

HYDROLOGIC ANALYSIS
OF THE CENTENNIAL WASH
IN MARICOPA COUNTY, ARIZONA

FOR

FEDERAL EMERGENCY MANAGEMENT AGENCY

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I. INTRODUCTION

Cella Barr Associates (CBA) has been contracted by the Federal Emergency Management Agency (FEMA) to perform a detailed Flood Insurance Re-Study of the Centennial Wash in Maricopa County, Arizona. The study reach for the Centennial Wash commences at the La Paz County line at the upstream limit and extends downstream, a distance of approximately 39.8 miles, to the confluence with the Gila River near Gillespie Dam. The confluence of the Gila River and the Centennial Wash lies approximately 42 miles southwest of Phoenix, Arizona as indicated on the Vicinity Map, Figure 1.

The purpose of this investigation is to estimate 100-year peak discharges at specific locations along the study reach of the Centennial Wash. These discharges are proposed for use in the completion of the Centennial Wash Flood Insurance Re-Study and are to be employed in determining water surface elevations, floodway boundaries and flood zones within the limits of the detailed study reach. For this particular Flood Insurance Re-Study, subsequent floodplain and floodway delineations will be based on the 100-year flood discharges only.

II. WATERSHED DESCRIPTION

General

The Centennial Wash headwaters are located in the Date Creek Mountains near Wickenburg, Arizona, approximately 50 miles northwest of Phoenix. Runoff generated within the 1,870-square mile Centennial Drainage Basin, at the Gila River, tends to flow in a southerly direction (southwesterly within the upper portions and southeasterly in the lower portions) and passes through Yavapai, Maricopa and La Paz Counties. Towns which lie within the subject watershed include Aguila, Wenden and Salome.

The Centennial Wash Drainage Basin is generally elongated in shape, with a total basin length of approximately 110 miles and widths ranging from 15 to 35 miles.

This watershed is characterized by mountain ranges of moderate elevation along the entire perimeter of the basin. The channels of the Centennial Wash drainage basin are characterized by numerous poorly defined, highly dynamic, braided washes that create a poorly defined dominant channel over the majority of the watershed. In most locations, the Centennial Wash channel is wide and shallow and only recognizable by dense stands of native vegetation. In several other locations, the channel loses its definition and becomes part of the irrigated farmland that is prominent in several areas. The channel does not have the capacity to convey major flows and large areas of the watershed are susceptible to shallow flooding due to the flatness of the valley floor. The runoff rate derived from the watershed is attenuated significantly as sheet flow passes downstream across the watershed due to the effects of overbank storage. Channel slopes range from about 0.34 percent to 2.65 percent. Moderate to heavy growth of native vegetation covers most reaches of the channels and associated overbank areas.

The Centennial Wash headwaters at an elevation of 4,506 feet in the Date Creek Mountains and elevations become variably lower, reaching an elevation of about 787 feet at the confluence with the Gila River. This results in a mean slope of 0.64 percent for the overall Centennial Drainage Basin.

Within the Centennial Wash drainage basin, there are five major soils association types: Cherioni-Rock outcrop, Antho-Valencia, Muhall-Leveen, Gilman-Estrella-Avondale and Laveen-Coolidge. Four of the soil associations are of hydrologic Soils Group B and one is of hydrologic Soil Group D, as defined below:

Cherioni-Rock outcrop, of hydrologic Soil Group D, has a high runoff potential and a very slow rate of water transmission. These very shallow soils, over nearly impervious material, are found in the mountainous portion of the Centennial Wash drainage basin. Approximately 10 to 20 percent of the study areas are comprised of this Soil Group D.

The other four soil associations, comprising about 80 to 90 percent of the study area, are found in the braided channel regions and valley plains. These soils, of Soil Group B, have moderate infiltration rates when wetted and a moderate rate of water transmission. The soils are moderately deep to deep, moderately well to well drained with moderately fine to moderately coarse textures.

The natural vegetation cover in the study area is desert brush. Desert brush includes such plants as mesquite, palo verde, brittle brush, creosote bush, catclaw, ironwood, salt bush, saguaro and mixed cactus, etc. This hydrologic cover type is typical of lower elevations with low annual precipitation. Some irrigated farmlands are located in the study area along the Centennial Wash.

The study area is situated in a semi-arid climatic zone characterized by hot summers lasting from May to September with maximum daily

temperatures averaging more than 90°F (33°C), and mild winters with early morning temperatures usually above the freezing point and afternoon temperatures generally between 65°F (18°C) and 70°F (22°C).

Average annual precipitation varies from 11 inches at Wickenburg to 7 inches at the lower limit of the watershed. The study watershed is subject to three major wet seasons. The first season, during the winter months, is invoked by pacific storms producing gentle, widespread showers. The second season, during the summer months, is characterized by thunderstorms produced by moist air from the Gulf of Mexico. During the third season, from late summer to fall, rain is also delivered through tropical thunderstorms arising in the Pacific.

Physical Characteristics

For the purpose of this study, the Centennial Wash Drainage Basin has been divided into an upper and lower region. As shown on Figure 2, the upper watershed has been designated as that portion of the contributing drainage basin which lies above the Narrows Dam (southeast of Salome) and is represented by Sub-basins 1 through 18. The lower watershed extends from the dam, downstream to the Gila River and is comprised of Sub-basins 19 to 37. General basin characteristics for the two regions are presented in the following text.

Upper Centennial Basin

The Upper Centennial Basin is bounded on the north by the Harcuvar and Date Creek Mountains and on the south by the Harquahala and Vulture Mountains. The Centennial Wash in this vicinity flows in a southwesterly direction through the McMullen Valley before making an abrupt turn near Salome and continuing in a southeasterly direction to the Narrows Dam. The general slope for this reach of the Centennial Wash is 0.96 percent.

A majority of the Upper Centennial Basin remains in a natural state and is currently only impacted by minor flood control measures, highways, railroads and low intensity, irrigated cropland.

A vegetative cover density of about 20 percent Desert Brush extends throughout the undisturbed valley region of this basin, with densities in the mountain regions decreasing due to rocky outcrops.

Lower Centennial Basin

The Lower Centennial Basin is bounded on the north by the Harquahala, Big Horn and Belmont Mountains and on the south by the Little Harquahala, Eagletail and Gila Bend Mountains.

The Centennial Wash in this vicinity flows in a southeasterly direction through the Harquahala Plain before converging on the Gila River. The general slope for this reach is 0.31 percent.

Over the years, the natural condition for the Lower Centennial Basin has been considerably impacted by various land uses and improvements. A large portion of this area has been converted to irrigated cropland. Numerous regional flood control measures have also been instituted within the Lower Centennial Basin. A major segment of the Central Arizona Project, the Granite Reef Aqueduct, crosses the entire basin and passes under the Centennial Wash near Lone Mountain. In addition, major highway and railroad construction has taken place.

A natural, vegetative cover density of about 10 percent Desert Brush exists in the undisturbed valley regions with cover densities decreasing in the mountain regions.

The channel for the Centennial Wash in the Lower Centennial Basin is wide and shallow and sometimes only recognizable by the dense stands of Salt Cedar lining this reach of the watercourse. Runoff from large portions of the Lower Centennial Basin has been stored and/or diverted by flood control structures and is delivered at various locations along the main drainage course. Major tributaries in this region are Tiger Wash and Old Camp/Winters Wash.

Structural Features

Structural features within the watershed include major roadways, railroads, flood control structures and water distribution systems. The installation of these improvements has involved the participation of many Federal, State and Local agencies and represent a considerable investment.

The following text contains more specific information regarding the more prominent facilities.

Upper Centennial Basin

In the Upper Centennial Basin, major roadways include U.S. Highway 60/70 and State Highway 71. These roads traverse the basin from northeast to southwest through the central portion of the subject watershed. At the two locations where these highways cross the Centennial channel, concrete box culverts have been installed to convey runoff beneath the roadway. For the purpose of this study, it has been assumed that these structures will have no affect on the determination of the 100-year peak discharge at the confluence with the Gila River.

The Atchison-Topeka and Santa Fe Railroad traverses the Upper Centennial Watershed adjacent to the previously cited highways. The primary drainage channel crosses the railway near Wenden. Field investigation yielded that a new bridge crossing was under construction. It was the opinion of CBA that this new structure would be capable of passing the 100-year peak discharge and would provide no significant stormwater detention, therefore routings for this structure were not included in the hydrologic analysis.

Flood control structures within the Upper Centennial Basin range from minor features such as spreading dikes and local diversion systems to flood-retarding structures and levees. As determined through field investigation and reconnaissance, many of these structures were not designed to, or are not capable of, storing or diverting the 100-year storm runoff. It was assumed that these structures would have little consequence on the estimated 100-year peak discharge downstream and, therefore, minor features such as spreading dikes and irrigation systems were not modeled in the hydrologic analysis. The following is a discussion regarding the more significant flood control structures in the Upper Centennial Basin.

Ritter Dam is located in the northeastern portion of the upper watershed and was not incorporated into the hydrologic analysis due to its relatively small size and long distance from the downstream concentration points investigated along the study reach.

The Upper Centennial Flood Retarding Structures (FRS) are located along the primary channel between the Towns of Wenden and Aguila. These facilities were constructed in 1956 by the Bureau of Land Management and consist of seven detention structures. During field investigation, it was evident that the two upstream structures had been breached at some time in the past. In light of the current condition of these facilities and the potential for future failures

of the system, CBA has elected not to incorporate the potential flood control effects of the Upper Centennial FRS in this investigation.

The Narrows Dam is located approximately eight miles southeast of Salome in the Little Harquahala Mountains. This facility is under the jurisdiction of the Bureau of Land Management and Maricopa County. As evidenced during field investigation, the dam is presently in good condition and functioning appropriately. However, we believe its capacity to control the 100-year discharge is marginal. Based on its significant size and storage area, however, we have incorporated the impact of this facility into the hydrologic analysis.

Lower Centennial Basin

In the Lower Centennial Basin, major roadways include the Salome-Buckeye Highway and Interstate 10. These roads traverse the basin from northwest to southeast through the central portion of the subject watershed. At locations where these highways cross the Centennial channel, concrete box culverts and dip sections have been installed to convey runoff beneath and/or across the roadway. For the purpose of this study, it has been assumed that these roadway crossings will have no affect on the determination of the 100-year peak discharges along the study reach.

The Southern Pacific Railroad traverses the Lower Centennial Watershed near the downstream study limits. The railroad crosses the primary drainage channel, and from field investigation, it appears that this facility will not function as a flood detention structure and, therefore, will have minimal affect on the downstream 100-year discharge.

Numerous flood control structures have been constructed within the Lower Centennial Basin. These facilities are depicted on Figure 3.

The Tiger Wash Detention Structure is located north of I-10, in the central region of the Centennial Drainage Basin. This facility detains peak runoff generated by the Tiger Wash Drainage Basin (Sub-Basin 21) and outlets into the Centennial Wash just upstream of I-10. For the purpose of this study, data relevant to the performance of this detention structure was obtained from the McArthur Report (Reference 8) and utilized in the hydrologic analysis.

The Harquahala Flood Retention Structure (FRS) is also located north of I-10 and lies to the east of the Tiger Wash Detention Structure. This facility detains peak runoff generated in the Big Horn Mountains (Sub-Basin 29). Outflow from this structure is conveyed to the Centennial Wash by means of the Saddleback Diversion System.

The Saddleback FRS lies south of I-10 and intercepts runoff from the Harquahala FRS and Sub-Basin 30. Stormwater from this structure is conveyed to the Centennial Wash by the Saddleback Diversion Channel as shown on Figure 3.

Adjacent to the primary drainage channel and south of I-10 is the Centennial Levee Reach I. This levee diverts runoff generated by the area just south of I-10 and upstream from the left overbank of the Centennial Wash.

Information relevant to the performance of the Harquahala FRS, the Saddleback FRS, the Saddleback Diversion and the Centennial Levee was obtained from the "Harquahala Valley Watershed - Supplemental Watershed Work Plan No. 1," March 1977 (Reference 9). These structures are included in the hydrologic analysis.

As illustrated on Figure 3, approximately 40 miles of the Central Arizona Project Canal (Granite Reef Aqueduct) passes through the Centennial watershed. The Granite Reef Aqueduct traverses the basin from west to east through the central portion of the subject basin and lies immediately south of the previously referenced Tiger Wash Detention Basin and Harquahala FRS. Structural data pertaining to this system was obtained from material provided by the U.S. Bureau of Reclamation.

In addition, a lateral branch of the Central Arizona Project (CAP) canal known as the Eagletail Reach passes in a southeasterly direction through Sub-Basins 26 and 27. Structural and performance data for this structure was obtained from the consulting firm Franzoy, Corey and Associates, and is incorporated in the HEC-1 analysis.

III. AVAILABLE INFORMATION

The Centennial Wash Drainage Basin has been the subject of considerable investigation in the past. For the preparation of this report, numerous federal, state and local government agencies and private parties were contacted and relevant material was requested and reviewed. A bibliographic listing of references and personal contacts utilized in the preparation of this report has been provided herein.

The primary reference source employed in this investigation was prepared by Mr. Robin McArthur of the U.S. Soil Conservation Service (Reference 8). This report is titled "Harquahala Valley Watershed, Hydrologic Studies for the Design of Reach 1 and Reach 2 of the Centennial Levee," January 1984. This study did not include the lower portion of Centennial Wash which is designated for Flood Insurance Re-Study as part of this report.

The U.S. Geological Survey has historically maintained several stream gauge recording stations at various locations along the Centennial Wash. For the purpose of this study, only the station that contributed sufficient data was considered relative to estimating a 100-year peak discharge. Of particular interest to this investigation is the water-stage recording station, U.S.G.S. No. 09517500, situated along the Centennial Wash near Arlington, Arizona. This station was located at the Centennial channel crossing of the former U.S. Highway 80, approximately three (3) miles upstream of Gillespie Dam and provides a continuous record of annual peak discharges for the period from January 1961 to September 1979. A Log-Pearson Type III Statistical Analysis was performed for the 19 years of recorded data using procedures outlined in Reference 4. The 100-year peak discharge value estimated by this procedure for the Centennial Wash, at a location approximately 3 miles upstream of Gillespie Dam (Gila River), was 39,200 cfs. This peak discharge is based on a contributing drainage basin area of 1,810 square miles, resulting in an average runoff generation rate of 21.7 cfs per square mile (0.3 cfs per acre) for the watershed.

It is the opinion of CBA, that the analysis of this particular stream gauge data resulted in a value which is too low to correctly represent the potential 100-year peak discharge for the Centennial Wash. This conclusion was reached after consideration of several primary factors:

- The statistical analysis was performed on only 19 years of recorded data.
- The location of the stream gauge recording station is approximately 130 miles from the headwaters of the Centennial Wash. This stream gauge data may not reflect contributions from all portions of the watershed.
- A 100-year storm runoff value of 21.7 cfs per square mile was low by comparison to the results of the McArthur Report (Reference 8).

It is, therefore, the intent of CBA not to utilize the results of the Flood Frequency Analysis presented herein for the current Centennial Wash Flood Insurance Study.

IV. HYDROLOGIC ANALYSIS

General

For the purpose of this study, the Army Corps of Engineers "HEC-1" computer program was utilized to estimate 100-year peak discharges along the study reach of the Centennial Wash. Use of this method was determined to be the preferred procedure during a meeting held on February 25, 1988 at CBA's Phoenix office which included representatives of CBA, the Flood Control District of Maricopa County, the Arizona Department of Water Resources, and several other agencies. The methodology employed for the selection of input parameters for the HEC-1 program is presented in the following text.

Drainage Basins

The Centennial Wash Drainage Basin was delineated using U.S. Geological Survey Quadrangle Maps, aerial photography and field investigation. The exterior watershed boundary and the interior sub-basin boundaries were delineated utilizing the USGS topographic map 7-1/2-minute series (1" = 24,000'). Drainage area sizes, elevations and length of watercourse were also determined using these maps. Due to the large size of these maps, they were not included in this report. The overall watershed was subdivided into sub-basins selected based upon topography and interpretation of characteristic unit flow contributions. The Centennial Wash Drainage Basin divisions and sub-basins are represented on Figure 2 of this report. Table 1 provides a summary of the estimated sub-basin areas.

In order to eliminate duplication of effort and to provide a means of comparison for the CBA results, concentration points for this study were selected to coincide with the concentration points of the McArthur Report within overlapping portions of the study reach.

Drainage Basin boundaries were then determined independently of the McArthur Report and basin areas were estimated with the use of a computer-assisted, digitizing table and manual planimeter. The entire Centennial Wash Watershed was divided into 40 sub-basins ranging in size from 8 to 185 square miles. A comparison was then made between the drainage basins determined by CBA and those estimated in the McArthur report. An acceptable correlation between the two computed basin areas exists. The difference in computed overall watershed area between this study and the McArthur Report, computed at the downstream concentration point for the McArthur Report (CBA Basins 1 through 22) is 0.6 square miles.

Curve Numbers

As determined through field investigation, a generally uniform vegetative cover of desert brush extends throughout the Centennial Wash watershed. Cover density within the valley portions of the study area varies from an estimated 20 percent in the upper regions of the watershed to about 10 percent for the lower regions. The surrounding mountain slopes have somewhat lower vegetative cover densities than the valley areas.

The determination of hydrologic soils groups was performed with the use of the Soil Conservation Service General Soil Maps for Maricopa and Yuma Counties (References 12 and 13). Sub-basins were differentiated and proportioned with respect to the appropriate soils groups.

Based on the above-referenced parameters, weighted curve numbers were then determined for each sub-basin according to the method presented in "Urban Hydrology for Small Watersheds, Soil Conservation Service Technical Release 55," 1986 (Reference 17). Curve numbers for each sub-basin are contained in Table 1 of this report.

Precipitation

Rainfall values were computed utilizing "NOAA Atlas 2, Precipitation - Frequency Atlas of the Western United States, volume VIII, Arizona" (Reference 18).

As mentioned earlier in this report, Mr. Robin McArthur of the U.S. Soil Conservation Service has prepared a detailed Hydrologic Study of the Centennial Wash for Centennial Levee Reaches I and II. After reviewing this report, CBA computed rainfall values for three sample basins using the same procedure as in the McArthur Study. CBA's results for sub-basins 8, 13 and 20 were then compared with the computed values in the McArthur Study, and the correlation was found to be acceptable.

To avoid duplication of effort, CBA utilized the precipitation values contained in the McArthur Report for sub-basins 1 through 25, 28, 30 and 31 (CBA's sub-basin sizes approximately match those presented in the McArthur Report). Precipitation values for the remaining drainage sub-basins, 26, 27, 29 and 32 through 37, which were not included in McArthur's Report, have been computed by CBA using the same method contained in said report. The procedure Mr. McArthur followed to compute the 100-year 24-hour rainfall values is attached as Appendix III in this report.

Lag Times

As described in the SCS "National Engineering Handbook, Section 4, Hydrology", Chapter 15 (Reference 19), the Time of Concentration for each sub-basin was computed using the Upland method and lag times for the subject basins were estimated in terms of T_c using the empirical relation of:

$$L = 0.6T_c$$

$$L = \text{Lag Time (hours)}$$

$$T_c = \text{Time of Concentration (hours)}$$

Table I presents the computed Lag Times used in the HEC-1 analysis.

Stream Channel Routings

Stream channel profiles are utilized by the HEC-1 program to route storm runoff through prescribed channel reaches. For the purpose of this study, channel routings were performed using the normal depth option of the HEC-1 program. Channel cross-sections for the lower portions of the Centennial Wash watershed were developed from topographic mapping (1 in. = 400 ft.; C.I. = 2 ft. and 4 ft., 1978 and 1988) and U.S.G.S. quadrangle sheets, 7.5 minute series topographic maps (1:24,000). Field cross-sections were developed for the upper portion of the watershed and the U.S.G.S. topographic maps (1:24,000) were utilized to determine approximate elevations of the cross-sections.

Flood Control Structures

Structural data, as well as any available hydrologic and hydraulic information, was obtained from local engineering firms and governmental agencies having jurisdiction over the existing structures. For the purpose of this investigation, 100-year composite hydrographs generated by the HEC-1 program were routed through the existing structures, based on data provided to CBA regarding previously performed analyses of the individual structures.

The following is a list of those structures which were modeled for the HEC-1 analysis: Lower Centennial Dam (Narrows Dam), Bureau of Land Management; Tiger Wash Detention Structure, Bureau of Reclamation; Harquahala Flood Retention Structure (FRS), Saddleback FRS, Maricopa County; Old Camp Wash Crossing of the Granite Reef Aqueduct, Central Arizona Water Conservation District; Eagletail Reach (CAP Lateral), Harquahala Valley Irrigation District.

Features which were not included in the HEC-1 analysis include local irrigation canals, levees and spreading dikes. These structures are presumed to effectively control runoff during low flow events, as those

that are produced by a 10- or 25-year storm; however, such structures would have little impact on the 100-year runoff.

Other structures not modeled for this analysis are the Upper Centennial Flood Retarding Structures, due to the structural failures evidenced during field reconnaissance and the Ritter Dam because of its location in the upper watershed and great distance from downstream concentration points.

V. RESULTS

The intent of this investigation is to determine 100-year peak discharges at designated locations along the Centennial Wash. These discharges are proposed for use in the completion of the Centennial Wash Flood Insurance Re-Study and are to be employed in determining water surface elevations, floodway boundaries and flood zones within the limits of the detailed study reach shown on Figures 1, 2, and 3.

The Flood Flow Frequency computer model provided by the U.S. Army Corps of Engineers was used to perform a Log-Pearson Type III Statistical Analysis on the 19 years of recorded gauge data (U.S.G.S. Gauge No. 09517500) located along the Centennial Wash, roughly 3 miles upstream of Gillespie Dam (Gila River). The results of this analysis yielded a 100-year peak discharge of 39,000 cfs. This value appears to be a low estimate, resulting from the relatively short period of record and the downstream location of the recording station with respect to a total basin length of 135 miles for the Centennial watershed. It is our opinion that 19 years of available stream flow data is not adequate to determine the 100-year peak discharge for the Centennial Wash at the Gila River.

Detailed hydrologic analysis of Centennial Wash at Centennial Levee Reaches I and II was performed by Mr. Robin McArthur (Reference 8). His study utilized the Soil Conservation Service TR-20 hydrologic model to determine 100-year discharges along portions of the Centennial Wash, but did not evaluate the lower reaches of the Centennial Wash designated for this Flood Insurance Re-Study.

The drainage areas at the Maricopa County/La Paz County line and at Centennial Levee Reaches I and II calculated by CBA correlate very closely with the drainage areas at the same locations in Mr. McArthur's Report. The 100-year peak discharge values at these locations computed by CBA (using the HEC-1 computer program) also correlate well, but are approximately five to seven percent (5 to 7%) lower when compared with the 100-year discharges presented in McArthur's Report (see Table 2).

