAMETERS WERE PROVIDED FOR THIS BASIN
2349 KM L= .9 Lca= .5 S= 11.6 Kn= .020 LAG= 12.8
2350 KM PHOENIX VALLEY S-GRAPH WAS USED FOR THIS BASIN
2351 BA .49
2352 LG .10 .25 5.00 .40 80.00
2353 UI 275. 809. 1385. 833. 327. 116. 40. 0. 0. 0.
2354 UI 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.
*
2355 KK R87B
2356 KM RETAIN 100 YR 2 HR RUNOFF VOLUME
2357 DT D87B 49
2358 DI 0 10000
2359 DQ 0 10000
*
2360 KK C87
2361 KM COMBINE FLOW FROM SUBBASINS 87A AND 87B
2362 HC 2
*
2363 KK 87T88B
2364 KM ROUTE S87 TO S88 VIA GERMANN ROAD
2365 RS 6 FLOW -1
2366 RC .045 .025 .045 5280 .002
2367 RX 0 1000 1005 1010 1050 1060 1560 2060
2368 RY 14 13 18 12 11 14 14.5 15
*

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1
 SCHEMATIC DIAGRAM OF STREAM NETWORK

INPUT LINE	(V) ROUTING	(--->) DIVERSION OR PUMP FLOW
NO.	(.) CONNECTOR	(<---) RETURN OF DIVERTED OR PUMPED FLOW
201	SOSS V	
206	RSOSS V	
212	.	59A
224	.	-----> D59A
222	.	R59A
227	C59A.....	
230	V V 59A59B	
237	.	59B
251	.	-----> D59B
249	.	R59B
254	C59B.....	
257	V V 59BT60	
263	.	60
280	.	-----> D60
278	.	R60
283	EMFGUA.....	
286	V V GUATEL	
292	.	64
308	.	-----> D64
303	.	R64
311	EMFELL.....	
314	V V ELTWAR	
320	.	62B
331	.	-----> D62B
329	.	R62B
334	V V 62BTD	
340	.	62D
353	.	-----> D62D
351	.	R62D
356	CP62D.....	
359	V V 62DTF	

365	.	.	62F	.	.
377
375	.	.	R62F	----->	D62F
380	.	.	CP62F
	.	.	V	.	.
	.	.	V	.	.
383	.	.	62T63	.	.
390
	.	.	63	.	.
402
400	.	.	R63	----->	D63
405	.	.	CP63
	.	.	V	.	.
	.	.	V	.	.
408	.	.	63T71	.	.
415
	.	.	68B	.	.
426
424	.	.	R68	----->	D68B
	.	.	V	.	.
	.	.	V	.	.
429	.	.	68BT69	.	.
435
	69
446
444
	R69
	----->
	D69
449	.	.	C69
	.	.	V	.	.
	.	.	V	.	.
452	.	.	69T71	.	.
458
	71
470
468
	R71
	----->
	D71
473	.	.	C71
	.	.	V	.	.
	.	.	V	.	.
479	.	.	71T72	.	.
486
	72
498
496	.	.	R72	----->	D72
501	.	.	CPWAR
503	EMFWAR
	V
	V
506	WARTKN
512	.	.	70B	.	.
524
522	.	.	R70B	----->	D70B
	.	.	V	.	.
	.	.	V	.	.
527	.	.	70BT76	.	.

804
801

809
807

812
810

815

820
818

821

830

833

836

839

844
842

845

852

856

859

864

871

883
881

886

889

895

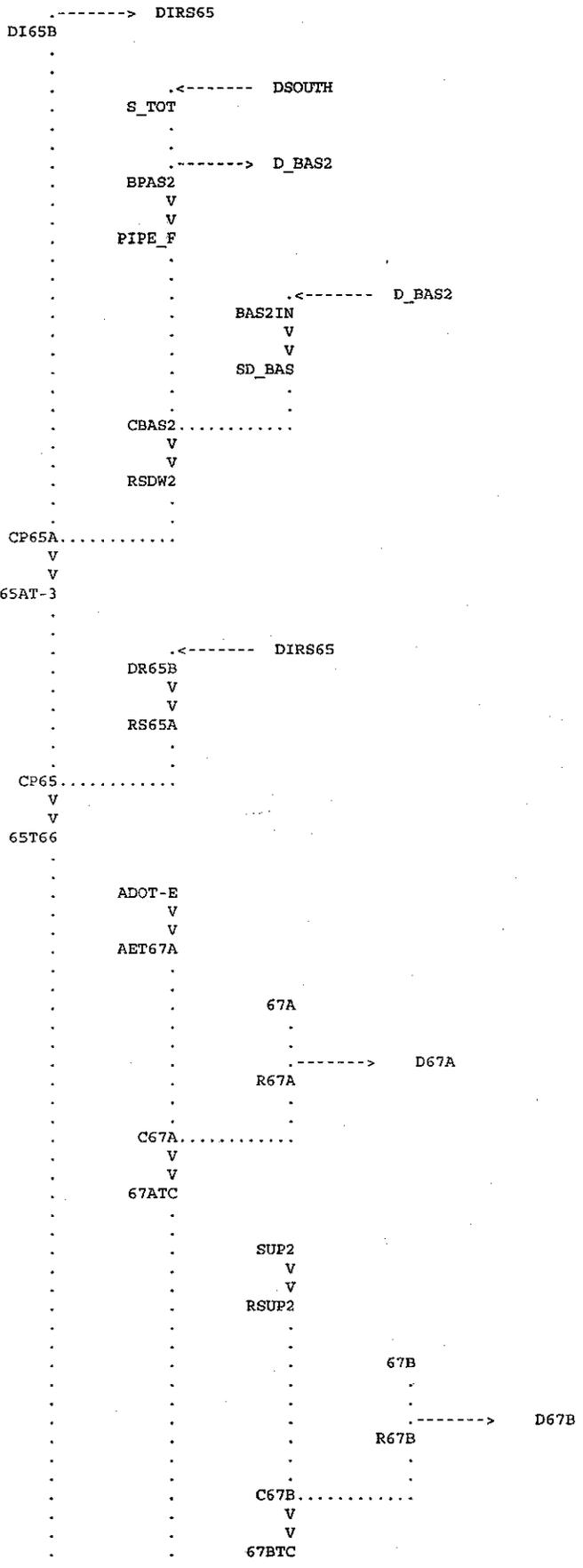
899

906

918
916

921

924



932	.	.	.	67C	
945	.	.	.	----->	D67C
943	.	.	.	R67C	
948	
	.	.	.	C67C.....	
	.	.	.	V	
	.	.	.	V	
951	.	.	.	67CT67	
959	
	.	.	.	67D	
971	.	.	.	----->	D67D
969	.	.	.	R67D	
974	
	.	.	.	C67D.....	
	.	.	.	V	
	.	.	.	V	
977	.	.	.	67DT66	
985	
	.	.	.	66A	
997	.	.	.	----->	D66A
995	.	.	.	R66A	
	.	.	.	V	
	.	.	.	V	
1000	.	.	.	66ATB	
1006	
	.	.	.	66B	
1019	.	.	.	----->	D66B
1017	.	.	.	R66B	
1022	
	.	.	.	CP66B.....	
	.	.	.	V	
	.	.	.	V	
1025	.	.	.	66BTC	
1031	
	.	.	.	66C	
1043	.	.	.	----->	D66C
1041	.	.	.	R66C	
1046	
	.	.	.	CP66C1.....	
1050	
	.	.	.	CP66C2.....	
1057	.	.	.	----->	DB66
1054	.	.	.	DI66	
	.	.	.	V	
	.	.	.	V	
1060	.	.	.	66C1T2	
1068	
	.	.	.	CP66C.....	
	.	.	.	V	
	.	.	.	V	
1071	.	.	.	66CTD	
1079	.	.	.	-----<	DB66
1077	.	.	.	DR66	
	.	.	.	V	
	.	.	.	V	
1080	.	.	.	RS66D1	
1093	.	.	.	----->	D-WB
1090	.	.	.	B-WA	
1096	
	.	.	.	C-WA.....	

1099	V			
	V			
	RC-WA			
1105				
1103		<-----	D-WB	
	DR-WA			
	V			
	V			
1106	RS66D2			
1116	CP66D		
	V			
	V			
1120	66T66D			
	V			
	V			
1124	66-66D			
1128		66D		
1140		----->	D66D	
1137		R66D		
1143		61A		
1155				
1153		----->	D61A	
		R61A		
		V		
		V		
1158		61ATB		
1164			61B	
1177				
1175			----->	D61B
			R61B	
1180		CP61B	
		V		
		V		
1183		61T66D		
1190			67E	
1203				
1200			----->	D67E
			R67E	
1206				
		C67E	
1209	C66D		
	V			
	V			
1212	66T70A			
	V			
	V			
1215	66T70B			
	V			
	V			
1218	CULVT			
	V			
	V			
1221	66T70C			
1228		62A		
1239		----->	D62A	
1237		R62A		
		V		
		V		
1242		62ATC		
1249			62C	

1431	CP73B.....		
	V		
	V		
1434	73BTC		
	.		
1441		73C	
	.		
1452		----->	D73C
1450		R73C	
	.		
1455	CP73C.....		
	V		
	V		
1458	73T74C		
	.		
1464		74A	
	.	V	
	.	V	
1478		DB74A	
	.	V	
	.	V	
1488		74ATB	
	.		
1494		74B	
	.		
1506		----->	D74B
1504		R74B	
	.		
1509	CP74B.....		
	V		
	V		
1512	74BTC		
	.		
1518		74C	
	.		
1529		----->	D74C
1527		R74C	
	.		
1532	CP74C.....		
	V		
	V		
1535	74CT75		
	.		
1542		75	
	.		
1563	C75.....		
	V		
	V		
1566	75TPC		
	.		
1572		77A	
	.	V	
	.	V	
1588		77ATB	
	.		
1594		77B	
	.		
1606		----->	D77B
1604		R77B	
	.		
1609	CP77B.....		
	V		
	V		
1612	77BTC		
	.		
1619		77C	
	.		

1630	.	.	----->	D77C
1628	.	R77C	.	
1633	.	C77C.....	.	
	.	V	.	
	.	V	.	
1636	.	77CT78	.	
1641	.	78A	.	
	.	V	.	
	.	V	.	
1657	.	78ATB	.	
1663	.	.	78B	
	.	.	.	
1675	.	C78B.....	.	
	.	V	.	
	.	V	.	
1678	.	78BTC	.	
1685	.	.	78C	
1696	.	.	----->	D78C
1694	.	.	R78C	
1699	.	C78C.....	.	
	.	V	.	
	.	V	.	
1702	.	78CT79	.	
1709	.	79A	.	
1724	.	C79A1.....	.	
1727	.	78F	.	
1744	.	.	82A1	
1759	.	C82A1.....	.	
	.	V	.	
	.	V	.	
1763	.	DB82A1	.	
	.	V	.	
	.	V	.	
1772	.	PS-9	.	
1779	.	.	CAP2	
	.	.	V	
	.	.	V	
1788	.	.	RCAP2	
1795	.	.	.	82A2
1815	.	.	CP82A2.....	
1818	.	.	.	82A4
	.	.	V	
	.	.	V	
1834	.	.	82A4T3	
1841	.	.	.	82A3
1858	.	.	CP82A3.....	
1861	.	.	CP82A5.....	
	.	.	V	
	.	.	V	
1864	.	.	DB82B	

1873			V	
			V	
			MN-1	
1880		CP82A6	
			V	
1883		82TBOX		
			V	
1890		BOXCLV		
			V	
1897		BOXT78		
1904			78D	
1916			----->	D78D
1914			R78D	
1919				82B
1931			----->	D82
1929			R82	
1937			----->	TRW
1934			DTRW	
1940		C78D	
			V	
1944		78DTE		
1951			78E	
1963				83
1975			----->	D83
1973			R83	
1978		C78E	
			V	
1981		78ET84		
1989			84	
2001			----->	D84
1999			R84	
2004		C84	
			V	
2008		84T79B		
2017			79B	
2030		C79B1	
			V	
2033		79BTB2		
2041		C79B2	
			V	
2044		79TPC2		
2052	CPPWR	V	

2056	.	V		
	.	PWRT80		
2063	.		80A	
	.		.	
2077	.		----->	D80A
2075	.		R80A	
	.		.	
2080	.	CP80A	
	.	V		
	.	V		
2084	.	PWRSAN		
	.			
2091	.	EMFPOM	
	.	V		
	.	V		
2094	.	POWTWI		
	.			
2101	.		80B	
	.		.	
2114	.		----->	D80B
2112	.		R80B	
	.		.	
2117	.		81B	
	.		.	
2129	.		----->	D81B
2127	.		R81B	
	.		.	
2132	.		80B81B	
	
2135	.	EMFWIL	
	.	V		
	.	V		
2138	.	WILTSP		
	.			
2145	.	SUB258		
	.	V		
	.	V		
2167	.	RO259		
	.			
2170	.		SUB260	
	.		.	
2183	.		----->	RETDIV
2181	.		RETAIN	
	.		.	
2186	.	CO262	
	.	V		
	.	V		
2189	.	RO263		
	.			
2192	.		SUB264	
	.		.	
2206	.		----->	RETDIV
2204	.		RETAIN	
	.		.	
2209	.	CO266	
	.	V		
	.	V		
2212	.	RO267		
	.			
2215	.		SUB268	
	.		.	
2230	.		----->	RETDIV
2228	.		RETAIN	
	.		.	
2233	.	CO270	
	.	V		

2236	V RO283		
2239		88A	
2250			D88A
2248		R88A	
		V	
		V	
2253		88AT89	
2261			89A
2272			D89A
2270		R89A	
2275		C89A	
		V	
		V	
2278		89ATRI	
2286	C283		
	V		
	V		
2290	283T90		
2301		90A	
2312			D90A
2310		R90A	
2315	C90A		
	V		
	V		
2318	90ATB		
2326		87A	
2337			D87A
2335		R87A	
		V	
		V	
2340		87ATB	
2346			87B
2357			D87B
2355		R87B	
2360		C87	
		V	
		V	
2363		87T88B	
2369			88B
2380			D88B
2378		R88B	
2383		C88B	
		V	
		V	
2386		88T89B	
2392			89B
2403			D89B
2401		R89B	

Basin B is on south west corner of Mountain Road and Siphon Draw Wash;
Basin C is on west of Mountain Road and north of Siphon Draw Wash.
No detention basins within Pinal County.

MODEL REVISED 9/12/02 TO CHANGE ZW CARD TO ZR CARD AT HYDROGRAPH CAP1B (CWR)

ID Kirkham Michael:
Last Revised Date: 5/14/02
Filename: WS4-SEM.DAT

Comments Dated 5/14/02 (CJ)

This model should be used for the Rittenhouse and Chandler Heights Basin
Design Project - 30% Design Analyses.

This model is one of several models that represent the EMF watershed.
This model covers the Southeast Mesa Area and should reference as a DSS
the watershed model for the Northeast Mesa Area (Filename WS2-NEM.DAT).

This model is necessary to determine the input hydrographs for the
Rittenhouse Basin Design HEC-RAS Unsteady State analysis. To develop
the necessary input hydrographs the following models should be run in order.
Because the files utilize a TAPE21 file to export import hydrographs
between models, prior to running the FIRST model (WS1-NWM.DAT) any existing
TAPE21 file in the directory should be deleted. The run procedure order is:

- 1) WS1-NWM.DAT
- 2) WS2-NEM.DAT
- 3) WS3-QCSW.DAT
- 4) WS4-SEM.DAT (referencing WS2-NEM.DSS for the DSS file)
- 5) RT1-BASE.DAT

The necessary input hydrographs for the Rittenhouse Basin analysis
are determined in RT1-BASE. In that output file, the hydrograph at
RWFLD1 should be exported and used as the input hydrograph at the
EMF Reach 4 Cross Section 17.082. And the hydrograph at RITTEN should
be exported and used as the input hydrograph for the Rittenhouse Main
Channel at Cross Section 820.00

**** NOTE BY PRIMATECH ENGINEERS: ****
**** DATE: 06/12/2001 ****
**** THE NEW FILE NAME IS: SEBTALT2.DAT ****
**** THE FILE WAS RENAMED AS <<RTBTALT2.DAT>> FOR THE EAST MARICOPA ****
**** FLOODWAY CAPACITY MITIGATION PROJECT, BY FLOOD CONTROL DISTRICT OF ****
**** MARICOPA COUNTY. ****
**** THE FILE WAS RENAMED <<RTBTALT3.DAT>> AND UPDATED USING GREEN AND ****
**** AMPT FUTURE CONDITIONS FOR BASINS 258 TO 268. ****

THIS MODEL WAS ORIGINALLY MIDDOUT.DAT
IT HAS BEEN MODIFIED BY CPE (7/2000)
FOR ALTERNATIVE 2 FOR THE EAST MARICOPA FLOWWAY
CAPACITY MITIGATION AND MULTI-USE CORRIDOR STUDY
TO ROUTE BOTH THE POWERLINE FLOWWAY
AND THE SANTAN FREEWAY CHANNEL INTO THE RAY BASIN PRIOR THEIR OUTFALL
INTO THE EMF

Model files changed by Collins/Pina Engineering
to reflect multi-use design concepts (recreation
and environment) proposed throughout the entire
EMF Corridor. July 2000

VERSION 8.06 CPE 7/31/00

FILENAME: MIDDOUT.DAT

ALL CIP INFRASTRUCTURE IS IN PLACE, FUTURE CONDITIONS LANDUSE IS IN PLACE
FLOW IS ROUTED UP ELLSWORTH ROAD IN A EARTH LINED CHANNEL

PRODUCED BY DIBBLE AND ASSOCIATES AND HOSKIN ENGINEERING CONSULTANTS.
File Name: Final8.Dat
Revised - Jan. 2000 by SZ (Wood/Patel) From Final7.dat - new Z-V & Sideweir
Revised - Jan. 2000 by SZ (Wood/Patel) from Final6.dat - 60% review comments

Revised - Dec. 1999 by SZ (Wood/Patel) from Final5.dat
Revised - Dec. 1999 by SZ (Wood/Patel) from Final4.dat
Revised - Nov. 1999 by SZ (Wood/Patel) from Final3.dat
Revised - June 1999 by SZ (Wood/Patel) for Final Model from Opt1.dat.
Revised - May 1999 by SZ (Wood/Patel) for Option 1, Based on Model SDIB.DAT
REVISED - MAY, 1999 BY VAS TO INCORPORATE INCREASE OF SUBBASIN RETENTION AND
REVISIONS TO THE REGIONAL DETENTION BASIN STORAGE
REVISED - FEB, 1999 BY VALERIE SWICK, FCD OF MARICOPA COUNTY
REVISED - MAY, 1998 BY D&A

REVISED BY VALERIE SWICK, FEB. 26, 1998

FLows FROM DETENTION BASIN LOCATED AT NE CORNER OF ELLIOT AND ELLSWORTH ROADS IS ROUTED TO THE SOUTHWEST BY SIPHON DRAW TO SUBBASIN 70A. FROM THERE THEY WILL BE ROUTED BY A CHANNEL TO THE EMF. FLOWS FROM SUBBASINS ADJACENT TO SANTAN FREEWAY ALIGNMENT WILL BE ROUTED SOUTH TO SUBBASIN 70A WHERE THEY WILL BE COMBINED WITH FLOW IN SIPHON DRAW.

EAST MESA AREA DRAINAGE MASTER PLAN
AREA SOUTH OF SUPERSTITION (U.S. HWY 60)
AUGUST 1997
SOUTHEAST MESA HIGH RESOLUTION MODEL

*****FUTURE CONDITION MODEL OF THE WATERSHED*****

*****ATTENTION*****
SUBBASINS 75, 79A, 79B, 78E, LANDUSES WERE NOT CHANGED BECAUSE IT WAS FELT THAT THEIR FUTURE CONDITIONS LANDUSES WOULD BE SIMILAR TO THE EXISTING CONDITIONS LANDUSES. RETENTION VOLUMES WILL ALSO NOT BE UTILIZED FOR SUBBASINS 75, 79A, 79B, 78E SOME QUEEN CREEK SUBBASINS WILL ALSO NOT HAVE RETENTION VOLUMES, EITHER BECAUSE THEY LIE IN PINAL COUNTY AND WE DONT KNOW PINAL COUNTIES PLANS OR THEY LIE IN THE SANTAN MOUNTAINS AND WON'T GET DEVELOPED WILLIAMS GATEWAY AIRPORT (SUBBASINS 80A, 80B, 81A, AND 81B) ARE MODELED AS FUTURE CONDITIONS AND HAVE RETENTION VOLUMES FOR THE 100YR 2HR STORM

FILENAME: SDIBB.DAT

THIS MODEL REPRESENTS THE FUTURE CONDITION OF THE WATERSHED.
TOTAL DRAINAGE AREA IS APPROXIMATELY 213 SQ. MI.
THIS MODEL USES A K_n VALUE OF 0.09 FOR DESERT LAND USE DUE TO SHEET FLOW CONDITIONS.

100-YEAR 24-HOUR FREQUENCY
AREAL REDUCTIONS FROM FCD HYDROLOGY MANUAL
THIS MODEL INCLUDES INFLOW FROM NORTH OF THE SUPERSTITION FREEWAY AND EAST OF THE CAP

DATA FROM THE QUEEN CREEK ADMS HAS BEEN ADDED TO CALCULATE FLOWS INTO THE EMF. MUSKINGUM ROUTING NSTEPS WERE ADJUSTED TO BE WITHIN THE SUGGESTED RANGE.

METHODOLOGY
THE US CORPS OF ENGINEERS FLOOD HYDROLOGY MODEL HEC-1 DATED SEP1990 VER 4.0
SCS TYPE II RAINFALL DISTRIBUTION
S-GRAPH HYDROGRAPH
GREEN AND AMPT INFILTRATION EQUATION USED FOR CALCULATING LOSSES
NORMAL DEPTH STORAGE CHANNEL ROUTING
APPROXIMATE DIRECTION, LOCATION, AND LENGTH OF THE WASHES HAVE BEEN EVALUATED BASED ON FIELD INVESTIGATION, USGS MAPS, LANDIS AERIAL SURVEYS DATED 1994
THE NOAA TECHNICAL MEMORANDUM NOAA ATLAS 2 DEPTH AREA RATIOS

ORIGINAL STUDY PERFORMED BY LISA C. YOUNG AND AFSHIN AHOURAIYAN, UPDATED BY DAVID DEGERNESS (OCT-DEC, 1996). REVIEWED BY VALERIE A. SWICK AND AMIR MOTAMEDI OF THE FLOOD CONTROL DISTRICT HYDROLOGY BRANCH ENGINEERING DIVISION, FLOOD CONTROL DISTRICT OF MARICOPA COUNTY, DECEMBER - JULY 1995.

ASSUMED VELOCITY OF 1 FT/SEC FOR SHEET FLOW, 2-3 FT/SEC FOR WASH/NATURAL CHANNEL, 3 FT/SEC FOR ROAD AND GRASS CHANNEL, 10FT/SEC FOR CONCRETE CHANNEL

VELOCITIES FOR ADMP IMPROVEMENT CHANNELS FROM DIBBLE AND ASSOCIATES SUGGESTED ALTERNATIVES (JULY 1, 1997)

*** THE FOLLOWING NOTE WAS ADDED BY PRIMATECH ENGINEERS ON 06-12-2001 ***

NOTE: MUST USE NEBUILD.DSS AS THE DSS FILE TO IMPORT FLOWS ACROSS THE SUPERSTITION FREEWAY.

NOTE: MUST USE NDIBF.DSS AS THE DSS FILE TO IMPORT FLOWS ACROSS THE SUPERSTITION FREEWAY.

DDM MCHUP2 SE MESA ADMP - SOUTH OF SUPERSTITION FWY, FUTURE CONDITIONS

182 IO

OUTPUT CONTROL VARIABLES

IPRNT 5 PRINT CONTROL
IPLOT 0 PLOT CONTROL
QSCAL 0. HYDROGRAPH PLOT SCALE

IT

HYDROGRAPH TIME DATA

NMIN 5 MINUTES IN COMPUTATION INTERVAL
IDATE 1APR97 STARTING DATE
ITIME 0000 STARTING TIME
NQ 1000 NUMBER OF HYDROGRAPH ORDINATES
NDDATE 4APR97 ENDING DATE
NDTIME 1115 ENDING TIME
ICENT 19 CENTURY MARK

COMPUTATION INTERVAL .08 HOURS
TOTAL TIME BASE 83.25 HOURS

ENGLISH UNITS

DRAINAGE AREA SQUARE MILES
PRECIPITATION DEPTH INCHES
LENGTH, ELEVATION FEET
FLOW CUBIC FEET PER SECOND
STORAGE VOLUME ACRE- FEET
SURFACE AREA ACRES
TEMPERATURE DEGREES FAHRENHEIT

184 JD

INDEX STORM NO. 1

STRM 3.60 PRECIPITATION DEPTH
TRDA .01 TRANSPOSITION DRAINAGE AREA

185 PI

PRECIPITATION PATTERN

Table with 10 columns of precipitation pattern data for storm 1, showing values ranging from 0.00 to 0.03.

195 JD

INDEX STORM NO. 2

STRM 3.38 PRECIPITATION DEPTH
TRDA 10.00 TRANSPOSITION DRAINAGE AREA

0 PI

PRECIPITATION PATTERN

Table with 10 columns of precipitation pattern data for storm 2, showing values ranging from 0.00 to 0.03.