Utilizing the HEC-1 computer program, a 100-year peak discharge of 67,300 cfs was estimated for the Centennial Wash at the confluence with the Gila River. Discharges determined at this location and other key locations along the study reach utilizing the HEC-1 computer model are depicted on Table 2. CBA proposes to use the peak discharges determined by the HEC-1 analysis to estimate the 100-year water surface elevations and floodway boundaries for the portions of the Centennial Wash designated for detailed study per this Flood Insurance Re-Study.

TABLE 1
CENTENNIAL WASH
PARAMETERS USED IN THE HEC-1 ANALYSIS

Drainage Sub-Basins	Drainage Area (square miles)	Lag Time (hours)	Curve Number	Precipitation (24 hrs. - 100-yr.) (in.)
1	67.0	5.80	78.0	3.88
2	185.0	15.70	81.0	3.88
3	43.0	4.81	80.0	3.88
4	21.6	3.27	81.0	3.88
4A	18.5	3.40	81.0	3.88
5	48.7	2.90	79.0	3.88
6	23.7	2.59	81.0	3.88
7	44.7	4.01	79.0	3.88
8	31.3	2.89	80.0	3.88
8A	16.2	2.33	80.0	3.88
9	52.2	3.02	80.0	3.88
10	17.0	2.50	79.0	3.88
11	20.2	3.04	78.0	3.88
12	10.6	1.98	79.0	3.88
12A	9.9	1.62	79.0	3.88
13	68.7	3.96	78.0	3.88
14	13.7	0.71	84.0	3.88
15	18.6	3.41	79.0	3.88
16	19.7	1.59	82.0	3.88
17	21.0	1.98	80.0	3.88
18	24.4	2.89	80.0	3.88

TABLE 1 (Cont.)

CENTENNIAL WASH
PARAMETERS USED IN THE HEC-1 ANALYSIS

Drainage Sub-Basins	Drainage Area (square miles)	Lag Time (hours)	Curve Number	Precipitation (24 hrs. - 100-yr.) (in.)
19	10.6	1.18	77.0	3.88
20	50.7	6.63	80.0	3.88
21	76.8	7.00	84.0	3.88
21A	84.4	7.20	84.0	3.88
22	25.1	2.51	79.0	3.88
23	7.9	2.00	77.0	3.88
24	11.2	2.51	77.0	3.88
25	24.9	4.21	77.0	3.88
26	42.4	3.19	79.0	4.12
27	62.1	4.35	77.0	4.12
28	84.4	15.06	75.0	4.12
29	101.3	4.85	83.0	3.93
30	28.6	4.75	78.0	3.93
31	12.0	0.96	86.0	4.12
32	52.4	5.05	79.0	3.93
33	47.0	4.00	80.0	3.93
34	64.3	4.64	81.0	3.93
35	99.6	4.73	87.0	3.93
36	163.1	12.25	79.0	3.93
37	45.8	4.43	83.0	3.93

TABLE 2
 ESTIMATES OF 100-YEAR
 PEAK DISCHARGES ALONG CENTENNIAL WASH
 FROM HEC-1

Location Along Centennial Wash	<u>CBA's Study</u>		<u>Mr. McArthur's Study</u>	
	Drainage Area (square miles)	100-Year Discharge (cfs)	Drainage Area (square miles)	100-Year Discharge (cfs)
At Maricopa County/ La Paz County Line (Concentration Point 12 - Figure 2)	1,031.20	52,200	1,031.21	56,400
At Centennial Levee Reach 2 (Concentration Point 14 - Figure 2)	1,109.70	52,300	1,110.3	55,300
Near Base Line Road (Concentration Point 15, - Figure 2)	1,398.1	58,100		NO DATA
At Railroad Bridge near Arlington (Concentration Point 17 - Figure 2)	1,824.53	67,300		NO DATA
At Confluence with Gila River (Concentration Point 18 - Figure 2)	1,870.33	67,300		NO DATA

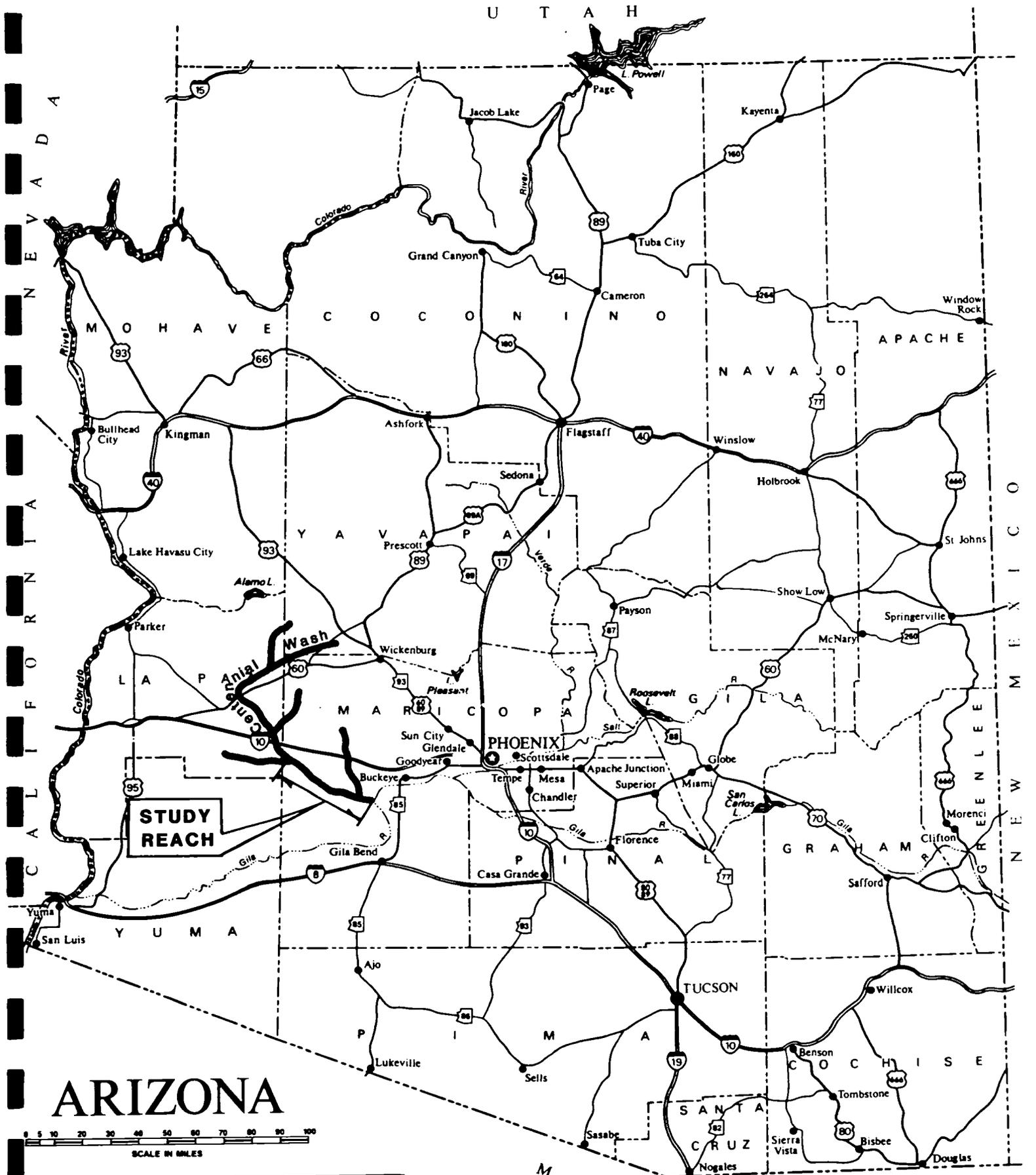
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7.5 minute series topographic maps (1:24,000).
Date Creek Ranch, Date Creek Ranch S. E., Date Creek Ranch S. W., O'Neal Pass, Congress S. W., Smith Peak and E. C. P. Peak.

15. United States Department of Interior, Geological Survey; 15 minute series topographic maps (1:62,500). Salome, Gladden, Aguila, Vulture Mountains, Hope, Lone Mountain, Big Horn Mountains, Belmont Mountains, Eagletail Mountains, Cortez Peak, Arlington, Woolsey Peak and Dendora Valley.
16. "Water Resources Data - Arizona," U. S. Geological Survey Water Data Report; Water Years, 1978 - 1984.
17. "Urban Hydrology for Small Watersheds," Soil Conservation Service Technical Release 55, June 1986.
18. "NOAA Atlas 2, Precipitation - Frequency Atlas of the Western United States, Volume VIII, Arizona," U.S. Department of Commerce.
19. "National Engineering Handbook, Section 4, Hydrology," Soil Conservation Service, Chapter 15.

VII. PERSONAL CONTACTS

1. HEATTON, L.
Bureau of Reclamation, Phoenix, Arizona.
2. KOENEKAMP, T.
Franzoy-Corey Engineering Company, Tempe, Arizona
3. McLAUGHLIN, D.
Bureau of Land Management, Phoenix, Arizona
4. WALSH, R.
Bureau of Reclamation, Phoenix, Arizona.
5. MCARTHUR, R.
Soil Conservation Service, Phoenix, Arizona



ARIZONA

0 5 10 20 30 40 50 60 70 80 90 100
SCALE IN MILES

CENTENNIAL WASH LOCATION MAP

CBA CELLA BARR
ASSOCIATES

2075 North Sixth Avenue
Tucson, Arizona 85705
(602) 624-7404

FILE NO. 04856-03-74
SEPTEMBER, 1988

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FIGURE 1

Model Library / other Hydrograph Cent

CW-noRL.out
 modified for
 HEC-1 version 4.1

RUNOFF SUMMARY
 FLOW IN CUBIC FEET PER SECOND
 TIME IN HOURS, AREA IN SQUARE MILES

OPERATION	STATION	PEAK FLOW	TIME OF PEAK	AVERAGE FLOW FOR MAXIMUM PERIOD			BASIN AREA	MAXIMUM STAGE	TIME OF MAX STAGE
				6-HOUR	24-HOUR	72-HOUR			
HYDROGRAPH AT	SUB 1	7279.	18.50	6534.	3185.	1076.	67.00		
ROUTED TO	1	7110.	21.00	6359.	3128.	1076.	67.00	2202.93	21.00
HYDROGRAPH AT	SUB 2	10486.	29.00	10267.	7910.	3346.	145.00		
HYDROGRAPH AT	SUB 3	5890.	17.00	5043.	2234.	749.	43.00		
3 COMBINED AT	1	16524.	22.50	16120.	12502.	5170.	295.00		
ROUTED TO	2	16448.	24.50	16074.	12464.	5162.	295.00	2085.05	24.50
HYDROGRAPH AT	SUB 4	4125.	15.50	3101.	1172.	391.	21.00		
HYDROGRAPH AT	SUB4A	3440.	15.50	2626.	1004.	335.	18.50		
3 COMBINED AT	2	17869.	23.50	17398.	13460.	5881.	335.10		
HYDROGRAPH AT	SUB 5	9311.	15.00	6690.	2443.	815.	46.70		
2 COMBINED AT	3	19492.	22.50	18933.	15478.	6695.	383.80		
ROUTED TO	3	19500.	23.50	18915.	15434.	6688.	383.80	2024.21	23.50
HYDROGRAPH AT	SUB 6	5325.	14.50	3647.	1287.	429.	23.70		
HYDROGRAPH AT	SUB 7	6689.	16.50	5465.	2238.	748.	44.70		
3 COMBINED AT	4	27317.	16.00	23263.	18446.	7857.	452.20		
ROUTED TO	4	26730.	18.00	22948.	18260.	7831.	452.20	1944.79	18.00
HYDROGRAPH AT	SUB 8	6274.	15.00	4494.	1634.	545.	31.30		
HYDROGRAPH AT	SUB8A	3764.	14.50	2457.	846.	282.	16.20		
3 COMBINED AT	5	31129.	18.00	26075.	19518.	8640.	499.70		
HYDROGRAPH AT	SUB 9	10078.	15.00	7392.	2725.	909.	52.20		
2 COMBINED AT	6	36933.	17.50	30285.	21848.	9547.	551.90		
ROUTED TO	5	36672.	18.50	30200.	21783.	9544.	551.90	1889.47	18.50
HYDROGRAPH AT	SUB10	3610.	14.50	2426.	853.	284.	17.00		
HYDROGRAPH AT	SUB11	3531.	15.00	2614.	973.	324.	20.20		
3 COMBINED AT	7	39628.	18.50	32636.	23237.	10149.	589.10		
ROUTED TO	6	38617.	19.50	32556.	23156.	10132.	589.10	1825.04	19.50
HYDROGRAPH AT	SUB12	2648.	14.00	1582.	532.	177.	10.60		

+	HYDROGRAPH AT	SUB12A	2816.	13.50	1527.	497.	166.	9.50		
+	3 COMBINED AT	8	39507.	19.50	33372.	23595.	10459.	609.70		
+	HYDROGRAPH AT	SUB13	9907.	16.50	8076.	3303.	1103.	68.70		
+	2 COMBINED AT	9	45374.	19.50	39335.	26471.	11562.	678.30		
+	HYDROGRAPH AT	SUB15	3154.	15.50	2423.	932.	311.	18.60		
+	2 COMBINED AT	10	46817.	19.50	40947.	27296.	11873.	696.90		
+	ROUTED TO	7	44035.	22.00	40136.	27146.	11865.	696.90	1771.32	22.00
+	HYDROGRAPH AT	SUB14	7985.	12.50	2675.	835.	276.	13.70		
+	HYDROGRAPH AT	SUB16	6509.	13.50	3441.	1113.	371.	19.70		
+	3 COMBINED AT	11	44994.	21.50	41166.	27682.	12491.	740.20		
+	ROUTED TO	1	44253.	22.50	40704.	27584.	12386.	730.30	258.69	22.50
+	ROUTED TO	8	43686.	24.50	40447.	27523.	12373.	730.30	1610.99	24.50
+	HYDROGRAPH AT	SUB17	5489.	14.00	3273.	1097.	366.	21.00		
+	HYDROGRAPH AT	SUB18	4891.	15.00	3503.	1274.	425.	24.40		
+	3 COMBINED AT	12	44911.	24.50	41594.	28176.	13127.	775.70		
+	ROUTED TO	12	44838.	25.00	41549.	28164.	13114.	775.70	1444.86	25.00
+	HYDROGRAPH AT	SUB19	3328.	13.00	1534.	490.	163.	10.60		
+	HYDROGRAPH AT	SUB20	5433.	19.00	4992.	2588.	883.	50.70		
+	3 COMBINED AT	13	48175.	24.50	44900.	30404.	14142.	837.60		
+	HYDROGRAPH AT	SUB21A	9999.	20.00	9289.	4981.	1714.	84.40		
+	ROUTED TO	10	9161.	24.50	8443.	4948.	1714.	84.40	1724.49	24.50
+	HYDROGRAPH AT	SUB21	9332.	19.50	8609.	4546.	1560.	76.80		
+	2 COMBINED AT	14	15639.	23.00	14636.	9426.	3274.	161.20		
+	ROUTED TO	2	7507.	31.50	7309.	5256.	2674.	161.20	1408.11	31.50
+	2 COMBINED AT	15	51943.	25.00	48692.	35130.	16795.	998.20		
+	ROUTED TO	10	51880.	25.50	48701.	35125.	16780.	998.20	1365.29	25.50
+	HYDROGRAPH AT	SUB22	5310.	14.50	3578.	1259.	420.	25.10		
+	2 COMBINED AT	16	52429.	25.50	49145.	35492.	17153.	1023.30		
+	ROUTED TO	11	52218.	26.00	49064.	35482.	17135.	1023.30	1286.23	26.00

+		SUB34	9666.	17.00	8192.	3549.	1189.	64.30		
+	3 COMBINED AT									
+		24	57548.	36.50	55116.	42280.	23781.	1561.60		
+	HYDROGRAPH AT									
+		SUB35	18727.	17.00	15876.	6837.	2297.	99.60		
+	ROUTED TO									
+		6	6206.	23.50	6113.	4938.	2281.	99.60	68.29	23.50
+	ROUTED TO									
+		19	6175.	29.00	6078.	4878.	2265.	99.60	1902.75	29.00
+	HYDROGRAPH AT									
+		SUB36	10654.	25.50	10351.	7239.	2787.	163.10		
+	3 COMBINED AT									
+		25	67676.	36.00	65108.	51887.	28812.	1824.50		
+	ROUTED TO									
+		20	67263.	37.50	64808.	51845.	28778.	1824.50	850.26	37.50
+	HYDROGRAPH AT									
+		SUB37	7718.	16.50	6451.	2726.	914.	45.80		
+	2 COMBINED AT									
+		26	67298.	37.50	64851.	52134.	29583.	1470.30		

*** NORMAL END OF HEC-1 ***

```

X
X FLOOD HYDROGRAPH PACKAGE (HEC-1) X
X FEBRUARY 1981 X
X REVISED 30 OCT 81 X
X
X RUN DATE WED, JAN 04 1989 TIME 14:07:22 X
X
XXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXX

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X
X U.S. ARMY CORPS OF ENGINEERS X
X THE HYDROLOGIC ENGINEERING CENTER X
X 609 SECOND STREET X
X DAVIS, CALIFORNIA 95616 X
X (916) 440-3285 OR (FTS) 448-3285 X
X
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X X XXXXXXXX XXXXX X
X X X X X XX
X X X X X
XXXXXXXXX XXXX X XXXXX X
X X X X X
X X X X X
X X XXXXXXXX XXXXX XXX

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THIS PROGRAM REPLACES ALL PREVIOUS VERSIONS OF HEC-1 KNOWN AS HEC1 (JAN 73), HEC1GS, HEC1DB, AND HEC1KW.

THE DEFINITIONS OF VARIABLES -RTIMP- AND -RTIOP- HAVE CHANGED FROM THOSE USED WITH THE 1973-STYLE INPUT STRUCTURE. THE DEFINITION OF -ANSSK- ON RM-CARD WAS CHANGED WITH REVISIONS DATED 28 SEP 81. SEE SEPTEMBER 1981 INPUT DESCRIPTION FOR NEW DEFINITIONS.

HEC-1 INPUT

```

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10
1 ID MARICOPA CO. FLOOD INSURANCE STUDY
2 ID CENTENNIAL WASH HEC-1 (FILE CNHEC1F)
3 ID CURVE NUMBERS DETERMINED BY UPLAND METHOD
4 ID CBA JOB NO. 04856-03-74 5 JANUARY 89
5 IT 30 18APR88 1200 200
6 IO 3 0
7 IN 15 18APR88 1200

8 KK SUB 1
9 KM RUNOFF FROM SUBBASIN 1
10 KM SCS DIMENSIONLESS UNIT HYDROGRAPH
11 KM LAGTIME = HYDRAULIC LENGTH, AVG. WATERSHED SLOPE AND C.N.
12 KM RAINFALL DISTRUBUTION = SCS TYPE II
13 BA 67.0
14 PB 3.88
15 PC 0 0.002 0.005 0.008 0.011 0.014 0.017 0.020 0.023 0.026
16 PC 0.029 0.032 0.035 0.038 0.041 0.044 0.048 0.052 0.056 0.060
17 PC 0.064 0.068 0.072 0.076 0.080 0.085 0.090 0.095 0.100 0.105
18 PC 0.110 0.115 0.120 0.126 0.133 0.140 0.147 0.155 0.163 0.172
19 PC 0.181 0.191 0.203 0.218 0.236 0.257 0.283 0.387 0.663 0.707
20 PC 0.735 0.758 0.776 0.791 0.804 0.815 0.825 0.834 0.842 0.849
21 PC 0.856 0.863 0.869 0.875 0.881 0.887 0.893 0.898 0.903 0.908
22 PC 0.913 0.918 0.922 0.926 0.930 0.934 0.938 0.942 0.946 0.950
23 PC 0.953 0.956 0.959 0.962 0.965 0.968 0.971 0.974 0.977 0.980
24 PC 0.983 0.986 0.989 0.992 0.995 0.998 1.000

```


158 KM RUNOFF FROM SUBBASIN 13
 159 BA 68.7
 160 LS 78.0
 161 UD 3.96

162 KK 9 COMBINE 1-12A WITH 13
 163 KM ADD TWO HYDROGRAPHS
 164 HC 2

165 KK SUB15
 166 KM RUNOFF FROM SUBBASIN 15
 167 BA 18.6
 168 LS 79
 169 UD 3.41

HEC-1 INPUT

PAGE 5

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

170 KK 10 COMBINE 1-13 WITH 15
 171 KM ADD TWO HYDROGRAPHS
 172 HC 2

173 KK 7 ROUTE HYDROGRAPH 1-15 THROUGH REACH 7
 174 RL 0.2
 175 RS 6 FLOW -1
 176 RC 0.055 0.040 0.055 26562 0.005
 177 RX 0 900 950 1110 1130 1140 1400 1700
 178 RY 1764 1763 1760 1760 1764 1762 1762 1764

179 KK SUB14
 180 KM RUNOFF FROM SUBBASIN 14
 181 BA 13.7
 182 LS 84.0
 183 UD 0.71

184 KK SUB16
 185 KM RUNOFF FROM SUBBASIN 16
 186 BA 19.7
 187 LS 82.0
 188 UD 1.59

189 KK 11 COMBINE 1-15 WITH 14 AND 16
 190 KM ADD THREE HYDROGRAPHS
 191 HC 3

192 KK 1 ROUTE (LOWER CENTENNIAL - WARROWS DAM)
 193 KM ROUTE TOTAL HYDROGRAPH THROUGH RESERVOIR
 194 RS 1 ELEV 224
 195 SV 0 0 0 500 850 1240 1700 2300 2620 2900
 196 SE 224 232 236.86 243.86 245.86 247.86 249.86 251.86 252.86 253.86
 197 SQ 0 20 160 547 1030 3150 7550 14000 18000 22500

198 KK 8 ROUTE HYDROGRAPH 1-16 THROUGH REACH 8
 199 RL 0.2
 200 RS 10 FLOW -1
 201 RC 0.050 0.035 0.050 49479 0.004
 202 RX 0 500 550 640 650 800 820 1500

292 KK 12 ROUTE HYDROGRAPH 1-23 THROUGH REACH 12
 293 RL 0.2
 294 RS 4 FLOW -1
 295 RC 0.050 0.035 0.050 17708 0.0027
 296 RX 0 800 1000 2300 2600 3500 4000 4500
 297 RY 1274 1269 1267 1265 1267 1269 1271 1275
 HEC-1 INPUT

PAGE 8

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

298 KK SUB24
 299 KM RUNOFF FROM SUBBASIN 24
 300 BA 11.2
 301 LS 77
 302 UD 2.51

303 KK SUB25
 304 KM RUNOFF FROM SUBBASIN 25
 305 BA 24.9
 306 LS 77.0
 307 UD 4.21

308 KK 18 COMBINE 1-23 WITH 24 AND 25
 309 KM ADD THREE HYDROGRAPHS
 310 HC 3

311 KK 13 ROUTE HYDROGRAPH 1-25 THROUGH REACH 13
 312 RL 0.2
 313 RS 6 FLOW -1
 314 RC 0.050 0.035 0.050 21354 0.0027
 315 RX 0 2200 2900 4100 4300 5300 6250 7470
 316 RY 1184 1182 1180 1179.9 1180 1182 1180 1181.4

317 KK SUB26
 318 KM RUNOFF FROM SUBBASIN 26
 319 BA 42.4
 320 PB 4.12
 321 PC 0 0.002 0.005 0.008 0.011 0.014 0.017 0.020 0.023 0.026
 322 PC 0.029 0.032 0.035 0.038 0.041 0.044 0.048 0.052 0.056 0.060
 323 PC 0.064 0.068 0.072 0.076 0.080 0.085 0.090 0.095 0.100 0.105
 324 PC 0.110 0.115 0.120 0.126 0.133 0.140 0.147 0.155 0.163 0.172
 325 PC 0.181 0.191 0.203 0.218 0.236 0.257 0.283 0.387 0.663 0.707
 326 PC 0.735 0.758 0.776 0.791 0.804 0.815 0.825 0.834 0.842 0.849
 327 PC 0.856 0.863 0.869 0.875 0.881 0.887 0.893 0.898 0.903 0.908
 328 PC 0.913 0.918 0.922 0.926 0.930 0.934 0.938 0.942 0.946 0.950
 329 PC 0.953 0.956 0.959 0.962 0.965 0.968 0.971 0.974 0.977 0.980
 330 PC 0.983 0.986 0.989 0.992 0.995 0.998 1.000
 331 LS 79
 332 UD 3.19

333 KK 19 COMBINE 1-25 WITH 26
 334 KM ADD TWO HYDROGRAPHS
 335 HC 2

336 KK 14 ROUTE HYDROGRAPH 1-26 THROUGH REACH 14
 337 RL 0.2

338	RS	14	FLOW	-1						
339	RC	0.065	0.045	0.065	46354	0.0027				
340	RX	0	2900	5300	5440	5630	5720	5730	8140	
341	RY	1100	1096	1092	1090	1088	1090	1092	1098	

HEC-1 INPUT

PAGE 9

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

342	KK	SUB27								
343	KM	RUNOFF FROM SUBBASIN 27								
344	BA	62.1								
345	LS	77.0								
346	UD	4.35								

347	KK	3 ROUTE (EAGLETAIL REACH)									
348	KM	ROUTE HYDROGRAPH THROUGH DETENTION STRUCTURE									
349	RS	1	ELEV 1252.5								
350	RV	5	50	155	310	525	790	1110	1485	1905	2355
351	RE	1252.5	1253.0	1253.5	1254.0	1254.5	1255.0	1255.5	1256.0	1256.5	1257.0
352	RO	0	20	50	90	140	200	260	325	400	475

353	KK	20 COMBINE 1-26 WITH 27								
354	KM	ADD TWO HYDROGRAPHS								
355	HC	2								

356	KK	SUB28								
357	KM	RUNOFF FROM SUBBASIN 28								
358	BA	84.4								
359	LS	75.0								
360	UD	15.06								

361	KK	SUB31								
362	KM	RUNOFF FROM SUBBASIN 31								
363	BA	12.0								
364	LS	86.0								
365	UD	.96								

366	KK	21 COMBINE 1-27 WITH 28 AND 31								
367	KM	ADD THREE HYDROGRAPHS								
368	HC	3								

369	KK	SUB29									
370	KM	RUNOFF FROM SUBBASIN 29									
371	BA	101.3									
372	PB	3.93									
373	PC	0	0.002	0.005	0.008	0.011	0.014	0.017	0.020	0.023	0.026
374	PC	0.029	0.032	0.035	0.038	0.041	0.044	0.048	0.052	0.056	0.060
375	PC	0.064	0.068	0.072	0.076	0.080	0.085	0.090	0.095	0.100	0.105
376	PC	0.110	0.115	0.120	0.126	0.133	0.140	0.147	0.155	0.163	0.172
377	PC	0.181	0.191	0.203	0.218	0.236	0.257	0.283	0.387	0.663	0.707
378	PC	0.735	0.758	0.776	0.791	0.804	0.815	0.825	0.834	0.842	0.849
379	PC	0.856	0.863	0.869	0.875	0.881	0.887	0.893	0.898	0.903	0.908
380	PC	0.913	0.918	0.922	0.926	0.930	0.934	0.938	0.942	0.946	0.950
381	PC	0.953	0.956	0.959	0.962	0.965	0.968	0.971	0.974	0.977	0.980
382	PC	0.983	0.986	0.989	0.992	0.995	0.998	1.000			
383	LS	83.0									
384	UD	4.85									