.00 .00 .00 .00 .00 .00 .00 .00 .00 .00
.00 .00 .00 .00 .00 .00 .00 .00 .00 .00
.00 .00 .00 .00 .00 .00 .00 .00 .00 .00
.00 .00 .00 .00 .00 .00 .00 .00 .00 .00
.00 .00 .00 .00 .00 .00 .00 .00 .00 .00
.00 .00 .00 .00 .00 .00 .00 .00 .00 .00
.00 .00 .00 .00 .00 .00 .00 .00 .00 .00
.00 .00 .00 .00 .00 .00 .00 .00 .00 .00
.00 .00 .00 .00 .00 .00 .00 .00 .00 .00
.00 .00 .00 .00 .00 .00 .00 .00 .00 .00

-----DSS---ZOPEN: Existing File Opened, File: N60EM.DSS
Unit: 71; DSS Version: 6-JG

----- Entering ZRRTSX for unit 71 -----

Pathname: /SOSSAMAN DRAIN/AT SUPERSTITION/FLOW//5MIN/100YR/
Time Window set. Interval: 5 Number of data values: 1
Starting date and time: Mar 31, 1997 2400 (35519 1440)
Ending date and time: Mar 31, 1997 2400 (35519 1440)
Input time offset: 0
After ZRDINF, Record found: T
Pathname: /SOSSAMAN DRAIN/AT SUPERSTITION/FLOW/31MAR1997/5MIN/100YR/
Number of actual data: 288 Header length: 0
Compression: 0 Quality: 0

-----DSS--- ZREAD Unit 71; Vers. 4: /SOSSAMAN DRAIN/AT SUPERSTITION/FLOW/31MAR1997/5MIN/100YR/

-----DSS---Debug: Enter ZRRTSB; Unit: 71
NSTART: 1 NVALS: 1 JULS: 35519 ISTE: 1440
NLDATA: 288 JULSD: 35519
JULS: 31MAR97 JULSD: 31MAR97
Quality Read: F, Quality Requested: F
---ZRRTSB Calculations: NPOS: 288 NDATA: 288 NREAD: 1 ILIM: 1
----- Exiting ZRRTS, Number of data values: 1, Status: 0
Offset: 0, Units: CFS, Type:INST-VAL

----- Entering ZRRTSX for unit 71 -----

Pathname: /SOSSAMAN DRAIN/AT SUPERSTITION/FLOW//5MIN/100YR/
Time Window set. Interval: 5 Number of data values: 744
Starting date and time: Mar 31, 1997 2400 (35519 1440)
Ending date and time: Apr 3, 1997 1355 (35522 835)
Input time offset: 0
After ZRDINF, Record found: T
Pathname: /SOSSAMAN DRAIN/AT SUPERSTITION/FLOW/31MAR1997/5MIN/100YR/
Number of actual data: 288 Header length: 0
Compression: 0 Quality: 0

-----DSS--- ZREAD Unit 71; Vers. 4: /SOSSAMAN DRAIN/AT SUPERSTITION/FLOW/31MAR1997/5MIN/100YR/

-----DSS---Debug: Enter ZRRTSB; Unit: 71
NSTART: 1 NVALS: 744 JULS: 35519 ISTE: 1440
NLDATA: 288 JULSD: 35519
JULS: 31MAR97 JULSD: 31MAR97
Quality Read: F, Quality Requested: F
---ZRRTSB Calculations: NPOS: 288 NDATA: 288 NREAD: 1 ILIM: 1
After ZRDINF, Record found: T
Pathname: /SOSSAMAN DRAIN/AT SUPERSTITION/FLOW/01APR1997/5MIN/100YR/
Number of actual data: 288 Header length: 0
Compression: 0 Quality: 0

-----DSS--- ZREAD Unit 71; Vers. 4: /SOSSAMAN DRAIN/AT SUPERSTITION/FLOW/01APR1997/5MIN/100YR/

-----DSS---Debug: Enter ZRRTSB; Unit: 71
NSTART: 2 NVALS: 744 JULS: 35519 ISTE: 1440
NLDATA: 288 JULSD: 35520
JULS: 31MAR97 JULSD: 01APR97
Quality Read: F, Quality Requested: F
---ZRRTSB Calculations: NPOS: 1 NDATA: 288 NREAD: 288 ILIM: 289
After ZRDINF, Record found: T
Pathname: /SOSSAMAN DRAIN/AT SUPERSTITION/FLOW/02APR1997/5MIN/100YR/
Number of actual data: 288 Header length: 0
Compression: 0 Quality: 0

-----DSS--- ZREAD Unit 71; Vers. 4: /SOSSAMAN DRAIN/AT SUPERSTITION/FLOW/02APR1997/5MIN/100YR/

-----DSS---Debug: Enter ZRRTSB; Unit: 71
NSTART: 290 NVALS: 744 JULS: 35519 ISTE: 1440
NLDATA: 288 JULSD: 35521
JULS: 31MAR97 JULSD: 02APR97
Quality Read: F, Quality Requested: F
---ZRRTSB Calculations: NPOS: 1 NDATA: 288 NREAD: 288 ILIM: 577
After ZRDINF, Record found: T
Pathname: /SOSSAMAN DRAIN/AT SUPERSTITION/FLOW/03APR1997/5MIN/100YR/
Number of actual data: 288 Header length: 0
Compression: 0 Quality: 0

-----DSS--- ZREAD Unit 71; Vers. 4: /SOSSAMAN DRAIN/AT SUPERSTITION/FLOW/03APR1997/5MIN/100YR/

-----DSS---Debug: Enter ZRRTSB; Unit: 71
NSTART: 578 NVALS: 744 JULS: 35519 ISTE: 1440
NLDATA: 288 JULSD: 35522
JULS: 31MAR97 JULSD: 03APR97
Quality Read: F, Quality Requested: F
---ZRRTSB Calculations: NPOS: 1 NDATA: 288 NREAD: 167 ILIM: 744
----- Exiting ZRRTS, Number of data values: 744, Status: 0
Offset: 0, Units: CFS, Type:INST-VAL

----- Entering ZRRTSX for unit 71 -----

Pathname: /SOSSAMAN DRAIN/AT SUPERSTITION/FLOW//5MIN/100YR/
Time Window set. Interval: 5 Number of data values: 1
Starting date and time: Apr 3, 1997 1355 (35522 835)

Ending date and time: Apr 3, 1997 1355 (35522 835)
Input time offset: 0
After ZRDINF, Record found: T
Pathname: /SOSSAMAN DRAIN/AT SUPERSTITION/FLOW/03APR1997/SMIN/100YR/
Number of actual data: 288 Header length: 0
Compression: 0 Quality: 0
-----DSS--- ZREAD Unit 71; Vers. 4: /SOSSAMAN DRAIN/AT SUPERSTITION/FLOW/03APR1997/SMIN/100YR/
-----DSS---Debug: Enter ZRRTSB; Unit: 71
NSTART: 1 NVALS: 1 JULS: 35522 ISTE: 835
NLDATA: 288 JULSD: 35522
JULS: 03APR97 JULSD: 03APR97
Quality Read: F, Quality Requested: F
---ZRRTSB Calculations: NPOS: 167 NDATA: 288 NREAD: 1 ILIM: 1
----- Exiting ZRRTS, Number of data values: 1, Status: 0
Offset: 0, Units: CFS, Type: INST-VAL
----- Entering ZRRTSX for unit 71 -----
Pathname: /SOSSAMAN DRAIN/AT SUPERSTITION/FLOW//SMIN/100YR/
Time Window set. Interval: 5 Number of data values: 744
Starting date and time: Apr 3, 1997 1355 (35522 835)
Ending date and time: Apr 6, 1997 0350 (35525 230)
Input time offset: 0
After ZRDINF, Record found: T
Pathname: /SOSSAMAN DRAIN/AT SUPERSTITION/FLOW/03APR1997/SMIN/100YR/
Number of actual data: 288 Header length: 0
Compression: 0 Quality: 0
-----DSS--- ZREAD Unit 71; Vers. 4: /SOSSAMAN DRAIN/AT SUPERSTITION/FLOW/03APR1997/SMIN/100YR/
-----DSS---Debug: Enter ZRRTSB; Unit: 71
NSTART: 1 NVALS: 744 JULS: 35522 ISTE: 835
NLDATA: 288 JULSD: 35522
JULS: 03APR97 JULSD: 03APR97
Quality Read: F, Quality Requested: F
---ZRRTSB Calculations: NPOS: 167 NDATA: 288 NREAD: 122 ILIM: 122
After ZRDINF, Record found: T
Pathname: /SOSSAMAN DRAIN/AT SUPERSTITION/FLOW/04APR1997/SMIN/100YR/
Number of actual data: 288 Header length: 0
Compression: 0 Quality: 0
-----DSS--- ZREAD Unit 71; Vers. 4: /SOSSAMAN DRAIN/AT SUPERSTITION/FLOW/04APR1997/SMIN/100YR/
-----DSS---Debug: Enter ZRRTSB; Unit: 71
NSTART: 123 NVALS: 744 JULS: 35522 ISTE: 835
NLDATA: 288 JULSD: 35523
JULS: 03APR97 JULSD: 04APR97
Quality Read: F, Quality Requested: F
---ZRRTSB Calculations: NPOS: 1 NDATA: 288 NREAD: 288 ILIM: 410
After ZRDINF, Record found: F
Pathname: /SOSSAMAN DRAIN/AT SUPERSTITION/FLOW/05APR1997/SMIN/100YR/
-----DSS---Debug: Enter ZRRTSB; Unit: 71
NSTART: 411 NVALS: 744 JULS: 35522 ISTE: 835
NLDATA: 288 JULSD: 35524
JULS: 03APR97 JULSD: 05APR97
Quality Read: F, Quality Requested: F
---ZRRTSB Calculations: NPOS: 1 NDATA: 288 NREAD: 288 ILIM: 698
-----DSS*** ZRRTS: CAUTION - Data block not found in file. Unit: 71
Pathname: /SOSSAMAN DRAIN/AT SUPERSTITION/FLOW/05APR1997/SMIN/100YR/
After ZRDINF, Record found: F
Pathname: /SOSSAMAN DRAIN/AT SUPERSTITION/FLOW/06APR1997/SMIN/100YR/
-----DSS---Debug: Enter ZRRTSB; Unit: 71
NSTART: 699 NVALS: 744 JULS: 35522 ISTE: 835
NLDATA: 288 JULSD: 35525
JULS: 03APR97 JULSD: 06APR97
Quality Read: F, Quality Requested: F
---ZRRTSB Calculations: NPOS: 1 NDATA: 288 NREAD: 46 ILIM: 744
----- Exiting ZRRTS, Number of data values: 744, Status: 3
Offset: 0, Units: CFS, Type: INST-VAL
----- Entering ZRRTSX for unit 71 -----
Pathname: /CAP1A/OVERCHUTE/FLOW//SMIN/100YEAR/
Time Window set. Interval: 5 Number of data values: 1
Starting date and time: Mar 31, 1997 2400 (35519 1440)
Ending date and time: Mar 31, 1997 2400 (35519 1440)
Input time offset: 0
After ZRDINF, Record found: T
Pathname: /CAP1A/OVERCHUTE/FLOW/31MAR1997/SMIN/100YEAR/
Number of actual data: 288 Header length: 0
Compression: 0 Quality: 0
-----DSS--- ZREAD Unit 71; Vers. 4: /CAP1A/OVERCHUTE/FLOW/31MAR1997/SMIN/100YEAR/
-----DSS---Debug: Enter ZRRTSB; Unit: 71
NSTART: 1 NVALS: 1 JULS: 35519 ISTE: 1440
NLDATA: 288 JULSD: 35519
JULS: 31MAR97 JULSD: 31MAR97
Quality Read: F, Quality Requested: F
---ZRRTSB Calculations: NPOS: 288 NDATA: 288 NREAD: 1 ILIM: 1
----- Exiting ZRRTS, Number of data values: 1, Status: 0
Offset: 0, Units: CFS, Type: INST-VAL
----- Entering ZRRTSX for unit 71 -----
Pathname: /CAP1A/OVERCHUTE/FLOW//SMIN/100YEAR/
Time Window set. Interval: 5 Number of data values: 744
Starting date and time: Mar 31, 1997 2400 (35519 1440)
Ending date and time: Apr 3, 1997 1355 (35522 835)

Input time offset: 0
After ZRDINF, Record found: T
Pathname: /CAP1A/OVERCHUTE/FLOW/31MAR1997/SMIN/100YEAR/
Number of actual data: 288 Header length: 0
Compression: 0 Quality: 0
-----DSS--- ZREAD Unit 71; Vers. 4: /CAP1A/OVERCHUTE/FLOW/31MAR1997/SMIN/100YEAR/
-----DSS---Debug: Enter ZRRTSB; Unit: 71
NSTART: 1 NVALS: 744 JULS: 35519 ISTE: 1440
NLDATA: 288 JULSD: 35519
JULS: 31MAR97 JULSD: 31MAR97
Quality Read: F, Quality Requested: F
---ZRRTSB Calculations: NPOS: 288 NDATA: 288 NREAD: 1 ILIM: 1
After ZRDINF, Record found: T
Pathname: /CAP1A/OVERCHUTE/FLOW/01APR1997/SMIN/100YEAR/
Number of actual data: 288 Header length: 0
Compression: 0 Quality: 0
-----DSS--- ZREAD Unit 71; Vers. 4: /CAP1A/OVERCHUTE/FLOW/01APR1997/SMIN/100YEAR/
-----DSS---Debug: Enter ZRRTSB; Unit: 71
NSTART: 2 NVALS: 744 JULS: 35519 ISTE: 1440
NLDATA: 288 JULSD: 35520
JULS: 31MAR97 JULSD: 01APR97
Quality Read: F, Quality Requested: F
---ZRRTSB Calculations: NPOS: 1 NDATA: 288 NREAD: 288 ILIM: 289
After ZRDINF, Record found: T
Pathname: /CAP1A/OVERCHUTE/FLOW/02APR1997/SMIN/100YEAR/
Number of actual data: 288 Header length: 0
Compression: 0 Quality: 0
-----DSS--- ZREAD Unit 71; Vers. 4: /CAP1A/OVERCHUTE/FLOW/02APR1997/SMIN/100YEAR/
-----DSS---Debug: Enter ZRRTSB; Unit: 71
NSTART: 290 NVALS: 744 JULS: 35519 ISTE: 1440
NLDATA: 288 JULSD: 35521
JULS: 31MAR97 JULSD: 02APR97
Quality Read: F, Quality Requested: F
---ZRRTSB Calculations: NPOS: 1 NDATA: 288 NREAD: 288 ILIM: 577
After ZRDINF, Record found: T
Pathname: /CAP1A/OVERCHUTE/FLOW/03APR1997/SMIN/100YEAR/
Number of actual data: 288 Header length: 0
Compression: 0 Quality: 0
-----DSS--- ZREAD Unit 71; Vers. 4: /CAP1A/OVERCHUTE/FLOW/03APR1997/SMIN/100YEAR/
-----DSS---Debug: Enter ZRRTSB; Unit: 71
NSTART: 578 NVALS: 744 JULS: 35519 ISTE: 1440
NLDATA: 288 JULSD: 35522
JULS: 31MAR97 JULSD: 03APR97
Quality Read: F, Quality Requested: F
---ZRRTSB Calculations: NPOS: 1 NDATA: 288 NREAD: 167 ILIM: 744
----- Exiting ZRRTS, Number of data values: 744, Status: 0
Offset: 0, Units: CFS, Type: INST-VAL
----- Entering ZRRTSX for unit 71 -----
Pathname: /CAP1A/OVERCHUTE/FLOW//5MIN/100YEAR/
Time Window set. Interval: 5 Number of data values: 1
Starting date and time: Apr 3, 1997 1355 (35522 835)
Ending date and time: Apr 3, 1997 1355 (35522 835)
Input time offset: 0
After ZRDINF, Record found: T
Pathname: /CAP1A/OVERCHUTE/FLOW/03APR1997/SMIN/100YEAR/
Number of actual data: 288 Header length: 0
Compression: 0 Quality: 0
-----DSS--- ZREAD Unit 71; Vers. 4: /CAP1A/OVERCHUTE/FLOW/03APR1997/SMIN/100YEAR/
-----DSS---Debug: Enter ZRRTSB; Unit: 71
NSTART: 1 NVALS: 1 JULS: 35522 ISTE: 835
NLDATA: 288 JULSD: 35522
JULS: 03APR97 JULSD: 03APR97
Quality Read: F, Quality Requested: F
---ZRRTSB Calculations: NPOS: 167 NDATA: 288 NREAD: 1 ILIM: 1
----- Exiting ZRRTS, Number of data values: 1, Status: 0
Offset: 0, Units: CFS, Type: INST-VAL
----- Entering ZRRTSX for unit 71 -----
Pathname: /CAP1A/OVERCHUTE/FLOW//5MIN/100YEAR/
Time Window set. Interval: 5 Number of data values: 744
Starting date and time: Apr 3, 1997 1355 (35522 835)
Ending date and time: Apr 6, 1997 0350 (35525 230)
Input time offset: 0
After ZRDINF, Record found: T
Pathname: /CAP1A/OVERCHUTE/FLOW/03APR1997/SMIN/100YEAR/
Number of actual data: 288 Header length: 0
Compression: 0 Quality: 0
-----DSS--- ZREAD Unit 71; Vers. 4: /CAP1A/OVERCHUTE/FLOW/03APR1997/SMIN/100YEAR/
-----DSS---Debug: Enter ZRRTSB; Unit: 71
NSTART: 1 NVALS: 744 JULS: 35522 ISTE: 835
NLDATA: 288 JULSD: 35522
JULS: 03APR97 JULSD: 03APR97
Quality Read: F, Quality Requested: F
---ZRRTSB Calculations: NPOS: 167 NDATA: 288 NREAD: 122 ILIM: 122
After ZRDINF, Record found: T
Pathname: /CAP1A/OVERCHUTE/FLOW/04APR1997/SMIN/100YEAR/
Number of actual data: 288 Header length: 0
Compression: 0 Quality: 0

-----DSS--- ZREAD Unit 71; Vers. 4: /CAP1A/OVERCHUTE/FLOW/04APR1997/5MIN/100YEAR/
-----DSS---Debug: Enter ZRRTSB; Unit: 71
NSTART: 123 NVALS: 744 JULS: 35522 ISTE: 835
NLDATA: 288 JULSD: 35523
JULS: 03APR97 JULSD: 04APR97
Quality Read: F, Quality Requested: F
---ZRRTSB Calculations: NPOS: 1 NDATA: 288 NREAD: 288 ILIM: 410
After ZRDINF, Record found: F
Pathname: /CAP1A/OVERCHUTE/FLOW/05APR1997/5MIN/100YEAR/
-----DSS---Debug: Enter ZRRTSB; Unit: 71
NSTART: 411 NVALS: 744 JULS: 35522 ISTE: 835
NLDATA: 288 JULSD: 35524
JULS: 03APR97 JULSD: 05APR97
Quality Read: F, Quality Requested: F
---ZRRTSB Calculations: NPOS: 1 NDATA: 288 NREAD: 288 ILIM: 698
-----DSS*** ZRRTS: CAUTION - Data block not found in file. Unit: 71
Pathname: /CAP1A/OVERCHUTE/FLOW/05APR1997/5MIN/100YEAR/
After ZRDINF, Record found: F
Pathname: /CAP1A/OVERCHUTE/FLOW/06APR1997/5MIN/100YEAR/
-----DSS---Debug: Enter ZRRTSB; Unit: 71
NSTART: 699 NVALS: 744 JULS: 35522 ISTE: 835
NLDATA: 288 JULSD: 35525
JULS: 03APR97 JULSD: 06APR97
Quality Read: F, Quality Requested: F
---ZRRTSB Calculations: NPOS: 1 NDATA: 288 NREAD: 46 ILIM: 744
----- Exiting ZRRTS, Number of data values: 744, Status: 3
Offset: 0, Units: CFS, Type: INST-VAL
----- Entering ZRRTSX for unit 71 -----
Pathname: /CAP1B/OVERCHUTE/FLOW//5MIN/100YEAR/
Time Window set. Interval: 5 Number of data values: 1
Starting date and time: Mar 31, 1997 2400 (35519 1440)
Ending date and time: Mar 31, 1997 2400 (35519 1440)
Input time offset: 0
After ZRDINF, Record found: T
Pathname: /CAP1B/OVERCHUTE/FLOW/31MAR1997/5MIN/100YEAR/
Number of actual data: 288 Header length: 0
Compression: 0 Quality: 0
-----DSS--- ZREAD Unit 71; Vers. 4: /CAP1B/OVERCHUTE/FLOW/31MAR1997/5MIN/100YEAR/
-----DSS---Debug: Enter ZRRTSB; Unit: 71
NSTART: 1 NVALS: 1 JULS: 35519 ISTE: 1440
NLDATA: 288 JULSD: 35519
JULS: 31MAR97 JULSD: 31MAR97
Quality Read: F, Quality Requested: F
---ZRRTSB Calculations: NPOS: 288 NDATA: 288 NREAD: 1 ILIM: 1
----- Exiting ZRRTS, Number of data values: 1, Status: 0
Offset: 0, Units: CFS, Type: INST-VAL
----- Entering ZRRTSX for unit 71 -----
Pathname: /CAP1B/OVERCHUTE/FLOW//5MIN/100YEAR/
Time Window set. Interval: 5 Number of data values: 744
Starting date and time: Mar 31, 1997 2400 (35519 1440)
Ending date and time: Apr 3, 1997 1355 (35522 835)
Input time offset: 0
After ZRDINF, Record found: T
Pathname: /CAP1B/OVERCHUTE/FLOW/31MAR1997/5MIN/100YEAR/
Number of actual data: 288 Header length: 0
Compression: 0 Quality: 0
-----DSS--- ZREAD Unit 71; Vers. 4: /CAP1B/OVERCHUTE/FLOW/31MAR1997/5MIN/100YEAR/
-----DSS---Debug: Enter ZRRTSB; Unit: 71
NSTART: 1 NVALS: 744 JULS: 35519 ISTE: 1440
NLDATA: 288 JULSD: 35519
JULS: 31MAR97 JULSD: 31MAR97
Quality Read: F, Quality Requested: F
---ZRRTSB Calculations: NPOS: 288 NDATA: 288 NREAD: 1 ILIM: 1
After ZRDINF, Record found: T
Pathname: /CAP1B/OVERCHUTE/FLOW/01APR1997/5MIN/100YEAR/
Number of actual data: 288 Header length: 0
Compression: 0 Quality: 0
-----DSS--- ZREAD Unit 71; Vers. 4: /CAP1B/OVERCHUTE/FLOW/01APR1997/5MIN/100YEAR/
-----DSS---Debug: Enter ZRRTSB; Unit: 71
NSTART: 2 NVALS: 744 JULS: 35519 ISTE: 1440
NLDATA: 288 JULSD: 35520
JULS: 31MAR97 JULSD: 01APR97
Quality Read: F, Quality Requested: F
---ZRRTSB Calculations: NPOS: 1 NDATA: 288 NREAD: 288 ILIM: 289
After ZRDINF, Record found: T
Pathname: /CAP1B/OVERCHUTE/FLOW/02APR1997/5MIN/100YEAR/
Number of actual data: 288 Header length: 0
Compression: 0 Quality: 0
-----DSS--- ZREAD Unit 71; Vers. 4: /CAP1B/OVERCHUTE/FLOW/02APR1997/5MIN/100YEAR/
-----DSS---Debug: Enter ZRRTSB; Unit: 71
NSTART: 290 NVALS: 744 JULS: 35519 ISTE: 1440
NLDATA: 288 JULSD: 35521
JULS: 31MAR97 JULSD: 02APR97
Quality Read: F, Quality Requested: F
---ZRRTSB Calculations: NPOS: 1 NDATA: 288 NREAD: 288 ILIM: 577
After ZRDINF, Record found: T
Pathname: /CAP1B/OVERCHUTE/FLOW/03APR1997/5MIN/100YEAR/