```

472      KK      25 COMBINE 1-34 WITH 35 AND 36
473      KM      ADD THREE HYDROGRAPHS
474      HC      3

475      KK      20 ROUTE HYDROGRAPH 1-36 THROUGH REACH 20
476      RL      0.2
477      RS      6      FLOW      -1
478      RC      0.065  0.040  0.060  30729  0.0024
479      RX      0      400      800      1000      1100      1500      1800      2300
480      RY      848      846      842      840      840      842      846      848

481      KK      SUB37
482      KM      RUNOFF FROM SUBBASIN 37
483      BA      45.8
484      LS      83.0
485      UD      4.43

486      KK      26 COMBINE 1-36 WITH 37
487      KM      ADD TWO HYDROGRAPHS
488      HC      2
489      ZZ
  
```

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*****
*
* FLOOD HYDROGRAPH PACKAGE (HEC-1)
* FEBRUARY 1981
* REVISED 30 OCT 81
*
* RUN DATE WED, JAN 04 1989 TIME 14:07:22
*
*****
  
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*****
*
* U.S. ARMY CORPS OF ENGINEERS
* THE HYDROLOGIC ENGINEERING CENTER
* 609 SECOND STREET
* DAVIS, CALIFORNIA 95616
* (916) 440-3285 DR (FTS) 448-3285
*
*****
  
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MARICOPA CO. FLOOD INSURANCE STUDY
 CENTENNIAL WASH HEC-1 (FILE CWHEC1F)
 CURVE NUMBERS DETERMINED BY UPLAND METHOD
 CBA JOB NO. 04856-03-74 5 JANUARY 89

```

6 IO      OUTPUT CONTROL VARIABLES
          IPRNT      3      PRINT CONTROL
          IPLOT      0      PLOT CONTROL
          QSCAL      0.      HYDROGRAPH PLOT SCALE

IT        HYDROGRAPH TIME DATA
          MNIN      30      MINUTES IN COMPUTATION INTERVAL
          IDATE      18APR88  STARTING DATE
          ITIME      1200     STARTING TIME
          NQ         200     NUMBER OF HYDROGRAPH ORDINATES
          MDDATE     22APR88  ENDING DATE
          NDTIME     1530     ENDING TIME