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Number of actual data: 288 Header length: 0
Compression: 0 Quality: 0
-----DSS--- ZREAD Unit 71; Vers. 4: /CAP1B/OVERCHUTE/FLOW/03APR1997/5MIN/100YEAR/
-----DSS---Debug: Enter ZRRTSB; Unit: 71
NSTART: 578 NVALS: 744 JULS: 35519 IOSTIME: 1440
NLDATA: 288 JULSD: 35522
JULS: 31MAR97 JULSD: 03APR97
Quality Read: F, Quality Requested: F
---ZRRTSB Calculations: NPOS: 1 NDATA: 288 NREAD: 167 ILIM: 744
----- Exiting ZRRTS, Number of data values: 744, Status: 0
Offset: 0, Units: CFS, Type:INST-VAL
----- Entering ZRRTSX for unit 71 -----
Pathname: /CAP1B/OVERCHUTE/FLOW//5MIN/100YEAR/
Time Window set. Interval: 5 Number of data values: 1
Starting date and time: Apr 3, 1997 1355 ( 35522 835)
Ending date and time: Apr 3, 1997 1355 ( 35522 835)
Input time offset: 0
After ZRDINF, Record found: T
Pathname: /CAP1B/OVERCHUTE/FLOW/03APR1997/5MIN/100YEAR/
Number of actual data: 288 Header length: 0
Compression: 0 Quality: 0
-----DSS--- ZREAD Unit 71; Vers. 4: /CAP1B/OVERCHUTE/FLOW/03APR1997/5MIN/100YEAR/
-----DSS---Debug: Enter ZRRTSB; Unit: 71
NSTART: 1 NVALS: 1 JULS: 35522 IOSTIME: 835
NLDATA: 288 JULSD: 35522
JULS: 03APR97 JULSD: 03APR97
Quality Read: F, Quality Requested: F
---ZRRTSB Calculations: NPOS: 167 NDATA: 288 NREAD: 1 ILIM: 1
----- Exiting ZRRTS, Number of data values: 1, Status: 0
Offset: 0, Units: CFS, Type:INST-VAL
----- Entering ZRRTSX for unit 71 -----
Pathname: /CAP1B/OVERCHUTE/FLOW//5MIN/100YEAR/
Time Window set. Interval: 5 Number of data values: 744
Starting date and time: Apr 3, 1997 1355 ( 35522 835)
Ending date and time: Apr 6, 1997 0350 ( 35525 230)
Input time offset: 0
After ZRDINF, Record found: T
Pathname: /CAP1B/OVERCHUTE/FLOW/03APR1997/5MIN/100YEAR/
Number of actual data: 288 Header length: 0
Compression: 0 Quality: 0
-----DSS--- ZREAD Unit 71; Vers. 4: /CAP1B/OVERCHUTE/FLOW/03APR1997/5MIN/100YEAR/
-----DSS---Debug: Enter ZRRTSB; Unit: 71
NSTART: 1 NVALS: 744 JULS: 35522 IOSTIME: 835
NLDATA: 288 JULSD: 35522
JULS: 03APR97 JULSD: 03APR97
Quality Read: F, Quality Requested: F
---ZRRTSB Calculations: NPOS: 167 NDATA: 288 NREAD: 122 ILIM: 122
After ZRDINF, Record found: T
Pathname: /CAP1B/OVERCHUTE/FLOW/04APR1997/5MIN/100YEAR/
Number of actual data: 288 Header length: 0
Compression: 0 Quality: 0
-----DSS--- ZREAD Unit 71; Vers. 4: /CAP1B/OVERCHUTE/FLOW/04APR1997/5MIN/100YEAR/
-----DSS---Debug: Enter ZRRTSB; Unit: 71
NSTART: 123 NVALS: 744 JULS: 35522 IOSTIME: 835
NLDATA: 288 JULSD: 35523
JULS: 03APR97 JULSD: 04APR97
Quality Read: F, Quality Requested: F
---ZRRTSB Calculations: NPOS: 1 NDATA: 288 NREAD: 288 ILIM: 410
After ZRDINF, Record found: F
Pathname: /CAP1B/OVERCHUTE/FLOW/05APR1997/5MIN/100YEAR/
-----DSS---Debug: Enter ZRRTSB; Unit: 71
NSTART: 411 NVALS: 744 JULS: 35522 IOSTIME: 835
NLDATA: 288 JULSD: 35524
JULS: 03APR97 JULSD: 05APR97
Quality Read: F, Quality Requested: F
---ZRRTSB Calculations: NPOS: 1 NDATA: 288 NREAD: 288 ILIM: 698
-----DSS*** ZRRTS: CAUTION - Data block not found in file. Unit: 71
Pathname: /CAP1B/OVERCHUTE/FLOW/05APR1997/5MIN/100YEAR/
After ZRDINF, Record found: F
Pathname: /CAP1B/OVERCHUTE/FLOW/06APR1997/5MIN/100YEAR/
-----DSS---Debug: Enter ZRRTSB; Unit: 71
NSTART: 699 NVALS: 744 JULS: 35522 IOSTIME: 835
NLDATA: 288 JULSD: 35525
JULS: 03APR97 JULSD: 06APR97
Quality Read: F, Quality Requested: F
---ZRRTSB Calculations: NPOS: 1 NDATA: 288 NREAD: 46 ILIM: 744
----- Exiting ZRRTS, Number of data values: 744, Status: 3
Offset: 0, Units: CFS, Type:INST-VAL
----- Entering ZRRTSX for unit 71 -----
Pathname: /ADOT EAST BASIN/AT SUPERSTITION/FLOW//5MIN/100YR/
Time Window set. Interval: 5 Number of data values: 1
Starting date and time: Mar 31, 1997 2400 ( 35519 1440)
Ending date and time: Mar 31, 1997 2400 ( 35519 1440)
Input time offset: 0
After ZRDINF, Record found: T
Pathname: /ADOT EAST BASIN/AT SUPERSTITION/FLOW/31MAR1997/5MIN/100YR/
Number of actual data: 288 Header length: 0
```

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Compression: 0 Quality: 0
-----DSS--- ZREAD Unit 71; Vers. 2: /ADOT EAST BASIN/AT SUPERSTITION/FLOW/31MAR1997/5MIN/100YR/
-----DSS---Debug: Enter ZRRTSB; Unit: 71
NSTART: 1 NVALS: 1 JULS: 35519 ISTE: 1440
NLDATA: 288 JULSD: 35519
JULS: 31MAR97 JULSD: 31MAR97
Quality Read: F, Quality Requested: F
---ZRRTSB Calculations: NPOS: 288 NDATA: 288 NREAD: 1 ILIM: 1
----- Exiting ZRRTS, Number of data values: 1, Status: 0
Offset: 0, Units: CFS, Type:INST-VAL
----- Entering ZRRTSX for unit 71 -----
Pathname: /ADOT EAST BASIN/AT SUPERSTITION/FLOW//5MIN/100YR/
Time Window set. Interval: 5 Number of data values: 744
Starting date and time: Mar 31, 1997 2400 ( 35519 1440)
Ending date and time: Apr 3, 1997 1355 ( 35522 835)
Input time offset: 0
After ZRDINF, Record found: T
Pathname: /ADOT EAST BASIN/AT SUPERSTITION/FLOW/31MAR1997/5MIN/100YR/
Number of actual data: 288 Header length: 0
Compression: 0 Quality: 0
-----DSS--- ZREAD Unit 71; Vers. 2: /ADOT EAST BASIN/AT SUPERSTITION/FLOW/31MAR1997/5MIN/100YR/
-----DSS---Debug: Enter ZRRTSB; Unit: 71
NSTART: 1 NVALS: 744 JULS: 35519 ISTE: 1440
NLDATA: 288 JULSD: 35519
JULS: 31MAR97 JULSD: 31MAR97
Quality Read: F, Quality Requested: F
---ZRRTSB Calculations: NPOS: 288 NDATA: 288 NREAD: 1 ILIM: 1
After ZRDINF, Record found: T
Pathname: /ADOT EAST BASIN/AT SUPERSTITION/FLOW/01APR1997/5MIN/100YR/
Number of actual data: 288 Header length: 0
Compression: 0 Quality: 0
-----DSS--- ZREAD Unit 71; Vers. 2: /ADOT EAST BASIN/AT SUPERSTITION/FLOW/01APR1997/5MIN/100YR/
-----DSS---Debug: Enter ZRRTSB; Unit: 71
NSTART: 2 NVALS: 744 JULS: 35519 ISTE: 1440
NLDATA: 288 JULSD: 35520
JULS: 31MAR97 JULSD: 01APR97
Quality Read: F, Quality Requested: F
---ZRRTSB Calculations: NPOS: 1 NDATA: 288 NREAD: 288 ILIM: 289
After ZRDINF, Record found: T
Pathname: /ADOT EAST BASIN/AT SUPERSTITION/FLOW/02APR1997/5MIN/100YR/
Number of actual data: 288 Header length: 0
Compression: 0 Quality: 0
-----DSS--- ZREAD Unit 71; Vers. 2: /ADOT EAST BASIN/AT SUPERSTITION/FLOW/02APR1997/5MIN/100YR/
-----DSS---Debug: Enter ZRRTSB; Unit: 71
NSTART: 290 NVALS: 744 JULS: 35519 ISTE: 1440
NLDATA: 288 JULSD: 35521
JULS: 31MAR97 JULSD: 02APR97
Quality Read: F, Quality Requested: F
---ZRRTSB Calculations: NPOS: 1 NDATA: 288 NREAD: 288 ILIM: 577
After ZRDINF, Record found: T
Pathname: /ADOT EAST BASIN/AT SUPERSTITION/FLOW/03APR1997/5MIN/100YR/
Number of actual data: 288 Header length: 0
Compression: 0 Quality: 0
-----DSS--- ZREAD Unit 71; Vers. 2: /ADOT EAST BASIN/AT SUPERSTITION/FLOW/03APR1997/5MIN/100YR/
-----DSS---Debug: Enter ZRRTSB; Unit: 71
NSTART: 578 NVALS: 744 JULS: 35519 ISTE: 1440
NLDATA: 288 JULSD: 35522
JULS: 31MAR97 JULSD: 03APR97
Quality Read: F, Quality Requested: F
---ZRRTSB Calculations: NPOS: 1 NDATA: 288 NREAD: 167 ILIM: 744
----- Exiting ZRRTS, Number of data values: 744, Status: 0
Offset: 0, Units: CFS, Type:INST-VAL
----- Entering ZRRTSX for unit 71 -----
Pathname: /ADOT EAST BASIN/AT SUPERSTITION/FLOW//5MIN/100YR/
Time Window set. Interval: 5 Number of data values: 1
Starting date and time: Apr 3, 1997 1355 ( 35522 835)
Ending date and time: Apr 3, 1997 1355 ( 35522 835)
Input time offset: 0
After ZRDINF, Record found: T
Pathname: /ADOT EAST BASIN/AT SUPERSTITION/FLOW/03APR1997/5MIN/100YR/
Number of actual data: 288 Header length: 0
Compression: 0 Quality: 0
-----DSS--- ZREAD Unit 71; Vers. 2: /ADOT EAST BASIN/AT SUPERSTITION/FLOW/03APR1997/5MIN/100YR/
-----DSS---Debug: Enter ZRRTSB; Unit: 71
NSTART: 1 NVALS: 1 JULS: 35522 ISTE: 835
NLDATA: 288 JULSD: 35522
JULS: 03APR97 JULSD: 03APR97
Quality Read: F, Quality Requested: F
---ZRRTSB Calculations: NPOS: 167 NDATA: 288 NREAD: 1 ILIM: 1
----- Exiting ZRRTS, Number of data values: 1, Status: 0
Offset: 0, Units: CFS, Type:INST-VAL
----- Entering ZRRTSX for unit 71 -----
Pathname: /ADOT EAST BASIN/AT SUPERSTITION/FLOW//5MIN/100YR/
Time Window set. Interval: 5 Number of data values: 744
Starting date and time: Apr 3, 1997 1355 ( 35522 835)
Ending date and time: Apr 6, 1997 0350 ( 35525 230)
Input time offset: 0
```

```
After ZRDINF, Record found: T
Pathname: /ADOT EAST BASIN/AT SUPERSTITION/FLOW/03APR1997/5MIN/100YR/
Number of actual data: 288 Header length: 0
Compression: 0 Quality: 0
-----DSS--- ZREAD Unit 71; Vers. 2: /ADOT EAST BASIN/AT SUPERSTITION/FLOW/03APR1997/5MIN/100YR/
-----DSS---Debug: Enter ZRRTSB; Unit: 71
NSTART: 1 NVALS: 744 JULS: 35522 ISTE: 835
NLDATA: 288 JULSD: 35522
JULS: 03APR97 JULSD: 03APR97
Quality Read: F, Quality Requested: F
---ZRRTSB Calculations: NPOS: 167 NDATA: 288 NREAD: 122 ILIM: 122
After ZRDINF, Record found: T
Pathname: /ADOT EAST BASIN/AT SUPERSTITION/FLOW/04APR1997/5MIN/100YR/
Number of actual data: 288 Header length: 0
Compression: 0 Quality: 0
-----DSS--- ZREAD Unit 71; Vers. 2: /ADOT EAST BASIN/AT SUPERSTITION/FLOW/04APR1997/5MIN/100YR/
-----DSS---Debug: Enter ZRRTSB; Unit: 71
NSTART: 123 NVALS: 744 JULS: 35522 ISTE: 835
NLDATA: 288 JULSD: 35523
JULS: 03APR97 JULSD: 04APR97
Quality Read: F, Quality Requested: F
---ZRRTSB Calculations: NPOS: 1 NDATA: 288 NREAD: 288 ILIM: 410
After ZRDINF, Record found: F
Pathname: /ADOT EAST BASIN/AT SUPERSTITION/FLOW/05APR1997/5MIN/100YR/
-----DSS---Debug: Enter ZRRTSB; Unit: 71
NSTART: 411 NVALS: 744 JULS: 35522 ISTE: 835
NLDATA: 288 JULSD: 35524
JULS: 03APR97 JULSD: 05APR97
Quality Read: F, Quality Requested: F
---ZRRTSB Calculations: NPOS: 1 NDATA: 288 NREAD: 288 ILIM: 698
-----DSS*** ZRRTS: CAUTION - Data block not found in file. Unit: 71
Pathname: /ADOT EAST BASIN/AT SUPERSTITION/FLOW/05APR1997/5MIN/100YR/
After ZRDINF, Record found: F
Pathname: /ADOT EAST BASIN/AT SUPERSTITION/FLOW/06APR1997/5MIN/100YR/
-----DSS---Debug: Enter ZRRTSB; Unit: 71
NSTART: 699 NVALS: 744 JULS: 35522 ISTE: 835
NLDATA: 288 JULSD: 35525
JULS: 03APR97 JULSD: 06APR97
Quality Read: F, Quality Requested: F
---ZRRTSB Calculations: NPOS: 1 NDATA: 288 NREAD: 46 ILIM: 744
----- Exiting ZRRTS, Number of data values: 744, Status: 3
Offset: 0, Units: CFS, Type: INST-VAL
----- Entering ZRRTSX for unit 71 -----
Pathname: /ADOT WEST BASIN/AT SUPERSTITION/FLOW//5MIN/100YR/
Time Window set. Interval: 5 Number of data values: 1
Starting date and time: Mar 31, 1997 2400 ( 35519 1440)
Ending date and time: Mar 31, 1997 2400 ( 35519 1440)
Input time offset: 0
After ZRDINF, Record found: T
Pathname: /ADOT WEST BASIN/AT SUPERSTITION/FLOW/31MAR1997/5MIN/100YR/
Number of actual data: 288 Header length: 0
Compression: 0 Quality: 0
-----DSS--- ZREAD Unit 71; Vers. 2: /ADOT WEST BASIN/AT SUPERSTITION/FLOW/31MAR1997/5MIN/100YR/
-----DSS---Debug: Enter ZRRTSB; Unit: 71
NSTART: 1 NVALS: 1 JULS: 35519 ISTE: 1440
NLDATA: 288 JULSD: 35519
JULS: 31MAR97 JULSD: 31MAR97
Quality Read: F, Quality Requested: F
---ZRRTSB Calculations: NPOS: 288 NDATA: 288 NREAD: 1 ILIM: 1
----- Exiting ZRRTS, Number of data values: 1, Status: 0
Offset: 0, Units: CFS, Type: INST-VAL
----- Entering ZRRTSX for unit 71 -----
Pathname: /ADOT WEST BASIN/AT SUPERSTITION/FLOW//5MIN/100YR/
Time Window set. Interval: 5 Number of data values: 744
Starting date and time: Mar 31, 1997 2400 ( 35519 1440)
Ending date and time: Apr 3, 1997 1355 ( 35522 835)
Input time offset: 0
After ZRDINF, Record found: T
Pathname: /ADOT WEST BASIN/AT SUPERSTITION/FLOW/31MAR1997/5MIN/100YR/
Number of actual data: 288 Header length: 0
Compression: 0 Quality: 0
-----DSS--- ZREAD Unit 71; Vers. 2: /ADOT WEST BASIN/AT SUPERSTITION/FLOW/31MAR1997/5MIN/100YR/
-----DSS---Debug: Enter ZRRTSB; Unit: 71
NSTART: 1 NVALS: 744 JULS: 35519 ISTE: 1440
NLDATA: 288 JULSD: 35519
JULS: 31MAR97 JULSD: 31MAR97
Quality Read: F, Quality Requested: F
---ZRRTSB Calculations: NPOS: 288 NDATA: 288 NREAD: 1 ILIM: 1
After ZRDINF, Record found: T
Pathname: /ADOT WEST BASIN/AT SUPERSTITION/FLOW/01APR1997/5MIN/100YR/
Number of actual data: 288 Header length: 0
Compression: 0 Quality: 0
-----DSS--- ZREAD Unit 71; Vers. 2: /ADOT WEST BASIN/AT SUPERSTITION/FLOW/01APR1997/5MIN/100YR/
-----DSS---Debug: Enter ZRRTSB; Unit: 71
NSTART: 2 NVALS: 744 JULS: 35519 ISTE: 1440
NLDATA: 288 JULSD: 35520
JULS: 31MAR97 JULSD: 01APR97
```

Quality Read: F, Quality Requested: F
---ZRRTSB Calculations: NPOS: 1 NDATA: 288 NREAD: 288 ILIM: 289
After ZRDINF, Record found: T
Pathname: /ADOT WEST BASIN/AT SUPERSTITION/FLOW/02APR1997/5MIN/100YR/
Number of actual data: 288 Header length: 0
Compression: 0 Quality: 0
-----DSS--- ZREAD Unit 71; Vers. 2: /ADOT WEST BASIN/AT SUPERSTITION/FLOW/02APR1997/5MIN/100YR/
-----DSS---Debug: Enter ZRRTSB; Unit: 71
NSTART: 290 NVALS: 744 JULS: 35519 ISTE: 1440
NLDATA: 288 JULSD: 35521
JULS: 31MAR97 JULSD: 02APR97
Quality Read: F, Quality Requested: F
---ZRRTSB Calculations: NPOS: 1 NDATA: 288 NREAD: 288 ILIM: 577
After ZRDINF, Record found: T
Pathname: /ADOT WEST BASIN/AT SUPERSTITION/FLOW/03APR1997/5MIN/100YR/
Number of actual data: 288 Header length: 0
Compression: 0 Quality: 0
-----DSS--- ZREAD Unit 71; Vers. 2: /ADOT WEST BASIN/AT SUPERSTITION/FLOW/03APR1997/5MIN/100YR/
-----DSS---Debug: Enter ZRRTSB; Unit: 71
NSTART: 578 NVALS: 744 JULS: 35519 ISTE: 1440
NLDATA: 288 JULSD: 35522
JULS: 31MAR97 JULSD: 03APR97
Quality Read: F, Quality Requested: F
---ZRRTSB Calculations: NPOS: 1 NDATA: 288 NREAD: 167 ILIM: 744
----- Exiting ZRRTS, Number of data values: 744, Status: 0
Offset: 0, Units: CFS, Type: INST-VAL
----- Entering ZRRTSX for unit 71 -----
Pathname: /ADOT WEST BASIN/AT SUPERSTITION/FLOW//5MIN/100YR/
Time Window set. Interval: 5 Number of data values: 1
Starting date and time: Apr 3, 1997 1355 (35522 835)
Ending date and time: Apr 3, 1997 1355 (35522 835)
Input time offset: 0
After ZRDINF, Record found: T
Pathname: /ADOT WEST BASIN/AT SUPERSTITION/FLOW/03APR1997/5MIN/100YR/
Number of actual data: 288 Header length: 0
Compression: 0 Quality: 0
-----DSS--- ZREAD Unit 71; Vers. 2: /ADOT WEST BASIN/AT SUPERSTITION/FLOW/03APR1997/5MIN/100YR/
-----DSS---Debug: Enter ZRRTSB; Unit: 71
NSTART: 1 NVALS: 1 JULS: 35522 ISTE: 835
NLDATA: 288 JULSD: 35522
JULS: 03APR97 JULSD: 03APR97
Quality Read: F, Quality Requested: F
---ZRRTSB Calculations: NPOS: 167 NDATA: 288 NREAD: 1 ILIM: 1
----- Exiting ZRRTS, Number of data values: 1, Status: 0
Offset: 0, Units: CFS, Type: INST-VAL
----- Entering ZRRTSX for unit 71 -----
Pathname: /ADOT WEST BASIN/AT SUPERSTITION/FLOW//5MIN/100YR/
Time Window set. Interval: 5 Number of data values: 744
Starting date and time: Apr 3, 1997 1355 (35522 835)
Ending date and time: Apr 6, 1997 0350 (35525 230)
Input time offset: 0
After ZRDINF, Record found: T
Pathname: /ADOT WEST BASIN/AT SUPERSTITION/FLOW/03APR1997/5MIN/100YR/
Number of actual data: 288 Header length: 0
Compression: 0 Quality: 0
-----DSS--- ZREAD Unit 71; Vers. 2: /ADOT WEST BASIN/AT SUPERSTITION/FLOW/03APR1997/5MIN/100YR/
-----DSS---Debug: Enter ZRRTSB; Unit: 71
NSTART: 1 NVALS: 744 JULS: 35522 ISTE: 835
NLDATA: 288 JULSD: 35522
JULS: 03APR97 JULSD: 03APR97
Quality Read: F, Quality Requested: F
---ZRRTSB Calculations: NPOS: 167 NDATA: 288 NREAD: 122 ILIM: 122
After ZRDINF, Record found: T
Pathname: /ADOT WEST BASIN/AT SUPERSTITION/FLOW/04APR1997/5MIN/100YR/
Number of actual data: 288 Header length: 0
Compression: 0 Quality: 0
-----DSS--- ZREAD Unit 71; Vers. 2: /ADOT WEST BASIN/AT SUPERSTITION/FLOW/04APR1997/5MIN/100YR/
-----DSS---Debug: Enter ZRRTSB; Unit: 71
NSTART: 123 NVALS: 744 JULS: 35522 ISTE: 835
NLDATA: 288 JULSD: 35523
JULS: 03APR97 JULSD: 04APR97
Quality Read: F, Quality Requested: F
---ZRRTSB Calculations: NPOS: 1 NDATA: 288 NREAD: 288 ILIM: 410
After ZRDINF, Record found: F
Pathname: /ADOT WEST BASIN/AT SUPERSTITION/FLOW/05APR1997/5MIN/100YR/
-----DSS---Debug: Enter ZRRTSB; Unit: 71
NSTART: 411 NVALS: 744 JULS: 35522 ISTE: 835
NLDATA: 288 JULSD: 35524
JULS: 03APR97 JULSD: 05APR97
Quality Read: F, Quality Requested: F
---ZRRTSB Calculations: NPOS: 1 NDATA: 288 NREAD: 288 ILIM: 698
-----DSS*** ZRRTS: CAUTION - Data block not found in file. Unit: 71
Pathname: /ADOT WEST BASIN/AT SUPERSTITION/FLOW/05APR1997/5MIN/100YR/
After ZRDINF, Record found: F
Pathname: /ADOT WEST BASIN/AT SUPERSTITION/FLOW/06APR1997/5MIN/100YR/
-----DSS---Debug: Enter ZRRTSB; Unit: 71
NSTART: 699 NVALS: 744 JULS: 35522 ISTE: 835

HYDROGRAPH AT	72	1370.	12.17	195.	67.	22.	.84
DIVERSION TO	D72	1370.	12.17	147.	42.	14.	.84
HYDROGRAPH AT	R72	667.	12.42	80.	25.	8.	.84
2 COMBINED AT	CPWAR	924.	12.50	289.	107.	36.	4.13
2 COMBINED AT	EMFWAR	1599.	14.08	843.	346.	130.	20.94
ROUTED TO	WARTKN	1534.	14.25	840.	345.	129.	20.94
HYDROGRAPH AT	70B	628.	12.17	89.	30.	10.	.38
DIVERSION TO	D70B	628.	12.17	68.	19.	6.	.38
HYDROGRAPH AT	R70B	249.	12.50	35.	11.	4.	.38
ROUTED TO	70BT76	79.	14.00	34.	11.	4.	.38
HYDROGRAPH AT	76B	1119.	12.17	153.	52.	17.	.64
DIVERSION TO	D76B	1119.	12.17	118.	33.	11.	.64
HYDROGRAPH AT	R76B	457.	12.42	59.	18.	6.	.64
2 COMBINED AT	KNOX	447.	12.42	91.	29.	10.	1.02
2 COMBINED AT	EMFKNX	1634.	14.25	890.	369.	137.	21.96
HYDROGRAPH AT	65A	1235.	13.00	244.	62.	21.	2.54
DIVERSION TO	65A1	457.	13.00	90.	23.	8.	2.54
HYDROGRAPH AT	REMA1	778.	13.00	154.	39.	13.	2.54
DIVERSION TO	65A2a	223.	13.00	44.	11.	4.	2.54
HYDROGRAPH AT	REMA2	556.	13.00	110.	28.	9.	2.54
DIVERSION TO	65A2b	87.	13.00	17.	4.	1.	2.54
HYDROGRAPH AT	REMA3	469.	13.00	93.	23.	8.	2.54
DIVERSION TO	65A2c	62.	13.00	12.	3.	1.	2.54
HYDROGRAPH AT	65A3	407.	13.00	80.	20.	7.	2.54
HYDROGRAPH AT	CAP1A	710.	12.67	132.	36.	12.	.96
ROUTED TO	RCAP1A	625.	13.33	132.	36.	12.	.96
ROUTED TO	RRCP1A	618.	13.33	132.	36.	12.	.96
2 COMBINED AT	C65A1	959.	13.00	213.	56.	19.	1.81
ROUTED TO	RCHL1	958.	13.08	213.	56.	19.	1.81

DIVERSION TO	D66D	633.	12.08	55.	16.	5.	.31
HYDROGRAPH AT	R66D	283.	12.25	30.	9.	3.	.31
HYDROGRAPH AT	61A	764.	12.17	92.	30.	10.	.52
DIVERSION TO	D61A	764.	12.17	76.	21.	7.	.52
HYDROGRAPH AT	R61A	150.	12.42	28.	9.	3.	.52
ROUTED TO	61ATB	80.	13.33	27.	9.	3.	.52
HYDROGRAPH AT	61B	1122.	12.42	168.	52.	17.	1.09
DIVERSION TO	D61B	1122.	12.42	150.	41.	14.	1.09
HYDROGRAPH AT	R61B	156.	13.08	36.	12.	4.	1.09
2 COMBINED AT	CP61B	204.	13.08	60.	20.	7.	1.61
ROUTED TO	61T66D	155.	13.83	58.	20.	7.	1.61
HYDROGRAPH AT	67E	743.	12.33	107.	35.	12.	.58
DIVERSION TO	D67E	743.	12.33	91.	25.	8.	.58
HYDROGRAPH AT	R67E	140.	12.83	30.	10.	3.	.58
2 COMBINED AT	C67E	185.	13.83	82.	29.	10.	2.19
3 COMBINED AT	C66D	957.	14.25	774.	456.	167.	18.55
ROUTED TO	66T70A	954.	14.33	773.	456.	167.	18.55
ROUTED TO	66T70B	954.	14.33	772.	456.	167.	18.55
ROUTED TO	CULVT	947.	14.33	769.	456.	167.	18.55
ROUTED TO	66T70C	944.	14.58	767.	455.	167.	18.55
HYDROGRAPH AT	62A	782.	12.00	86.	30.	10.	.38
DIVERSION TO	D62A	782.	12.00	58.	17.	6.	.38
HYDROGRAPH AT	R62A	509.	12.17	43.	13.	4.	.38
ROUTED TO	62ATC	345.	12.33	42.	13.	4.	.38
HYDROGRAPH AT	62C	803.	12.17	95.	31.	10.	.55
DIVERSION TO	D62C	724.	12.08	55.	16.	5.	.55
HYDROGRAPH AT	R62C	763.	12.25	52.	15.	5.	.55
2 COMBINED AT	C62C	1045.	12.25	93.	28.	9.	.93
ROUTED TO	62CTE	649.	12.50	91.	28.	9.	.93

+	HYDROGRAPH AT	62E	223.	12.17	27.	9.	3.	.15
+	DIVERSION TO	D62E	223.	12.17	22.	6.	2.	.15
+	HYDROGRAPH AT	R62E	50.	12.50	8.	2.	1.	.15
+	2 COMBINED AT	CP62E	678.	12.50	98.	30.	10.	1.08
+	ROUTED TO	62T68A	363.	12.92	94.	30.	10.	1.08
+	HYDROGRAPH AT	68A	675.	12.08	74.	25.	8.	.35
+	DIVERSION TO	D68A	675.	12.08	55.	16.	5.	.35
+	HYDROGRAPH AT	R68A	309.	12.25	30.	9.	3.	.35
+	2 COMBINED AT	CP68A	404.	12.92	120.	38.	13.	1.43
+	ROUTED TO	68T70A	350.	13.17	117.	38.	13.	1.43
+	2 COMBINED AT	OUTLET	1064.	13.42	858.	488.	178.	19.98
+	ROUTED TO	R-OUT	1063.	14.42	857.	488.	178.	19.98
+	HYDROGRAPH AT	70A	1107.	12.08	120.	40.	13.	.54
+	DIVERSION TO	D70A	1107.	12.08	94.	26.	9.	.54
+	HYDROGRAPH AT	R70A	353.	12.25	43.	13.	4.	.54
+	2 COMBINED AT	C70A	1099.	13.50	877.	498.	181.	20.52
+	ROUTED TO	70T76A	1081.	14.67	870.	497.	181.	20.52
+	HYDROGRAPH AT	76A	2065.	12.58	411.	131.	44.	1.91
+	DIVERSION TO	D76A	2065.	12.58	338.	93.	31.	1.91
+	HYDROGRAPH AT	R76A	751.	13.08	123.	38.	13.	1.91
+	2 COMBINED AT	C76A	1235.	13.83	954.	527.	191.	22.43
+	ROUTED TO	76ATPR	1175.	14.42	938.	526.	191.	22.43
+	2 COMBINED AT	EMFSTN	2575.	14.33	1698.	845.	311.	44.39
+	ROUTED TO	KNXTRY	2510.	14.58	1688.	843.	311.	44.39
+	HYDROGRAPH AT	73A	355.	13.33	90.	23.	8.	.95
+	ROUTED TO	73ATB	286.	13.83	90.	23.	8.	.95
+	HYDROGRAPH AT	73B	562.	12.17	59.	17.	6.	.42
+	DIVERSION TO	D73B	562.	12.17	55.	15.	5.	.42
+	HYDROGRAPH AT	R73B	47.	12.58	7.	2.	1.	.42

2 COMBINED AT	CP73B	294.	13.83	96.	25.	8.	1.37
ROUTED TO	73BTC	293.	14.00	95.	25.	8.	1.37
HYDROGRAPH AT	73C	1353.	12.00	134.	46.	15.	.58
DIVERSION TO	D73C	1353.	12.00	103.	29.	10.	.58
HYDROGRAPH AT	R73C	410.	12.17	54.	17.	6.	.58
2 COMBINED AT	CP73C	405.	12.17	146.	41.	14.	1.95
ROUTED TO	73T74C	322.	14.33	144.	41.	14.	1.95
HYDROGRAPH AT	74A	289.	13.33	72.	18.	6.	.75
ROUTED TO	DB74A	56.	14.42	37.	18.	6.	.75
ROUTED TO	74ATB	47.	15.25	36.	18.	6.	.75
HYDROGRAPH AT	74B	435.	12.25	41.	11.	4.	.33
DIVERSION TO	D74B	435.	12.25	41.	11.	4.	.33
HYDROGRAPH AT	R74B	1.	15.33	0.	0.	0.	.33
2 COMBINED AT	CP74B	47.	15.33	37.	18.	6.	1.08
ROUTED TO	74BTC	46.	15.58	36.	18.	6.	1.08
HYDROGRAPH AT	74C	746.	12.00	82.	28.	9.	.34
DIVERSION TO	D74C	746.	12.00	62.	18.	6.	.34
HYDROGRAPH AT	R74C	335.	12.17	34.	10.	3.	.34
3 COMBINED AT	CP74C	374.	14.33	201.	68.	23.	3.37
ROUTED TO	74CT75	369.	14.50	199.	68.	23.	3.37
HYDROGRAPH AT	75	251.	14.75	114.	30.	10.	4.01
2 COMBINED AT	C75	604.	14.67	307.	96.	32.	7.38
ROUTED TO	75TPC	602.	14.75	306.	96.	32.	7.38
HYDROGRAPH AT	77A	526.	13.75	164.	41.	14.	1.74
ROUTED TO	77ATB	451.	14.17	163.	41.	14.	1.74
HYDROGRAPH AT	77B	504.	12.17	42.	11.	4.	.36
DIVERSION TO	D77B	504.	12.17	32.	8.	3.	.36
HYDROGRAPH AT	R77B	257.	12.33	11.	3.	1.	.36
2 COMBINED AT	CP77B	452.	14.17	173.	44.	15.	2.10

ROUTED TO	77BTC	443.	14.50	172.	44.	15.	2.10
HYDROGRAPH AT	77C	660.	12.00	65.	22.	7.	.28
DIVERSION TO	D77C	660.	12.00	50.	14.	5.	.28
HYDROGRAPH AT	R77C	212.	12.08	26.	8.	3.	.28
2 COMBINED AT	C77C	458.	14.50	195.	51.	17.	2.38
ROUTED TO	77CT78	452.	14.67	194.	51.	17.	2.38
HYDROGRAPH AT	78A	568.	13.75	177.	44.	15.	1.88
ROUTED TO	78ATB	466.	14.42	177.	44.	15.	1.88
HYDROGRAPH AT	78B	610.	12.17	65.	17.	6.	.39
2 COMBINED AT	C78B	610.	12.25	237.	60.	20.	2.27
ROUTED TO	78BTC	503.	12.50	237.	60.	20.	2.27
HYDROGRAPH AT	78C	646.	12.00	69.	22.	7.	.28
DIVERSION TO	D78C	610.	12.00	43.	12.	4.	.28
HYDROGRAPH AT	R78C	512.	12.08	35.	10.	3.	.28
3 COMBINED AT	C78C	907.	14.67	456.	120.	40.	4.93
ROUTED TO	78CT79	899.	14.92	453.	119.	40.	4.93
HYDROGRAPH AT	79A	902.	13.50	266.	67.	22.	2.01
2 COMBINED AT	C79A1	1422.	13.58	708.	184.	61.	6.94
HYDROGRAPH AT	78F	1028.	14.17	384.	97.	32.	4.19
HYDROGRAPH AT	82A1	1069.	13.50	291.	73.	24.	3.12
2 COMBINED AT	C82A1	1637.	13.50	667.	168.	56.	7.31
ROUTED TO	DB82A1	163.	15.92	153.	134.	97.	7.31
ROUTED TO	PS-9	163.	15.92	153.	134.	97.	7.31
HYDROGRAPH AT	CAF2	64.	20.00	64.	64.	62.	.01
ROUTED TO	RCAP2	64.	21.17	64.	64.	61.	.01
HYDROGRAPH AT	82A2	839.	14.75	374.	99.	33.	4.13
2 COMBINED AT	CP82A2	876.	14.75	414.	155.	94.	4.14
HYDROGRAPH AT	82A4	597.	13.83	199.	50.	17.	2.13
ROUTED TO	82A4T3	590.	14.00	198.	50.	17.	2.13