          COMPUTATION INTERVAL  0.50 HOURS
          TOTAL TIME BASE      99.50 HOURS
  
```

ENGLISH UNITS
 DRAINAGE AREA SQUARE MILES

PRECIPITATION DEPTH INCHES
 LENGTH, ELEVATION FEET
 FLOW CUBIC FEET PER SECOND
 STORAGE VOLUME ACRE-Feet
 SURFACE AREA ACRES
 TEMPERATURE DEGREES FAHRENHEIT

7
 8 KA 1 SUB 1 1
 1 1

RUNOFF FROM SUBBASIN 1
 SCS DIMENSIONLESS UNIT HYDROGRAPH
 LAGTIME = HYDRAULIC LENGTH, AVG. WATERSHED SLOPE AND C.N.
 RAINFALL DISTRIBUTION = SCS TYPE II

7 IN TIME DATA FOR INPUT TIME SERIES
 JXMIN 15 TIME INTERVAL IN MINUTES
 JXDATE 18APR88 STARTING DATE
 JXTIME 1200 STARTING TIME

SUBBASIN RUNOFF DATA

13 BA SUBBASIN CHARACTERISTICS
 TAREA 67.00 SUBBASIN AREA

PRECIPITATION DATA

14 PB STORM 3.88 BASIN TOTAL PRECIPITATION

15 PI INCREMENTAL PRECIPITATION PATTERN

0.00	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01
0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.02	0.02
0.02	0.03	0.05	0.38	0.07	0.04	0.03	0.02	0.02	0.02	0.01
0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01
0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01

25 LS SCS LOSS RATE
 STRTL 0.56 INITIAL ABSTRACTION
 CRYNBR 78.00 CURVE NUMBER
 RTIMP 0.00 PERCENT IMPERVIOUS AREA

26 UD SCS DIMENSIONLESS UNITGRAPH
 TLAG 5.80 LAG

UNIT HYDROGRAPH
 60 END-OF-PERIOD ORDINATES

133.	405.	766.	1213.	1772.	2479.	3313.	4055.	4645.	5061.
5302.	5346.	5311.	5114.	4827.	4506.	4147.	3705.	3187.	2713.
2328.	2028.	1763.	1542.	1369.	1208.	1057.	924.	792.	702.

38 KK * SUB 3 *
* *
XXXXXXXXXXXX

RUNOFF FROM SUBBASIN 3

SUBBASIN RUNOFF DATA

40 BA SUBBASIN CHARACTERISTICS
TAREA 43.00 SUBBASIN AREA

PRECIPITATION DATA

14 PB STORM 3.88 BASIN TOTAL PRECIPITATION

15 P1 INCREMENTAL PRECIPITATION PATTERN

0.00	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01
0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.02	0.02
0.02	0.03	0.05	0.38	0.07	0.04	0.03	0.02	0.02	0.02	0.01
0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01
0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01		

41 LS SCS LOSS RATE
STRTL 0.50 INITIAL ABSTRACTION
CRVNR 80.00 CURVE NUMBER
RTIMP 0.00 PERCENT IMPERVIOUS AREA

42 UD SCS DIMENSIONLESS UNITGRAPH
TLAG 4.81 LAG

UNIT HYDROGRAPH
50 END-OF-PERIOD ORDINATES

122.	404.	767.	1249.	1891.	2654.	3312.	3775.	4038.	4101.
4070.	3853.	3575.	3257.	2865.	2393.	1971.	1663.	1410.	1198.
1037.	889.	760.	639.	546.	465.	397.	337.	286.	242.
206.	176.	150.	128.	109.	93.	79.	67.	57.	49.
43.	38.	33.	28.	23.	19.	15.	11.	6.	2.

*** *** *** *** ***
HYDROGRAPH AT STATION SUB 3

TOTAL RAINFALL = 3.86, TOTAL LOSS = 1.94, TOTAL EXCESS = 1.94

PEAK FLOW (CFS)	TIME (HR)	MAXIMUM AVERAGE FLOW				
		6-HR	24-HR	72-HR	99.50-HR	
5890.	17.00	5043.	2234.	749.	542.	
		(INCHES)	1.090	1.932	1.943	1.943
		(AC-FT)	2501.	4431.	4456.	4456.

CUMULATIVE AREA = 43.00 SQ MI

3195. 2447. 1918. 1496. 1141. 889. 689. 528. 402. 313.
 244. 189. 145. 111. 87. 71. 57. 42. 30. 18.
 6.

*** HYDROGRAPH AT STATION SUB 5

TOTAL RAINFALL = 3.88, TOTAL LOSS = 2.01, TOTAL EXCESS = 1.87

PEAK FLOW (CFS)	TIME (HR)	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	99.50-HR
9311.	15.00	6690.	2443.	815.	590.
		(INCHES) 1.277	1.866	1.867	1.867
		(AC-FT) 3318.	4845.	4848.	4848.

CUMULATIVE AREA = 48.70 SQ MI

70 KK 3 COMBINE 1-4A WITH 5

ADD TWO HYDROGRAPHS

72 HC HYDROGRAPH COMBINATION
 ICOMP 2 NUMBER OF HYDROGRAPHS TO COMBINE

*** HYDROGRAPH AT STATION 3

PEAK FLOW (CFS)	TIME (HR)	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	99.50-HR
19493.	22.50	18935.	15480.	6696.	4859.
		(INCHES) 0.459	1.500	1.946	1.952
		(AC-FT) 9389.	30705.	39842.	39955.

CUMULATIVE AREA = 383.80 SQ MI

73 KK 3 ROUTE HYDROGRAPH 1-5 THROUGH REACH 3

HYDROGRAPH ROUTING DATA

74 RL ROUTING LOSSES
 QLOSS 0.00 INITIAL LOSS
 CLOSS 0.00 ADDITIONAL FRACTION LOST

75 RS STORAGE ROUTING
 NSTPS 10 NUMBER OF SUBREACHES
 ITYP FLOW TYPE OF INITIAL CONDITION
 RSVRIC -1.00 INITIAL CONDITION
 X 0.00 WORKING A AND B COEFFICIENT

76 RC NORMAL DEPTH CHANNEL ROUTING
 ANL 0.055 LEFT OVBANK N-VALUE
 ANCH 0.040 MAIN CHANNEL N-VALUE
 ANR 0.055 RIGHT OVBANK N-VALUE
 RLNTH 18750. REACH LENGTH
 SEL 0.0022 ENERGY SLOPE
 ELMAX 0.0 MAX. ELEV. FOR STORAGE/OUTFLOW CALCULATION

CROSS-SECTION DATA

	--- LEFT OVBANK ---		+ ----- MAIN CHANNEL -----				+ --- RIGHT OVBANK ---	
78 RY ELEVATION	2028.00	2026.00	2023.00	2020.00	2020.00	2023.00	2026.00	2028.00
77 RX DISTANCE	0.00	300.00	400.00	800.00	1500.00	1800.00	2400.00	2700.00

COMPUTED STORAGE-OUTFLOW-ELEVATION DATA

STORAGE	0.00	135.77	289.35	460.73	649.91	856.91	1081.71	1324.31	1584.72	1862.94
OUTFLOW	0.00	296.75	969.34	1963.72	3272.12	4898.21	6850.42	9139.56	12173.77	15648.70
ELEVATION	2020.00	2020.42	2020.84	2021.26	2021.68	2022.11	2022.53	2022.95	2023.37	2023.79
STORAGE	2158.96	2472.79	2804.42	3153.86	3521.11	3907.59	4316.81	4748.92	5203.92	5681.82
OUTFLOW	19521.01	23794.42	28476.41	33573.50	39092.34	45021.45	51412.53	58273.07	65610.36	73434.74
ELEVATION	2024.21	2024.63	2025.05	2025.47	2025.89	2026.32	2026.74	2027.16	2027.58	2028.00

*** WARNING *** MODIFIED PULS ROUTING MAY BE NUMERICALLY UNSTABLE FOR OUTFLOWS BETWEEN 969. TO 73435.
 THE ROUTED HYDROGRAPH SHOULD BE EXAMINED FOR OSCILLATIONS OR OUTFLOWS GREATER THAN PEAK INFLOWS.
 THIS CAN BE CORRECTED BY DECREASING THE TIME INTERVAL OR INCREASING STORAGE (USE A LONGER REACH.)

*** HYDROGRAPH AT STATION 3 ***

PEAK FLOW	TIME	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	99.50-HR
+ (CFS)	(HR)				
+ 19501.	23.50	18917.	15436.	6689.	4857.
	(INCHES)	0.458	1.496	1.944	1.951
	(AC-FT)	9380.	30617.	39802.	39938.
PEAK STORAGE	TIME	MAXIMUM AVERAGE STORAGE			
		6-HR	24-HR	72-HR	99.50-HR
+ (AC-FT)	(HR)				
+ 216.	23.50	211.	184.	92.	67.
PEAK STAGE	TIME	MAXIMUM AVERAGE STAGE			

JTYP FLOW TYPE OF INITIAL CONDITION
 RSVRIC -1.00 INITIAL CONDITION
 X 0.00 WORKING R AND D COEFFICIENT

95 RC NORMAL DEPTH CHANNEL ROUTING
 ANL 0.050 LEFT OVBANK N-VALUE
 ANCH 0.040 MAIN CHANNEL N-VALUE
 ANR 0.055 RIGHT OVBANK N-VALUE
 RLNTH 3854.1 REACH LENGTH
 SEL 0.0023 ENERGY SLOPE
 ELMAX 0.0 MAX. ELEV. FOR STORAGE/OUTFLOW CALCULATION

CROSS-SECTION DATA

	--- LEFT OVBANK ---	+ -----	MAIN CHANNEL	----- +	--- RIGHT OVBANK ---
97 RY ELEVATION	1946.00	1944.00	1942.00	1940.00	1940.00 1942.00 1944.00 1946.00
96 RX DISTANCE	0.00	200.00	500.00	800.00	1500.00 1700.00 1900.00 2100.00

COMPUTED STORAGE-OUTFLOW-ELEVATION DATA

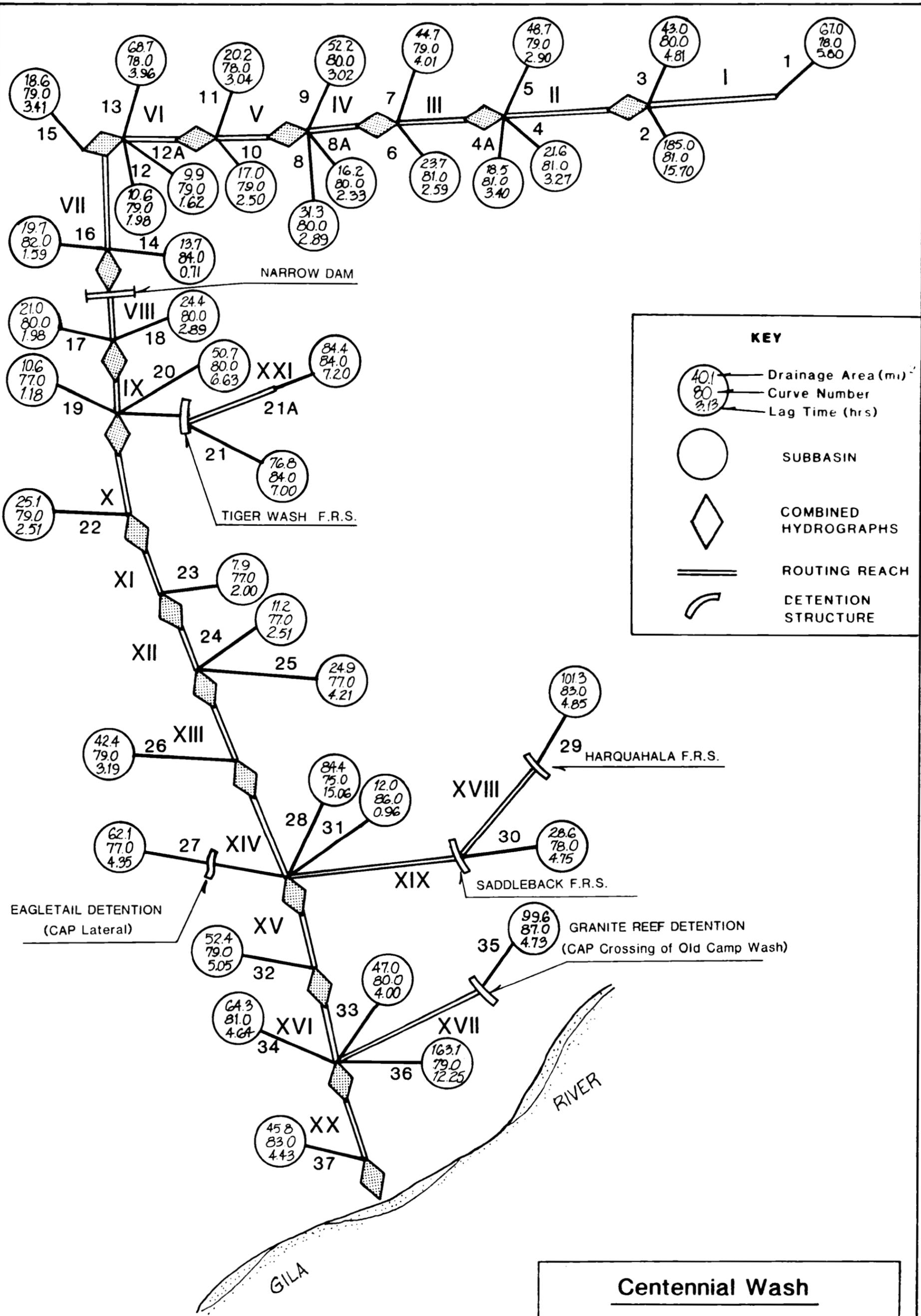
STORAGE	0.00	206.61	435.28	686.01	958.80	1253.64	1570.55	1909.51	2270.53	2653.60
OUTFLOW	0.00	186.86	606.63	1220.96	2021.18	3006.15	4178.09	5679.08	7458.64	9454.66
ELEVATION	1940.00	1940.32	1940.63	1940.95	1941.26	1941.58	1941.89	1942.21	1942.53	1942.84
STORAGE	3058.74	3485.93	3935.19	4406.25	4895.94	5403.28	5928.27	6470.90	7031.18	7609.10
OUTFLOW	11670.34	14108.74	16773.07	19671.49	22809.45	26177.34	29776.47	33608.45	37675.14	41978.54
ELEVATION	1943.16	1943.47	1943.79	1944.11	1944.42	1944.74	1945.05	1945.37	1945.68	1946.00

*** WARNING *** MODIFIED PULS ROUTING MAY BE NUMERICALLY UNSTABLE FOR OUTFLOWS BETWEEN 5679. TO 41979.
 THE ROUTED HYDROGRAPH SHOULD BE EXAMINED FOR OSCILLATIONS OR OUTFLOWS GREATER THAN PEAK INFLOWS.
 THIS CAN BE CORRECTED BY DECREASING THE TIME INTERVAL OR INCREASING STORAGE (USE A LONGER REACH.)

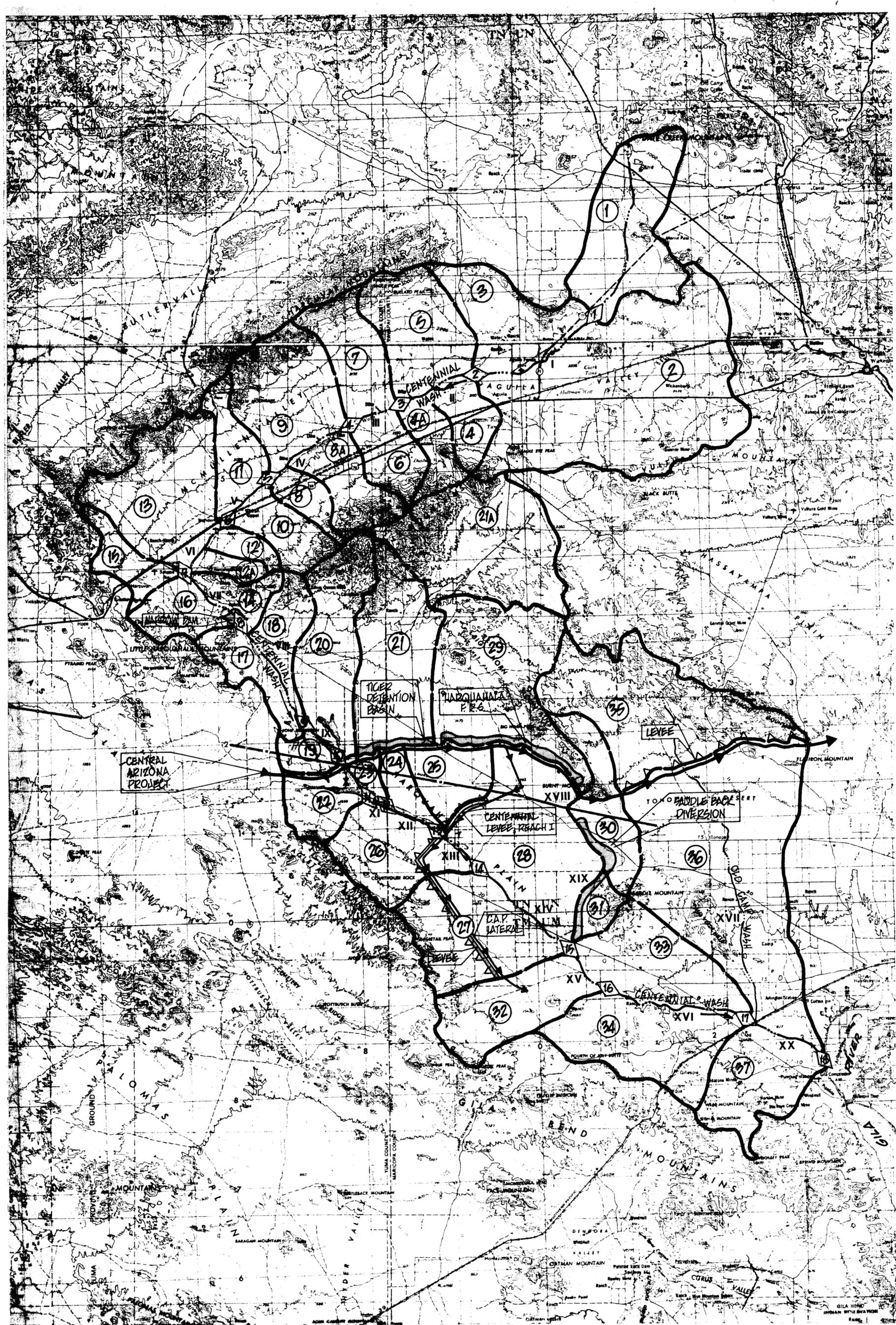
*** HYDROGRAPH AT STATION 4 ***

PEAK FLOW	TIME	MAXIMUM AVERAGE FLOW				
		6-HR	24-HR	72-HR	99.50-HR	
+ (CFS)	(HR)					
+ 26730.	18.00	22948.	18262.	7832.	5693.	
		(INCHES)	0.472	1.502	1.932	1.941
		(AC-FT)	11379.	36221.	46601.	46814.
PEAK STORAGE	TIME	MAXIMUM AVERAGE STORAGE				
		6-HR	24-HR	72-HR	99.50-HR	
+ (AC-FT)	(HR)					
+ 548.	18.00	491.	416.	207.	152.	
PEAK STAGE	TIME	MAXIMUM AVERAGE STAGE				
		6-HR	24-HR	72-HR	99.50-HR	
+ (FEET)	(HR)					
+ 1944.79	18.00	1944.43	1943.92	1942.15	1941.60	

CUMULATIVE AREA = 452.20 SQ MI



Centennial Wash
SCHEMATIC OF THE
BASIN LOGIC USED IN HEC-1



NOTE: EXTERIOR DRAINAGE BASIN BOUNDARIES AND INTERIOR SUB-BASIN BOUNDARIES WERE DELINEATED ON U.S.G.S. TOPOGRAPHIC MAPS (7.5 MINUTE, 1:24000). THIS MAP IS AN EXHIBIT TO SHOW BASIN DIVISIONS AS INPUT IN THE HEC-1 ANALYSIS AND SHOULD NOT BE USED TO CHECK BASIN DELINEATION OR AREA.

KEY

-  CONCENTRATION POINT
-  DRAINAGE BASIN BOUNDARY
-  SUBBASIN BOUNDARY
-  SUBBASIN NUMBER
-  ROUTING REACH NUMBER
-  CENTRAL ARIZONA PROJECT
-  DETENTION STRUCTURE



1" = 4 miles
Approximate Scale

FIGURE 3

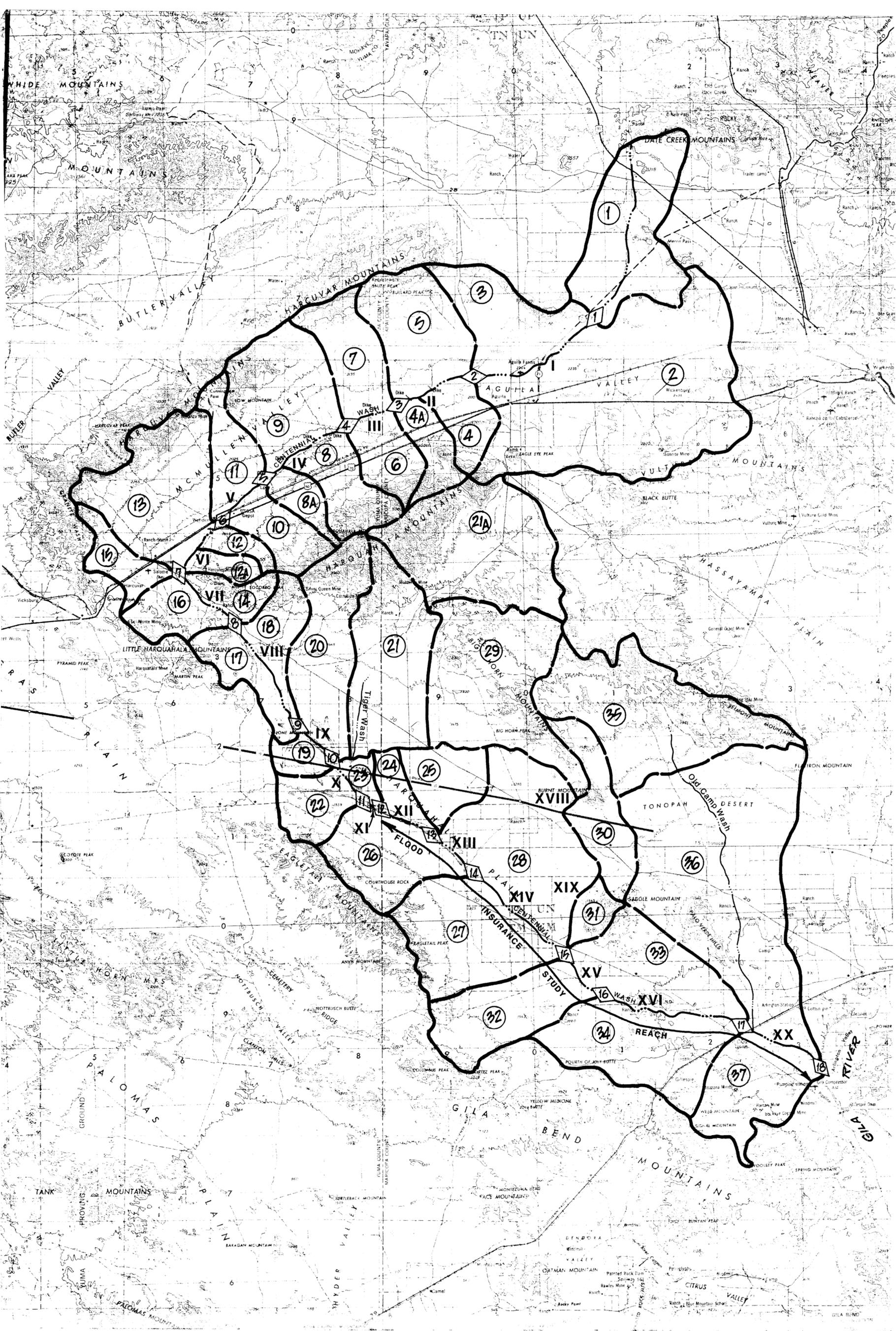
**Centennial Wash
Flood Insurance Study
Maricopa County, Arizona**

TOPOGRAPHIC MAP SHOWING
FLOOD CONTROL STRUCTURES
ALONG CENTENNIAL WASH

CBA CELLA BARR ASSOCIATES 2075 North Sixth Avenue
Tucson, Arizona 85705
(602) 624-7401

JOB NO. 04856-04-74 SEPTEMBER 1988

REVISED DECEMBER 1988



NOTE: EXTERIOR DRAINAGE BASIN BOUNDARIES AND INTERIOR SUB-BASIN BOUNDARIES WERE DELINEATED ON U.S.G.S. TOPOGRAPHIC MAPS (7.5 MINUTE, 1:24000). THIS MAP IS AN EXHIBIT TO SHOW BASIN DIVISIONS AS INPUT IN THE HEC-1 ANALYSIS AND SHOULD NOT BE USED TO CHECK BASIN DELINEATION OR AREA.

KEY	
	CONCENTRATION POINTS
	DRAINAGE BASIN BOUNDARY
	SUBBASIN BOUNDARY
	SUBBASIN NUMBER
	ROUTING REACH NUMBER



FIGURE 2

**Centennial Wash
Flood Insurance Study
Maricopa County, Arizona**

TOPOGRAPHIC MAP SHOWING
CENTENNIAL WASH SUBBASINS

CBA CELLA BARR ASSOCIATES 2075 North Sixth Avenue
Tucson, Arizona 85705
(602) 624-7401

JOB NO. 04856-04-74 SEPTEMBER 1988

REVISED DECEMBER, 1988

CUMULATIVE AREA = 551.90 SQ MI

*** ** ** ** **

XXXXXXXXXXXXXXXXXXXX

119 RK ROUTE HYDROGRAPH 1-9 THROUGH REACH 5

XXXXXXXXXXXXXXXXXXXX

HYDROGRAPH ROUTING DATA

120 RL ROUTING LOSSES
QLOSS 0.00 INITIAL LOSS
CLOSS 0.00 ADDITIONAL FRACTION LOST

121 RS STORAGE ROUTING
NSTPS 6 NUMBER OF SUBREACHES
ITYP FLOW TYPE OF INITIAL CONDITION
RSVRC -1.00 INITIAL CONDITION
X 0.00 WORKING R AND D COEFFICIENT

122 RC NORMAL DEPTH CHANNEL ROUTING
ANL 0.055 LEFT OVERBANK N-VALUE
ANCH 0.040 MAIN CHANNEL N-VALUE
ANR 0.055 RIGHT OVERBANK N-VALUE
RLNTH 18229. REACH LENGTH
SEL 0.0033 ENERGY SLOPE
ELMAX 0.0 MAX. ELEV. FOR STORAGE/OUTFLOW CALCULATION

CROSS-SECTION DATA

Table with 9 columns: ELEVATION, DISTANCE, and 7 values for channel and overbank elevations. Rows 124 RY and 123 RX.

COMPUTED STORAGE-OUTFLOW-ELEVATION DATA

Table with 11 columns: STORAGE, OUTFLOW, ELEVATION. Rows for STORAGE, OUTFLOW, ELEVATION at two different points.

*** WARNING *** MODIFIED PULS ROUTING MAY BE NUMERICALLY UNSTABLE FOR OUTFLOWS BETWEEN 330. TO 23353.
THE ROUTED HYDROGRAPH SHOULD BE EXAMINED FOR OSCILLATIONS OR OUTFLOWS GREATER THAN PEAK INFLOWS.
THIS CAN BE CORRECTED BY DECREASING THE TIME INTERVAL OR INCREASING STORAGE (USE A LONGER REACH.)

*** ** ** ** **
HYDROGRAPH AT STATION 5

PEAK FLOW TIME MAXIMUM AVERAGE FLOW
6-HR 24-HR 72-HR 99.50-HR

HYDROGRAPH ROUTING DATA

139 RL ROUTING LOSSES
QLOSS 0.00 INITIAL LOSS
CLOSS 0.00 ADDITIONAL FRACTION LOST

140 RS STORAGE ROUTING
NSTPS 6 NUMBER OF SUBREACHES
ITYP FLOW TYPE OF INITIAL CONDITION
RSVNIC -1.00 INITIAL CONDITION
X 0.00 WORKING P AND D COEFFICIENT

141 RC NORMAL DEPTH CHANNEL ROUTING
ANL 0.050 LEFT OVBANK N-VALUE
ANM 0.035 MAIN CHANNEL N-VALUE
ANR 0.050 RIGHT OVBANK N-VALUE
RLNTH 26042. REACH LENGTH
SEL 0.0029 ENERGY SLOPE
ELMAZ 0.0 MAX. ELEV. FOR STORAGE/OUTFLOW CALCULATION

CROSS-SECTION DATA

Table with 9 columns: ELEVATION, DISTANCE, and 7 intermediate values. Rows 143 R) and 142 RX.

COMPUTED STORAGE-OUTFLOW-ELEVATION DATA

Table with 11 columns: STORAGE, OUTFLOW, ELEVATION, STORAGE, OUTFLOW, ELEVATION. Rows 1-6.

*** WARNING *** MODIFIED PULS ROUTING MAY BE NUMERICALLY UNSTABLE FOR OUTFLOWS BETWEEN 12024. TO 78251.
THE ROUTED HYDROGRAPH SHOULD BE EXAMINED FOR OSCILLATIONS OR OUTFLOWS GREATER THAN PEAK INFLOWS.
THIS CAN BE CORRECTED BY DECREASING THE TIME INTERVAL OR INCREASING STORAGE (USE A LONGER REACH.)

*** HYDROGRAPH AT STATION 6 ***

Table with 6 columns: PEAK FLOW TIME, MAXIMUM AVERAGE FLOW (6-HR, 24-HR, 72-HR, 99.50-HR), PEAK STORAGE TIME, MAXIMUM AVERAGE STORAGE (6-HR, 24-HR, 72-HR, 99.50-HR). Rows 1-4.

(CFS)	(HR)	(CFS)	6-HR	24-HR	72-HR	99.50-HR
2648.	14.00		1582.	532.	177.	128.
		(INCHES)	1.388	1.866	1.867	1.867
		(AC-FT)	785.	1055.	1055.	1055.

CUMULATIVE AREA = 10.60 SQ MI

*** ** ** ** **

147 KA SUB12A
 RUNOFF FROM SUBBASIN 12A

SUBBASIN RUNOFF DATA

151 BA SUBBASIN CHARACTERISTICS
 TAREA 9.90 SUBBASIN AREA

PRECIPITATION DATA

14 PB STORM 3.86 BASIN TOTAL PRECIPITATION

15 PI INCREMENTAL PRECIPITATION PATTERN

0.00	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01
0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.02
0.02	0.03	0.05	0.38	0.07	0.04	0.03	0.02	0.02	0.01
0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01
0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01

152 LS SCS LOSS RATE
 STRTL 0.53 INITIAL ABSTRACTION
 CRVMBF 79.00 CURVE NUMBER
 RTIMP 0.00 PERCENT IMPERVIOUS AREA

153 UD SCS DIMENSIONLESS UNITGRAPH
 TLAG 1.62 LAG

UNIT HYDROGRAPH
 18 END-OF-PERIOD ORDINATES

411.	1372.	2383.	2541.	2125.	1422.	888.	587.	373.	245.
157.	101.	66.	43.	28.	20.	12.	5.		

*** ** ** **
 HYDROGRAPH AT STATION SUB12A

TOTAL RAINFALL = 3.88, TOTAL LOSS = 2.01, TOTAL EXCESS = 1.87

PEAK FLOW TIME MAXIMUM AVERAGE FLOW

HYDROGRAPH AT STATION 9

PEAK FLOW (CFS)	TIME (HR)	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	99.50-HR
45375.	19.50	39335.	26472.	11563.	8419.
		(INCHES) 0.539	1.451	1.902	1.914
		(AC-FT) 19505.	52506.	68803.	69227.

CUMULATIVE AREA = 678.30 SQ MI

XX

165 KK
 SUB15

RUNOFF FROM SUBBASIN 15

SUBBASIN RUNOFF DATA

167 BA
 SUBBASIN CHARACTERISTICS
 TAREA 18.60 SUBBASIN AREA

PRECIPITATION DATA

14 PB STORM 3.86 BASIN TOTAL PRECIPITATION

15 PI INCREMENTAL PRECIPITATION PATTERN

0.00	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01
0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.02	0.02
0.02	0.03	0.05	0.38	0.07	0.04	0.03	0.02	0.02	0.02	0.01
0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01
0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01

168 LS SCS LOSS RATE
 STRTL 0.53 INITIAL ABSTRACTION
 CRVNR 79.00 CURVE NUMBER
 RTIMP 0.00 PERCENT IMPERVIOUS AREA

169 WD SCS DIMENSIONLESS UNITGRAPH
 TLAG 3.41 LAG

UNIT HYDROGRAPH
36 END-OF-PERIOD ORDINATES

137.	407.	799.	1370.	1946.	2311.	2443.	2431.	2232.	1981.
1661.	1278.	998.	794.	643.	521.	418.	332.	265.	214.
170.	134.	109.	88.	70.	56.	45.	36.	29.	24.
20.	16.	12.	9.	5.	2.				

QLOSS 0.00 INITIAL LOSS
 CLOSS 0.00 ADDITIONAL FRACTION LOST

175 RS STORAGE ROUTING
 NSTPS 6 NUMBER OF SUBREACHES
 ITYP FLOW TYPE OF INITIAL CONDITION
 RSVRIC -1.00 INITIAL CONDITION
 X 0.00 WORKING R AND D COEFFICIENT

176 RC NORMAL DEPTH CHANNEL ROUTING
 ANL 0.055 LEFT OVBANK N-VALUE
 ANCH 0.040 MAIN CHANNEL N-VALUE
 ANP 0.055 RIGHT OVBANK N-VALUE
 RLNTH 2652. REACH LENGTH
 SEL 0.0050 ENERGY SLOPE
 ELMAX 0.0 MAX. ELEV. FOR STORAGE/OUTFLOW CALCULATION

CROSS-SECTION DATA

	--- LEFT OVBANK ---	+	----- MAIN CHANNEL -----	+	--- RIGHT OVBANK ---
176 RY ELEVATION	1764.00		1763.00 1760.00 1760.00 1764.00		1762.00 1764.00
177 RX DISTANCE	0.00		900.00 950.00 1110.00 1130.00		1140.00 1400.00 1700.00

COMPUTED STORAGE-OUTFLOW-ELEVATION DATA

STORAGE	0.00	20.83	42.25	64.25	86.84	110.02	133.78	158.13	183.06	208.58
OUTFLOW	0.00	31.59	100.89	199.49	324.14	472.95	644.69	838.45	1053.60	1289.61
ELEVATION	1760.00	1760.21	1760.42	1760.63	1760.84	1761.05	1761.26	1761.47	1761.68	1761.89
STORAGE	251.89	316.15	385.17	458.98	537.56	627.63	745.59	892.21	1067.48	1271.39
OUTFLOW	1555.05	1885.39	2285.68	2740.63	3253.63	3721.45	4361.61	5130.76	6037.80	7098.64
ELEVATION	1762.11	1762.32	1762.53	1762.74	1762.95	1763.16	1763.37	1763.58	1763.79	1764.00

*** WARNING *** MODIFIED PULS ROUTING MAY BE NUMERICALLY UNSTABLE FOR OUTFLOWS BETWEEN 838. TO 1290.
 THE ROUTED HYDROGRAPH SHOULD BE EXAMINED FOR OSCILLATIONS OR OUTFLOWS GREATER THAN PEAK INFLOWS.
 THIS CAN BE CORRECTED BY DECREASING THE TIME INTERVAL OR INCREASING STORAGE (USE A LONGER REACH.)

*** HYDROGRAPH AT STATION 7 ***

PEAK FLOW	TIME	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	99.50-HR
(CFS)	(HR)				
44034.	22.00	40136.	27147.	11866.	8635.
(INCHES)		0.535	1.449	1.900	1.911
(AC-FT)		19902.	53846.	70608.	71011.
PEAK STORAGE	TIME	MAXIMUM AVERAGE STORAGE			
		6-HR	24-HR	72-HR	99.50-HR
(AC-FT)	(HR)				
1395.	22.00	1270.	854.	369.	269.
PEAK STAGE	TIME	MAXIMUM AVERAGE STAGE			
		6-HR	24-HR	72-HR	99.50-HR
(FEET)	(HR)				
1771.33	22.00	1770.56	1767.98	1764.57	1763.43

CUMULATIVE AREA = 696.90 SQ MI

*** **

X X
179 KK X SUB14 X
X X

RUNOFF FROM SUBBASIN 14

SUBBASIN RUNOFF DATA

181 BA SUBBASIN CHARACTERISTICS
TAREA 13.70 SUBBASIN AREA

PRECIPITATION DATA

14 PB STORM 3.88 BASIN TOTAL PRECIPITATION

15 FI INCREMENTAL PRECIPITATION PATTERN

0.00	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01
0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.02	0.02
0.02	0.03	0.05	0.38	0.07	0.04	0.03	0.02	0.02	0.02	0.01
0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01
0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01

182 LS SCS LOSS RATE
STRTL 0.38 INITIAL ABSTRACTION
CRVMBR 84.00 CURVE NUMBER
RTIMP 0.00 PERCENT IMPERVIOUS AREA

183 UD SCS DIMENSIONLESS UNITGRAPH
TLAG 0.71 LAG

UNIT HYDROGRAPH
9 END-OF-PERIOD ORDINATES

3542. 6922. 4205. 1735. 739. 317. 136. 63. 22.

*** **

HYDROGRAPH AT STATION SUB14

TOTAL RAINFALL = 3.88, TOTAL LOSS = 1.61, TOTAL EXCESS = 2.27

PEAK FLOW (CFS)	TIME (HR)	MAXIMUM AVERAGE FLOW				
		6-HR	24-HR	72-HR	99.50-HR	
7985.	12.50	2675.	835.	278.	201.	
		(INCHES)	1.815	2.266	2.266	2.266
		(AC-FT)	1326.	1655.	1655.	1655.

CUMULATIVE AREA = 13.70 SQ MI

*** ** ** ** **

XXXXXXXXXXXXX
184 KK
SUB16
XXXXXXXXXXXXX

RUNOFF FROM SUBBASIN 16

SUBBASIN RUNOFF DATA

166 BA
SUBBASIN CHARACTERISTICS
AREA 19.70 SUBBASIN AREA

PRECIPITATION DATA

14 PE STORM 3.88 BASIN TOTAL PRECIPITATION

15 PI INCREMENTAL PRECIPITATION PATTERN

0.00	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01
0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.02	0.02
0.02	0.03	0.05	0.38	0.07	0.04	0.03	0.02	0.02	0.02	0.01
0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01
0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01

167 LS
SCS LOSE RATE
STRTL 0.44 INITIAL ABSTRACTION
CRVMBR 82.00 CURVE NUMBER
RTIMP 0.00 PERCENT IMPERVIOUS AREA

188 UD
SCS DIMENSIONLESS UNITGRAPH
TLAG 1.59 LAG

UNIT HYDROGRAPH
18 END-OF-PERIOD ORDINATES

851.	2858.	4857.	5127.	4205.	2739.	1701.	1120.	713.	462.
291.	190.	123.	77.	52.	35.	20.	6.		

HYDROGRAPH AT STATION SUB16

TOTAL RAINFALL = 3.88, TOTAL LOSS = 1.78, TOTAL EXCESS = 2.10

PEAK FLOW (CFS)	TIME (HR)	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	99.50-HR
6509.	13.50	3441.	1113.	371.	268.
	(INCHES)	1.624	2.101	2.101	2.101
	(AC-FT)	1706.	2207.	2207.	2207.

CUMULATIVE AREA = 19.70 SQ MI

ANL 0.050 LEFT OVBANK N-VALUE
 ANCH 0.035 MAIN CHANNEL N-VALUE
 ANR 0.050 RIGHT OVBANK N-VALUE
 RLNTH 49479. REACH LENGTH
 SEL 0.0040 ENERGY SLOPE
 ELMAX 0.0 MAX. ELEV. FOR STORAGE/OUTFLOW CALCULATION

CROSS-SECTION DATA

	--- LEFT OVBANK ---	+ ----- MAIN CHANNEL -----	+ --- RIGHT OVBANK ---
203 RY ELEVATION	1610.00	1600.00 1606.00 1604.00	1600.00 1605.00 1608.00
202 RA DISTANCE	0.00	500.00 550.00 640.00	650.00 800.00 820.00 1500.00

COMPUTED STORAGE-OUTFLOW-ELEVATION DATA

STORAGE	0.00	90.70	183.44	278.23	375.06	473.94	574.86	677.83	783.92	903.01
OUTFLOW	0.00	139.13	443.50	875.27	1419.54	2067.47	2813.11	3652.14	4434.17	5123.70
ELEVATION	1600.00	1600.53	1601.05	1601.58	1602.11	1602.63	1603.16	1603.68	1604.21	1604.74
STORAGE	1046.28	1266.27	1570.14	1951.28	2408.99	2943.27	3531.28	4125.97	4787.49	5605.58
OUTFLOW	5866.66	6897.23	8506.76	10635.66	13151.84	16100.30	19781.32	23982.34	28531.96	33805.11
ELEVATION	1605.26	1605.79	1606.32	1606.84	1607.37	1607.89	1608.42	1608.95	1609.47	1610.00

*** WARNING *** MODIFIED PULS ROUTING MAY BE NUMERICALLY UNSTABLE FOR OUTFLOWS BETWEEN 875. TO 33805.
 THE ROUTED HYDROGRAPH SHOULD BE EXAMINED FOR OSCILLATIONS OR OUTFLOWS GREATER THAN PEAK INFLOWS.
 THIS CAN BE CORRECTED BY DECREASING THE TIME INTERVAL OR INCREASING STORAGE (USE A LONGER REACH.)

*** HYDROGRAPH AT STATION 8 ***

PEAK FLOW	TIME	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	99.50-HR
(CFS)	(HR)				
+ 43685.	24.50	40446.	27524.	12374.	9038.
(INCHES)		0.515	1.402	1.890	1.908
(AC-FT)		20056.	54592.	73630.	74324.

PEAK STORAGE	TIME	MAXIMUM AVERAGE STORAGE			
		6-HR	24-HR	72-HR	99.50-HR
(AC-FT)	(HR)				
+ 714.	24.50	664.	467.	217.	160.

PEAK STAGE	TIME	MAXIMUM AVERAGE STAGE			
		6-HR	24-HR	72-HR	99.50-HR
(FEET)	(HR)				
+ 1610.99	24.50	1610.66	1609.26	1605.74	1604.33

CUMULATIVE AREA = 730.30 SQ MI

 204 KA I SUB17 I
 I I

221 RX DISTANCE 0.00 800.00 1000.00 2300.00 2600.00 3500.00 4000.00 4500.00

COMPUTED STORAGE-OUTFLOW-ELEVATION DATA

STORAGE	0.00	45.04	180.17	405.39	720.02	1101.77	1539.82	2034.18	2583.03	3169.73
OUTFLOW	0.00	119.41	758.20	2235.44	4876.97	9015.96	14459.10	21291.91	30263.53	41806.25
ELEVATION	1440.00	1440.53	1441.05	1441.58	1442.11	1442.63	1443.16	1443.68	1444.21	1444.74
STORAGE	3790.21	4444.47	5135.55	5875.96	6667.04	7508.79	8392.21	9298.72	10227.74	11179.29
OUTFLOW	54948.43	69683.13	85864.91	103680.58	123241.28	144565.05	168049.84	193406.25	220520.98	249380.84
ELEVATION	1445.26	1445.79	1446.32	1446.84	1447.37	1447.89	1448.42	1448.95	1449.47	1450.00

*** WARNING *** MODIFIED PULS ROUTING MAY BE NUMERICALLY UNSTABLE FOR OUTFLOWS BETWEEN 9016. TO 249381. THE ROUTED HYDROGRAPH SHOULD BE EXAMINED FOR OSCILLATIONS OR OUTFLOWS GREATER THAN PEAK INFLOWS. THIS CAN BE CORRECTED BY DECREASING THE TIME INTERVAL OR INCREASING STORAGE (USE A LONGER REACH.)

*** HYDROGRAPH AT STATION 12 ***

PEAK FLOW (CFS)	TIME (HR)	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	99.50-HR
44836.	25.00	41548.	28165.	13115.	9595.
		(INCHES) 0.498	1.350	1.886	1.907
		(AC-FT) 20602.	55864.	78039.	78901.

PEAK STORAGE (AC-FT)	TIME (HR)	MAXIMUM AVERAGE STORAGE			
		6-HR	24-HR	72-HR	99.50-HR
826.	25.00	788.	602.	322.	239.

PEAK STAGE (FEET)	TIME (HR)	MAXIMUM AVERAGE STAGE			
		6-HR	24-HR	72-HR	99.50-HR
1444.86	25.00	1444.72	1444.02	1442.69	1442.09

CUMULATIVE AREA = 775.70 SQ MI

223 KK SUB19

RUNOFF FROM SUBBASIN 19

SUBBASIN RUNOFF DATA

225 BA SUBBASIN CHARACTERISTICS TAREA 10.60 SUBBASIN AREA

PRECIPITATION DATA

QLOSS 0.00 INITIAL LOSS
 CLOSS 0.00 ADDITIONAL FRACTION LOST

266 RS STORAGE ROUTING
 MSTPS 10 NUMBER OF SUBREACHES
 ITYP FLOW TYPE OF INITIAL CONDITION
 RSVRIC -1.00 INITIAL CONDITION
 X 0.00 WORKING R AND D COEFFICIENT

267 RC NORMAL DEPTH CHANNEL ROUTING
 ANL 0.055 LEFT OVBANK M-VALUE
 ANCH 0.035 MAIN CHANNEL M-VALUE
 ANR 0.060 RIGHT OVBANK M-VALUE
 RLNTH 19271. REACH LENGTH
 SEL 0.0033 ENERGY SLOPE
 ELMAX 0.0 MAX. ELEV. FOR STORAGE/OUTFLOW CALCULATION

CROSS-SECTION DATA

	---	LEFT OVBANK	---	+	-----	MAIN CHANNEL	-----	+	---	RIGHT OVBANK	---
269 RY	ELEVATION	1370.00	1366.00	1362.00	1360.00	1362.00	1364.00	1366.00	1368.00		