HYDROGRAPH AT	82A3	508.	14.17	188.	48.	16.	2.02
2 COMBINED AT	CP82A3	1057.	14.08	381.	96.	32.	4.15
2 COMBINED AT	CP82A5	1594.	14.08	784.	249.	125.	8.29
ROUTED TO	DB82B	329.	16.83	312.	244.	125.	8.29
ROUTED TO	MN-1	329.	16.92	312.	244.	125.	8.29
2 COMBINED AT	CP82A6	470.	17.00	454.	369.	219.	15.60
ROUTED TO	82TBOX	470.	17.00	454.	369.	219.	15.60
ROUTED TO	BOXCLV	470.	17.08	454.	369.	219.	15.60
ROUTED TO	BOXT78	470.	17.33	454.	369.	218.	15.60
HYDROGRAPH AT	78D	1488.	12.17	186.	60.	20.	.89
DIVERSION TO	D78D	1488.	12.17	154.	42.	14.	.89
HYDROGRAPH AT	R78D	334.	12.50	55.	17.	6.	.89
HYDROGRAPH AT	82B	1496.	12.17	174.	57.	19.	.92
DIVERSION TO	D82	14.	1.08	2.	1.	0.	.92
HYDROGRAPH AT	R82	1496.	12.17	174.	57.	19.	.92
DIVERSION TO	TRW	1496.	12.17	174.	55.	18.	.92
HYDROGRAPH AT	DTTRW	6.	18.92	5.	1.	0.	.92
3 COMBINED AT	C78D	488.	17.33	471.	380.	223.	17.41
ROUTED TO	78DTE	488.	17.67	471.	379.	222.	17.41
HYDROGRAPH AT	78E	799.	12.75	149.	38.	13.	1.01
HYDROGRAPH AT	83	1321.	12.25	190.	62.	21.	1.01
DIVERSION TO	D83	1321.	12.25	150.	42.	14.	1.01
HYDROGRAPH AT	R83	523.	12.58	66.	21.	7.	1.01
3 COMBINED AT	C78E	1178.	12.75	557.	423.	239.	19.43
ROUTED TO	78ET84	966.	13.17	550.	422.	238.	19.43
HYDROGRAPH AT	84	1254.	12.25	185.	61.	20.	.99
DIVERSION TO	D84	1254.	12.25	154.	43.	14.	.99
HYDROGRAPH AT	R84	314.	12.75	57.	18.	6.	.99
2 COMBINED AT	C84	1085.	13.08	593.	436.	243.	20.42

ROUTED TO	84T79B	1059.	13.17	591.	436.	243.	20.42
+							
HYDROGRAPH AT	79B	643.	13.08	152.	38.	13.	1.00
+							
2 COMBINED AT	C79B1	1638.	13.17	704.	467.	254.	21.42
+							
ROUTED TO	79BTB2	1596.	13.25	703.	467.	254.	21.42
+							
2 COMBINED AT	C79B2	2748.	13.50	1322.	627.	307.	28.36
+							
ROUTED TO	79TPC2	2718.	13.50	1318.	627.	307.	28.36
+							
2 COMBINED AT	CPPWR	2918.	13.58	1552.	698.	330.	35.74
+							
ROUTED TO	PWRT80	2899.	13.67	1549.	698.	330.	35.74
+							
HYDROGRAPH AT	80A	2344.	12.83	575.	182.	61.	2.64
+							
DIVERSION TO	D80A	71.	8.83	53.	17.	6.	2.64
+							
HYDROGRAPH AT	R80A	2344.	12.83	575.	166.	55.	2.64
+							
2 COMBINED AT	CP80A	3709.	13.33	1997.	831.	377.	38.38
+							
ROUTED TO	PWRSAN	3700.	13.42	1994.	831.	377.	38.38
+							
2 COMBINED AT	EMFPOW	4662.	14.50	3180.	1576.	652.	82.77
+							
ROUTED TO	PONTWI	4644.	14.67	3173.	1575.	652.	82.77
+							
HYDROGRAPH AT	80B	1150.	12.50	214.	68.	23.	1.12
+							
DIVERSION TO	D80B	15.	4.00	8.	2.	1.	1.12
+							
HYDROGRAPH AT	R80B	1150.	12.50	214.	66.	22.	1.12
+							
HYDROGRAPH AT	81B	1168.	12.25	173.	59.	20.	.84
+							
DIVERSION TO	D81B	444.	11.92	55.	18.	6.	.84
+							
HYDROGRAPH AT	R81B	1168.	12.25	145.	41.	14.	.84
+							
2 COMBINED AT	80B81B	2033.	12.33	350.	106.	35.	1.96
+							
2 COMBINED AT	EMFWIL	4712.	14.67	3333.	1654.	680.	84.73
+							
ROUTED TO	WILTSP	4681.	14.92	3320.	1653.	680.	84.73
+							
HYDROGRAPH AT	SUB258	983.	13.50	444.	131.	44.	3.65
+							
ROUTED TO	RO259	894.	15.17	441.	131.	44.	3.65
+							
HYDROGRAPH AT	SUB260	1097.	12.33	134.	40.	13.	.98
+							
DIVERSION TO	RETDIV	1097.	12.33	129.	35.	12.	.98
+							
HYDROGRAPH AT	RETAIN	42.	12.92	14.	5.	2.	.98

2 COMBINED AT	CO262	906.	15.17	450.	135.	45.	4.63
ROUTED TO	RO263	846.	16.75	448.	135.	45.	4.63
HYDROGRAPH AT	SUB264	1016.	12.42	146.	45.	15.	1.00
DIVERSION TO	RETDIV	1016.	12.42	136.	37.	12.	1.00
HYDROGRAPH AT	RETAIN	79.	12.92	24.	8.	3.	1.00
2 COMBINED AT	CO266	860.	16.75	461.	141.	47.	5.63
ROUTED TO	RO267	761.	20.08	454.	141.	47.	5.63
HYDROGRAPH AT	SUB268	702.	12.75	147.	46.	15.	.97
DIVERSION TO	RETDIV	702.	12.75	126.	34.	11.	.97
HYDROGRAPH AT	RETAIN	191.	13.50	36.	11.	4.	.97
2 COMBINED AT	CO270	770.	20.08	464.	151.	51.	6.60
ROUTED TO	RO283	714.	22.92	458.	151.	51.	6.60
HYDROGRAPH AT	88A	1103.	12.00	115.	40.	13.	.50
DIVERSION TO	D88A	1103.	12.00	89.	25.	8.	.50
HYDROGRAPH AT	R88A	426.	12.17	46.	14.	5.	.50
ROUTED TO	88AT89	103.	12.50	43.	14.	5.	.50
HYDROGRAPH AT	89A	976.	12.08	114.	39.	13.	.50
DIVERSION TO	D89A	976.	12.08	89.	25.	8.	.50
HYDROGRAPH AT	R89A	316.	12.33	45.	14.	5.	.50
2 COMBINED AT	C89A	400.	12.33	86.	28.	9.	1.00
ROUTED TO	89ATRI	197.	12.67	83.	28.	9.	1.00
2 COMBINED AT	C283	734.	22.92	474.	177.	59.	7.60
ROUTED TO	283T90	726.	23.17	471.	177.	59.	7.60
HYDROGRAPH AT	90A	907.	12.08	94.	32.	11.	.48
DIVERSION TO	D90A	907.	12.08	76.	21.	7.	.48
HYDROGRAPH AT	R90A	236.	12.33	33.	10.	3.	.48
2 COMBINED AT	C90A	733.	23.17	475.	186.	63.	8.08
ROUTED TO	90ATB	724.	23.42	473.	186.	63.	8.08
HYDROGRAPH AT	87A	1008.	12.08	112.	39.	13.	.49

DIVERSION TO	D87A	1008.	12.08	87.	25.	8.	.49
HYDROGRAPH AT	R87A	387.	12.25	44.	14.	5.	.49
ROUTED TO	87ATB	117.	12.83	42.	14.	5.	.49
HYDROGRAPH AT	87B	989.	12.08	113.	39.	13.	.49
DIVERSION TO	D87B	989.	12.08	87.	25.	8.	.49
HYDROGRAPH AT	R87B	373.	12.25	45.	14.	5.	.49
2 COMBINED AT	C87	350.	12.25	84.	28.	9.	.98
ROUTED TO	87T88B	200.	12.83	82.	28.	9.	.98
HYDROGRAPH AT	88B	1001.	12.08	114.	39.	13.	.50
DIVERSION TO	D88B	1001.	12.08	89.	25.	8.	.50
HYDROGRAPH AT	R88B	344.	12.25	45.	14.	5.	.50
2 COMBINED AT	C88B	295.	12.25	122.	41.	14.	1.48
ROUTED TO	88T89B	264.	13.08	119.	41.	14.	1.48
HYDROGRAPH AT	89B	1014.	12.08	113.	39.	13.	.50
DIVERSION TO	D89B	1014.	12.08	87.	25.	8.	.50
HYDROGRAPH AT	R89B	415.	12.25	45.	14.	5.	.50
2 COMBINED AT	C89B	407.	12.25	159.	55.	18.	1.98
ROUTED TO	89TB90	311.	13.75	154.	55.	18.	1.98
HYDROGRAPH AT	90B	637.	12.67	122.	37.	12.	.82
DIVERSION TO	D90B	377.	12.33	39.	12.	4.	.82
HYDROGRAPH AT	R90B	637.	12.67	95.	26.	9.	.82
3 COMBINED AT	C90	774.	12.75	509.	260.	87.	10.88
ROUTED TO	90T91	740.	23.83	504.	259.	87.	10.88
HYDROGRAPH AT	85	1270.	12.25	184.	61.	20.	1.00
DIVERSION TO	D85	1270.	12.25	152.	42.	14.	1.00
HYDROGRAPH AT	R85	358.	12.67	58.	19.	6.	1.00
ROUTED TO	85T86	213.	13.00	56.	19.	6.	1.00
HYDROGRAPH AT	86	1284.	12.25	185.	61.	20.	1.00
DIVERSION TO	D86	1284.	12.25	154.	43.	14.	1.00

+	HYDROGRAPH AT	R86	331.	12.67	57.	18.	6.	1.00
	2 COMBINED AT	C86	353.	13.00	110.	36.	12.	2.00
+	ROUTED TO	86T91	329.	13.25	106.	36.	12.	2.00
	HYDROGRAPH AT	91	644.	12.25	86.	28.	9.	.46
+	DIVERSION TO	D91	644.	12.25	69.	19.	6.	.46
	HYDROGRAPH AT	RET91	200.	12.58	30.	9.	3.	.46
+	HYDROGRAPH AT	81A	1594.	12.67	346.	115.	38.	1.81
	DIVERSION TO	D81A	28.	2.67	10.	3.	1.	1.81
+	HYDROGRAPH AT	R81A	1594.	12.67	346.	112.	37.	1.81
	4 COMBINED AT	CP91	1815.	12.67	718.	386.	134.	15.15
+	ROUTED TO	91TEMF	1797.	12.75	717.	386.	134.	15.15
	2 COMBINED AT	EMFRIT	5018.	14.92	3751.	1930.	774.	99.88

SUMMARY OF KINEMATIC WAVE - MUSKINGUM-CUNGE ROUTING
(FLOW IS DIRECT RUNOFF WITHOUT BASE FLOW)

ISTAQ	ELEMENT	DT (MIN)	PEAK (CFS)	TIME TO PEAK (MIN)	VOLUME (IN)	DT (MIN)	INTERPOLATED TO COMPUTATION INTERVAL		VOLUME (IN)
							PEAK (CFS)	TIME TO PEAK (MIN)	
FOR STORM = 1	STORM AREA (SQ MI) =			.01					
PIPE_F	MANE	.75	80.00	741.75	.07	5.00	80.00	745.00	.07
CONTINUITY SUMMARY (AC-FT) - INFLOW= .1668E+02 EXCESS= .0000E+00 OUTFLOW= .1668E+02 BASIN STORAGE= .1312E-12 PERCENT ERROR= .0									
FOR STORM = 2	STORM AREA (SQ MI) =			10.00					
PIPE_F	MANE	.63	80.00	741.47	.06	5.00	80.00	745.00	.06
CONTINUITY SUMMARY (AC-FT) - INFLOW= .1635E+02 EXCESS= .0000E+00 OUTFLOW= .1635E+02 BASIN STORAGE= .1315E-12 PERCENT ERROR= .0									
FOR STORM = 3	STORM AREA (SQ MI) =			30.00					
PIPE_F	MANE	.63	80.00	741.47	.06	5.00	80.00	745.00	.06
CONTINUITY SUMMARY (AC-FT) - INFLOW= .1621E+02 EXCESS= .0000E+00 OUTFLOW= .1622E+02 BASIN STORAGE= .1309E-12 PERCENT ERROR= .0									
FOR STORM = 4	STORM AREA (SQ MI) =			60.00					
PIPE_F	MANE	.63	80.00	746.54	.06	5.00	80.00	750.00	.06
CONTINUITY SUMMARY (AC-FT) - INFLOW= .1610E+02 EXCESS= .0000E+00 OUTFLOW= .1610E+02 BASIN STORAGE= .1346E-12 PERCENT ERROR= .0									
FOR STORM = 5	STORM AREA (SQ MI) =			90.00					
PIPE_F	MANE	.65	80.00	746.85	.06	5.00	80.00	750.00	.06
CONTINUITY SUMMARY (AC-FT) - INFLOW= .1597E+02 EXCESS= .0000E+00 OUTFLOW= .1597E+02 BASIN STORAGE= .1324E-12 PERCENT ERROR= .0									
FOR STORM = 6	STORM AREA (SQ MI) =			120.00					
PIPE_F	MANE	.67	80.00	746.57	.06	5.00	80.00	750.00	.06
CONTINUITY SUMMARY (AC-FT) - INFLOW= .1583E+02 EXCESS= .0000E+00 OUTFLOW= .1583E+02 BASIN STORAGE= .1328E-12 PERCENT ERROR= .0									
FOR STORM = 7	STORM AREA (SQ MI) =			150.00					
PIPE_F	MANE	.69	80.00	746.88	.06	5.00	80.00	750.00	.06
CONTINUITY SUMMARY (AC-FT) - INFLOW= .1574E+02 EXCESS= .0000E+00 OUTFLOW= .1574E+02 BASIN STORAGE= .1330E-12 PERCENT ERROR= .0									
FOR STORM = 1	STORM AREA (SQ MI) =			.01					
RSDW2	MANE	5.00	152.68	875.00	.39	5.00	152.68	875.00	.39
CONTINUITY SUMMARY (AC-FT) - INFLOW= .9926E+02 EXCESS= .0000E+00 OUTFLOW= .9926E+02 BASIN STORAGE= .8423E-03 PERCENT ERROR= .0									
FOR STORM = 2	STORM AREA (SQ MI) =			10.00					
RSDW2	MANE	5.00	148.86	875.00	.35	5.00	148.86	875.00	.35
CONTINUITY SUMMARY (AC-FT) - INFLOW= .8950E+02 EXCESS= .0000E+00 OUTFLOW= .8951E+02 BASIN STORAGE= .8437E-03 PERCENT ERROR= .0									
FOR STORM = 3	STORM AREA (SQ MI) =			30.00					
RSDW2	MANE	5.00	146.22	875.00	.33	5.00	146.22	875.00	.33
CONTINUITY SUMMARY (AC-FT) - INFLOW= .8321E+02 EXCESS= .0000E+00 OUTFLOW= .8322E+02 BASIN STORAGE= .8372E-03 PERCENT ERROR= .0									
FOR STORM = 4	STORM AREA (SQ MI) =			60.00					
RSDW2	MANE	5.00	143.56	875.00	.30	5.00	143.56	875.00	.30
CONTINUITY SUMMARY (AC-FT) - INFLOW= .7686E+02 EXCESS= .0000E+00 OUTFLOW= .7687E+02 BASIN STORAGE= .8406E-03 PERCENT ERROR= .0									

FOR STORM = 5 STORM AREA (SQ MI) = 90.00
RSDW2 MANE 5.00 142.64 870.00 .29 5.00 142.64 870.00 .29

CONTINUITY SUMMARY (AC-FT) - INFLOW= .7458E+02 EXCESS= .0000E+00 OUTFLOW= .7459E+02 BASIN STORAGE= .8218E-03 PERCENT ERROR= .0

FOR STORM = 6 STORM AREA (SQ MI) = 120.00
RSDW2 MANE 5.00 141.62 870.00 .28 5.00 141.62 870.00 .28

CONTINUITY SUMMARY (AC-FT) - INFLOW= .7228E+02 EXCESS= .0000E+00 OUTFLOW= .7229E+02 BASIN STORAGE= .8345E-03 PERCENT ERROR= .0

FOR STORM = 7 STORM AREA (SQ MI) = 150.00
RSDW2 MANE 5.00 140.95 870.00 .28 5.00 140.95 870.00 .28

CONTINUITY SUMMARY (AC-FT) - INFLOW= .7089E+02 EXCESS= .0000E+00 OUTFLOW= .7090E+02 BASIN STORAGE= .8418E-03 PERCENT ERROR= .0

FOR STORM = 1 STORM AREA (SQ MI) = .01
65AT-3 MANE 1.33 498.85 790.67 .65 5.00 498.60 790.00 .65

CONTINUITY SUMMARY (AC-FT) - INFLOW= .3237E+03 EXCESS= .0000E+00 OUTFLOW= .3237E+03 BASIN STORAGE= .1732E-03 PERCENT ERROR= .0

FOR STORM = 2 STORM AREA (SQ MI) = 10.00
65AT-3 MANE 1.33 492.52 790.21 .60 5.00 492.51 790.00 .60

CONTINUITY SUMMARY (AC-FT) - INFLOW= .3003E+03 EXCESS= .0000E+00 OUTFLOW= .3003E+03 BASIN STORAGE= .1738E-03 PERCENT ERROR= .0

FOR STORM = 3 STORM AREA (SQ MI) = 30.00
65AT-3 MANE 1.33 488.54 786.08 .57 5.00 487.93 785.00 .57

CONTINUITY SUMMARY (AC-FT) - INFLOW= .2852E+03 EXCESS= .0000E+00 OUTFLOW= .2852E+03 BASIN STORAGE= .1734E-03 PERCENT ERROR= .0

FOR STORM = 4 STORM AREA (SQ MI) = 60.00
65AT-3 MANE 1.33 485.03 785.68 .54 5.00 484.50 785.00 .54

CONTINUITY SUMMARY (AC-FT) - INFLOW= .2696E+03 EXCESS= .0000E+00 OUTFLOW= .2696E+03 BASIN STORAGE= .1731E-03 PERCENT ERROR= .0

FOR STORM = 5 STORM AREA (SQ MI) = 90.00
65AT-3 MANE 1.33 483.64 786.14 .53 5.00 483.31 785.00 .53

CONTINUITY SUMMARY (AC-FT) - INFLOW= .2634E+03 EXCESS= .0000E+00 OUTFLOW= .2634E+03 BASIN STORAGE= .1738E-03 PERCENT ERROR= .0

FOR STORM = 6 STORM AREA (SQ MI) = 120.00
65AT-3 MANE 1.34 482.10 785.27 .52 5.00 481.84 785.00 .52

CONTINUITY SUMMARY (AC-FT) - INFLOW= .2580E+03 EXCESS= .0000E+00 OUTFLOW= .2580E+03 BASIN STORAGE= .1738E-03 PERCENT ERROR= .0

FOR STORM = 7 STORM AREA (SQ MI) = 150.00
65AT-3 MANE 1.34 481.19 785.57 .51 5.00 480.85 785.00 .51

CONTINUITY SUMMARY (AC-FT) - INFLOW= .2541E+03 EXCESS= .0000E+00 OUTFLOW= .2541E+03 BASIN STORAGE= .1736E-03 PERCENT ERROR= .0

FOR STORM = 1 STORM AREA (SQ MI) = .01
65T66 MANE 1.05 506.38 790.65 .67 5.00 506.18 790.00 .67

CONTINUITY SUMMARY (AC-FT) - INFLOW= .3353E+03 EXCESS= .0000E+00 OUTFLOW= .3353E+03 BASIN STORAGE= .1373E-03 PERCENT ERROR= .0

FOR STORM = 2 STORM AREA (SQ MI) = 10.00
65T66 MANE 1.05 499.59 789.72 .61 5.00 499.46 790.00 .61

CONTINUITY SUMMARY (AC-FT) - INFLOW= .3073E+03 EXCESS= .0000E+00 OUTFLOW= .3073E+03 BASIN STORAGE= .1378E-03 PERCENT ERROR= .0

FOR STORM = 3 STORM AREA (SQ MI) = 30.00
65T66 MANE 1.05 493.84 786.25 .58 5.00 493.26 785.00 .58

CONTINUITY SUMMARY (AC-FT) - INFLOW= .2890E+03 EXCESS= .0000E+00 OUTFLOW= .2890E+03 BASIN STORAGE= .1378E-03 PERCENT ERROR= .0

FOR STORM = 4 STORM AREA (SQ MI) = 60.00
65T66 MANE 1.06 487.33 786.13 .54 5.00 487.00 785.00 .54

CONTINUITY SUMMARY (AC-FT) - INFLOW= .2708E+03 EXCESS= .0000E+00 OUTFLOW= .2708E+03 BASIN STORAGE= .1376E-03 PERCENT ERROR= .0

FOR STORM = 5 STORM AREA (SQ MI) = 90.00
65T66 MANE 1.06 485.36 785.66 .53 5.00 484.94 785.00 .53

CONTINUITY SUMMARY (AC-FT) - INFLOW= .2643E+03 EXCESS= .0000E+00 OUTFLOW= .2643E+03 BASIN STORAGE= .1374E-03 PERCENT ERROR= .0

FOR STORM = 6 STORM AREA (SQ MI) = 120.00
65T66 MANE 1.06 480.95 786.05 .52 5.00 480.87 785.00 .52

CONTINUITY SUMMARY (AC-FT) - INFLOW= .2580E+03 EXCESS= .0000E+00 OUTFLOW= .2580E+03 BASIN STORAGE= .1374E-03 PERCENT ERROR= .0

FOR STORM = 7 STORM AREA (SQ MI) = 150.00
65T66 MANE 1.06 479.95 785.32 .51 5.00 479.74 785.00 .51

CONTINUITY SUMMARY (AC-FT) - INFLOW= .2541E+03 EXCESS= .0000E+00 OUTFLOW= .2541E+03 BASIN STORAGE= .1373E-03 PERCENT ERROR= .0

FOR STORM = 1 STORM AREA (SQ MI) = .01
66C1T2 MANE .75 410.00 886.49 2.94 5.00 410.00 890.00 2.94

CONTINUITY SUMMARY (AC-FT) - INFLOW= .5231E+03 EXCESS= .0000E+00 OUTFLOW= .5231E+03 BASIN STORAGE= .9382E-04 PERCENT ERROR= .0

FOR STORM = 2 STORM AREA (SQ MI) = 10.00
66C1T2 MANE .75 410.00 896.19 2.87 5.00 410.00 900.00 2.87

CONTINUITY SUMMARY (AC-FT) - INFLOW= .5121E+03 EXCESS= .0000E+00 OUTFLOW= .5121E+03 BASIN STORAGE= .9382E-04 PERCENT ERROR= .0

FOR STORM = 3 STORM AREA (SQ MI) = 30.00
66C1T2 MANE .75 409.96 916.36 2.82 5.00 409.95 915.00 2.82

CONTINUITY SUMMARY (AC-FT) - INFLOW= .5020E+03 EXCESS= .0000E+00 OUTFLOW= .5020E+03 BASIN STORAGE= .9381E-04 PERCENT ERROR= .0

FOR STORM = 4 STORM AREA (SQ MI) = 60.00
66C1T2 MANE .75 406.85 961.09 2.78 5.00 406.84 965.00 2.78

CONTINUITY SUMMARY (AC-FT) - INFLOW= .4948E+03 EXCESS= .0000E+00 OUTFLOW= .4948E+03 BASIN STORAGE= .9375E-04 PERCENT ERROR= .0

FOR STORM = 5 STORM AREA (SQ MI) = 90.00
66C1T2 MANE .75 406.11 930.06 2.76 5.00 406.11 930.00 2.76

CONTINUITY SUMMARY (AC-FT) - INFLOW= .4924E+03 EXCESS= .0000E+00 OUTFLOW= .4924E+03 BASIN STORAGE= .9378E-04 PERCENT ERROR= .0

FOR STORM = 6 STORM AREA (SQ MI) = 120.00
66C1T2 MANE .75 406.05 930.08 2.75 5.00 406.05 930.00 2.75

CONTINUITY SUMMARY (AC-FT) - INFLOW= .4906E+03 EXCESS= .0000E+00 OUTFLOW= .4906E+03 BASIN STORAGE= .9377E-04 PERCENT ERROR= .0

FOR STORM = 7 STORM AREA (SQ MI) = 150.00
66C1T2 MANE .75 406.02 930.08 2.75 5.00 406.02 930.00 2.75

CONTINUITY SUMMARY (AC-FT) - INFLOW= .4895E+03 EXCESS= .0000E+00 OUTFLOW= .4895E+03 BASIN STORAGE= .9377E-04 PERCENT ERROR= .0

FOR STORM = 1 STORM AREA (SQ MI) = .01
66C1D MANE .81 898.70 790.07 1.27 5.00 898.70 790.00 1.27

CONTINUITY SUMMARY (AC-FT) - INFLOW= .8585E+03 EXCESS= .0000E+00 OUTFLOW= .8584E+03 BASIN STORAGE= .1199E-03 PERCENT ERROR= .0

FOR STORM = 2 STORM AREA (SQ MI) = 10.00
66CTD MANE .83 830.82 857.19 1.21 5.00 830.74 860.00 1.21

CONTINUITY SUMMARY (AC-FT) - INFLOW= .8195E+03 EXCESS= .0000E+00 OUTFLOW= .8194E+03 BASIN STORAGE= .1197E-03 PERCENT ERROR= .0

FOR STORM = 3 STORM AREA (SQ MI) = 30.00
66CTD MANE .83 799.36 879.70 1.17 5.00 799.25 880.00 1.17

CONTINUITY SUMMARY (AC-FT) - INFLOW= .7910E+03 EXCESS= .0000E+00 OUTFLOW= .7910E+03 BASIN STORAGE= .1197E-03 PERCENT ERROR= .0

FOR STORM = 4 STORM AREA (SQ MI) = 60.00
66CTD MANE .84 784.50 869.62 1.13 5.00 784.49 870.00 1.13

CONTINUITY SUMMARY (AC-FT) - INFLOW= .7656E+03 EXCESS= .0000E+00 OUTFLOW= .7656E+03 BASIN STORAGE= .1198E-03 PERCENT ERROR= .0

FOR STORM = 5 STORM AREA (SQ MI) = 90.00
66CTD MANE .84 781.94 869.35 1.12 5.00 781.94 870.00 1.12

CONTINUITY SUMMARY (AC-FT) - INFLOW= .7567E+03 EXCESS= .0000E+00 OUTFLOW= .7567E+03 BASIN STORAGE= .1195E-03 PERCENT ERROR= .0

FOR STORM = 6 STORM AREA (SQ MI) = 120.00
66CTD MANE .84 777.99 871.04 1.10 5.00 777.96 870.00 1.10

CONTINUITY SUMMARY (AC-FT) - INFLOW= .7486E+03 EXCESS= .0000E+00 OUTFLOW= .7486E+03 BASIN STORAGE= .1196E-03 PERCENT ERROR= .0

FOR STORM = 7 STORM AREA (SQ MI) = 150.00
66CTD MANE .84 775.52 869.95 1.10 5.00 775.52 870.00 1.10

CONTINUITY SUMMARY (AC-FT) - INFLOW= .7437E+03 EXCESS= .0000E+00 OUTFLOW= .7436E+03 BASIN STORAGE= .1196E-03 PERCENT ERROR= .0

FOR STORM = 1 STORM AREA (SQ MI) = .01
RC-WA MANE .70 909.58 815.44 1.08 5.00 909.54 815.00 1.08

CONTINUITY SUMMARY (AC-FT) - INFLOW= .9235E+03 EXCESS= .0000E+00 OUTFLOW= .9235E+03 BASIN STORAGE= .1037E-03 PERCENT ERROR= .0

FOR STORM = 2 STORM AREA (SQ MI) = 10.00
RC-WA MANE .71 845.53 861.07 1.03 5.00 845.43 860.00 1.03

CONTINUITY SUMMARY (AC-FT) - INFLOW= .8829E+03 EXCESS= .0000E+00 OUTFLOW= .8829E+03 BASIN STORAGE= .1038E-03 PERCENT ERROR= .0

FOR STORM = 3 STORM AREA (SQ MI) = 30.00
RC-WA MANE .72 815.02 880.38 1.00 5.00 815.01 880.00 1.00

CONTINUITY SUMMARY (AC-FT) - INFLOW= .8540E+03 EXCESS= .0000E+00 OUTFLOW= .8540E+03 BASIN STORAGE= .1037E-03 PERCENT ERROR= .0

FOR STORM = 4 STORM AREA (SQ MI) = 60.00
RC-WA MANE .72 798.36 876.09 .97 5.00 798.31 875.00 .97

CONTINUITY SUMMARY (AC-FT) - INFLOW= .8282E+03 EXCESS= .0000E+00 OUTFLOW= .8282E+03 BASIN STORAGE= .1036E-03 PERCENT ERROR= .0

FOR STORM = 5 STORM AREA (SQ MI) = 90.00
RC-WA MANE .72 795.59 875.95 .96 5.00 795.56 875.00 .96

CONTINUITY SUMMARY (AC-FT) - INFLOW= .8192E+03 EXCESS= .0000E+00 OUTFLOW= .8192E+03 BASIN STORAGE= .1038E-03 PERCENT ERROR= .0

FOR STORM = 6 STORM AREA (SQ MI) = 120.00
RC-WA MANE .72 791.28 875.48 .95 5.00 791.23 875.00 .95

CONTINUITY SUMMARY (AC-FT) - INFLOW= .8108E+03 EXCESS= .0000E+00 OUTFLOW= .8108E+03 BASIN STORAGE= .1038E-03 PERCENT ERROR= .0

FOR STORM = 7 STORM AREA (SQ MI) = 150.00
RC-WA MANE .72 788.53 875.35 .94 5.00 788.50 875.00 .94

CONTINUITY SUMMARY (AC-FT) - INFLOW= .8057E+03 EXCESS= .0000E+00 OUTFLOW= .8057E+03 BASIN STORAGE= .1038E-03 PERCENT ERROR= .0

FOR STORM = 1 STORM AREA (SQ MI) = .01
66T66D MANE 1.02 909.19 791.03 1.18 5.00 909.13 790.00 1.18

CONTINUITY SUMMARY (AC-FT) - INFLOW= .1007E+04 EXCESS= .0000E+00 OUTFLOW= .1007E+04 BASIN STORAGE= .1498E-03 PERCENT ERROR= .0

FOR STORM = 2 STORM AREA (SQ MI) = 10.00
66T66D MANE 1.03 845.39 861.57 1.11 5.00 845.31 865.00 1.11

CONTINUITY SUMMARY (AC-FT) - INFLOW= .9528E+03 EXCESS= .0000E+00 OUTFLOW= .9528E+03 BASIN STORAGE= .1499E-03 PERCENT ERROR= .0

FOR STORM = 3 STORM AREA (SQ MI) = 30.00
66T66D MANE 1.04 814.84 880.39 1.07 5.00 814.80 880.00 1.07

CONTINUITY SUMMARY (AC-FT) - INFLOW= .9182E+03 EXCESS= .0000E+00 OUTFLOW= .9182E+03 BASIN STORAGE= .1498E-03 PERCENT ERROR= .0

FOR STORM = 4 STORM AREA (SQ MI) = 60.00
66T66D MANE 1.04 798.26 876.74 1.03 5.00 798.17 875.00 1.03

CONTINUITY SUMMARY (AC-FT) - INFLOW= .8836E+03 EXCESS= .0000E+00 OUTFLOW= .8836E+03 BASIN STORAGE= .1500E-03 PERCENT ERROR= .0

FOR STORM = 5 STORM AREA (SQ MI) = 90.00
66T66D MANE 1.05 795.49 876.30 1.02 5.00 795.42 875.00 1.02

CONTINUITY SUMMARY (AC-FT) - INFLOW= .8712E+03 EXCESS= .0000E+00 OUTFLOW= .8712E+03 BASIN STORAGE= .1501E-03 PERCENT ERROR= .0

FOR STORM = 6 STORM AREA (SQ MI) = 120.00
66T66D MANE 1.05 791.14 876.20 1.00 5.00 791.04 875.00 1.00

CONTINUITY SUMMARY (AC-FT) - INFLOW= .8590E+03 EXCESS= .0000E+00 OUTFLOW= .8590E+03 BASIN STORAGE= .1500E-03 PERCENT ERROR= .0

FOR STORM = 7 STORM AREA (SQ MI) = 150.00
66T66D MANE 1.05 788.40 875.77 .99 5.00 788.34 875.00 .99

CONTINUITY SUMMARY (AC-FT) - INFLOW= .8516E+03 EXCESS= .0000E+00 OUTFLOW= .8516E+03 BASIN STORAGE= .1501E-03 PERCENT ERROR= .0

FOR STORM = 1 STORM AREA (SQ MI) = .01
66-66D MANE 2.19 908.46 792.01 1.18 5.00 907.93 790.00 1.18

CONTINUITY SUMMARY (AC-FT) - INFLOW= .1007E+04 EXCESS= .0000E+00 OUTFLOW= .1007E+04 BASIN STORAGE= .3240E-03 PERCENT ERROR= .0

FOR STORM = 2 STORM AREA (SQ MI) = 10.00
66-66D MANE 2.23 845.24 865.95 1.11 5.00 845.22 865.00 1.11

CONTINUITY SUMMARY (AC-FT) - INFLOW= .9529E+03 EXCESS= .0000E+00 OUTFLOW= .9528E+03 BASIN STORAGE= .3228E-03 PERCENT ERROR= .0

FOR STORM = 3 STORM AREA (SQ MI) = 30.00
66-66D MANE 2.24 814.44 881.31 1.07 5.00 814.29 880.00 1.07

CONTINUITY SUMMARY (AC-FT) - INFLOW= .9182E+03 EXCESS= .0000E+00 OUTFLOW= .9182E+03 BASIN STORAGE= .3230E-03 PERCENT ERROR= .0

FOR STORM = 4 STORM AREA (SQ MI) = 60.00
66-66D MANE 2.25 798.12 878.20 1.03 5.00 798.05 880.00 1.03

CONTINUITY SUMMARY (AC-FT) - INFLOW= .8836E+03 EXCESS= .0000E+00 OUTFLOW= .8836E+03 BASIN STORAGE= .3230E-03 PERCENT ERROR= .0

FOR STORM = 5 STORM AREA (SQ MI) = 90.00
66-66D MANE 2.25 795.30 878.80 1.02 5.00 795.12 880.00 1.02

CONTINUITY SUMMARY (AC-FT) - INFLOW= .8712E+03 EXCESS= .0000E+00 OUTFLOW= .8712E+03 BASIN STORAGE= .3259E-03 PERCENT ERROR= .0

FOR STORM = 6 STORM AREA (SQ MI) = 120.00
66-66D MANE 2.26 790.91 877.52 1.00 5.00 790.76 880.00 1.00

CONTINUITY SUMMARY (AC-FT) - INFLOW= .8590E+03 EXCESS= .0000E+00 OUTFLOW= .8590E+03 BASIN STORAGE= .3257E-03 PERCENT ERROR= .0

FOR STORM = 7 STORM AREA (SQ MI) = 150.00
66-66D MANE 2.26 788.17 878.12 .99 5.00 787.96 875.00 .99

CONTINUITY SUMMARY (AC-FT) - INFLOW= .8516E+03 EXCESS= .0000E+00 OUTFLOW= .8516E+03 BASIN STORAGE= .3229E-03 PERCENT ERROR= .0

FOR STORM = 1 STORM AREA (SQ MI) = .01
66T70A MANE .87 1217.38 814.86 1.11 5.00 1217.29 815.00 1.11

CONTINUITY SUMMARY (AC-FT) - INFLOW= .1098E+04 EXCESS= .0000E+00 OUTFLOW= .1097E+04 BASIN STORAGE= .1355E-03 PERCENT ERROR= .1

FOR STORM = 2 STORM AREA (SQ MI) = 10.00
66T70A MANE .90 1007.12 845.15 1.03 5.00 1007.10 845.00 1.03

CONTINUITY SUMMARY (AC-FT) - INFLOW= .1023E+04 EXCESS= .0000E+00 OUTFLOW= .1022E+04 BASIN STORAGE= .1353E-03 PERCENT ERROR= .1

FOR STORM = 3 STORM AREA (SQ MI) = 30.00
66T70A MANE .92 925.18 867.02 .99 5.00 925.14 870.00 .99

CONTINUITY SUMMARY (AC-FT) - INFLOW= .9751E+03 EXCESS= .0000E+00 OUTFLOW= .9747E+03 BASIN STORAGE= .1358E-03 PERCENT ERROR= .0

FOR STORM = 4 STORM AREA (SQ MI) = 60.00
66T70A MANE .93 835.60 879.45 .94 5.00 835.57 880.00 .94

CONTINUITY SUMMARY (AC-FT) - INFLOW= .9272E+03 EXCESS= .0000E+00 OUTFLOW= .9269E+03 BASIN STORAGE= .1355E-03 PERCENT ERROR= .0

FOR STORM = 5 STORM AREA (SQ MI) = 90.00
66T70A MANE .94 829.86 872.94 .92 5.00 829.85 870.00 .92

CONTINUITY SUMMARY (AC-FT) - INFLOW= .9101E+03 EXCESS= .0000E+00 OUTFLOW= .9098E+03 BASIN STORAGE= .1355E-03 PERCENT ERROR= .0

FOR STORM = 6 STORM AREA (SQ MI) = 120.00
66T70A MANE .94 824.14 875.12 .90 5.00 824.13 875.00 .90

CONTINUITY SUMMARY (AC-FT) - INFLOW= .8931E+03 EXCESS= .0000E+00 OUTFLOW= .8928E+03 BASIN STORAGE= .1353E-03 PERCENT ERROR= .0

FOR STORM = 7 STORM AREA (SQ MI) = 150.00
66T70A MANE .94 805.75 889.63 .89 5.00 805.09 890.00 .89

CONTINUITY SUMMARY (AC-FT) - INFLOW= .8830E+03 EXCESS= .0000E+00 OUTFLOW= .8827E+03 BASIN STORAGE= .1358E-03 PERCENT ERROR= .0

FOR STORM = 1 STORM AREA (SQ MI) = .01
66T70B MANE 1.47 1214.22 816.66 1.11 5.00 1213.71 815.00 1.11

CONTINUITY SUMMARY (AC-FT) - INFLOW= .1097E+04 EXCESS= .0000E+00 OUTFLOW= .1097E+04 BASIN STORAGE= .2309E-03 PERCENT ERROR= .0

FOR STORM = 2 STORM AREA (SQ MI) = 10.00
66T70B MANE 1.53 1006.61 845.16 1.03 5.00 1006.56 845.00 1.03

CONTINUITY SUMMARY (AC-FT) - INFLOW= .1022E+04 EXCESS= .0000E+00 OUTFLOW= .1022E+04 BASIN STORAGE= .2300E-03 PERCENT ERROR= .0

FOR STORM = 3 STORM AREA (SQ MI) = 30.00
66T70B MANE 1.55 924.98 870.49 .99 5.00 924.97 870.00 .99

CONTINUITY SUMMARY (AC-FT) - INFLOW= .9747E+03 EXCESS= .0000E+00 OUTFLOW= .9747E+03 BASIN STORAGE= .2297E-03 PERCENT ERROR= .0

FOR STORM = 4 STORM AREA (SQ MI) = 60.00

66T70B MANE 1.58 835.39 880.48 .94 5.00 835.38 880.00 .94

CONTINUITY SUMMARY (AC-FT) - INFLOW= .9269E+03 EXCESS= .0000E+00 OUTFLOW= .9269E+03 BASIN STORAGE= .2292E-03 PERCENT ERROR= .0

FOR STORM = 5 STORM AREA (SQ MI) = 90.00
66T70B MANE 1.59 829.82 873.77 .92 5.00 829.79 875.00 .92

CONTINUITY SUMMARY (AC-FT) - INFLOW= .9098E+03 EXCESS= .0000E+00 OUTFLOW= .9097E+03 BASIN STORAGE= .2293E-03 PERCENT ERROR= .0

FOR STORM = 6 STORM AREA (SQ MI) = 120.00
66T70B MANE 1.59 823.64 876.56 .90 5.00 823.49 875.00 .90

CONTINUITY SUMMARY (AC-FT) - INFLOW= .8928E+03 EXCESS= .0000E+00 OUTFLOW= .8928E+03 BASIN STORAGE= .2298E-03 PERCENT ERROR= .0

FOR STORM = 7 STORM AREA (SQ MI) = 150.00
66T70B MANE 1.60 804.30 888.65 .89 5.00 804.28 890.00 .89

CONTINUITY SUMMARY (AC-FT) - INFLOW= .8826E+03 EXCESS= .0000E+00 OUTFLOW= .8826E+03 BASIN STORAGE= .2294E-03 PERCENT ERROR= .0

FOR STORM = 1 STORM AREA (SQ MI) = .01
CULVT MANE 1.11 1201.03 815.66 1.11 5.00 1200.39 815.00 1.11

CONTINUITY SUMMARY (AC-FT) - INFLOW= .1097E+04 EXCESS= .0000E+00 OUTFLOW= .1095E+04 BASIN STORAGE= .1738E-03 PERCENT ERROR= .1

FOR STORM = 2 STORM AREA (SQ MI) = 10.00
CULVT MANE 1.15 998.58 845.61 1.03 5.00 998.46 845.00 1.03

CONTINUITY SUMMARY (AC-FT) - INFLOW= .1022E+04 EXCESS= .0000E+00 OUTFLOW= .1021E+04 BASIN STORAGE= .1734E-03 PERCENT ERROR= .1

FOR STORM = 3 STORM AREA (SQ MI) = 30.00
CULVT MANE 1.17 919.19 870.57 .98 5.00 919.13 870.00 .98

CONTINUITY SUMMARY (AC-FT) - INFLOW= .9747E+03 EXCESS= .0000E+00 OUTFLOW= .9738E+03 BASIN STORAGE= .1731E-03 PERCENT ERROR= .1

FOR STORM = 4 STORM AREA (SQ MI) = 60.00
CULVT MANE 1.20 830.68 880.13 .94 5.00 830.67 880.00 .94

CONTINUITY SUMMARY (AC-FT) - INFLOW= .9269E+03 EXCESS= .0000E+00 OUTFLOW= .9261E+03 BASIN STORAGE= .1735E-03 PERCENT ERROR= .1

FOR STORM = 5 STORM AREA (SQ MI) = 90.00
CULVT MANE 1.20 824.52 875.31 .92 5.00 824.51 875.00 .92

CONTINUITY SUMMARY (AC-FT) - INFLOW= .9097E+03 EXCESS= .0000E+00 OUTFLOW= .9090E+03 BASIN STORAGE= .1738E-03 PERCENT ERROR= .1

FOR STORM = 6 STORM AREA (SQ MI) = 120.00
CULVT MANE 1.20 818.21 879.04 .90 5.00 818.12 880.00 .90

CONTINUITY SUMMARY (AC-FT) - INFLOW= .8928E+03 EXCESS= .0000E+00 OUTFLOW= .8921E+03 BASIN STORAGE= .1739E-03 PERCENT ERROR= .1

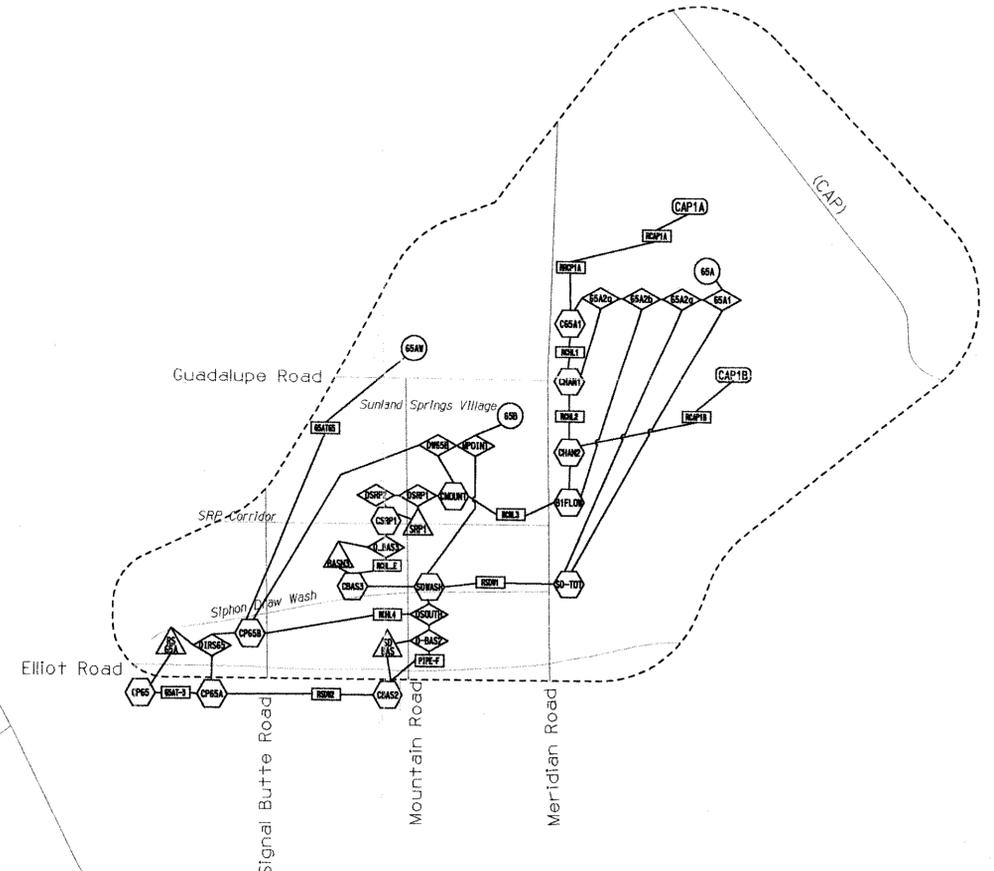
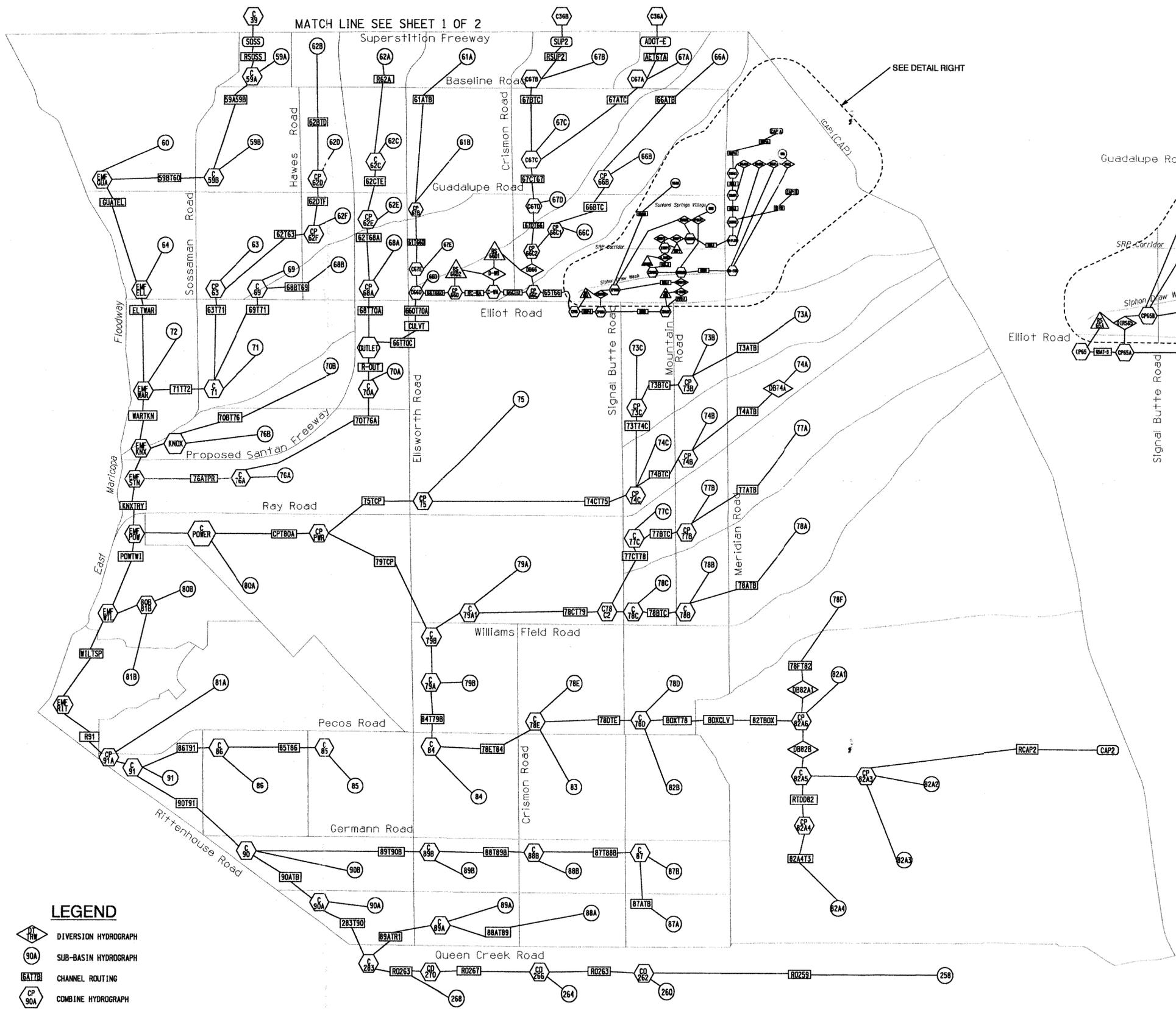
FOR STORM = 7 STORM AREA (SQ MI) = 150.00
CULVT MANE 1.20 799.77 889.24 .89 5.00 799.33 890.00 .89

CONTINUITY SUMMARY (AC-FT) - INFLOW= .8826E+03 EXCESS= .0000E+00 OUTFLOW= .8819E+03 BASIN STORAGE= .1729E-03 PERCENT ERROR= .1

*** NORMAL END OF HEC-1 ***

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Percent Inactive: .0

W:\2003\Projects\31902_Siphon Draw\Hydrology\CONC\JULY20_05\HEC1.dgn 07/20/2005 05:01:05 PM



DETAIL



LEGEND

- DIVERSION HYDROGRAPH
- SUB-BASIN HYDROGRAPH
- CHANNEL ROUTING
- COMBINE HYDROGRAPH
- INFLOW FROM OUTSIDE STUDY AREA
- STORAGE ROUTING

FLOOD CONTROL DISTRICT OF MARICOPA COUNTY			
PLAN	SZ	BY	DATE
PLANS CHK.		DH	JULY, '05
DRAWN BY		DH	JULY, '05
SUBMITTED BY:		DATE:	
SIPHON DRAW DRAINAGE IMPROVEMENTS CONTRACT FCD 2003-C019			HEC-1 SCHEMATIC FOR PREFERRED ALTERNATIVE SOUTH US 60 EAST MESA ADMP
			PLATE 12

APPENDIX B

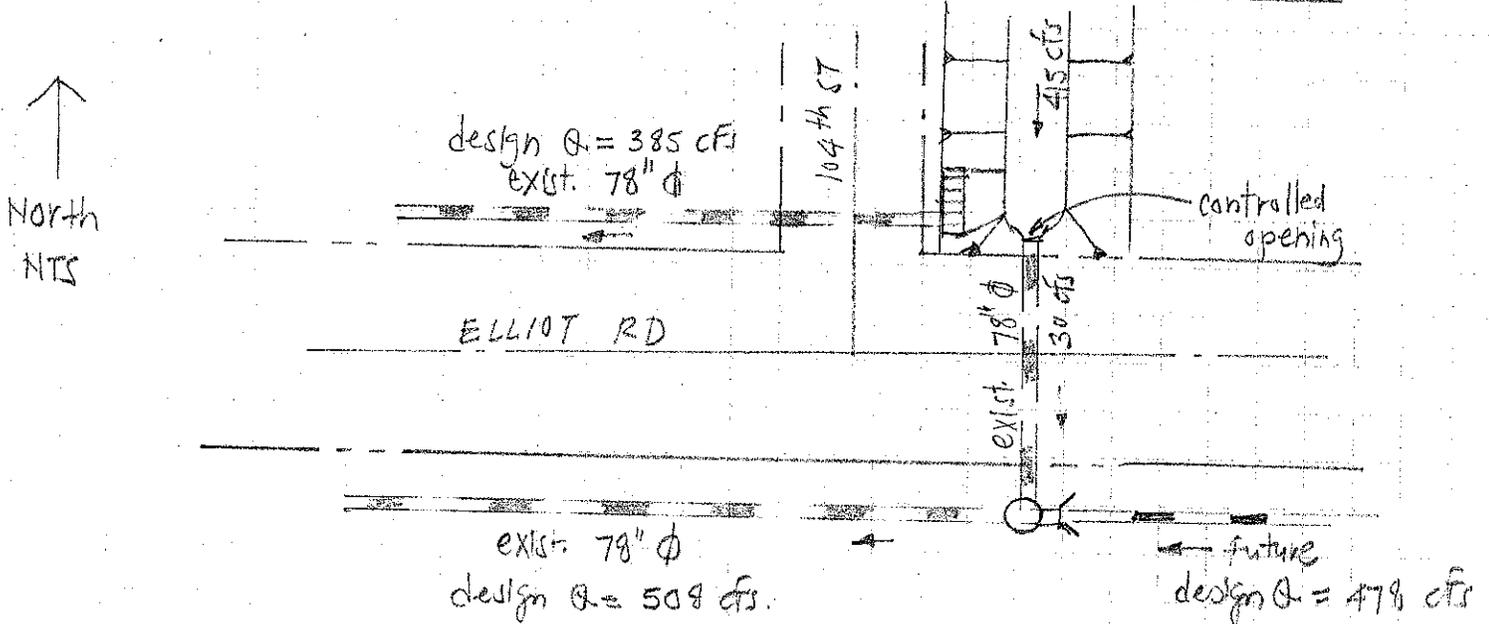
Summary of Peak Flows at 104th Street and Elliot Road

WOOD/PATEL

CIVIL ENGINEERS • HYDROLOGISTS • LAND SURVEYORS • CONSTRUCTION MANAGERS

PROJECT Siphon Draw No. _____
CALC'D BY ACP CHK'D BY _____
SHEET _____ OF _____ DATE 6/05

Summary of Peak Flows @ 104th St/Elliot Road.



EXISTING DESIGN PARAMETERS

WOOD/PATEL

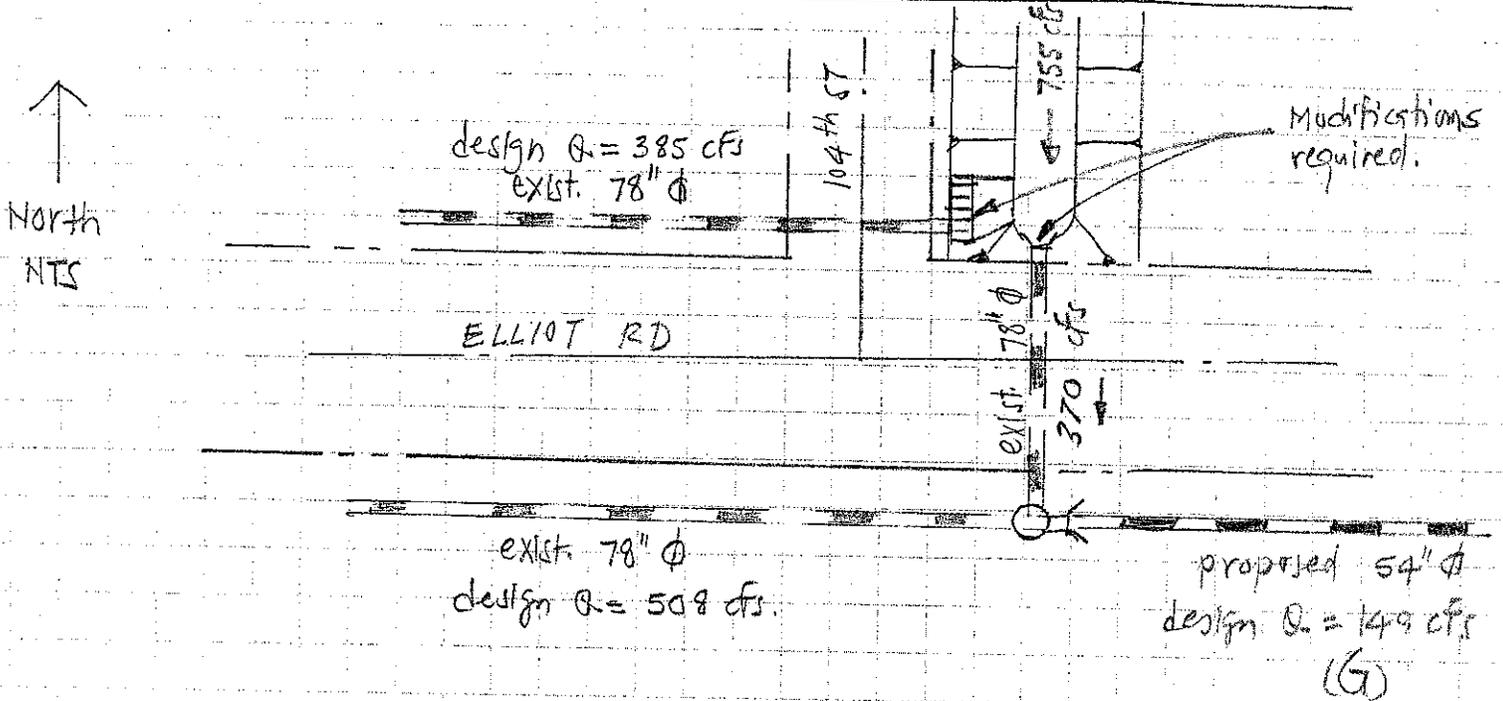
CIVIL ENGINEERS • HYDROLOGISTS • LAND SURVEYORS • CONSTRUCTION MANAGERS

PROJECT Siphon Draw No. _____

CALC'D BY ACP CHK'D BY _____

SHEET _____ OF _____ DATE 6/05

Summary of Peak Flows @ 104th St/Elliot Road



Modified Design Parameters

Phase I - Design Study

APPENDIX C

Supporting Cost Estimation Sheets

Siphon Draw Drainage Improvements Alternatives

Flood Control District of Maricopa County

W/P # 031902.04

FCD 2003 C019

Preliminary Opinion of Probable Cost for Alternative 1

MAJOR SYSTEM ELEMENTS

ITEM	DESCRIPTION	UNIT PRICE	UNIT	QUANTITY	AMOUNT
1	Channel "E" Excavation	\$6	CY	25,600	\$153,600
2	Drop Structure	\$310.00	CY	91	\$28,210
3	78" RGRCP Pipe "D"	\$270	LF	2,600	\$702,000
4	54" RGRCP Pipe "F"	\$150	LF	1,300	\$195,000
5	54" RGRCP Pipe "G"	\$150	LF	5,200	\$780,000
6	Basin "A" Excavation	\$6	CY	194,000	\$1,164,000
7	Basin "B" Excavation	\$6	CY	97,000	\$582,000
8	Manholes	\$6,000.00	EA	23	\$138,000
9	Diversion Structure	\$150,000	EA	2	\$300,000
10	Landscaping	\$0.50	SF	1,271,952	\$635,976

SUBTOTAL MAJOR SYSTEM ELEMENTS \$4,678,786

DRAFT

CONTINGENCIES:

Construction	25%	\$1,169,697
Design & Field Engineering	18%	\$1,052,727
Change Orders	7%	\$409,394

TOTAL MAJOR SYSTEM ELEMENTS \$7,310,603

1. Construction Contingencies @ 25% of the Total Construction Cost
2. Design and Field Engineering Costs @ 18% of the sum of Total Construction Cost and Construction Contingencies
3. Change Orders @ 7% of the sum of Total Construction Cost and Construction Contingencies
4. The cost estimates were performed for selected drainage elements only for alternative selection purposes. All common items were excluded from these costs. Therefore, the cost estimates shown are not the true total construction cost of any alternatives.