268 RX	DISTANCE	0.00	800.00	1000.00	2300.00	2600.00	3500.00	4000.00	4500.00		

COMPUTED STORAGE-OUTFLOW-ELEVATION DATA

STORAGE	0.00	49.02	196.08	441.18	783.58	1199.02	1675.73	2213.72	2811.03	3449.50
OUTFLOW	0.00	111.28	706.56	2083.18	4544.80	8401.18	13470.68	19831.90	28181.84	38918.44
ELEVATION	1360.00	1360.53	1361.05	1361.58	1362.11	1362.63	1363.16	1363.68	1364.21	1364.74
STORAGE	4124.75	4836.76	5588.84	6394.60	7255.50	8171.56	9132.95	10119.47	11130.50	12166.04
OUTFLOW	51129.51	64803.35	79812.03	96309.83	114387.55	134057.55	155657.00	178936.15	203795.94	230223.00
ELEVATION	1365.26	1365.79	1366.32	1366.84	1367.37	1367.89	1368.42	1368.95	1369.47	1370.00

*** WARNING *** MODIFIED PULS ROUTING MAY BE NUMERICALLY UNSTABLE FOR OUTFLOWS BETWEEN 707. TO 230223.
 THE ROUTED HYDROGRAPH SHOULD BE EXAMINED FOR OSCILLATIONS OR OUTFLOWS GREATER THAN PEAK INFLOWS.
 THIS CAN BE CORRECTED BY DECREASING THE TIME INTERVAL OR INCREASING STORAGE (USE A LONGER REACH.)

*** HYDROGRAPH AT STATION 10 ***

PEAK FLOW	TIME	MAXIMUM AVERAGE FLOW				
		6-HR	24-HR	72-HR	99.50-HR	
+ (CFS)	(HR)					
+ 51882.	25.50	48703.	35127.	16780.	12415.	
		(INCHES)	0.454	1.309	1.876	1.918
		(AC-FT)	24150.	69674.	99847.	102092.
PEAK STORAGE	TIME	MAXIMUM AVERAGE STORAGE				
		6-HR	24-HR	72-HR	99.50-HR	
+ (AC-FT)	(HR)					
416.	25.50	399.	319.	177.	134.	
PEAK STAGE	TIME	MAXIMUM AVERAGE STAGE				
		6-HR	24-HR	72-HR	99.50-HR	
+ (FEET)	(HR)					
1365.29	25.50	1365.16	1364.51	1363.10	1362.48	

CUMULATIVE AREA = 998.20 SQ MI

XXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXX

XXXXXXXXXXXX

270 KK

1 7
1 SUB22 1
1 1
XXXXXXXXXXXX

RUNOFF FROM SUBBASIN 22

SUBBASIN RUNOFF DATA

272 BA

SUBBASIN CHARACTERISTICS

TAREA 25.10 SUBBASIN AREA

PRECIPITATION DATA

14 PE

STORM 3.82 BASIN TOTAL PRECIPITATION

15 P1

INCREMENTAL PRECIPITATION PATTERN

0.00	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01
0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.02	0.02
0.02	0.03	0.05	0.38	0.07	0.04	0.03	0.02	0.02	0.02	0.01
0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01
0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01

273 LS

SCS LOSS RATE

STRTL 0.53 INITIAL ABSTRACTION
CRVMBR 79.00 CURVE NUMBER
RTIMP 0.00 PERCENT IMPERVIOUS AREA

274 UD

SCS DIMENSIONLESS UNITGRAPH

TLAG 2.51 LAG

UNIT HYDROGRAPH

27 END-OF-PERIOD ORDINATES

381.	1162.	2426.	3719.	4349.	4352.	3874.	3208.	2325.	1682.
1245.	951.	705.	526.	392.	290.	215.	161.	120.	89.
65.	50.	40.	30.	21.	13.	5.			

HYDROGRAPH AT STATION SUB22

TOTAL RAINFALL = 3.88, TOTAL LOSS = 2.01, TOTAL EXCESS = 1.87

PEAK FLOW

TIME

MAXIMUM AVERAGE FLOW

6-HR 24-HR 72-HR 99.50-HR

+ (CFS)

(HR)

(CFS)

+ 5310.

14.50

3578.

1259.

420.

304.

(INCHES)

1.325

1.866

1.867

1.867

(AC-FT)

1774.

2498.

2499.

2499.

CUMULATIVE AREA = 25.10 SQ MI

*** **

 275 KK 1 16 1 COMBINE 1-21 WITH 22

ADD TWO HYDROGRAPHS

277 HC HYDROGRAPH COMBINATION
 ICOMF 2 NUMBER OF HYDROGRAPHS TO COMBINE

*** **
 HYDROGRAPH AT STATION 16

PEAK FLOW (CFS)	TIME (HR)	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	99.50-HR
+ 52431.	25.50	49148.	35494.	17153.	12719.
		(INCHES) 0.447	1.290	1.870	1.916
		(AC-FT) 24371.	70401.	102068.	104591.

CUMULATIVE AREA = 1023.30 SQ MI

*** **

 278 KK 1 11 1 ROUTE HYDROGRAPH 1-22 THROUGH REACH 11

HYDROGRAPH ROUTING DATA

279 RL ROUTING LOSSES
 QLOSS 0.00 INITIAL LOSS
 CLOSS 0.00 ADDITIONAL FRACTION LOST

280 RS STORAGE ROUTING
 NSTPS 2 NUMBER OF SUBREACHES
 ITYP FLOW TYPE OF INITIAL CONDITION
 RSVRIC -1.00 INITIAL CONDITION
 X 0.00 WORKING R AND D COEFFICIENT

281 RC NORMAL DEPTH CHANNEL ROUTING
 ANL 0.055 LEFT OVBANK N-VALUE
 ANCH 0.035 MAIN CHANNEL N-VALUE

COMPUTED STORAGE-OUTFLOW-ELEVATION DATA

STORAGE	0.00	45.04	180.17	405.39	720.14	1105.82	1553.45	2063.00	2633.23	3252.47
OUTFLOW	0.00	100.65	639.11	1884.31	4110.99	7606.96	12224.02	18045.46	25701.19	35581.62
ELEVATION	1265.00	1265.53	1266.05	1266.58	1267.11	1267.63	1268.16	1268.68	1269.21	1269.74
STORAGE	3917.88	4639.46	5384.67	6173.11	6993.64	7846.26	8730.97	9647.78	10589.39	11545.16
OUTFLOW	46914.40	59716.16	74042.81	89870.61	107164.32	125923.40	146151.53	167855.28	191581.93	216821.03
ELEVATION	1270.26	1270.79	1271.32	1271.84	1272.37	1272.89	1273.42	1273.95	1274.47	1275.00

*** WARNING *** MODIFIED PULS ROUTING MAY BE NUMERICALLY UNSTABLE FOR OUTFLOWS BETWEEN 18045.70 216831.
 - THE ROUTED HYDROGRAPH SHOULD BE EXAMINED FOR OSCILLATIONS OR OUTFLOWS GREATER THAN PEAK INFLOWS.
 THIS CAN BE CORRECTED BY DECREASING THE TIME INTERVAL OR INCREASING STORAGE (USE A LONGER PEACH.)

*** HYDROGRAPH AT STATION 12 ***

PEAK FLOW	TIME	MAXIMUM AVERAGE FLOW
(CFS)	(HR)	6-HR 24-HR 72-HR 99.50-HR
+ 52163.	27.00	49995. 35560. 17215. 12724.
		(INCHES) 0.443 1.282 1.863 1.902
		(AC-FT) 24345. 70532. 102436. 104629.

PEAK STORAGE	TIME	MAXIMUM AVERAGE STORAGE
(AC-FT)	(HR)	6-HR 24-HR 72-HR 99.50-HR
+ 1052.	27.00	1009. 806. 451. 341.

PEAK STAGE	TIME	MAXIMUM AVERAGE STAGE
(FEET)	(HR)	6-HR 24-HR 72-HR 99.50-HR
+ 1270.48	27.00	1270.35 1269.69 1268.26 1267.59

CUMULATIVE AREA = 1031.20 SQ MI

 SUBBASIN RUNOFF DATA
 SUBBASIN CHARACTERISTICS
 TAREA 11.20 SUBBASIN AREA
 PRECIPITATION DATA
 STORM 3.88 BASIN TOTAL PRECIPITATION
 INCREMENTAL PRECIPITATION PATTERN
 0.00 0.01 0.01 0.01 0.01 0.01 0.01 0.01 0.01 0.01

300 BA
 SUBBASIN CHARACTERISTICS
 TAREA 11.20 SUBBASIN AREA
 PRECIPITATION DATA

14 PB STORM 3.88 BASIN TOTAL PRECIPITATION
 INCREMENTAL PRECIPITATION PATTERN
 0.00 0.01 0.01 0.01 0.01 0.01 0.01 0.01 0.01 0.01

15 P1 INCREMENTAL PRECIPITATION PATTERN
 0.00 0.01 0.01 0.01 0.01 0.01 0.01 0.01 0.01 0.01

PEAK FLOW + (CFS)	TIME (HR)	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	99.50-HR
+ 52898.	26.50	49842.	36228.	17709.	13126.
		(INCHES) 0.434	1.262	1.851	1.896
		(AC-FT) 24715.	71858.	105377.	107938.

CUMULATIVE AREA = 1067.30 SQ MI

*** **

 311 KK 2 13 2 ROUTE HYDROGRAPH 1-25 THROUGH REACH 13

HYDROGRAPH ROUTING DATA

312 RL ROUTING LOSSES
 QLOSS 0.00 INITIAL LOSS
 CLOSS 0.00 ADDITIONAL FRACTION LOST

313 RS STORAGE ROUTING
 NSTPS 6 NUMBER OF SUBREACHES
 ITYP FLOW TYPE OF INITIAL CONDITION
 RSVRIC -1.00 INITIAL CONDITION
 X 0.00 WORKING R AND D COEFFICIENT

314 RC NORMAL DEPTH CHANNEL ROUTING
 ANL 0.050 LEFT OVBANK N-VALUE
 ANCH 0.035 MAIN CHANNEL N-VALUE
 ANR 0.050 RIGHT OVBANK N-VALUE
 RLNTH 21354. REACH LENGTH
 SEL 0.0027 ENERGY SLOPE
 ELMAX 0.0 MAX. ELEV. FOR STORAGE/OUTFLOW CALCULATION

CROSS-SECTION DATA

	--- LEFT OVBANK ---	+ --- MAIN CHANNEL ---	+ --- RIGHT OVBANK ---
316 RY ELEVATION	1184.00	1182.00 1180.00 1179.90 1180.00	1182.00 1180.00 1181.40
315 RX DISTANCE	0.00	2200.00 2900.00 4100.00 4300.00	5300.00 6250.00 7470.00

COMPUTED STORAGE-OUTFLOW-ELEVATION DATA

STORAGE	0.00	121.00	321.07	571.28	871.63	1222.11	1622.74	2073.47	2563.46	3083.69
OUTFLOW	0.00	157.10	670.89	1509.85	2685.67	4218.42	6130.94	8452.32	11293.86	14555.89
ELEVATION	1179.90	1180.12	1180.33	1180.55	1180.76	1180.98	1181.19	1181.41	1181.63	1181.84
STORAGE	3633.98	4210.75	4812.64	5439.63	6091.74	6768.95	7471.28	8198.71	8951.26	9728.91
OUTFLOW	18335.17	22851.67	27872.09	33384.53	39385.46	45875.11	52855.70	60330.64	68304.11	76780.82
ELEVATION	1182.06	1182.27	1182.49	1182.71	1182.92	1183.14	1183.35	1183.57	1183.78	1184.00

*** WARNING *** MODIFIED PULS ROUTING MAY BE NUMERICALLY UNSTABLE FOR OUTFLOWS BETWEEN 22852. TO 76781.

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189 KK * 11 * COMBINE 1-15 WITH 14 AND 16

ADD THREE HYDROGRAPHS

191 MC HYDROGRAPH COMBINATION
ICOMP 3 NUMBER OF HYDROGRAPHS TO COMBINE

*** ** ** ** ** ** **
HYDROGRAPH AT STATION 11

PEAK FLOW (CFS)	TIME (HR)	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	99.50-HR
44991.	21.50	41164.	27682.	12492.	9105.
		(INCHES) 0.524	1.410	1.908	1.922
		(AC-FT) 20412.	54907.	74334.	74874.

CUMULATIVE AREA = 730.30 SQ MI

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192 KA * 1 * ROUTE (LOWER CENTENNIAL - NARROWS DAM)

ROUTE TOTAL HYDROGRAPH THROUGH RESERVOIR

HYDROGRAPH ROUTING DATA

174 RL ROUTING LOSSES
QLOSS 0.00 INITIAL LOSS
CLOSS 0.00 ADDITIONAL FRACTION LOST

194 RS STORAGE ROUTING
NSTPS 1 NUMBER OF SUBREACHES
ITYP ELEV TYPE OF INITIAL CONDITION
RSVRIC 224.00 INITIAL CONDITION
X 0.00 WORKING R AND D COEFFICIENT

195 SV STORAGE 0.0 0.0 0.0 500.0 850.0 1240.0 1700.0 2300.0 2620.0 2900.0

196 SE ELEVATION 224.00 232.00 236.86 243.86 245.86 247.86 249.86 251.86 252.86 253.86

(INCHES) 0.421 1.234 1.844 1.891
 (AC-FT) 24910. 73027. 109164. 111929.

CUMULATIVE AREA = 1109.70 SQ MI

*** ** ** ** **

 * *
 * 14 *
 * *

336 KK ROUTE HYDROGRAPH 1-26 THROUGH REACH 14

HYDROGRAPH ROUTING DATA

337 RL ROUTING LOSSES
 QLOSS 0.00 INITIAL LOSS
 CLOSS 0.00 ADDITIONAL FRACTION LOST

338 RS STORAGE ROUTING
 NSTPS 14 NUMBER OF SUBREACHES
 ITYP FLOW TYPE OF INITIAL CONDITION
 RSVRJC -1.00 INITIAL CONDITION
 X 0.00 MORNING R AND C COEFFICIENT

339 RC NORMAL DEPTH CHANNEL ROUTING
 ANL 0.065 LEFT OVBANK N-VALUE
 ANCH 0.045 MAIN CHANNEL N-VALUE
 ANR 0.065 RIGHT OVBANK N-VALUE
 RLWTH 46354. REACH LENGTH
 SEL 0.0027 ENERGY SLOPE
 ELMAX 0.0 MAX. ELEV. FOR STORAGE/OUTFLOW CALCULATION

CROSS-SECTION DATA

	---	LEFT OVBANK	---	+	-----	MAIN CHANNEL	-----	+	---	RIGHT OVBANK	---
341 RY	ELEVATION	1100.00	1096.00	1092.00	1090.00	1088.00	1090.00	1092.00	1098.00		
340 RX	DISTANCE	0.00	2900.00	5300.00	5440.00	5630.00	5720.00	5730.00	8140.00		

COMPUTED STORAGE-OUTFLOW-ELEVATION DATA

STORAGE	0.00	29.71	118.85	267.42	465.83	696.47	958.94	1340.65	2125.70	3335.93
OUTFLOW	0.00	22.28	141.44	417.03	932.09	1662.43	2610.83	3958.31	5971.77	8887.00
ELEVATION	1088.00	1088.63	1089.26	1089.89	1090.53	1091.16	1091.79	1092.42	1093.05	1093.68
STORAGE	4971.35	7031.95	9517.74	12431.66	15812.03	19670.65	24005.14	28706.59	33715.77	39032.71
OUTFLOW	12947.70	18367.96	25343.48	33994.13	44481.53	57141.51	72368.65	91153.86	112509.87	136501.47
ELEVATION	1094.32	1094.95	1095.58	1096.21	1096.84	1097.47	1098.11	1098.74	1099.37	1100.00

*** WARNING *** MODIFIED PULS ROUTING MAY BE NUMERICALLY UNSTABLE FOR OUTFLOWS BETWEEN 1662. TO 136501.
 THE ROUTED HYDROGRAPH SHOULD BE EXAMINED FOR OSCILLATIONS OR OUTFLOWS GREATER THAN PEAK INFLOWS.
 THIS CAN BE CORRECTED BY DECREASING THE TIME INTERVAL OR INCREASING STORAGE (USE A LONGER REACH.)

356 KK * SUB28 *
* *
XXXXXXXXXXXX

RUNOFF FROM SUBBASIN 28

SUBBASIN RUNOFF DATA

358 BA SUBBASIN CHARACTERISTICS
TAREA 84.40 SUBBASIN AREA

PRECIPITATION DATA

320 PB STORM 4.12 BASIN TOTAL PRECIPITATION

321 PI INCREMENTAL PRECIPITATION PATTERN

0.00	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01
0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.02	0.02
0.02	0.03	0.05	0.38	0.07	0.04	0.03	0.02	0.02	0.02	0.01
0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01
0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01

359 LS SCS LOSS RATE
STRTL 0.67 INITIAL ABSTRACTION
CRVNER 75.00 CURVE NUMBER
RTIMP 0.00 PERCENT IMPERVIOUS AREA

360 UD SCS DIMENSIONLESS UNITGRAPH
TLAG 15.06 LAG

UNIT HYDROGRAPH TRUNCATED FROM 153 TO 150 INTERVALS

UNIT HYDROGRAPH
150 END-OF-PERIOD ORDINATES
VOLUME = 1.00

26.	52.	78.	137.	198.	259.	335.	413.	491.	591.
695.	800.	930.	1069.	1209.	1366.	1531.	1696.	1845.	1984.
2124.	2238.	2334.	2429.	2503.	2555.	2608.	2641.	2649.	2658.
2660.	2651.	2643.	2620.	2568.	2516.	2461.	2400.	2340.	2277.
2207.	2138.	2066.	1979.	1892.	1804.	1699.	1595.	1491.	1404.
1317.	1230.	1168.	1107.	1046.	993.	940.	888.	843.	800.
756.	722.	690.	658.	626.	595.	563.	535.	509.	483.