Siphon Draw Drainage Improvements Alternatives

Flood Control District of Maricopa County

FCD 2003 C019

W/P # 031902.04

Preliminary Opinion of Probable Cost for Alternative 2MAJOR SYSTEM ELEMENTS

ITEM	DESCRIPTION	UNIT PRICE	UNIT	QUANTITY	AMOUNT
1	Channel "D" Excavation	\$6	CY	41,600	\$249,600
2	Channel "E" Excavation	\$6	CY	17,100	\$102,600
3	Drop Structure	\$310	CY	445	\$137,950
4	54" RGRCP Pipe "F"	\$150	LF	1,300	\$195,000
5	54" RGRCP Pipe "G"	\$150	LF	5,200	\$780,000
6	Basin "A" Excavation	\$6	CY	113,000	\$678,000
7	Basin "B" Excavation	\$6	CY	97,000	\$582,000
8	Basin "C" Excavation	\$6	CY	81,000	\$486,000
9	RCBC for Channel "D"	\$150,000.00	EA	1	\$150,000
10	Manholes	\$6,000.00	EA	17	\$102,000
11	Diversion Structure	\$150,000	EA	3	\$450,000
12	Landscaping	\$0.50	SF	1,812,096	\$906,048

SUBTOTAL MAJOR SYSTEM ELEMENTS \$4,819,198

CONTINGENCIES:

Construction	25%	\$1,204,800
Design & Field Engineering	18%	\$1,084,320
Change Orders	7%	\$421,680

TOTAL MAJOR SYSTEM ELEMENTS \$7,529,997

1. Construction Contingencies @ 25% of the Total Construction Cost
2. Design and Field Engineering Costs @ 18% of the sum of Total Construction Cost and Construction Contingencies
3. Change Orders @ 7% of the sum of Total Construction Cost and Construction Contingencies
4. The cost estimates were performed for selected drainage elements only for alternative selection purposes. All common items were excluded from these costs. Therefore, the cost estimates shown are not the true total construction cost of any alternatives.

Siphon Draw Drainage Improvements Alternatives

Flood Control District of Maricopa County

W/P # 031902.04

FCD 2003 C019

Preliminary Opinion of Probable Cost for Alternative 3MAJOR SYSTEM ELEMENTS

ITEM	DESCRIPTION	UNIT PRICE	UNIT	QUANTITY	AMOUNT
1	Channel "D" Excavation	\$6	CY	83,970	\$503,820
2	Channel "E" Excavation	\$6	CY	17,067	\$102,402
3	Drop Structure	\$310	CY	816	\$252,960
4	54" RGRCP Pipe "F"	\$150	LF	1,300	\$195,000
5	54" RGRCP Pipe "G"	\$150	LF	5,200	\$780,000
6	Basin "A" Excavation	\$6	CY	10,000	\$60,000
7	Basin "B" Excavation	\$6	CY	97,000	\$582,000
8	Basin "C" Excavation	\$6	CY	226,000	\$1,356,000
9	RCBC for Channel "D"	\$200,000.00	EA	1	\$200,000
10	Manholes	\$6,000.00	EA	17	\$102,000
11	Diversion Structure	\$150,000	EA	2	\$300,000
12	Landscaping	\$0.50	SF	1,328,580	\$664,290

SUBTOTAL MAJOR SYSTEM ELEMENTS \$5,098,472

CONTINGENCIES:

Construction	25%	\$1,274,618
Design & Field Engineering	18%	\$1,147,156
Change Orders	7%	\$446,116

TOTAL MAJOR SYSTEM ELEMENTS \$7,966,363

1. Construction Contingencies @ 25% of the Total Construction Cost
2. Design and Field Engineering Costs @ 18% of the sum of Total Construction Cost and Construction Contingencies
3. Change Orders @ 7% of the sum of Total Construction Cost and Construction Contingencies
4. The cost estimates were performed for selected drainage elements only for alternative selection purposes. All common items were excluded from these costs. Therefore, the cost estimates shown are not the true total construction cost of any alternatives.

Siphon Draw Drainage Improvements Alternatives

Flood Control District of Maricopa County

FCD 2003 C019

W/P # 031902

Preliminary Opinion of Probable Cost for Alternative 3a
Concrete Channel "D"MAJOR SYSTEM ELEMENTS

ITEM	DESCRIPTION	UNIT PRICE	UNIT	QUANTITY	AMOUNT
1	Channel "D" Excavation	\$6	CY	32,933	\$197,598
2	Concrete Channel "D"	\$310	CY	3,667	\$1,136,770
3	Channel "E" Excavation	\$6	CY	14,933	\$89,598
4	Drop Structure	\$310	CY	91	\$28,210
5	54" RGRCP Pipe "F"	\$150	LF	1,300	\$195,000
6	54" RGRCP Pipe "G"	\$150	LF	5,200	\$780,000
7	Basin "A" Excavation	\$6	CY	10,000	\$60,000
8	Basin "B" Excavation	\$6	CY	97,000	\$582,000
9	Basin "C" Excavation	\$6	CY	218,000	\$1,308,000
10	RCBC for Channel "D"	\$200,000.00	EA	1	\$200,000
11	Manholes	\$6,000.00	EA	17	\$102,000
12	Diversion Structure	\$150,000	EA	2	\$300,000
13	Landscaping	\$0.50	SF	1,328,580	\$664,290

SUBTOTAL MAJOR SYSTEM ELEMENTS \$5,643,466

CONTINGENCIES:

Construction	25%	\$1,410,867
Design & Field Engineering	18%	\$1,269,780
Change Orders	7%	\$493,803

TOTAL MAJOR SYSTEM ELEMENTS \$8,817,916

1. Construction Contingencies @ 25% of the Total Construction Cost
2. Design and Field Engineering Costs @ 18% of the sum of Total Construction Cost and Construction Contingencies
3. Change Orders @ 7% of the sum of Total Construction Cost and Construction Contingencies
4. The cost estimates were performed for selected drainage elements only for alternative selection purposes. All common items were excluded from these costs. Therefore, the cost estimates shown are not the true total construction cost of any alternatives.

Siphon Draw Drainage Improvements Alternatives

Flood Control District of Maricopa County

W/P # 031902.04

FCD 2003 C019

Preliminary Opinion of Probable Cost for Interim Alternative*MAJOR SYSTEM ELEMENTS*

ITEM	DESCRIPTION	UNIT PRICE	UNIT	QUANTITY	AMOUNT
1	Channel "D" Excavation	\$6	CY	83,970	\$503,820
2	Channel "H" Excavation	\$6	CY	90,430	\$542,580
3	Drop Structure	\$310	CY	1,470	\$455,700
4	Landscaping	\$0.50	SF	0	\$0
SUBTOTAL MAJOR SYSTEM ELEMENTS					\$1,502,100
<i>CONTINGENCIES</i>					
Construction				25%	\$375,525
Design & Field Engineering				18%	\$337,973
Change Orders				7%	\$131,434
TOTAL MAJOR SYSTEM ELEMENTS					\$2,347,031

1. Construction Contingencies @ 25% of the Total Construction Cost
2. Design and Field Engineering Costs @ 18% of the sum of Total Construction Cost and Construction Contingencies
3. Change Orders @ 7% of the sum of Total Construction Cost and Construction Contingencies

Siphon Draw Drainage Improvements Alternatives
 Flood Control District of Maricopa County
 FCD 2003 C019

W/P # 031902

Preliminary Opinion of Probable Cost for Interim Alternative-a
Concrete Channel "D" and "H"

MAJOR SYSTEM ELEMENTS

ITEM	DESCRIPTION	UNIT PRICE	UNIT	QUANTITY	AMOUNT
1	Channel "D" Excavation	\$6	CY	32,933	\$197,598
2	Channel "H" Excavation	\$6	CY	35,467	\$212,802
3	Concrete Channel "D"	\$310	CY	3,667	\$1,136,770
4	Concrete Channel "H"	\$310.00	CY	3,949	\$1,224,190
SUBTOTAL MAJOR SYSTEM ELEMENTS					\$2,771,360

CONTINGENCIES:

Construction	25%	\$692,840
Design & Field Engineering	18%	\$623,556
Change Orders	7%	\$242,494

TOTAL MAJOR SYSTEM ELEMENTS \$4,330,250

1. Construction Contingencies @ 25% of the Total Construction Cost
2. Design and Field Engineering Costs @ 18% of the sum of Total Construction Cost and Construction Contingencies
3. Change Orders @ 7% of the sum of Total Construction Cost and Construction Contingencies

APPENDIX D
Hydraulic Analysis

APPENDIX D

Hydraulic Analysis (Using simple normal depth calculations)

Channels

- C-1 Channel D = Earthen, at 3 sections (Q varies)
- C-2 Channel D = Concrete, at 3 sections (Q varies)
- C-3 Gunite Channel ($Q_{100} = 170$ cfs)
- C-4 Channel E = Earthen, ($Q_{100} = 310$ cfs)

Box Culverts

- B-1 3-10'x 5' RCBC, at SRP Maintenance Road ($Q_{100} = 1050$ cfs)
- B-2 1-10'x 6' RCBC, diversion to Basin C2 ($Q_{100} = 761$ cfs)
- B-3 1-10'x 4' RCBC, to Channel E ($Q_{100} = 310$ cfs)
- B-4 1-10'x 4' RCBC, equalizer box, Basin C1-C2 (Q varies)
- B-5 1-10'x 6' RCBC, diversion to Basin B2 ($Q_{100} = 740$ cfs)
- B-6 1-10'x 4' RCBC, equalizer box, Basin B1-B2 (Q varies)

Storm Drains

- S-1 Bleed-off pipes
- S-2 Storm Drain F ($Q_{100} = 80$ cfs)
- S-3 Storm Drain G ($Q_{100} = 149$ cfs)

Weirs

- W-1 Side weirs hydraulics

Sediment Basin

TRAPEZOIDAL CHANNEL ANALYSIS
NORMAL DEPTH COMPUTATION

July 18, 2005

PROGRAM INPUT DATA

DESCRIPTION	VALUE
Flow Rate (cfs).....	1,681.0
Channel Bottom Slope (ft/ft).....	0.001
Manning's Roughness Coefficient (n-value).....	0.035
Channel Left Side Slope (horizontal/vertical).....	3.0
Channel Right Side Slope (horizontal/vertical).....	3.0
Channel Bottom Width (ft).....	75.0

COMPUTATION RESULTS

DESCRIPTION	VALUE
Normal Depth (ft).....	5.18
Flow Velocity (fps).....	3.58
Froude Number.....	0.3
Velocity Head (ft).....	0.2
Energy Head (ft).....	5.38
Cross-Sectional Area of Flow (sq ft).....	469.5
Top Width of Flow (ft).....	106.11

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Phone: (281)440-3787, Fax: (281)440-4742, Email: software@dodson-hydro.com
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G1-4/3

TRAPEZOIDAL CHANNEL ANALYSIS
NORMAL DEPTH COMPUTATION

July 18, 2005

DESCRIPTION	PROGRAM INPUT DATA	VALUE
Flow Rate (cfs).....		1,465.0
Channel Bottom Slope (ft/ft).....		0.001
Manning's Roughness Coefficient (n-value).....		0.035
Channel Left Side Slope (horizontal/vertical).....		3.0
Channel Right Side Slope (horizontal/vertical).....		3.0
Channel Bottom Width (ft).....		65.0

DESCRIPTION	COMPUTATION RESULTS	VALUE
Normal Depth (ft).....		5.16
Flow Velocity (fps).....		3.53
Froude Number.....		0.299
Velocity Head (ft).....		0.19
Energy Head (ft).....		5.36
Cross-Sectional Area of Flow (sq ft).....		415.56
Top Width of Flow (ft).....		95.98

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TRAPEZOIDAL CHANNEL ANALYSIS
NORMAL DEPTH COMPUTATION

July 18, 2005

PROGRAM INPUT DATA

DESCRIPTION	VALUE
Flow Rate (cfs).....	1,265.0
Channel Bottom Slope (ft/ft).....	0.001
Manning's Roughness Coefficient (n-value).....	0.035
Channel Left Side Slope (horizontal/vertical).....	3.0
Channel Right Side Slope (horizontal/vertical).....	3.0
Channel Bottom Width (ft).....	50.0

COMPUTATION RESULTS

DESCRIPTION	VALUE
Normal Depth (ft).....	5.41
Flow Velocity (fps).....	3.53
Froude Number.....	0.298
Velocity Head (ft).....	0.19
Energy Head (ft).....	5.61
Cross-Sectional Area of Flow (sq ft).....	358.7
Top Width of Flow (ft).....	82.49

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TRAPEZOIDAL CHANNEL ANALYSIS
NORMAL DEPTH COMPUTATION

July 18, 2005

PROGRAM INPUT DATA

DESCRIPTION	VALUE
Flow Rate (cfs).....	1,681.0
Channel Bottom Slope (ft/ft).....	0.0062
Manning's Roughness Coefficient (n-value).....	0.016
Channel Left Side Slope (horizontal/vertical).....	2.0
Channel Right Side Slope (horizontal/vertical).....	2.0
Channel Bottom Width (ft).....	35.0

COMPUTATION RESULTS

DESCRIPTION	VALUE
Normal Depth (ft).....	3.01
Flow Velocity (fps).....	13.63
Froude Number.....	1.484
Velocity Head (ft).....	2.89
Energy Head (ft).....	5.89
Cross-Sectional Area of Flow (sq ft).....	123.32
Top Width of Flow (ft).....	47.03

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G-2 - 2/3

TRAPEZOIDAL CHANNEL ANALYSIS
NORMAL DEPTH COMPUTATION

July 18, 2005

DESCRIPTION	PROGRAM INPUT DATA	VALUE
Flow Rate (cfs).....		1,465.0
Channel Bottom Slope (ft/ft).....		0.0062
Manning's Roughness Coefficient (n-value).....		0.016
Channel Left Side Slope (horizontal/vertical).....		2.0
Channel Right Side Slope (horizontal/vertical).....		2.0
Channel Bottom Width (ft).....		30.0

DESCRIPTION	COMPUTATION RESULTS	VALUE
Normal Depth (ft).....		3.02
Flow Velocity (fps).....		13.47
Froude Number.....		1.476
Velocity Head (ft).....		2.82
Energy Head (ft).....		5.84
Cross-Sectional Area of Flow (sq ft).....		108.79
Top Width of Flow (ft).....		42.07

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G2-3/3

TRAPEZOIDAL CHANNEL ANALYSIS
NORMAL DEPTH COMPUTATION

July 18, 2005

PROGRAM INPUT DATA

DESCRIPTION	VALUE
Flow Rate (cfs).....	1,265.0
Channel Bottom Slope (ft/ft).....	0.0062
Manning's Roughness Coefficient (n-value).....	0.016
Channel Left Side Slope (horizontal/vertical).....	2.0
Channel Right Side Slope (horizontal/vertical).....	2.0
Channel Bottom Width (ft).....	25.0

COMPUTATION RESULTS

DESCRIPTION	VALUE
Normal Depth (ft).....	3.05
Flow Velocity (fps).....	13.31
Froude Number.....	1.469
Velocity Head (ft).....	2.75
Energy Head (ft).....	5.81
Cross-Sectional Area of Flow (sq ft).....	95.02
Top Width of Flow (ft).....	37.22

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TRAPEZOIDAL CHANNEL ANALYSIS
NORMAL DEPTH COMPUTATION

July 18, 2005

PROGRAM INPUT DATA

DESCRIPTION	VALUE
Flow Rate (cfs).....	170.0
Channel Bottom Slope (ft/ft).....	0.0066
Manning's Roughness Coefficient (n-value).....	0.015
Channel Left Side Slope (horizontal/vertical).....	2.0
Channel Right Side Slope (horizontal/vertical).....	2.0
Channel Bottom Width (ft).....	4.0

COMPUTATION RESULTS

DESCRIPTION	VALUE
Normal Depth (ft).....	2.14
Flow Velocity (fps).....	9.62
Froude Number.....	1.428
Velocity Head (ft).....	1.44
Energy Head (ft).....	3.57
Cross-Sectional Area of Flow (sq ft).....	17.68
Top Width of Flow (ft).....	12.55

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TRAPEZOIDAL CHANNEL ANALYSIS
NORMAL DEPTH COMPUTATION

July 18, 2005

PROGRAM INPUT DATA

DESCRIPTION	VALUE
Flow Rate (cfs).....	300.0
Channel Bottom Slope (ft/ft).....	0.0021
Manning's Roughness Coefficient (n-value).....	0.035
Channel Left Side Slope (horizontal/vertical).....	3.0
Channel Right Side Slope (horizontal/vertical).....	3.0
Channel Bottom Width (ft).....	25.0

COMPUTATION RESULTS

DESCRIPTION	VALUE
Normal Depth (ft).....	2.77
Flow Velocity (fps).....	3.26
Froude Number.....	0.386
Velocity Head (ft).....	0.16
Energy Head (ft).....	2.93
Cross-Sectional Area of Flow (sq ft).....	92.09
Top Width of Flow (ft).....	41.59

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BOX CULVERT ANALYSIS
COMPUTATION OF CULVERT PERFORMANCE CURVE

July 18, 2005

PROGRAM INPUT DATA

DESCRIPTION	VALUE
Culvert Span (ft)	10.0
Culvert Rise (ft)	5.0
FHWA Chart Number	8
FHWA Scale Number (Type of Culvert Entrance)	2
Manning's Roughness Coefficient (n-value)	0.013
Entrance Loss Coefficient of Culvert Opening	0.5
Culvert Length (ft)	60.0
Invert Elevation at Downstream end of Culvert (ft)	1,478.0
Invert Elevation at Upstream end of Culvert (ft)	1,478.2
Culvert Slope (ft/ft)	0.0033
Starting Flow Rate (cfs)	200.0
Incremental Flow Rate (cfs)	25.0
Ending Flow Rate (cfs)	350.0
Starting Tailwater Depth (ft)	1,480.0
Incremental Tailwater Depth (ft)	0.25
Ending Tailwater Depth (ft)	1,481.5

COMPUTATION RESULTS

Flow Rate (cfs)	Tailwater Depth (ft)	Headwater (ft)		Normal Depth (ft)	Critical Depth (ft)	Depth at Outlet (ft)	Outlet Velocity (fps)
		Inlet Control	Outlet Control				
200.0	1,480.0	3.94	1,480.21	2.26	2.32	5.0	4.0
225.0	1,480.25	4.27	1,480.57	2.45	2.51	5.0	4.5
250.0	1,480.5	4.58	1,480.94	2.63	2.69	5.0	5.0
275.0	1,480.75	4.89	1,481.33	2.82	2.86	5.0	5.5
300.0	1,481.0	5.18	1,481.72	2.99	3.04	5.0	6.0
325.0	1,481.25	5.47	1,482.13	3.17	3.2	5.0	6.5
350.0	1,481.5	5.76	1,482.56	3.34	3.36	5.0	7.0

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Design $Q_{100} = 1050$ cfs
 Use 3-10'x5' RCBC
 $Q/\text{barrel} = 1050/3 = 350$ cfs

BOX CULVERT ANALYSIS
COMPUTATION OF CULVERT PERFORMANCE CURVE

July 19, 2005

PROGRAM INPUT DATA

DESCRIPTION	VALUE
Culvert Span (ft)	10.0
Culvert Rise (ft)	6.0
FHWA Chart Number	8
FHWA Scale Number (Type of Culvert Entrance)	2
Manning's Roughness Coefficient (n-value)	0.013
Entrance Loss Coefficient of Culvert Opening	0.5
Culvert Length (ft)	130.0
Invert Elevation at Downstream end of Culvert (ft)	1,471.8
Invert Elevation at Upstream end of Culvert (ft)	1,472.0
Culvert Slope (ft/ft)	0.0015
Starting Flow Rate (cfs)	500.0
Incremental Flow Rate (cfs)	25.0
Ending Flow Rate (cfs)	750.0
Starting Tailwater Depth (ft)	1,473.0
Incremental Tailwater Depth (ft)	0.25
Ending Tailwater Depth (ft)	1,475.5

COMPUTATION RESULTS

Flow Rate (cfs)	Tailwater Depth (ft)	Headwater Inlet Control (ft)	Headwater Outlet Control (ft)	Normal Depth (ft)	Critical Depth (ft)	Depth at Outlet (ft)	Outlet Velocity (fps)
500.0	1,473.0	7.31	1,474.72	5.78	4.27	6.0	8.33
525.0	1,473.25	7.7	1,475.16	6.0	4.41	6.0	8.75
550.0	1,473.5	8.18	1,475.62	6.0	4.55	6.0	9.17
575.0	1,473.75	8.52	1,476.09	6.0	4.68	6.0	9.58
600.0	1,474.0	8.8	1,476.56	6.0	4.82	6.0	10.0
625.0	1,474.25	9.14	1,477.05	6.0	4.95	6.0	10.42
650.0	1,474.5	9.49	1,477.54	6.0	5.08	6.0	10.83
675.0	1,474.75	9.86	1,478.04	6.0	5.21	6.0	11.25
700.0	1,475.0	10.24	1,478.56	6.0	5.34	6.0	11.67
725.0	1,475.25	10.64	1,479.08	6.0	5.47	6.0	12.08
750.0	1,475.5	11.05	1,479.61	6.0	5.59	6.0	12.5

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BOX CULVERT ANALYSIS
COMPUTATION OF CULVERT PERFORMANCE CURVE

June 17, 2005

PROGRAM INPUT DATA

DESCRIPTION	VALUE
Culvert Span (ft)	10.0
Culvert Rise (ft)	4.0
FHWA Chart Number	8
FHWA Scale Number (Type of Culvert Entrance)	2
Manning's Roughness Coefficient (n-value)	0.013
Entrance Loss Coefficient of Culvert Opening	0.5
Culvert Length (ft)	60.0
Invert Elevation at Downstream end of Culvert (ft)	1,477.7
Invert Elevation at Upstream end of Culvert (ft)	1,477.8
Culvert Slope (ft/ft)	0.0017
Starting Flow Rate (cfs)	250.0
Incremental Flow Rate (cfs)	20.0
Ending Flow Rate (cfs)	370.0
Starting Tailwater Depth (ft)	1,479.0
Incremental Tailwater Depth (ft)	0.25
Ending Tailwater Depth (ft)	1,480.5

COMPUTATION RESULTS

Flow Rate (cfs)	Tailwater Depth (ft)	Headwater Inlet Control (ft)	Headwater Outlet Control (ft)	Normal Depth (ft)	Critical Depth (ft)	Depth at Outlet (ft)	Outlet Velocity (fps)
250.0	1,479.0	4.6	1,479.92	3.36	2.69	4.0	6.25
270.0	1,479.25	4.85	1,480.34	3.55	2.83	4.0	6.75
290.0	1,479.5	5.24	1,480.78	3.74	2.97	4.0	7.25
310.0	1,479.75	5.64	1,481.22	3.93	3.1	4.0	7.75
330.0	1,480.0	5.92	1,481.68	4.0	3.23	4.0	8.25
350.0	1,480.25	6.26	1,482.15	4.0	3.36	4.0	8.75
370.0	1,480.5	6.62	1,482.64	4.0	3.49	4.0	9.25

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BOX CULVERT ANALYSIS
COMPUTATION OF CULVERT PERFORMANCE CURVE

July 18, 2005

PROGRAM INPUT DATA

DESCRIPTION	VALUE
Culvert Span (ft)	10.0
Culvert Rise (ft)	4.0
FHWA Chart Number	8
FHWA Scale Number (Type of Culvert Entrance)	2
Manning's Roughness Coefficient (n-value)	0.013
Entrance Loss Coefficient of Culvert Opening	0.5
Culvert Length (ft)	60.0
Invert Elevation at Downstream end of Culvert (ft)	1,471.0
Invert Elevation at Upstream end of Culvert (ft)	1,471.05
Culvert Slope (ft/ft)	0.0008
Starting Flow Rate (cfs)	380.0
Incremental Flow Rate (cfs)	10.0
Ending Flow Rate (cfs)	440.0
Starting Tailwater Depth (ft)	1,473.0
Incremental Tailwater Depth (ft)	0.25
Ending Tailwater Depth (ft)	1,474.5

COMPUTATION RESULTS

Flow Rate (cfs)	Tailwater Depth (ft)	Headwater Inlet Control (ft)	Headwater Outlet Control (ft)	Normal Depth (ft)	Critical Depth (ft)	Depth at Outlet (ft)	Outlet Velocity (fps)
380.0	1,473.0	6.81	1,475.31	4.0	3.55	4.0	9.5
390.0	1,473.25	7.0	1,475.69	4.0	3.62	4.0	9.75
400.0	1,473.5	7.2	1,476.07	4.0	3.68	4.0	10.0
410.0	1,473.75	7.4	1,476.45	4.0	3.74	4.0	10.25
420.0	1,474.0	7.61	1,476.83	4.0	3.8	4.0	10.5
430.0	1,474.25	7.82	1,477.22	4.0	3.86	4.0	10.75
440.0	1,474.5	8.04	1,477.62	4.0	3.92	4.0	11.0

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BOX CULVERT ANALYSIS
COMPUTATION OF CULVERT PERFORMANCE CURVE

July 19, 2005

PROGRAM INPUT DATA

DESCRIPTION	VALUE
Culvert Span (ft)	10.0
Culvert Rise (ft)	6.0
FHWA Chart Number	8
FHWA Scale Number (Type of Culvert Entrance)	2
Manning's Roughness Coefficient (n-value)	0.013
Entrance Loss Coefficient of Culvert Opening	0.5
Culvert Length (ft)	60.0
Invert Elevation at Downstream end of Culvert (ft)	1,465.6
Invert Elevation at Upstream end of Culvert (ft)	1,465.8
Culvert Slope (ft/ft)	0.0033
Starting Flow Rate (cfs)	500.0
Incremental Flow Rate (cfs)	25.0
Ending Flow Rate (cfs)	750.0
Starting Tailwater Depth (ft)	1,468.0
Incremental Tailwater Depth (ft)	0.25
Ending Tailwater Depth (ft)	1,470.5

COMPUTATION RESULTS

Flow Rate (cfs)	Tailwater Depth (ft)	Headwater Inlet Control (ft)	Headwater Outlet Control (ft)	Normal Depth (ft)	Critical Depth (ft)	Depth at Outlet (ft)	Outlet Velocity (fps)
500.0	1,468.0	7.31	1,469.56	4.32	4.27	6.0	8.33
525.0	1,468.25	7.7	1,469.99	4.48	4.41	6.0	8.75
550.0	1,468.5	8.17	1,470.43	4.64	4.55	6.0	9.17
575.0	1,468.75	8.51	1,470.87	4.8	4.68	6.0	9.58
600.0	1,469.0	8.79	1,471.33	4.95	4.82	6.0	10.0
625.0	1,469.25	9.13	1,471.79	5.11	4.95	6.0	10.42
650.0	1,469.5	9.48	1,472.27	5.26	5.08	6.0	10.83
675.0	1,469.75	9.85	1,472.75	5.41	5.21	6.0	11.25
700.0	1,470.0	10.23	1,473.24	5.56	5.34	6.0	11.67
725.0	1,470.25	10.63	1,473.74	5.71	5.47	6.0	12.08
750.0	1,470.5	11.04	1,474.25	5.86	5.59	6.0	12.5

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BOX CULVERT ANALYSIS
COMPUTATION OF CULVERT PERFORMANCE CURVE

July 18, 2005

PROGRAM INPUT DATA

DESCRIPTION	VALUE
Culvert Span (ft).....	10.0
Culvert Rise (ft).....	4.0
FHWA Chart Number.....	8
FHWA Scale Number (Type of Culvert Entrance).....	2
Manning's Roughness Coefficient (n-value).....	0.013
Entrance Loss Coefficient of Culvert Opening.....	0.5
Culvert Length (ft).....	60.0
Invert Elevation at Downstream end of Culvert (ft).....	1,464.5
Invert Elevation at Upstream end of Culvert (ft).....	1,465.0
Culvert Slope (ft/ft).....	0.0083
Starting Flow Rate (cfs).....	380.0
Incremental Flow Rate (cfs).....	10.0
Ending Flow Rate (cfs).....	440.0
Starting Tailwater Depth (ft).....	1,467.0
Incremental Tailwater Depth (ft).....	0.25
Ending Tailwater Depth (ft).....	1,468.5

COMPUTATION RESULTS

Flow Rate (cfs)	Tailwater Depth (ft)	Headwater Inlet Control (ft)	Headwater Outlet Control (ft)	Normal Depth (ft)	Critical Depth (ft)	Depth at Outlet (ft)	Outlet Velocity (fps)
380.0	1,467.0	6.79	1,468.86	2.56	3.55	4.0	9.5
390.0	1,467.25	6.99	1,469.24	2.61	3.62	4.0	9.75
400.0	1,467.5	7.18	1,469.62	2.66	3.68	4.0	10.0
410.0	1,467.75	7.39	1,470.0	2.7	3.74	4.0	10.25
420.0	1,468.0	7.59	1,470.38	2.75	3.8	4.0	10.5
430.0	1,468.25	7.81	1,470.77	2.79	3.86	4.0	10.75
440.0	1,468.5	8.02	1,471.17	2.84	3.92	4.0	11.0

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PROJECT Siphon Draw No. _____
 CALC'D BY ACP CHK'D BY _____
 SHEET S-1 OF _____ DATE 6/05

STORM DRAIN ESTIMATESBleedoff Pipes:between SRP corridor (T₁ & T₂) basins

These basins function as basins in series in dynamic condition so head would be constantly changing. This condition was not evaluated due to the scope limitations.

Maximum storage is provided in Basin T₁ = 16 Acre-ft

Capacity required to drain the basin in 26 hour = $\frac{16 \times 43560}{36 \times 60 \times 60} = 5.38$ cfs

This capacity is feasible per S-1 1/2

Outlet from Basin C-2,

30" pipe used with rating table in HEC-1, provide larger size to evacuate the basin in shorter duration

Outlet from Basin B-1,

36" pipe used with rating table in HEC-1, — do —

PIPE CULVERT ANALYSIS
COMPUTATION OF CULVERT PERFORMANCE CURVE

July 19, 2005

PROGRAM INPUT DATA

DESCRIPTION	VALUE
Culvert Diameter (ft)	1.5
FHWA Chart Number	1
FHWA Scale Number (Type of Culvert Entrance)	2
Manning's Roughness Coefficient (n-value)	0.013
Entrance Loss Coefficient of Culvert Opening	0.5
Culvert Length (ft)	110.0
Invert Elevation at Downstream end of Culvert (ft)	1,482.0
Invert Elevation at Upstream end of Culvert (ft)	1,484.0
Culvert Slope (ft/ft)	0.0182
Starting Flow Rate (cfs)	2.0
Incremental Flow Rate (cfs)	1.0
Ending Flow Rate (cfs)	14.0
Starting Tailwater Depth (ft)	1.0
Incremental Tailwater Depth (ft)	0.5
Ending Tailwater Depth (ft)	7.0

COMPUTATION RESULTS

Flow Rate (cfs)	Tailwater Depth (ft)	Headwater Inlet Control (ft)	Headwater Outlet Control (ft)	Normal Depth (ft)	Critical Depth (ft)	Depth at Outlet (ft)	Outlet Velocity (fps)
2.0	1.0	0.73	0.0	0.38	0.53	0.38	5.65
3.0	1.5	0.92	-0.34	0.47	0.66	0.47	6.35
4.0	2.0	1.09	0.28	0.54	0.77	0.54	6.9
5.0	2.5	1.26	0.94	0.61	0.86	0.61	7.33
6.0	3.0	1.43	1.63	0.68	0.95	1.5	3.4
7.0	3.5	1.59	2.35	0.75	1.02	1.5	3.96
8.0	4.0	1.77	3.12	0.81	1.1	1.5	4.53
9.0	4.5	1.85	3.91	0.87	1.16	1.5	5.09
10.0	5.0	2.03	4.74	0.93	1.22	1.5	5.66
11.0	5.5	2.23	5.61	0.99	1.27	1.5	6.22
12.0	6.0	2.44	6.51	1.06	1.31	1.5	6.79
13.0	6.5	2.68	7.45	1.13	1.35	1.5	7.36
14.0	7.0	2.93	8.42	1.21	1.38	1.5	7.92

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CIRCULAR CHANNEL ANALYSIS
 RATING CURVE COMPUTATION

July 18, 2005

PROGRAM INPUT DATA

DESCRIPTION	VALUE
Channel Bottom Slope (ft/ft).....	0.0021
Manning's Roughness Coefficient (n-value).....	0.013
Channel Diameter (ft).....	4.5
Minimum Flow Depth (ft).....	3.0
Maximum Flow Depth (ft).....	4.5
Incremental Head (ft).....	0.25

COMPUTATION RESULTS

Flow Depth (ft)	Flow Rate (cfs)	Flow Velocity (fps)	Froude Number	Velocity Head (ft)	Energy Head (ft)	Flow Area (sq ft)	Top Width (ft)
3.0	70.64	6.27	0.679	0.611	3.611	11.26	4.24
3.25	78.52	6.38	0.644	0.633	3.883	12.3	4.03
3.5	85.57	6.45	0.603	0.646	4.146	13.27	3.74
3.75	91.41	6.45	0.554	0.647	4.397	14.16	3.35
4.0	95.5	6.39	0.49	0.635	4.635	14.94	2.83
4.25	96.9	6.23	0.4	0.603	4.853	15.56	2.06
4.5	0.0	0.0	0.0	0.0	0.0	0.0	0.0

COMPUTATION NOTES

*** Rating Curve terminated at flow depth = 4.50
 Flow depth equals or exceeds channel diameter (4.50)

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CIRCULAR CHANNEL ANALYSIS
RATING CURVE COMPUTATION

July 18, 2005

PROGRAM INPUT DATA

DESCRIPTION	VALUE
Channel Bottom Slope (ft/ft)	0.0073
Manning's Roughness Coefficient (n-value)	0.013
Channel Diameter (ft)	4.5
Minimum Flow Depth (ft)	3.0
Maximum Flow Depth (ft)	4.5
Incremental Head (ft)	0.25

COMPUTATION RESULTS

Flow Depth (ft)	Flow Rate (cfs)	Flow Velocity (fps)	Froude Number	Velocity Head (ft)	Energy Head (ft)	Flow Area (sq ft)	Top Width (ft)
3.0	131.7	11.69	1.265	2.124	5.124	11.26	4.24
3.25	146.39	11.9	1.201	2.201	5.451	12.3	4.03
3.5	159.55	12.02	1.125	2.245	5.745	13.27	3.74
3.75	170.44	12.03	1.033	2.251	6.001	14.16	3.35
4.0	178.06	11.92	0.914	2.208	6.208	14.94	2.83
4.25	180.67	11.61	0.745	2.096	6.346	15.56	2.06
4.5	0.0	0.0	0.0	0.0	0.0	0.0	0.0

COMPUTATION NOTES

*** Rating Curve terminated at flow depth = 4.50
Flow depth equals or exceeds channel diameter (4.50)

HYDROCALC Hydraulics for Windows, Version 1.2a Copyright (c) 1996
Dodson & Associates, Inc., 5629 FM 1960 West, Suite 314, Houston, TX 77069
Phone: (281)440-3787, Fax: (281)440-4742, Email: software@dodson-hydro.com
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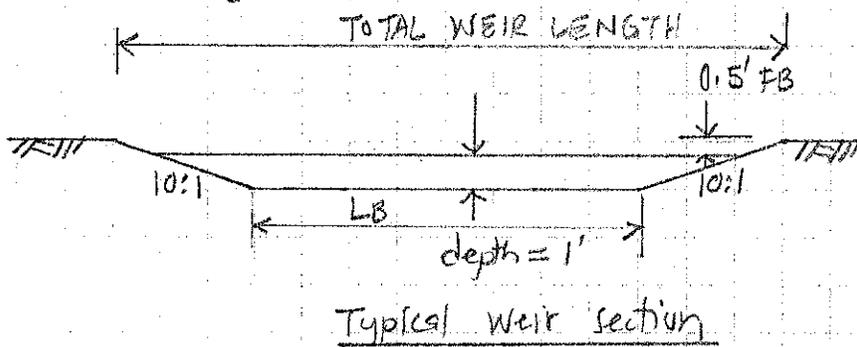
WOOD/PATEL

CIVIL ENGINEERS • HYDROLOGISTS • LAND SURVEYORS • CONSTRUCTION MANAGERS

PROJECT Siphon Draw No. _____
 CALC'D BY ACP CHK'D BY _____
 SHEET W-1 OF _____ DATE 6/05

Side weirs, SRP Corridor

Estimate Weir Length based on a simplified approach to approximate spillover structure sizes.



$$\text{Spillway Capacity} = C L H^{3/2}$$

$$Q = C (L_B + 1 \times 10) \times 1^{3/2} \quad \text{Where } C = 3$$

↑
approx.

$$Q = 3 L_B + 30$$

$$\therefore L_B = \frac{Q - 30}{3} = \frac{1}{3} (Q - 10)$$

Approximated weir lengths:

Basin	Design Q ₁₀₀	L _B	Total weir Length
T ₁	216	69'	99'
T ₂	200	63	93'
T ₃	100	30	60'

WOOD/PATEL

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PROJECT Siphon Draw No. _____
CALC'D BY ACP CHK'D BY _____
SHEET _____ OF _____ DATE 7/05

Sediment Basin:

Contributing Drainage Area = 3.09 sq. mi

Ave annual sediment production = 0.25 to 0.5 Acft/sq. mi/year

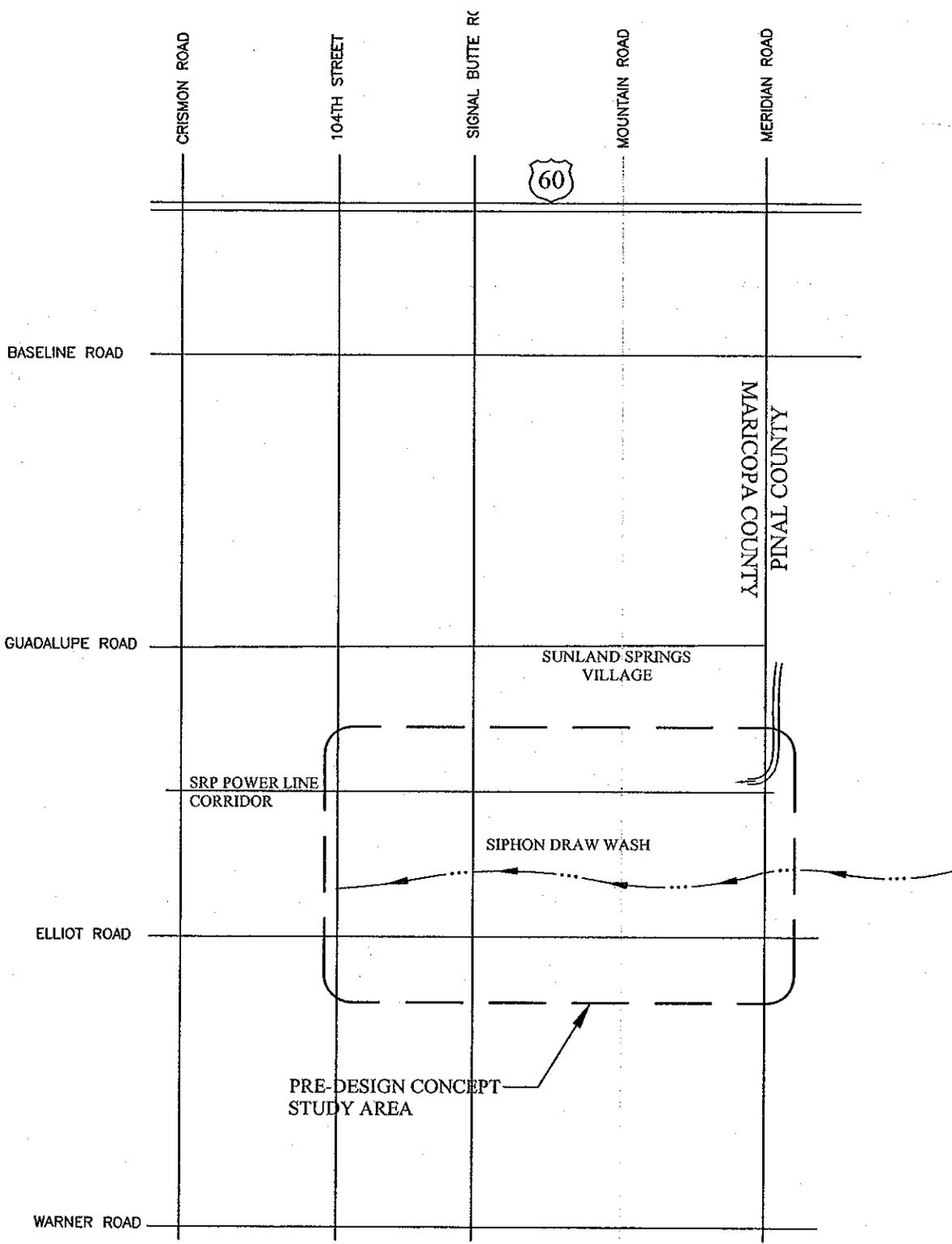
∴ sediment yield/yr = 0.375(ave) × 3.09 = 1.16 Acft

Volume provided = 4 Acft, cleaning frequency would vary -

On an average need cleaning $4/1.16 = 3.45$ ^{every} yr.

PLATE 1

Project Location and Vicinity Map



N.T.S.

**SIPHON DRAW DRAINAGE IMPROVEMENTS
PROJECT LOCATION AND VICINITY MAP**

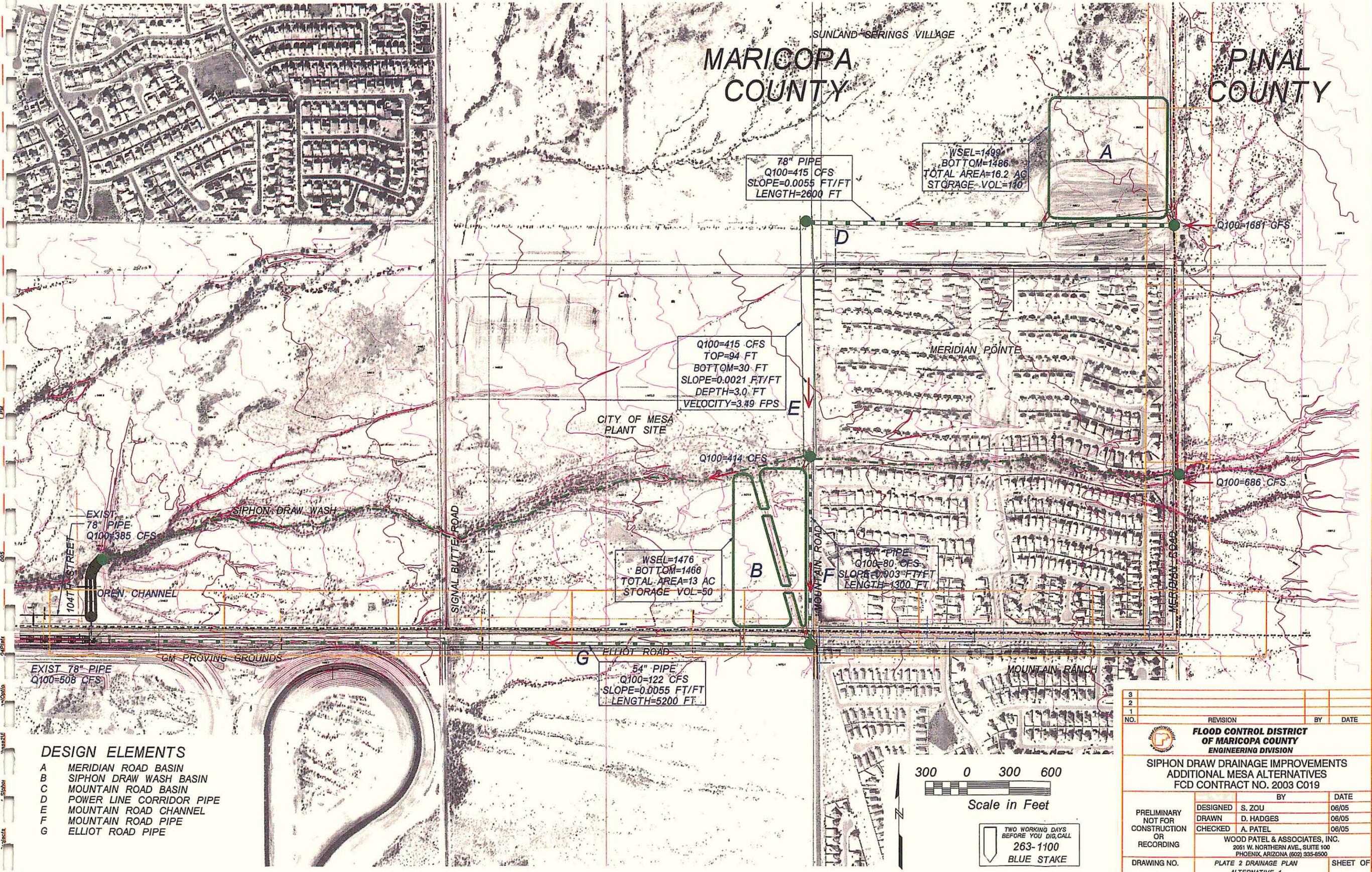
Wood, Patel & Associates, Inc.
2051 West Northern, Suite 100
Phoenix, Arizona 85021 (602) 335-8500

DRAWN BY: PAL
JOB NO: 031902.04 DATE: 6/05

PLATE 1

PLATE 2

Drainage Plan – Alternative 1



- DESIGN ELEMENTS**
- A MERIDIAN ROAD BASIN
 - B SIPHON DRAW WASH BASIN
 - C MOUNTAIN ROAD BASIN
 - D POWER LINE CORRIDOR PIPE
 - E MOUNTAIN ROAD CHANNEL
 - F MOUNTAIN ROAD PIPE
 - G ELLIOT ROAD PIPE

3			
2			
1			
NO.	REVISION	BY	DATE
<p>FLOOD CONTROL DISTRICT OF MARICOPA COUNTY ENGINEERING DIVISION</p>			
<p>SIPHON DRAW DRAINAGE IMPROVEMENTS ADDITIONAL MESA ALTERNATIVES FCD CONTRACT NO. 2003 C019</p>			
		BY	DATE
PRELIMINARY NOT FOR CONSTRUCTION OR RECORDING	DESIGNED	S. ZOU	06/05
	DRAWN	D. HADGES	06/05
	CHECKED	A. PATEL	06/05
	<p>WOOD PATEL & ASSOCIATES, INC. 2051 W. NORTHERN AVE., SUITE 100 PHOENIX, ARIZONA (602) 335-6500</p>		
DRAWING NO.	PLATE 2 DRAINAGE PLAN ALTERNATIVE 1		SHEET OF

PLATE 3

Drainage Plan – Alternative 2

MARICOPA COUNTY

PINAL COUNTY

SUNLAND SPRINGS VILLAGE

WSEL=1499
BOTTOM=1490
TOTAL AREA=13.0
STORAGE VOL=65

Q100=808 CFS
TOP=94 FT
BOTTOM=30 FT
SLOPE=0.0012 FT/FT
DEPTH=5.1 FT
VELOCITY=3.50 FPS

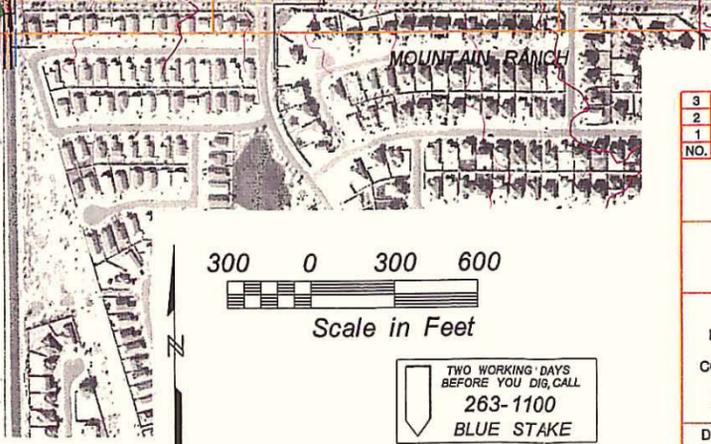
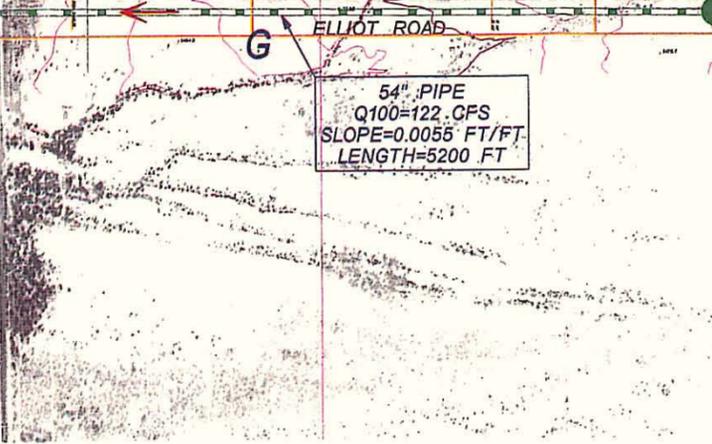
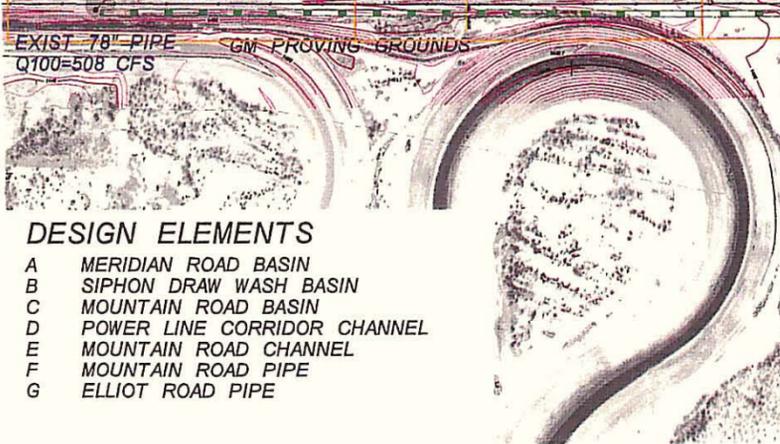
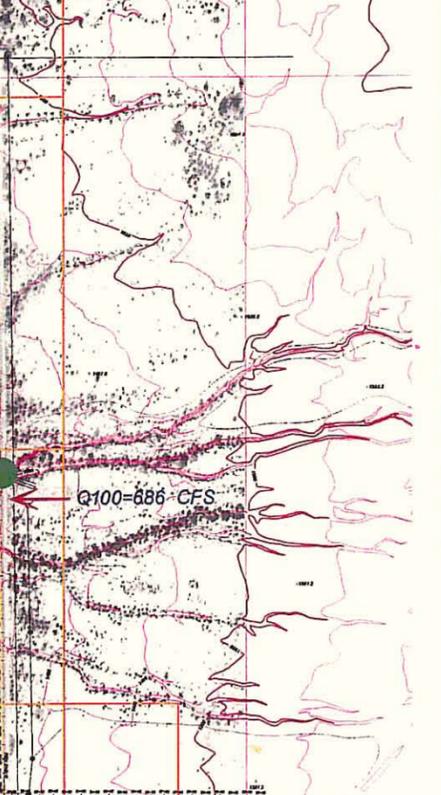
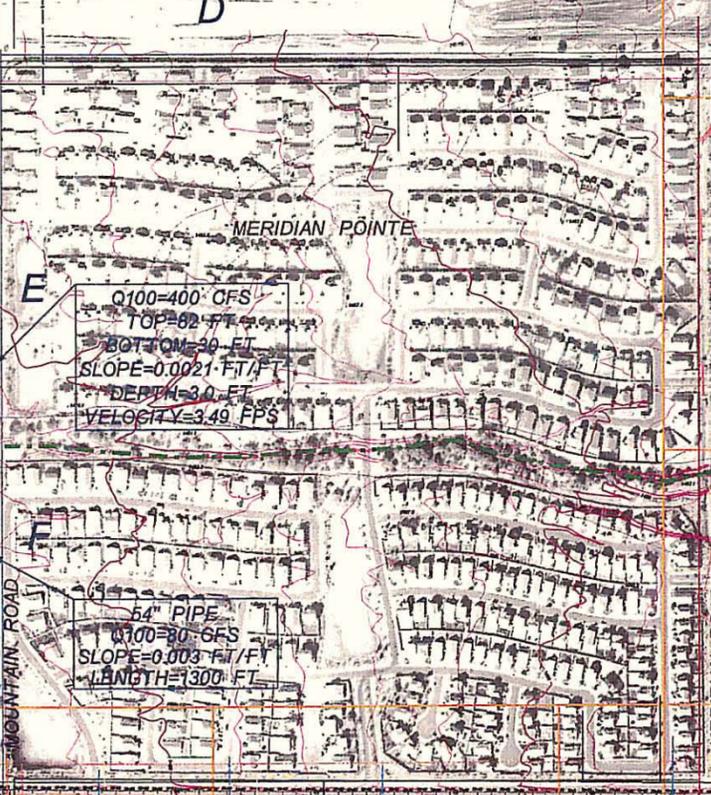
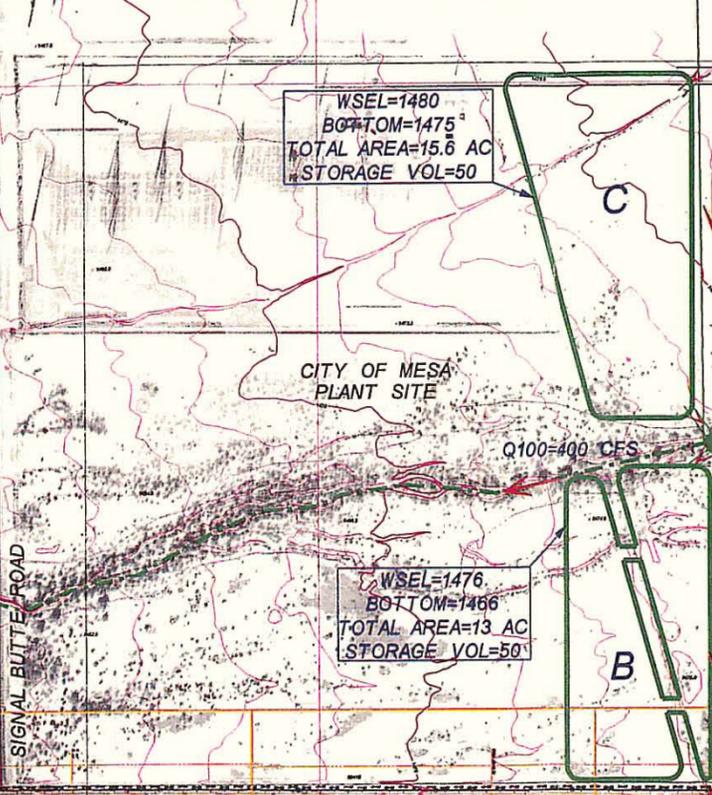
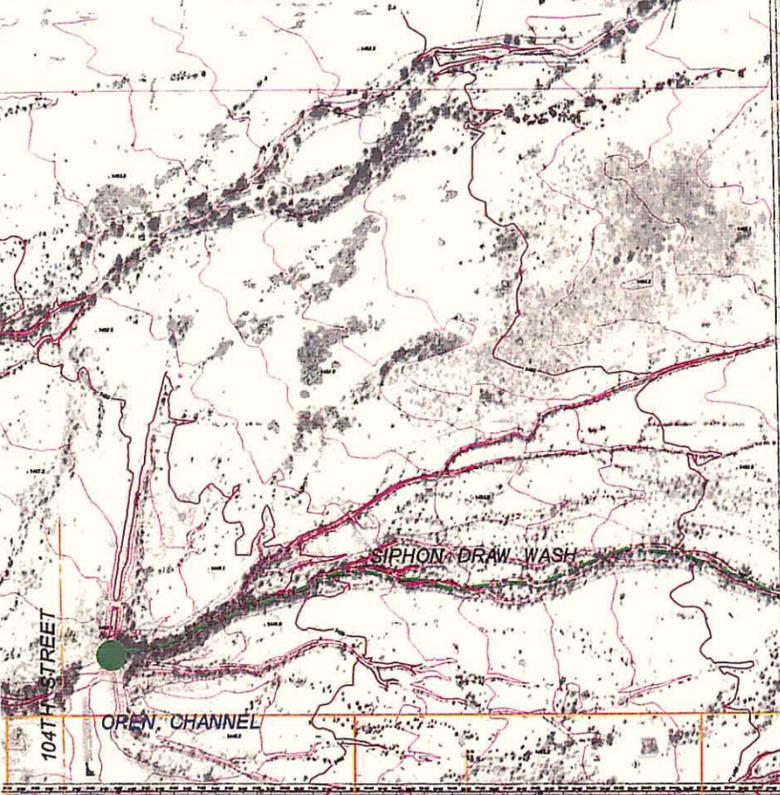
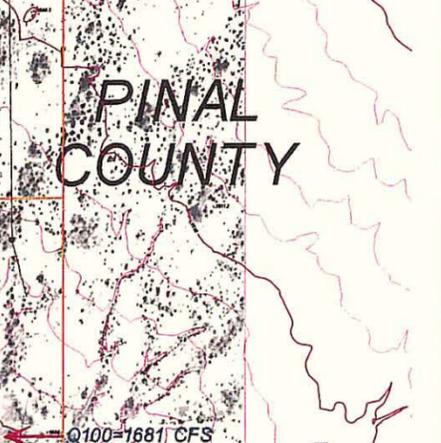
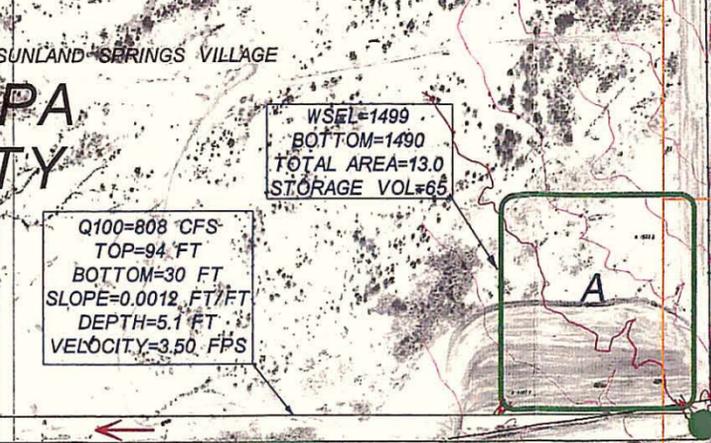
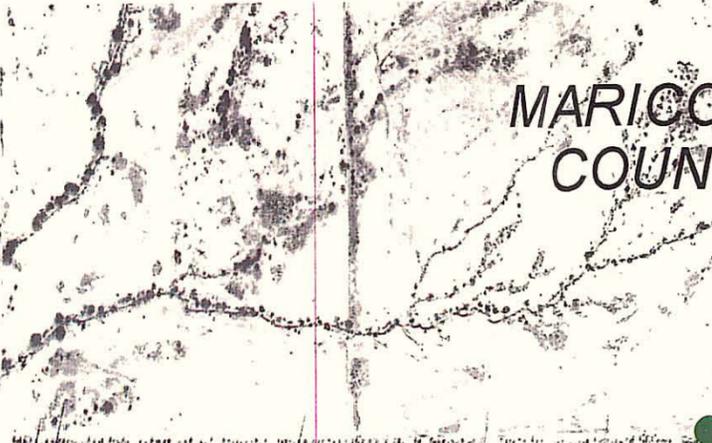
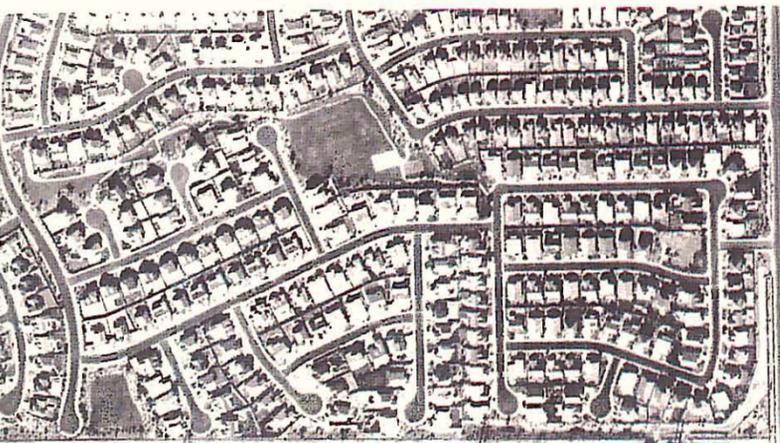
WSEL=1480
BOTTOM=1475
TOTAL AREA=15.6 AC
STORAGE VOL=50