456.	430.	404.	383.	365.	348.	330.	313.	296.	280.
267.	254.	241.	228.	215.	203.	193.	183.	174.	164.
155.	146.	139.	133.	126.	119.	113.	106.	102.	97.
92.	87.	83.	78.	74.	71.	67.	64.	60.	57.
54.	51.	49.	46.	43.	41.	39.	37.	35.	34.
32.	30.	29.	28.	27.	26.	25.	24.	22.	21.
20.	19.	18.	17.	16.	15.	14.	13.	12.	11.
11.	10.	9.	8.	7.	6.	5.	4.	4.	3.

HYDROGRAPH AT STATION SUB28

PEAK STAGE (FEET)	TIME (HR)	MAXIMUM AVERAGE STAGE			
		6-HR	24-HR	72-HR	99.50-HR
1201.80	36.00	1201.80	1201.80	1201.79	1201.48

CUMULATIVE AREA = 101.30 SQ MI

*** ** ** ** **

397 KA

* *
* SUB30 *
* *

RUNOFF FROM SUBBASIN 30

SUBBASIN RUNOFF DATA

399 BA

SUBBASIN CHARACTERISTICS
TAREA 28.60 SUBBASIN AREA

PRECIPITATION DATA

372 PB

STORM 3.93 BASIN TOTAL PRECIPITATION

373 PI

INCREMENTAL PRECIPITATION PATTERN

0.00	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01
0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.02	0.02
0.02	0.03	0.05	0.38	0.07	0.04	0.03	0.02	0.02	0.02	0.01
0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01
0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01

400 LS

SCS LOSS RATE

STRTL 0.56 INITIAL ABSTRACTION
CRVNR 78.00 CURVE NUMBER
RTIMP 0.00 PERCENT IMPERVIOUS AREA

401 UD

SCS DIMENSIONLESS UNITGRAPH

TLAG 4.75 LAG

UNIT HYDROGRAPH
50 END-OF-PERIOD ORDINATES

83.	276.	525.	857.	1299.	1824.	2266.	2570.	2735.	2763.
2735.	2570.	2376.	2155.	1879.	1547.	1271.	1078.	912.	774.
673.	572.	489.	406.	351.	296.	254.	213.	182.	152.
131.	111.	95.	80.	69.	58.	50.	41.	36.	30.
27.	24.	20.	17.	14.	11.	8.	6.	3.	0.

HYDROGRAPH AT STATION SUB30

TOTAL RAINFALL = 3.93, TOTAL LOSS = 2.10, TOTAL EXCESS = 1.83

+	(AC-FT)	(HR)				
	1987.	28.00	1968.	1743.	1030.	769.

PEAK STAGE	TIME		MAXIMUM AVERAGE STAGE		
		6-HR	24-HR	72-HR	99.50-HR

+	(FEET)	(HR)				
	1187.38	28.00	1187.35	1186.89	1184.76	1183.41

CUMULATIVE AREA = 28.60 SQ MI

*** ** ** ** **

XXXXXXXXXXXX

408 AK	7	16	7	ROUTE HYDROGRAPH 30 THROUGH REACH 19
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HYDROGRAPH ROUTING DATA

409 RL	ROUTING LOSSES		
	QLLOSS	0.00	INITIAL LOSS
	CLLOSS	0.00	ADDITIONAL FRACTION LOST

410 RS	STORAGE ROUTING		
	NSTPS	12	NUMBER OF SUBREACHES
	ITYP		FLOW TYPE OF INITIAL CONDITION
	RSVRIC	-1.00	INITIAL CONDITION
	X	0.00	WORKING R AND D COEFFICIENT

411 RC	NORMAL DEPTH CHANNEL ROUTING		
	ANL	0.040	LEFT OVBANK N-VALUE
	ANCH	0.030	MAIN CHANNEL N-VALUE
	ANR	0.040	RIGHT OVBANK N-VALUE
	RLNTH	43229.	REACH LENGTH
	SEL	0.0038	ENERGY SLOPE
	ELMAX	0.0	MAX. ELEV. FOR STORAGE/OUTFLOW CALCULATION

CROSS-SECTION DATA

		--- LEFT OVBANK ---	+	----- MAIN CHANNEL -----	+	--- RIGHT OVBANK ---			
413 RY	ELEVATION	1149.50	1149.50	1146.00	1146.00	1143.00	1143.00	1146.00	1147.00
412 RX	DISTANCE	0.00	10.00	24.00	64.00	73.00	205.00	220.00	270.00

COMPUTED STORAGE-OUTFLOW-ELEVATION DATA

STORAGE	0.00	45.28	91.49	138.63	186.69	235.69	285.61	336.47	388.25	444.24
OUTFLOW	0.00	67.90	216.45	427.17	692.77	1008.94	1372.76	1782.10	2235.35	2346.87
ELEVATION	1143.00	1143.34	1143.68	1144.03	1144.37	1144.71	1145.05	1145.39	1145.74	1146.08
STORAGE	515.37	592.77	676.16	761.41	847.13	933.31	1019.95	1107.06	1194.64	1282.68
OUTFLOW	2938.74	3608.89	4354.90	5188.90	6088.56	7051.61	8076.16	9160.64	10303.70	11504.13
ELEVATION	1146.42	1146.76	1147.11	1147.45	1147.79	1148.13	1148.47	1148.82	1149.16	1149.50

*** WARNING *** MODIFIED PULS ROUTING MAY BE NUMERICALLY UNSTABLE FOR OUTFLOWS BETWEEN 216. TO 11504.
 THE ROUTED HYDROGRAPH SHOULD BE EXAMINED FOR OSCILLATIONS OR OUTFLOWS GREATER THAN PEAK INFLOWS.

432 RL ROUTING LOSSES
 QLOSS 0.00 INITIAL LOSS
 CLOSS 0.00 ADDITIONAL FRACTION LOST

433 RS STORAGE ROUTING
 NSTPS 14 NUMBER OF SUBREACHES
 ITYP FLOW TYPE OF INITIAL CONDITION
 RSVRIC -1.00 INITIAL CONDITION
 X 0.00 WORKING R AND D COEFFICIENT

434 RC NORMAL DEPTH CHANNEL ROUTING
 ANL 0.065 LEFT OVERTANK M-VALUE
 ANCH 0.040 MAIN CHANNEL M-VALUE
 ANR 0.055 RIGHT OVERTANK M-VALUE
 RLNTH 58333. REACH LENGTH
 SEL 0.0024 ENERGY SLOPE
 ELMAX 0.0 MAX. ELEV. FOR STORAGE/OUTFLOW CALCULATION

CROSS-SECTION DATA

	---	LEFT OVERTANK	---	+	-----	MAIN CHANNEL	-----	+	---	RIGHT OVERTANK	---
435 RY	ELEVATION	930.00	928.00	924.00	920.00	920.00	924.00	928.00	930.00		
435 RY	DISTANCE	0.00	700.00	1300.00	1400.00	1550.00	1600.00	1800.00	2100.00		

COMPUTED STORAGE-OUTFLOW-ELEVATION DATA

STORAGE	0.00	112.68	239.25	379.76	534.17	702.49	884.72	1080.87	1295.74	1573.95
OUTFLOW	0.00	96.16	313.47	633.66	1053.60	1573.89	2196.77	2925.26	3824.26	4956.51
ELEVATION	920.00	920.53	921.05	921.58	922.11	922.63	923.16	923.68	924.21	924.74
STORAGE	1926.36	2352.96	2853.74	3428.72	4077.89	4801.25	5634.41	6650.82	7852.71	9240.08
OUTFLOW	6270.91	7795.27	9553.58	11567.64	13857.81	16443.37	19090.04	22172.76	25793.16	29990.18
ELEVATION	925.26	925.79	926.32	926.84	927.37	927.89	928.42	928.95	929.47	930.00

*** WARNING *** MODIFIED PULS ROUTING MAY BE NUMERICALLY UNSTABLE FOR OUTFLOWS BETWEEN 2197. TO 16443.
 THE ROUTED HYDROGRAPH SHOULD BE EXAMINED FOR OSCILLATIONS OR OUTFLOWS GREATER THAN PEAK INFLOWS.
 THIS CAN BE CORRECTED BY DECREASING THE TIME INTERVAL OR INCREASING STORAGE (USE A LONGER REACH.)

***		***		***		***		***	
		HYDROGRAPH AT STATION				18			
PEAK FLOW	TIME	MAXIMUM AVERAGE FLOW							
		6-HR	24-HR	72-HR	99.50-HR				
+	(CFS)	(HR)	(CFS)						
+	57432.	36.50	54966.	41911.	21986.	16294.			
			(INCHES)						
			(AC-FT)						
			27256.	83130.	130826.	133987.			
PEAK STORAGE	TIME	MAXIMUM AVERAGE STORAGE							
		6-HR	24-HR	72-HR	99.50-HR				
+	(AC-FT)	(HR)							
	1308.	36.50	1250.	941.	486.	361.			
PEAK STAGE	TIME	MAXIMUM AVERAGE STAGE							
		6-HR	24-HR	72-HR	99.50-HR				
+	(FEET)	(HR)							
	933.44	36.50	933.13	931.49	928.29	926.45			

CUMULATIVE AREA = 1450.50 SQ MI

*** **

XXXXXXXXXXXXXXXX

437 KK SUB33

RUNOFF FROM SUBBASIN 33

SUBBASIN RUNOFF DATA

439 BA SUBBASIN CHARACTERISTICS
AREA 47.00 SUBBASIN AREA

PRECIPITATION DATA

372 PE STORM 3.93 BASIN TOTAL PRECIPITATION

Table with 10 columns showing incremental precipitation pattern values ranging from 0.00 to 0.03.

440 LS SCS LOSS RATE
STRTL 0.50 INITIAL ABSTRACTION
CRVNEP 80.00 CURVE NUMBER
RTMP 0.00 PERCENT IMPERVIOUS AREA

441 UD SCS DIMENSIONLESS UNITGRAPH
TLAG 4.00 LAG

UNIT HYDROGRAPH
42 END-OF-PERIOD ORDINATES

Table with 10 columns of hydrograph ordinates values ranging from 9 to 4615.

*** **

HYDROGRAPH AT STATION SUB33

TOTAL RAINFALL = 3.93, TOTAL LOSS = 1.95, TOTAL EXCESS = 1.98

PEAK FLOW TIME MAXIMUM AVERAGE FLOW
6-HR 24-HR 72-HR 99.50-HR
(CFS) (HR) (CFS)

463 RS STORAGE ROUTING 24 NUMBER OF SUBREACHES
 NSTPS FLOW TYPE OF INITIAL CONDITION
 ITYP -1.00 INITIAL CONDITION
 RSRVIC X 0.00 WORKING R AND D COEFFICIENT

464 RC NORMAL DEPTH CHANNEL ROUTING
 ANL 0.060 LEFT OVERTANK N-VALUE
 ANCH 0.035 MAIN CHANNEL N-VALUE
 ANR 0.060 RIGHT OVERTANK N-VALUE
 RLNTH 105600. REACH LENGTH
 SEL 0.0049 ENERGY SLOPE
 ELMA 0.0 MAX. ELEV. FOR STORAGE/OUTFLOW CALCULATION

CROSS-SECTION DATA
 --- LEFT OVERTANK ---+----- MAIN CHANNEL -----+--- RIGHT OVERTANK ---
 465 BY ELEVATION 1004.00 1003.00 1002.00 1000.00 1000.00 1000.00 1000.00 1000.00 1000.00 1004.00
 465 B) DISTANCE 0.00 200.00 400.00 600.00 800.00 1000.00 1200.00 1400.00

COMPUTED STORAGE-OUTFLOW-ELEVATION DATA										
STORAGE	0.00	112.82	247.12	402.92	580.21	778.98	999.24	1241.00	1504.24	1788.97
OUTFLOW	0.00	46.19	153.35	315.88	534.96	813.27	1154.00	1560.57	2032.47	2555.11
ELEVATION	1000.00	1000.21	1000.42	1000.63	1000.84	1001.05	1001.26	1001.47	1001.68	1001.89
STORAGE	2097.88	2447.07	2835.25	3274.41	3752.54	4273.65	4837.74	5444.81	6094.85	6787.66
OUTFLOW	3278.39	4125.85	5067.71	6109.25	7255.04	8509.32	9876.11	11359.29	12922.61	14685.76
ELEVATION	1002.11	1002.32	1002.53	1002.74	1002.95	1003.16	1003.37	1003.58	1003.79	1004.00

*** WARNING *** MODIFIED PULS ROUTING MAY BE NUMERICALLY UNSTABLE FOR OUTFLOWS BETWEEN 2555. TO 14230.
 THE ROUTED HYDROGRAPH SHOULD BE EXAMINED FOR OSCILLATIONS OR OUTFLOWS GREATER THAN PEAK INFLOWS.
 THIS CAN BE CORRECTED BY DECREASING THE TIME INTERVAL OR INCREASING STORAGE (USE A LONGER REACH.)

PEAK FLOW	TIME	HYDROGRAPH AT STATION 19		
		6-HR	24-HR	72-HR
+ (CFS)	(HR)	6078.	4878.	2265.
+ 6175.	29.00	(INCHES) 0.567	1.821	2.537
		(AC-FT) 3014.	9675.	13476.
PEAK STORAGE		MAXIMUM AVERAGE STORAGE		
	TIME	6-HR	24-HR	72-HR
+ (AC-FT)	(HR)	136.	115.	61.
+ 138.	29.00	MAXIMUM AVERAGE STAGE		
	TIME	6-HR	24-HR	72-HR
+ (FEET)	(HR)	1002.73	1002.47	1001.50
+ 1002.75	29.00	CUMULATIVE AREA = 99.60 SQ MI		

PEAK FLOW (CFS)	TIME (HR)	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	99.50-HR
10654.	25.50	10351.	7239.	2787.	2017.
		(INCHES) 0.590	1.651	1.907	1.907
		(AC-FT) 5133.	14359.	16587.	16587.

CUMULATIVE AREA = 163.10 SQ MI

 472 JJ 25 2 COMBINE 1-34 WITH 35 AND 36

 ADD THREE HYDROGRAPHS

474 HC HYDROGRAPH COMBINATION
 1 COMB NUMBER OF HYDROGRAPHS TO COMBINE
 25

PEAK FLOW (CFS)	TIME (HR)	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	99.50-HR
67675.	36.00	65106.	51898.	28816.	21421.
		(INCHES) 0.332	1.058	1.762	1.810
		(AC-FT) 32284.	102939.	171481.	176149.

CUMULATIVE AREA = 1824.50 SQ MI

 475 KK 20 2 ROUTE HYDROGRAPH 1-36 THROUGH REACH 20

HYDROGRAPH ROUTING DATA

476 RL ROUTING LOSSES
 QLOSS 0.00 INITIAL LOSS
 CLOSS 0.00 ADDITIONAL FRACTION LOST

477 RS STORAGE ROUTING
 NSTPS 6 NUMBER OF SUBREACHES

	6	38617.	19.50	32557.	23158.	10133.	589.10		
								1825.04	19.50
HYDROGRAPH AT									
	SUB12	2645.	14.00	1582.	532.	177.	10.60		
HYDROGRAPH AT									
	SUB12A	2816.	13.50	1527.	497.	166.	9.90		
3 COMBINED AT									
	8	39508.	19.50	33273.	23597.	10460.	609.66		
HYDROGRAPH AT									
	SUB13	9907.	16.50	8076.	3303.	1103.	68.70		
2 COMBINED AT									
	9	45375.	19.50	39335.	26472.	11563.	678.30		
HYDROGRAPH AT									
	SUB15	3154.	15.50	2423.	932.	311.	18.60		
2 COMBINED AT									
	10	46218.	19.50	40947.	27297.	11673.	696.90		
ROUTED TO									
	7	44034.	22.00	40136.	27147.	11866.	696.90	1771.33	22.00
HYDROGRAPH AT									
	SUB14	7985.	12.50	2675.	835.	278.	13.70		
HYDROGRAPH AT									
	SUB16	6509.	13.50	3441.	1113.	371.	19.70		
3 COMBINED AT									
	11	44991.	21.50	41164.	27682.	12492.	730.30		
ROUTED TO									
	1	44251.	22.50	40703.	27585.	12386.	730.30	258.69	22.50
ROUTED TO									
	8	43685.	24.50	40446.	27524.	12374.	730.30	1610.99	24.50
HYDROGRAPH AT									
	SUB17	5489.	14.00	3273.	1097.	366.	21.00		
HYDROGRAPH AT									
	SUB18	4891.	15.00	3503.	1274.	425.	24.40		
3 COMBINED AT									
	12	44910.	24.50	41593.	28177.	13128.	775.70		
ROUTED TO									
	12	44836.	25.00	41548.	28165.	13115.	775.70	1444.86	25.00

HYDROGRAPH AT

SUB25 3294. 16.50 2718. 1148. 384. 24.90

3 COMBINED AT

18 52898. 26.50 49842. 36228. 17709. 1067.30

ROUTED TO

13 52664. 28.00 49623. 36197. 17681. 1067.30

1183.35 28.00

HYDROGRAPH AT

SUB26 6348. 15.50 6240. 2348. 783. 42.40

2 COMBINED AT

19 53202. 26.00 50235. 36818. 16346. 1109.70

ROUTED TO

14 52167. 31.50 49473. 36666. 18325. 1109.70

1097.23 31.50

HYDROGRAPH AT

SUB27 8912. 16.50 7451. 3172. 1061. 62.10

ROUTED TO

3 952. 28.00 946. 888. 695. 62.10

1260.15 28.00

2 COMBINED AT

20 53110. 31.50 50410. 37547. 19020. 1171.80

HYDROGRAPH AT

SUB28 4285. 28.50 4195. 3190. 1328. 84.40

HYDROGRAPH AT

SUB31 6754. 13.00 2721. 857. 286. 12.00

3 COMBINED AT

21 57229. 31.50 54405. 40531. 20427. 1268.20

HYDROGRAPH AT

SUB29 15962. 17.00 13675. 6022. 2021. 101.30

ROUTED TO

4 340. 35.00 340. 339. 336. 101.30

1408.67 35.00

ROUTED TO