Q100=400 CFS
TOP=82 FT
BOTTOM=30 FT
SLOPE=0.0021 FT/FT
DEPTH=30 FT
VELOCITY=3.49 FPS

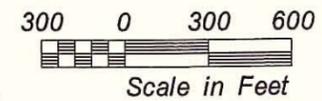
WSEL=1476
BOTTOM=1466
TOTAL AREA=13 AC
STORAGE VOL=50

54" PIPE
Q100=30 CFS
SLOPE=0.003 FT/FT
LENGTH=1300 FT

54" PIPE
Q100=122 CFS
SLOPE=0.0055 FT/FT
LENGTH=5200 FT



- DESIGN ELEMENTS**
- A MERIDIAN ROAD BASIN
 - B SIPHON DRAW WASH BASIN
 - C MOUNTAIN ROAD BASIN
 - D POWER LINE CORRIDOR CHANNEL
 - E MOUNTAIN ROAD CHANNEL
 - F MOUNTAIN ROAD PIPE
 - G ELLIOT ROAD PIPE



TWO WORKING DAYS
BEFORE YOU DIG, CALL
263-1100
BLUE STAKE

NO.	REVISION	BY	DATE
3			
2			
1			

FLOOD CONTROL DISTRICT OF MARICOPA COUNTY ENGINEERING DIVISION

SIPHON DRAW DRAINAGE IMPROVEMENTS
ADDITIONAL MESA ALTERNATIVES
FCD CONTRACT NO. 2003 C019

	BY	DATE
DESIGNED	S. ZOU	06/05
DRAWN	D. HADGES	06/05
CHECKED	A. PATEL	06/05

WOOD PATEL & ASSOCIATES, INC.
2051 W. NORTHERN AVE., SUITE 100
PHOENIX, ARIZONA (602) 335-8500

DRAWING NO. PLATE 3 DRAINAGE PLAN ALTERNATIVE 2 SHEET OF

PLATE 4

Drainage Plan – Alternative 3

MARICOPA COUNTY

PINAL COUNTY

SUNLAND SPRINGS VILLAGE

(CONCRETE)
 Q100=1681 CFS
 TOP=72 FT
 BOTTOM=20 FT
 SLOPE=0.0055 FT/FT
 DEPTH=6.8 FT
 VELOCITY=8.29 FPS

(EARTH)
 Q100=1681 CFS
 TOP=149
 BOTTOM=85 FT
 SLOPE=0.001 FT/FT
 DEPTH=4.90 FT
 VELOCITY=3.50 FPS

SEDIMENT BASIN
 AREA=1.1 AC

WSEL=1480
 BOTTOM=1470
 TOTAL AREA=16.5 AC
 STORAGE VOL=90 AC-FT

CITY OF MESA
 PLANT SITE

WSEL=1476
 BOTTOM=1466
 TOTAL AREA=13 AC
 STORAGE VOL=75 AC-FT

Q100=300 CFS
 TOP=82 FT
 BOTTOM=30 FT
 SLOPE=0.0021 FT/FT
 DEPTH=3.0 FT
 VELOCITY=3.79 FPS

54" PIPE
 Q100=100 CFS
 SLOPE=0.003 FT/FT
 LENGTH=1800 FT

54" PIPE
 Q100=149 CFS
 SLOPE=0.0055 FT/FT
 LENGTH=5200 FT

EXIST 78" PIPE

OPEN CHANNEL

EXIST 78" PIPE
 Q100=508 CFS

- DESIGN ELEMENTS**
- A MERIDIAN ROAD BASIN
 - B SIPHON DRAW WASH BASIN
 - C MOUNTAIN ROAD BASIN
 - D POWER LINE CORRIDOR CHANNEL
 - E MOUNTAIN ROAD CHANNEL
 - F MOUNTAIN ROAD PIPE
 - G ELLIOT ROAD PIPE

300 0 300 600

Scale in Feet

TWO WORKING DAYS
 BEFORE YOU DIG, CALL
 263-1100
 BLUE STAKE

3			
2			
1			
NO.	REVISION	BY	DATE

**FLOOD CONTROL DISTRICT
 OF MARICOPA COUNTY
 ENGINEERING DIVISION**

SIPHON DRAW DRAINAGE IMPROVEMENTS
 ADDITIONAL MESA ALTERNATIVES
 FCD CONTRACT NO. 2003 C019

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	DRAWN	D. HADGES	06/05
	CHECKED	A. PATEL	06/05
DRAWING NO.		PLATE 4 DRAINAGE PLAN ALTERNATIVE 3	SHEET OF

WOOD PATEL & ASSOCIATES, INC.
 2951 W. NORTHERN AVE., SUITE 100
 PHOENIX, ARIZONA (602) 335-8500

PLATE 5

Drainage Plan – Base Alternative

SUNLAND SPRINGS VILLAGE

MARICOPA COUNTY

PINAL COUNTY

(EARTH)
 Q100=1681 CFS
 TOP=149 FT
 BOTTOM=85 FT
 SLOPE=0.001 FT/FT
 DEPTH=4.9 FT
 VELOCITY=3.50 FPS

(CONCRETE)
 Q100=1681 CFS
 TOP=72 FT
 BOTTOM=20 FT
 SLOPE=0.0055 FT/FT
 DEPTH=6.3 FT
 VELOCITY=8.29 FPS

(EARTH)
 Q100=1681 CFS
 TOP=149 FT
 BOTTOM=85 FT
 SLOPE=0.001 FT/FT
 DEPTH=4.9 FT
 VELOCITY=3.50 FPS

(CONCRETE)
 Q100=1681 CFS
 TOP=72 FT
 BOTTOM=20 FT
 SLOPE=0.0055 FT/FT
 DEPTH=6.3 FT
 VELOCITY=8.29 FPS

Q100=1681 CFS

H

D

CITY OF MESA
 PLANT SITE

MERIDIAN POINTE

Q100=686 CFS

EXIST 78" PIPE
 Q100=385 CFS

OPEN CHANNEL

SIPHON DRAW WASH

SIGNAL BUTTE ROAD

MOUNTAIN ROAD

MERIDIAN ROAD

ELLIOT ROAD

EXIST 78" PIPE
 Q100=508 CFS

GM PROVING GROUNDS

MOUNTAIN RANCH

DESIGN ELEMENTS

- A MERIDIAN ROAD BASIN
- B SIPHON DRAW WASH BASIN
- C MOUNTAIN ROAD BASIN
- D POWER LINE CORRIDOR PIPE
- E MOUNTAIN ROAD CHANNEL
- F MOUNTAIN ROAD PIPE
- G ELLIOT ROAD PIPE

300 0 300 600

Scale in Feet

TWO WORKING DAYS
 BEFORE YOU DIG, CALL
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 BLUE STAKE

3			
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1			
NO.	REVISION	BY	DATE

**FLOOD CONTROL DISTRICT
 OF MARICOPA COUNTY
 ENGINEERING DIVISION**

SIPHON DRAW DRAINAGE IMPROVEMENTS
 ADDITIONAL MESA ALTERNATIVES
 FCD CONTRACT NO. 2003 C019

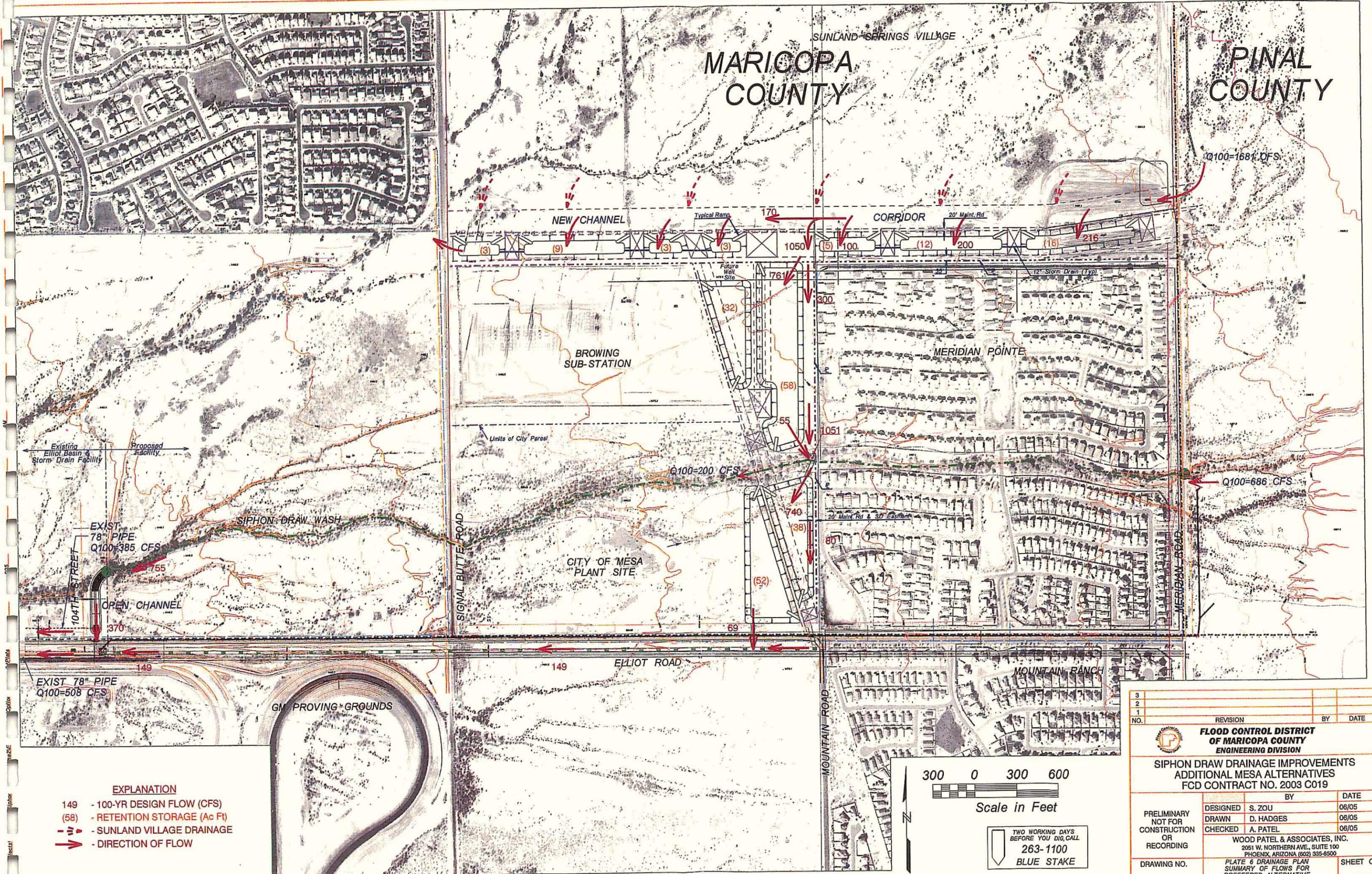
	DESIGNED	BY	DATE
PRELIMINARY NOT FOR CONSTRUCTION OR RECORDING	S. ZOU	S. ZOU	06/05
	D. HADGES	D. HADGES	06/05
	A. PATEL	A. PATEL	06/05
DRAWING NO.	WOOD PATEL & ASSOCIATES, INC. 2051 W. NORTHERN AVE., SUITE 100 PHOENIX, ARIZONA (602) 335-8500		
	PLATE 5 DRAINAGE SYSTEM BASE ALTERNATIVE		SHEET OF

PLATE 6

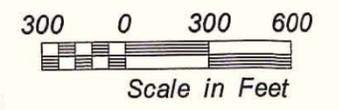
**Drainage Plan – Summary of Peak Flows
for Preferred Alternative**

SUNLAND SPRINGS VILLAGE
MARICOPA COUNTY

PINAL COUNTY



- EXPLANATION**
- 149 - 100-YR DESIGN FLOW (CFS)
 - (58) - RETENTION STORAGE (Ac Ft)
 - - - - SUNLAND VILLAGE DRAINAGE
 - - DIRECTION OF FLOW



NO.	REVISION	BY	DATE
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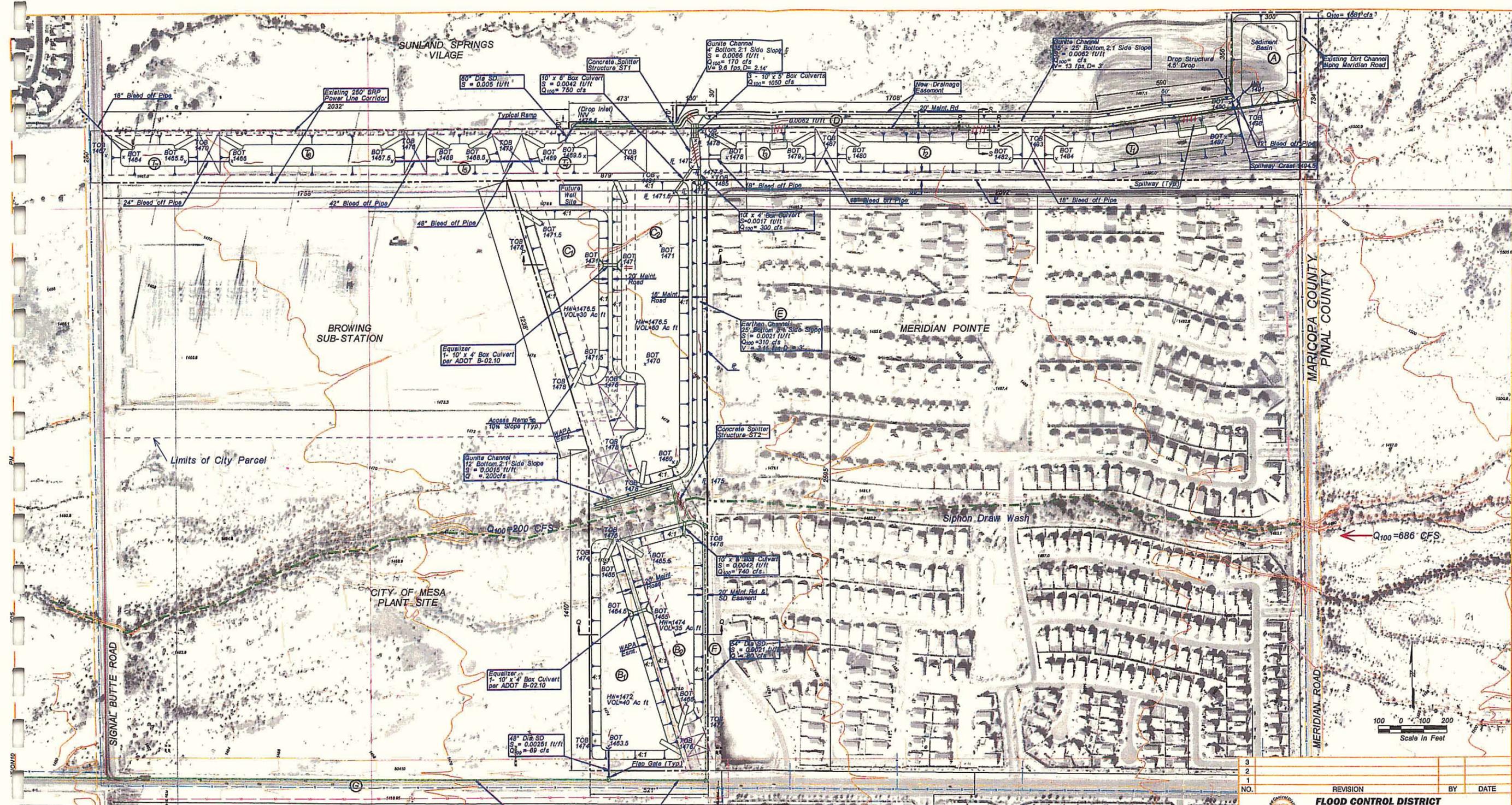
FLOOD CONTROL DISTRICT OF MARICOPA COUNTY
 ENGINEERING DIVISION

SIPHON DRAW DRAINAGE IMPROVEMENTS
 ADDITIONAL MESA ALTERNATIVES
 FCD CONTRACT NO. 2003 C019

	BY	DATE
DESIGNED	S. ZOU	06/05
DRAWN	D. HADGES	06/05
CHECKED	A. PATEL	06/05

PRELIMINARY NOT FOR CONSTRUCTION OR RECORDING
 WOOD PATEL & ASSOCIATES, INC.
 2051 W. NORTHERN AVE., SUITE 100
 PHOENIX, ARIZONA (602) 335-8500

DRAWING NO. PLATE 6 DRAINAGE PLAN SUMMARY OF FLOWS FOR PREFERRED ALTERNATIVE SHEET OF



EXPLANATION

- TOB Top of Basin Elevation
- BOT Bottom Basin Elevation
- INV Invert of Storm Drain
- ⊠ Proposed Pad for SRP Towers/Poles
- 4:1 Sideslope (4 Horizontal : 1 Vertical)
- Q Design flow 100 year storm in cfs
- Ⓐ Hydraulic Element Designation

NOTE: The dimensions depicted on this map are approximate

Topographic Data
 Source: FCDMC, South East Mesa ADMP
 Note that the 2' contour topo shown on this drawing has substantially changed due to improvements in Meridian Point, Sunland Springs Village, Elliot Road and Meridian Road.

NO.	REVISION	BY	DATE
3			
2			
1			

FLOOD CONTROL DISTRICT OF MARICOPA COUNTY ENGINEERING DIVISION

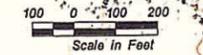
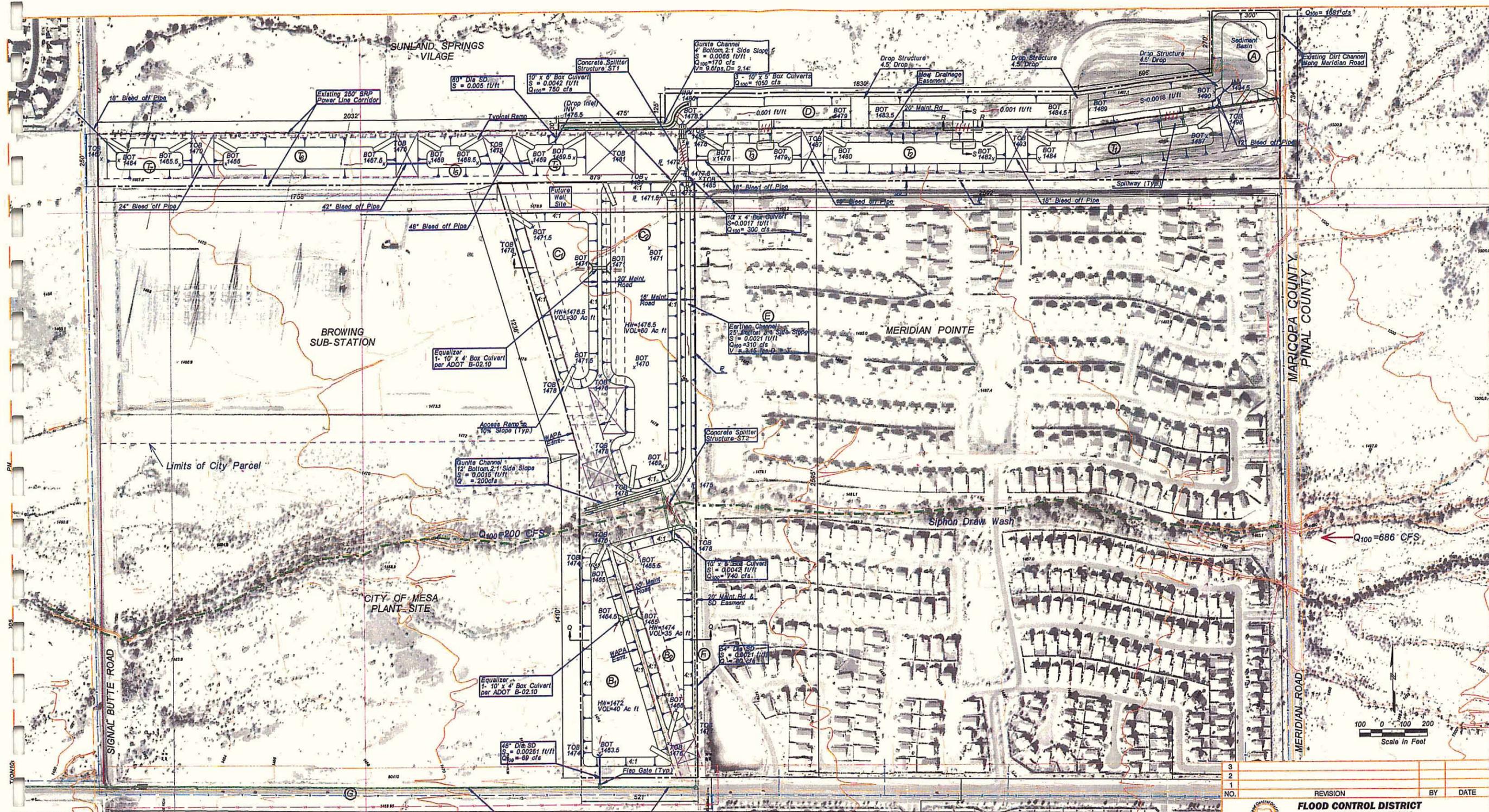
**SIPHON DRAW DRAINAGE IMPROVEMENTS
 ADDITIONAL MESA ALTERNATIVES
 FCD CONTRACT NO. 2003 C019**

	BY	DATE
DESIGNED	S. ZOU	03/05
DRAWN	C. FOWLER	03/05
CHECKED	A. PATEL	03/05

WOOD PATEL & ASSOCIATES, INC.
 2051 W. NORTHERN AVE., SUITE 100
 PHOENIX, ARIZONA (602) 335-8500

DRAWING NO. 1
 PREFFERED ALTERNATIVE (with Concrete Channel D)
 SHEET OF 1 of 1

TWO WORKING DAYS BEFORE RECORDING
 263-1100
 BLUE STAKE



EXPLANATION

- TOB Top of Basin Elevation
- BOT Bottom Basin Elevation
- INV Invert of Storm Drain
- ⊠ Proposed Pad for SRP Towers/Poles
- 4:1 Sideslope (4 Horizontal : 1 Vertical)
- Q Design flow 100 year storm in cfs
- Ⓐ Hydraulic Element Designation

NOTE: The dimensions depicted on this map are approximate

Topographic Data
 Source: FCDMC, South East Mesa ADMP
 Note that the 2' contour top shown on this drawing has substantially changed due to improvements in Meridian Point, Sunland Springs Village, Elliot Road and Meridian Road

NO.	REVISION	BY	DATE
3			
2			
1			

FLOOD CONTROL DISTRICT OF MARICOPA COUNTY
ENGINEERING DIVISION
SIPHON DRAW DRAINAGE IMPROVEMENTS
ADDITIONAL MESA ALTERNATIVES
FCD CONTRACT NO. 2003 C019

	BY	DATE
DESIGNED	S. ZOU	03/05
DRAWN	C. FOWLER	03/05
CHECKED	A. PATEL	03/05

WOOD PATEL & ASSOCIATES, INC.
 2051 W. NORTHERN AVE., SUITE 100
 PHOENIX, ARIZONA (602) 335-8500

DRAWING NO.	PREFERRED ALTERNATIVE	SHEET OF
1	(with Earthen Channel D)	1 of 1