15 340. 36.00 340. 339. 336. 101.30

1201.80 36.00

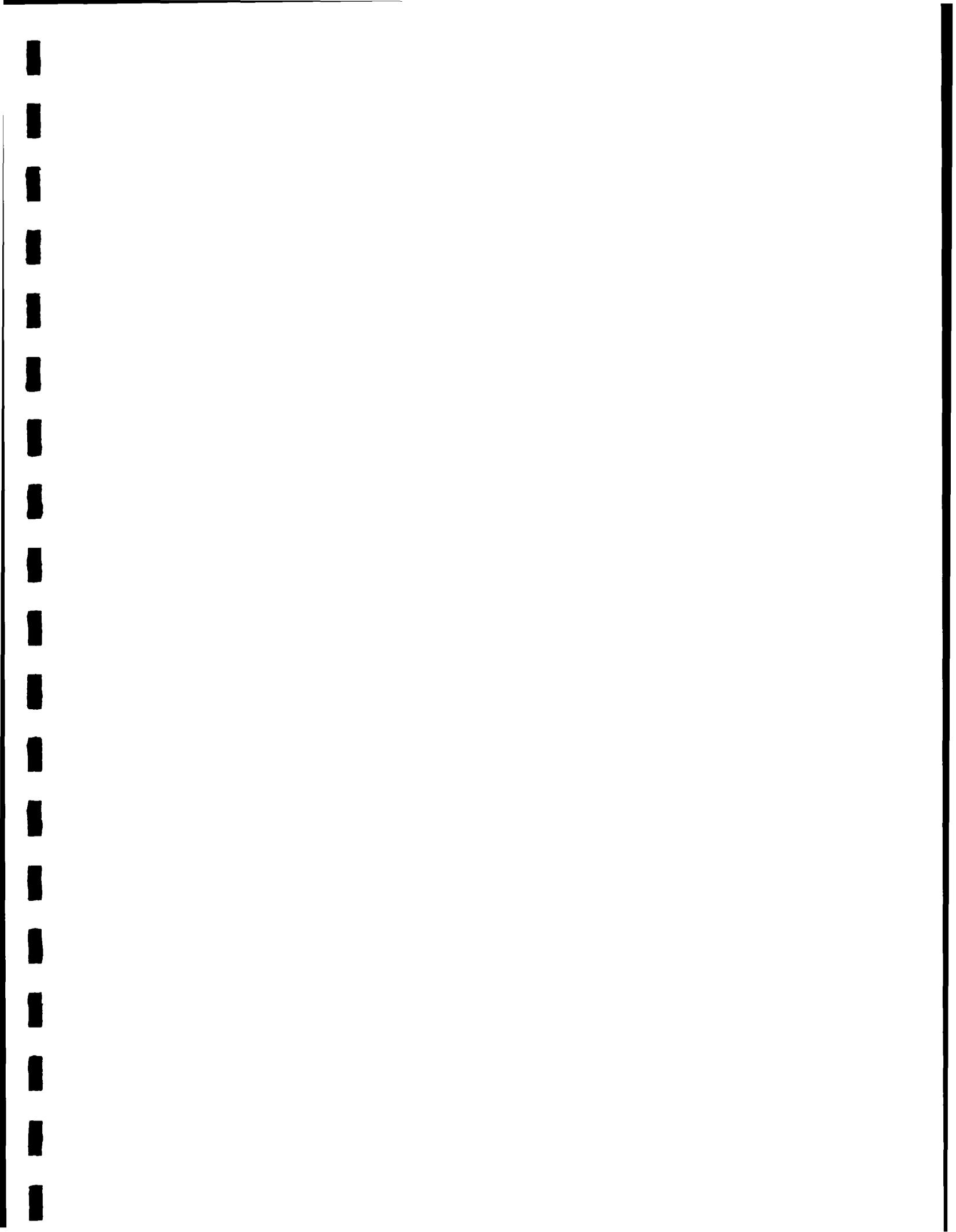
HYDROGRAPH AT

SUB30 3710. 17.00 3168. 1401. 469. 28.60

ROUTED TO

5 700. 28.00 698. 653. 431. 28.60

1187.39 28.00



STORM PRECIPITATION VOLUMES

R McArthur 1/84

PURPOSE: Precipitation volumes are required as input to the TR-20 rainfall-runoff model. The frequency of the precipitation amounts by duration are used as the basis of estimating the flood frequency.

PROCEDURE: The precipitation are taken from "NOAA Atlas 2, Precipitation-Frequency Atlas of the Western United States, Volume VIII, Arizona"; NWS, NOAA, USDC. Procedures outlined in this source were followed including using a grid to determine the average precipitation over the area of interest, conversion of partial duration to annual series, derivation of shorter duration precipitation volumes, and point-to-areal adjustments.

The recommendations of TSC TECHNICAL NOTE-HYDROLOGY-PO-6 (Rev.2) were also followed in using NOAA Atlas 2.

Due to the large size of the contributing area the area was divided into 5 subareas to determine if there was a significant difference in average point values by geographical location. The 5 subdivisions included:

- "A" - the area above Aguila (the upper part of the D.A.)
- "B" - the north side of McMullen Valley; bounded by Centennial Wash along the south side and the Harguavel mountains on the north side.
- "C" - the south side of McMullen Valley; bounded on the north by Centennial Wash and along the south by the Harguavel Mountains

"D" - the northwest part of Harguachala Valley including Tiger Wash; Centennial Wash as the southern boundary

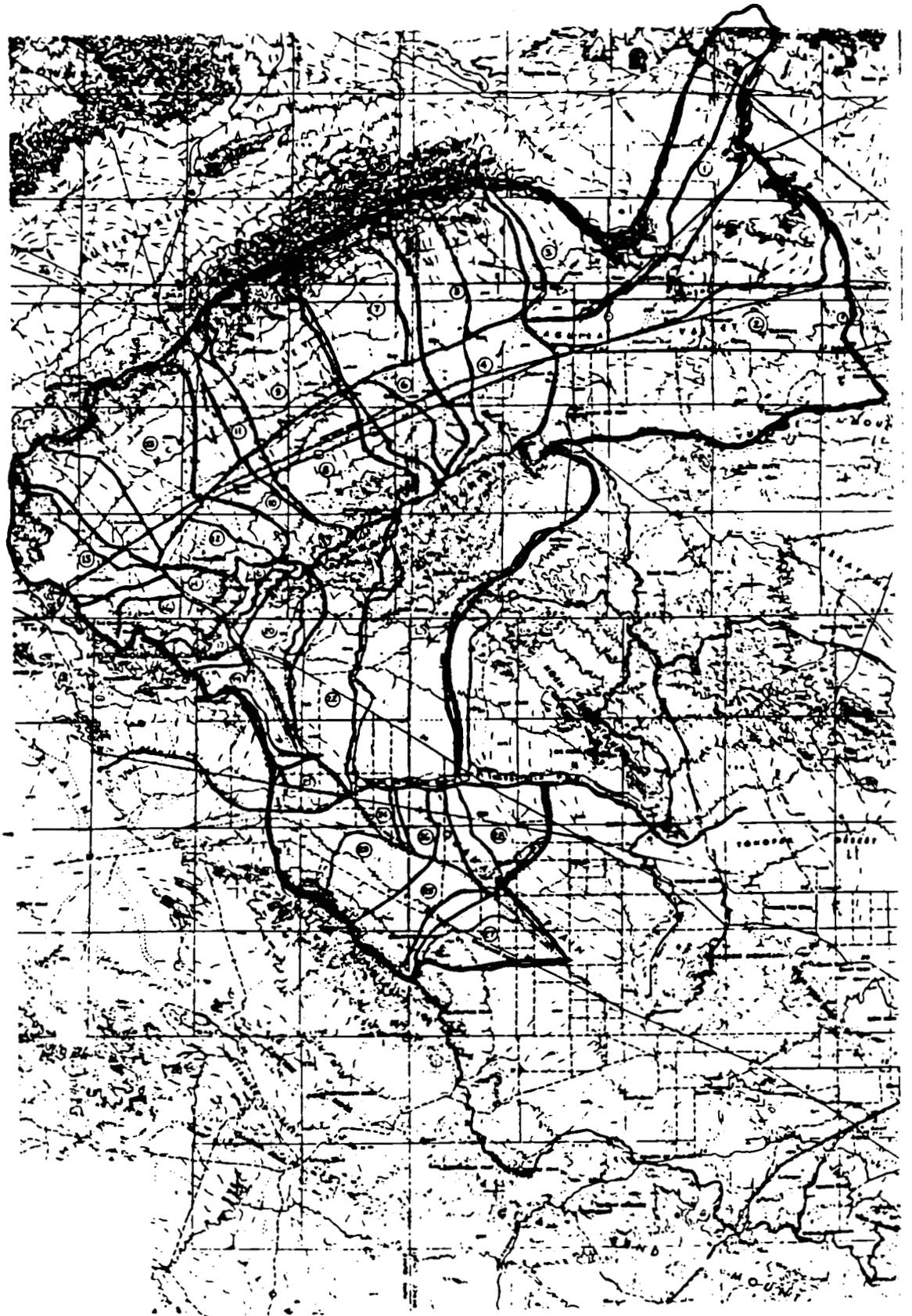
"E" the west part of Harguachala Valley bounded on the west by the low elevation extension of the Little Harguachala Mountains and the Eagletail Mountains and on the east by Centennial Wash

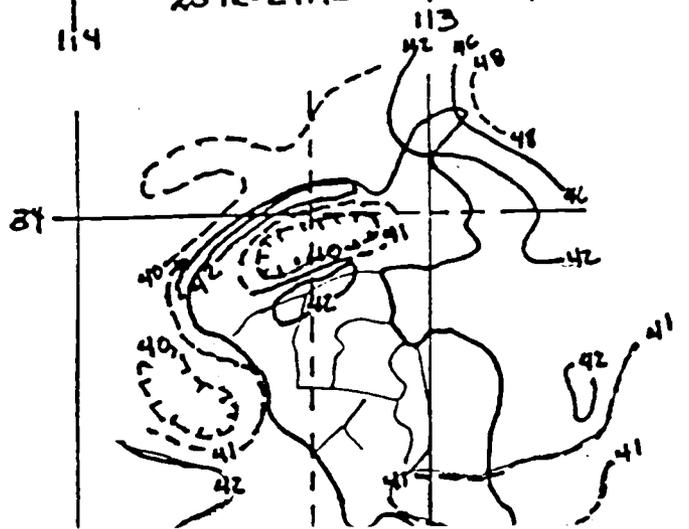
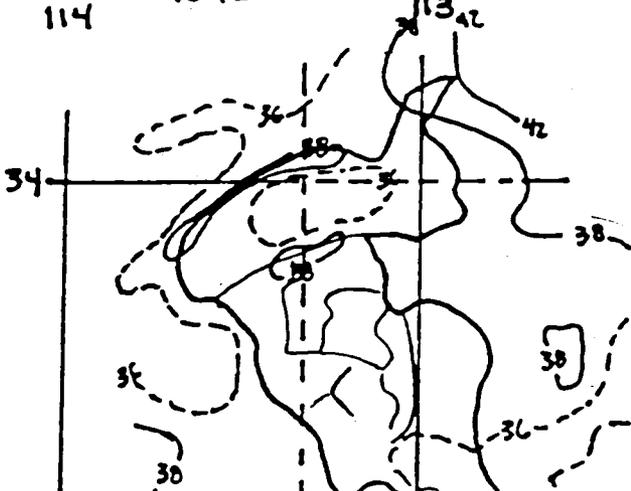
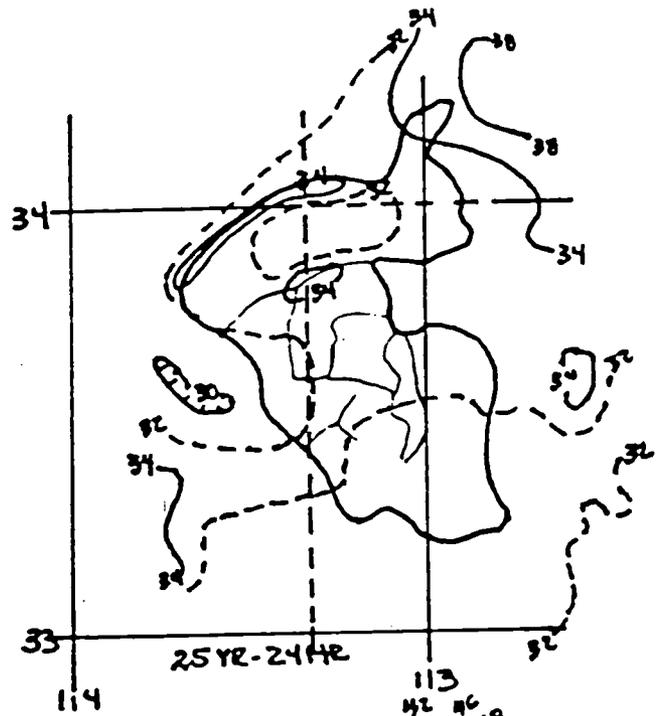
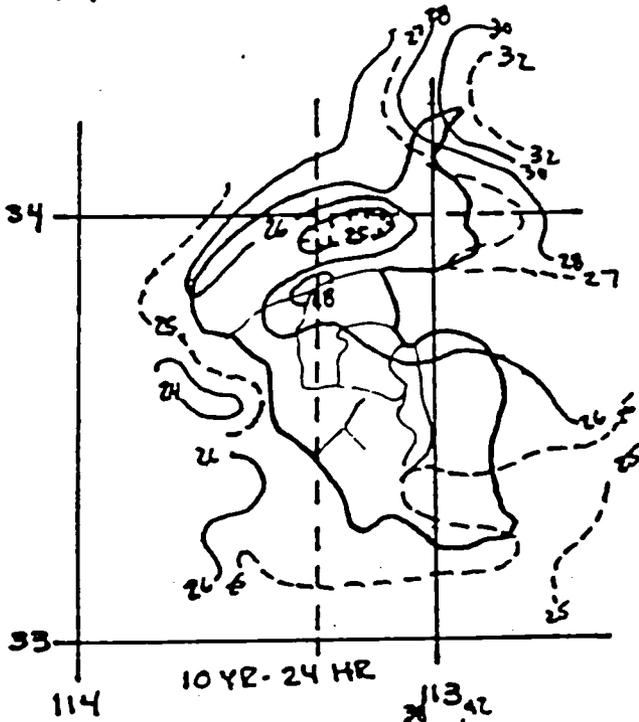
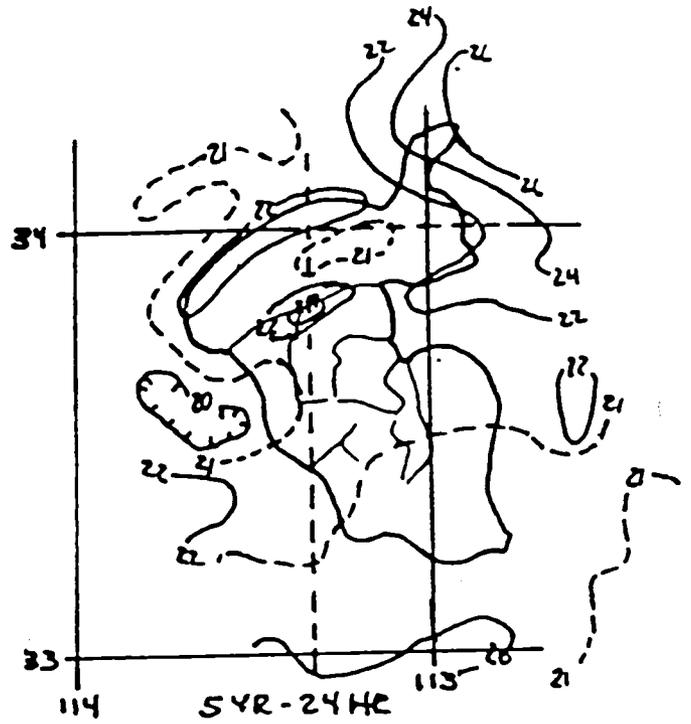
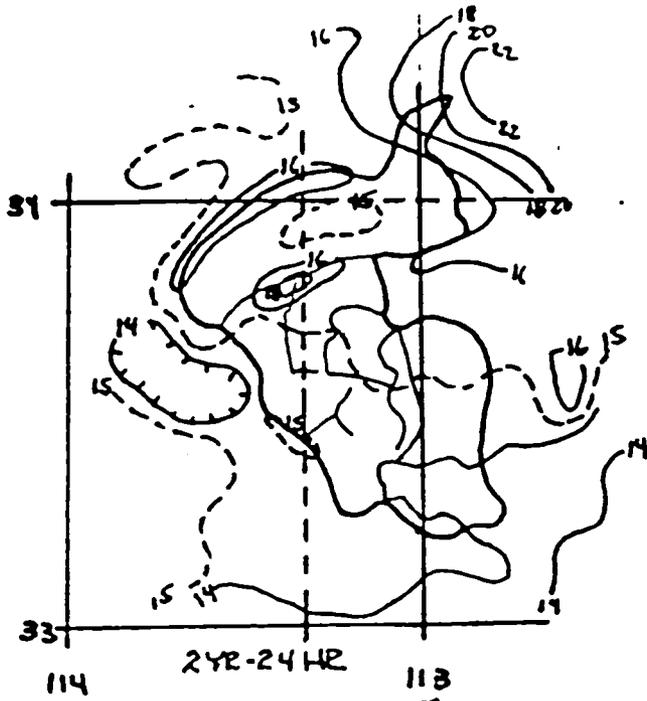
Fifty grid points were used to derive the average point precipitation volumes.

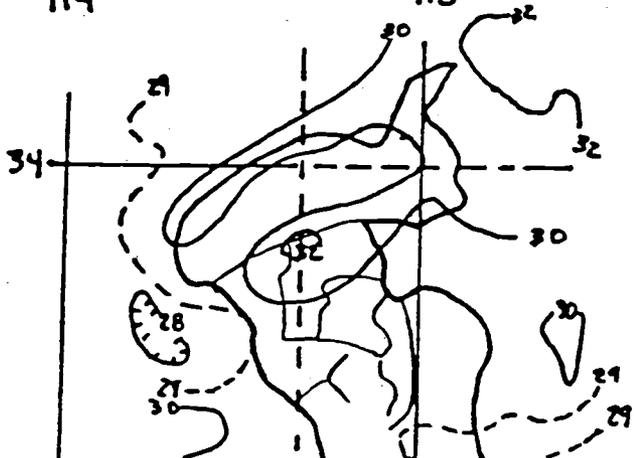
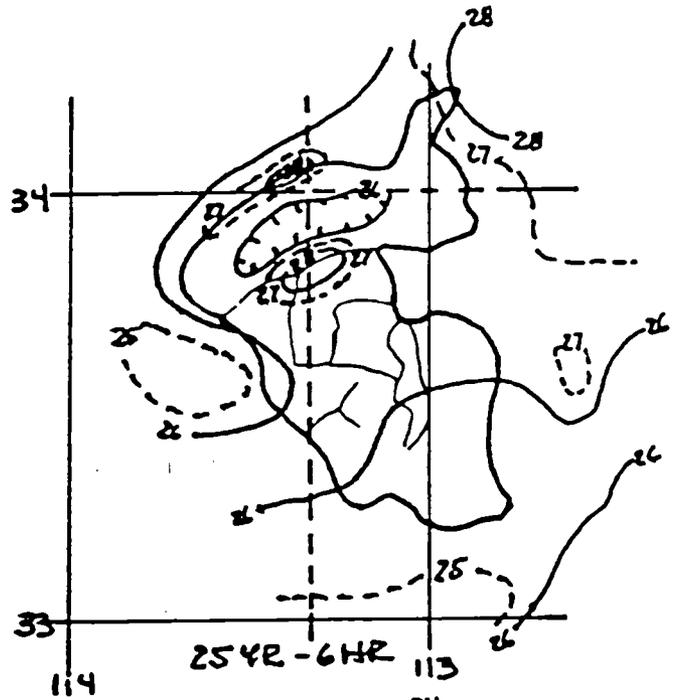
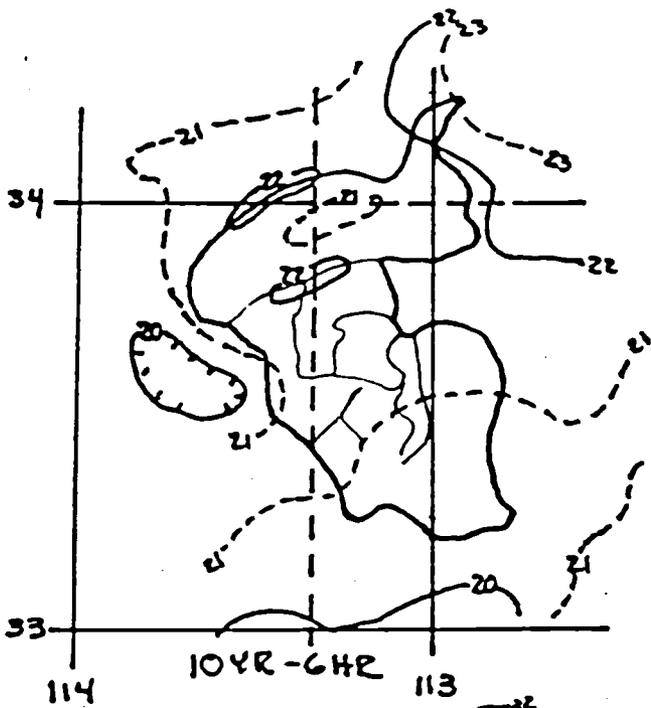
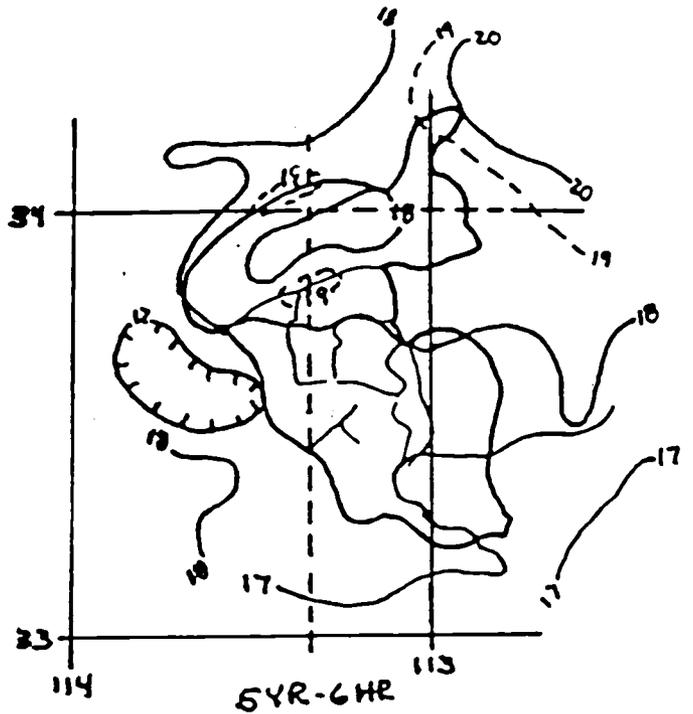
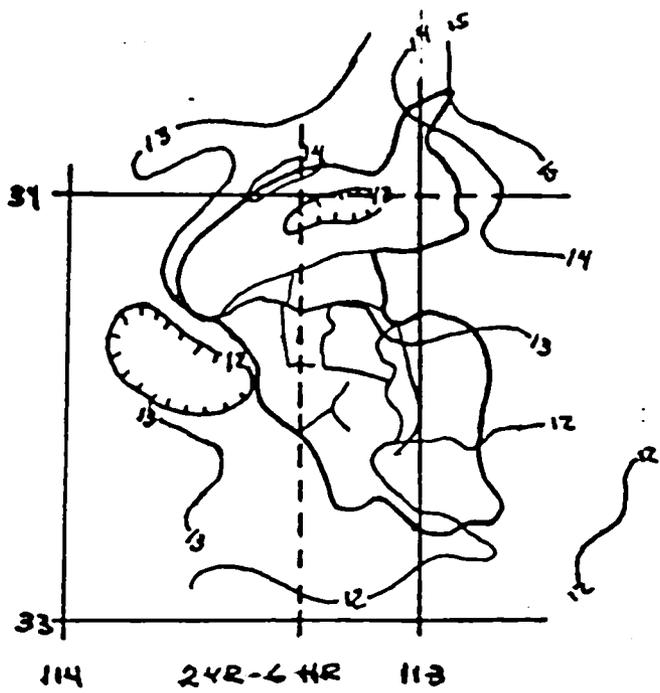
Point-to-area reduction was done by assume the center (maximum contribution) of the storm in the lower part of the watershed and decreasing amount in an upstream direction. The drainage areas were accumulated from the lowest point in the watershed to the highest point. Point-to-area factors were altered to reflect major changes in the accumulated drainage area. Beyond 300 M^2 there is little change in the point-to-area curve for the 24-hour duration storms.

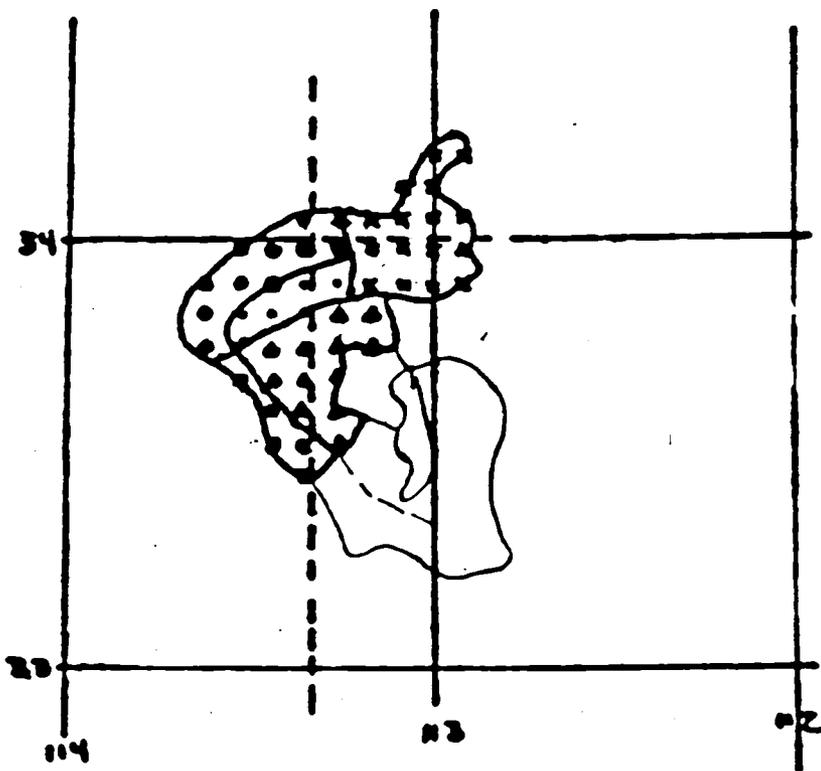
RESULTS & CONCLUSIONS: Derivation of average point precipitation for each of the 5 subdivisions concluded that there is not a significant difference with geographical location. There one average value was used for each frequency. The results are presented along with the areal reductions for the 24-hour and the 6-hour storm durations.

The 24-hour duration is best suited to fit the time of concentration for the entire area. The 6-hour duration









GRID FOR ESTIMATING
AVERAGE POINT PRECIPITATION
HARQUAHALA VALLEY WATERSHED

ARIZONA

HAROUAHALA VALLEY WATERSHED

R. McArthur 9/20/83

DERIVATION OF AREAL PRECIPITATION VOLUMES

TO BE USED IN THE TR-20 ANALYSIS OF CENTENNIAL WASH

PART OF DRAINAGE AREA		AREAL REDUCTION FACTOR	6-HOUR PRECIPITATION VOLUMES FOR SELECTED RETURN PERIODS													
			500-YR		100-YR		50-YR		25-YR		10-YR		5-YR		2-YR	
			POINT	AREAL	POINT	AREAL	POINT	AREAL	POINT	AREAL	POINT	AREAL	POINT	AREAL	POINT	AREAL
FR	TO															
001	122	0.832 848.13	4.5	3.74	3.76	2.88	3.01	2.50	2.62	2.18	2.10	1.75	1.72	1.43	1.16	0.97
015	132	0.840 279.89		3.78		2.91		2.53		2.20		1.76		1.44		0.97
020	151	0.849 217.04		3.82		2.94		2.56		2.22		1.78		1.46		0.98
027	152	0.962 (19.89M)		4.33		3.33		2.90		2.52		2.02		1.65		1.12

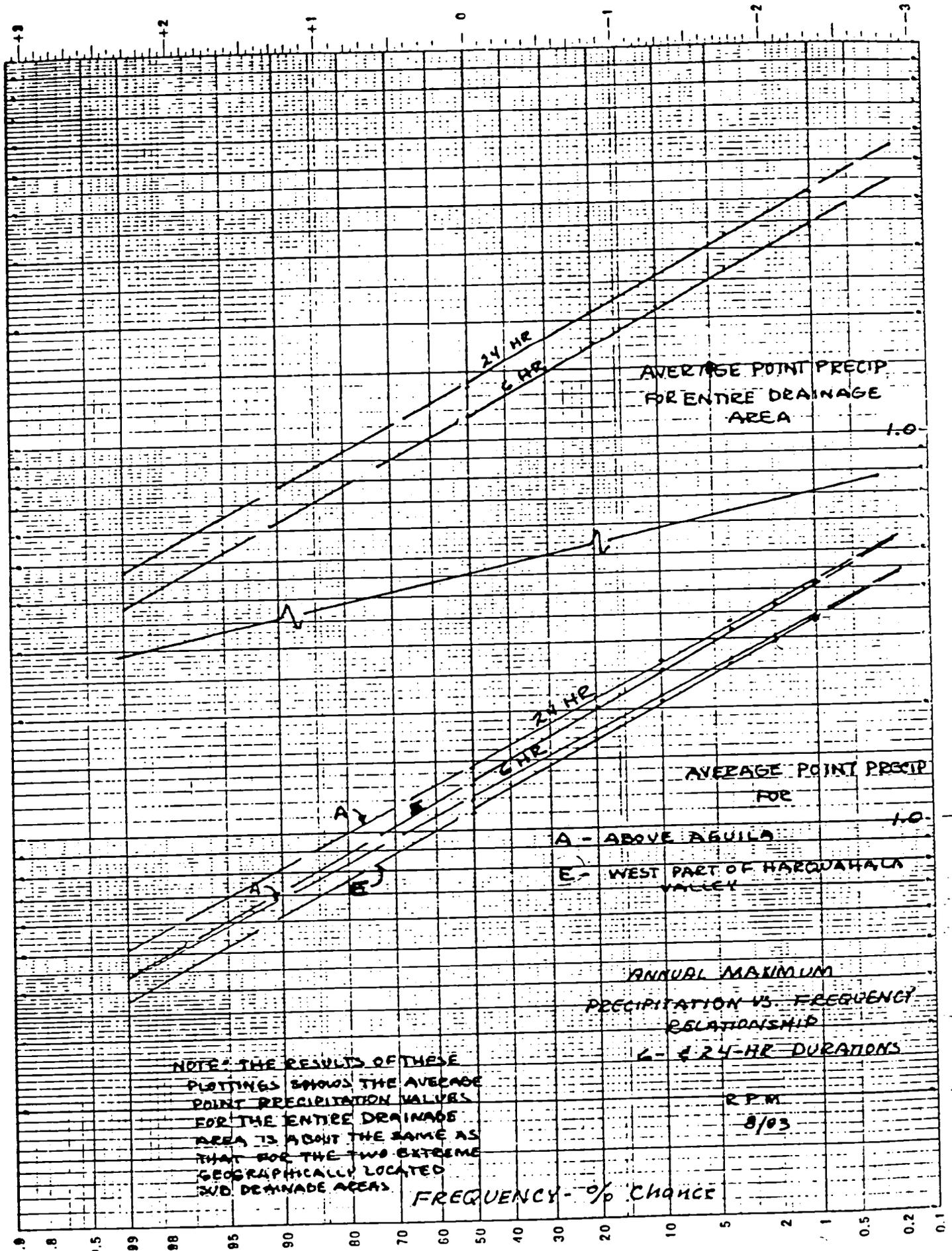
ARIZONA
R. McArthur 9/8/83

MARGUINALS VALLEY WATERSHED

DERIVATION OF AREAL PRECIPITATION VOLUMES
TO BE USED IN THE TR-20 ANALYSIS OF CENTENNIAL WASH

* PART OF DRAINAGE AREA		AREAL REDUCTION FACTOR	24 HOUR PRECIPITATION VOLUMES FOR SELECTED RETURN PERIODS													
			500-YR		100-YR		50-YR		25-YR		10-YR		5-YR		2-YR	
FR	TO		POINT	AREAL	POINT	AREAL	POINT	AREAL	POINT	AREAL	POINT	AREAL	POINT	AREAL	POINT	AREAL
001	122	0.908	5.5	4.99	4.25	3.84	3.37	2.91	2.58	2.34	1.91	1.27	1.40	1.28	1.28	1.28
015	132	0.912	5.02		3.88	3.38	2.92	2.35			1.92	1.28				
020	151	0.916	5.04		3.89	3.40	2.93	2.36			1.92	1.28				
027	152	0.969	5.33		4.12	3.60	3.10	2.50			2.03	1.36				

* REFER TO TR-20 FOR IDENTITY OF LOCATIONS



AVERAGE POINT PRECIP
FOR ENTIRE DRAINAGE
AREA 1.0

AVERAGE POINT PRECIP
FOR 1.0

A - ABOVE AGUILA
E - WEST PART OF HARQUAHALA
VALLEY

ANNUAL MAXIMUM
PRECIPITATION VS. FREQUENCY
RELATIONSHIP
L - 24-HR DURATIONS

R.P.M.
8/03

NOTE: THE RESULTS OF THESE
PLOTINGS SHOWS THE AVERAGE
POINT PRECIPITATION VALUES
FOR THE ENTIRE DRAINAGE
AREA IS ABOUT THE SAME AS
THAT FOR THE TWO EXTREME
GEOGRAPHICALLY LOCATED
SUB-DRAINAGE AREAS.

FREQUENCY - % CHANCE

ARIZONA

MARQUAHALA VALLEY WATERSHED

R. McArthur 8/26/83

COMPUTATION OF MAXIMUM ANNUAL PRECIPITATION 1 2
 FREQUENCY VALUES FOR 6 HR & 24 HR DURATIONS ANNUAL SERIES

FOR ENTIRE DRAINAGE AREA ABOVE PROPOSED
 CENTENNIAL WASH LEVEE

6 HOUR DURATION				
FREQUENCY (YRS)	MAP VALUE (INS)	FACTOR	PLOTTED VALUE (IN.)	VALUE USED (IN.)
(500)				(4.5)
100	3.36	1.00	3.36	3.46
50	3.01	1.00	3.01	3.01
25	2.66	1.00	2.66	2.62
10	2.16	0.99	2.14	2.10
5	1.82	0.96	1.75	1.72
2	1.32	0.88	1.16	1.16

USE

24 HOUR DURATION				
(500)				(5.5)
100	4.15	1.00	4.15	4.25
50	3.71	1.00	3.71	3.71
25	3.28	1.00	3.28	3.20
10	2.62	0.99	2.59	2.58
5	2.19	0.96	2.10	2.10
2	1.57	0.88	1.38	1.40

ARIZONA

HARQUAHUA VALLEY WATERSHED

R. McArthur 8/26/83

COMPUTATION OF MAXIMUM ANNUAL PRECIPITATION- 2 2
 FREQUENCY VOLUMES FOR 6HR & 24 HR DURATIONS ANNUAL SERIES

6 HOUR DURATION

FREQUENCY (Yrs)	FACTOR	A ABOVE AGUA			B NORTHSIDE OF MCMULLEN VALLEY			C SOUTHSIDE OF MCMULLEN VALLEY			D NW PART OF HARQUAHUA VALLEY			E W PART OF HARQUAHUA VALLEY		
		MAP VALUE (In)	PLOTTED VALUE (In)	VALUE USED (In)	MAP VALUE (In)	PLOTTED VALUE (In)	VALUE USED (In)	MAP VALUE (In)	PLOTTED VALUE (In)	VALUE USED (In)	MAP VALUE (In)	PLOTTED VALUE (In)	VALUE USED (In)	MAP VALUE (In)	PLOTTED VALUE (In)	VALUE USED (In)
(500)				(4.35)												(4.35)
100	1.00	3.36	3.36	3.42	3.35	3.35	3.36	3.36		3.37	3.37		3.31	3.31	3.35	
50	1.00	3.03	3.03	3.03	3.00	3.00	3.02	3.02		3.03	3.03		2.93	2.93	2.95	
25	1.00	2.66	2.66	2.66	2.67	2.67	2.66	2.66		2.68	2.68		2.60	2.60	2.58	
10	0.99	2.18	2.16	2.16	2.16	2.14	2.16	2.14		2.16	2.14		2.11	2.09	2.05	
5	0.96	1.84	1.77	1.77	1.83	1.76	1.83	1.76		1.80	1.73		1.74	1.67	1.68	
2	0.88	1.36	1.20	1.22	1.34	1.18	1.33	1.17		1.28	1.13		1.25	1.10	1.13	

24 HOUR DURATION

(500)		A			B		C		D		E		
				(5.4)									(5.4)
100	1.00	4.19	4.19	4.19	4.13	4.13	4.07	4.07	4.17	4.17	4.13	4.13	4.08
50	1.00	3.71	3.71	3.70	3.71	3.71	3.69	3.69	3.74	3.74	3.64	3.64	3.57
25	1.00	3.31	3.31	3.22	3.29	3.29	3.27	3.27	3.28	3.28	3.15	3.15	3.10
10	0.99	2.67	2.64	2.60	2.57	2.54	2.62	2.59	2.61	2.58	2.53	2.50	2.50
5	0.96	2.23	2.14	2.13	2.19	2.10	2.20	2.11	2.14	2.07	2.10	2.02	2.00
2	0.88	1.62	1.43	1.45	1.58	1.39	1.59	1.40	1.52	1.34	1.48	1.30	1.33

ARIZONA

HARQUAHALA VALLEY WATERSHED

R McARTHUR 8/26/83

DETERMINATION OF AVERAGE POINT PRECIPITATION

1/2

CENTENNIAL WASH

SOURCE: NOAA ATLAS 2 Vol. VIII Arizona Precip-Frequency Atlas of the United States

PART OF D.A.	GRID VALUES OF PRECIP & MEAN COMPUTATIONS											
	6 HR DURATION						24 HR DURATION					
	2 YR	5 YR	10 YR	25 YR	50 YR	100 YR	2 YR	5 YR	10 YR	25 YR	50 YR	100 YR
A ABOVE AGUILA	1.5	1.96	2.3	2.8	3.14	3.5	2.1	2.6	3.1	3.6	4.0	4.65
	1.38	1.86	2.19	2.68	3.08	3.36	1.59	2.25	2.68	3.32	3.72	4.16
	1.38	1.85	2.18	2.67	3.08	3.35	1.58	2.18	2.65	3.3	3.7	4.16
	1.38	1.85	2.18	2.66	3.12	3.34	1.6	2.23	2.7	3.32	3.72	4.17
	1.42	1.91	2.26	2.69	3.06	3.42	1.59	2.46	2.9	3.5	3.85	4.38
	1.38	1.87	2.2	2.68	3.01	3.38	1.54	2.3	2.7	3.38	3.78	4.19
	1.36	1.84	2.18	2.67	3.02	3.35	1.56	2.18	2.66	3.3	3.7	4.16
	1.34	1.83	2.17	2.65	2.92	3.33	1.63	2.15	2.62	3.28	3.68	4.13
	1.36	1.83	2.16	2.65	3.05	3.33	1.9	2.2	2.68	3.27	3.7	4.14
	1.36	1.85	2.18	2.66	2.99	3.36	1.6	2.22	2.68	3.33	3.62	4.18
	1.32	1.82	2.15	2.64	2.99	3.33	1.55	2.15	2.62	3.25	3.6	4.13
	1.3	1.8	2.12	2.62	3.05	3.3	1.52	2.1	2.55	3.2	3.75	4.11
	1.34	1.83	2.16	2.63	2.99	3.34	1.55	2.16	2.63	3.25	3.6	4.13
	1.32	1.8	2.13	2.62	2.98	3.36	1.52	2.2	2.62	3.25	3.55	4.16
	1.28	1.75	2.1	2.58	3.03	3.28	1.48	2.05	2.45	3.15	3.7	3.96
	1.33	1.81	2.18	2.65	3.00	3.4	1.59	2.15	2.61	3.22	3.6	4.12
	1.38	1.86	2.16	2.68	3.0	3.4	1.62	2.25	2.62	3.4	3.8	4.22
17	23.13	27.32	37.00	45.23	51.51	57.19	27.57	37.83	45.47	56.32	63.07	71.15
M	1.36	1.84	2.18	2.66	3.03	3.36	1.62	2.23	2.67	3.31	3.71	4.19
B NORTHSIDE OF McMULLEN VALLEY	1.28	1.75	2.08	2.6	2.98	3.28	1.48	2.1	2.45	3.18	3.55	3.95
	1.36	1.95	2.22	2.82	3.03	3.45	1.52	2.25	2.63	3.45	3.9	4.3
	1.25	1.82	2.12	2.6	2.98	3.28	1.63	2.18	2.53	3.18	3.58	4.05
	1.42	1.85	2.18	2.66	3.0	3.34	1.6	2.22	2.6	3.2	3.58	4.1
	1.32	1.78	2.12	2.53	2.97	3.25	1.53	2.16	2.54	3.18	3.58	3.95
	1.38	1.82	2.23	2.8	3.05	3.45	1.58	2.2	2.62	3.3	3.9	4.3
	1.32	1.86	2.18	2.68	3.0	3.33	1.45	2.2	2.59	3.3	3.68	4.1
	1.37	1.82	2.12	2.7	3.03	3.42	1.65	2.2	2.61	3.45	3.85	4.25
	1.35	1.82	2.14	2.66	3.0	3.29	1.55	2.22	2.6	3.38	3.78	4.18
	1.32	1.8	2.16	2.63	2.94	3.34	1.55	2.16	2.57	3.24	3.70	4.14
10	23.37	18.28	21.55	26.70	29.98	33.53	15.79	21.89	25.74	32.86	37.10	41.32
M	1.34	1.83	2.16	2.67	3.00	3.35	1.58	2.19	2.57	3.29	3.71	4.13
C SOUTH SIDE OF McMULLEN	1.32	1.82	2.15	2.65	3.0	3.3	1.52	2.16	2.6	3.2	3.6	4.0
	1.32	1.8	2.1	2.6	2.99	3.28	1.50	2.1	2.55	3.15	3.56	3.95
	1.33	1.9	2.22	2.82	3.2	3.6	1.80	2.4	2.85	3.45	3.85	4.2
	1.34	1.8	2.18	2.65	3.0	3.32	1.60	2.19	2.6	3.21	3.7	4.1

ARIZONA

HARQUAHALA VALLEY WATERSHED

R McArthur 8/26/83

DETERMINATION OF AVERAGE POINT PRECIPITATION

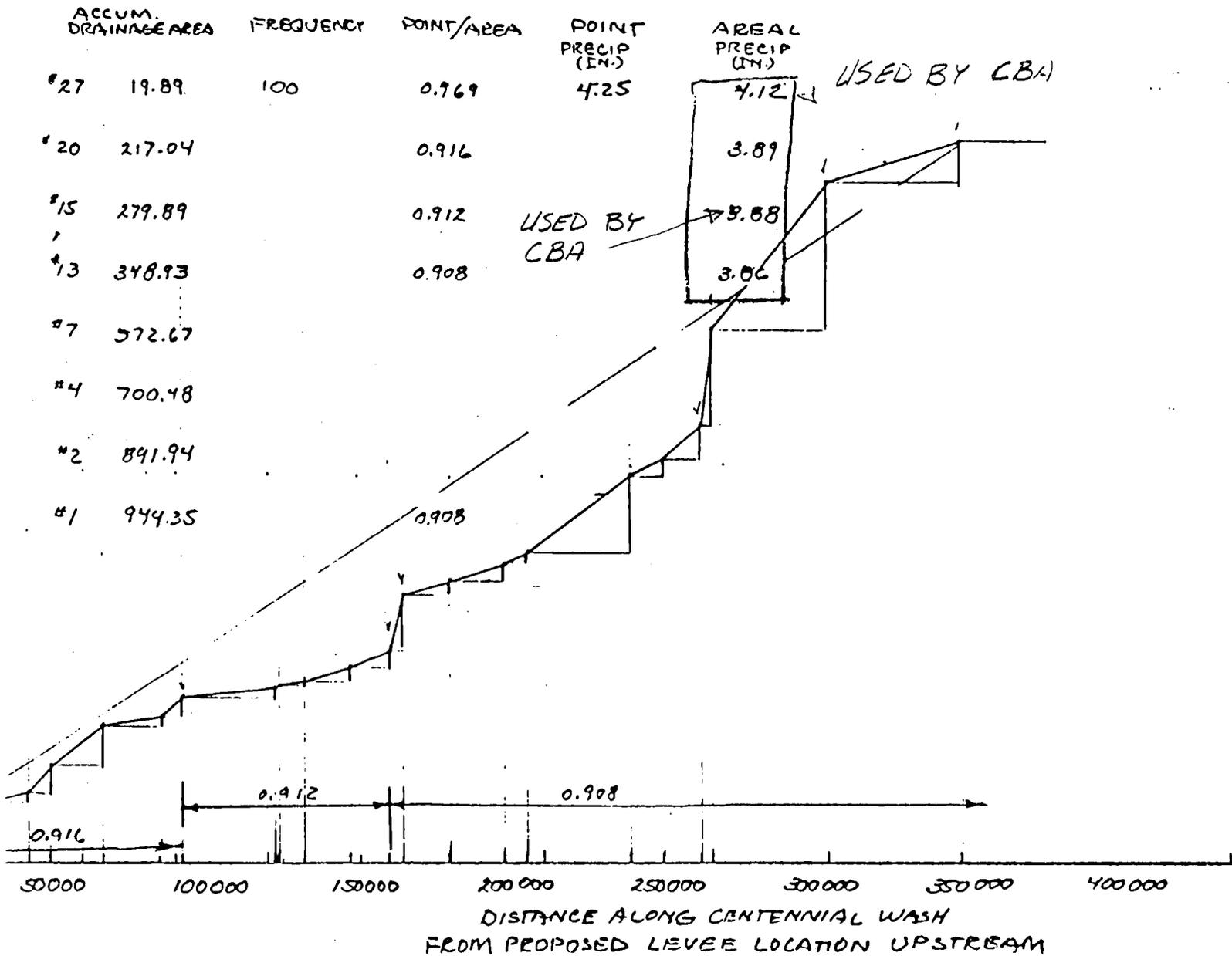
2/2

CENTENNIAL WASH

SOURCE: NOAA ATLAS 2 Vol. VIII Arizona Precip-Frequency Atlas of the United States

PART OF D.A.	GRID VALUES OF PRECIP & MEAN COMPUTATIONS												
	CHR DURATION						24 HR DURATION						
	24R	5YR	10YR	25YR	50YR	100YR	24R	5YR	10YR	25YR	50YR	100YR	
D N & PART OF HARQUAHALA VALLEY INCLUDE TIGER WASH	1.33	1.86	2.19	2.69	3.11	3.39	1.58	2.19	2.7	3.35	3.78	4.18	
	1.30	1.8	2.18	2.68	3.0	3.38	1.55	2.17	2.63	3.32	3.78	4.18	
	1.34	1.9	2.2	2.8	3.18	3.5	1.6	2.4	2.75	3.4	3.8	4.25	
	1.31	1.82	2.18	2.69	3.1	3.38	1.57	2.15	2.63	3.35	3.78	4.19	
	1.28	1.78	2.16	2.67	2.99	3.37	1.48	2.12	2.68	3.28	3.73	4.17	
	1.25	1.75	2.14	2.64	2.92	3.35	1.47	2.11	2.57	3.25	3.7	4.14	
	1.23	1.73	2.12	2.63	2.94	3.33	1.46	2.12	2.55	3.21	3.68	4.15	
	1.3	1.83	2.18	2.7	3.1	3.4	1.59	2.2	2.6	3.3	3.67	4.13	
	1.26	1.78	2.14	2.65	2.99	3.35	1.49	2.11	2.57	3.19	3.78	4.19	
	1.29	1.73	2.12	2.62	2.95	3.33	1.46	2.09	2.54	3.18	3.7	4.16	
	1.23	1.81	2.19	2.7	2.95	3.38	1.6	2.2	2.62	3.29	3.68	4.13	
	1.23	1.76	2.13	2.63	2.96	3.33	1.47	2.09	2.56	3.18	3.72	4.19	
Σ	15.25	21.55	25.93	32.10	36.32	40.49	18.29	25.95	31.30	39.30	44.90	50.06	
M	1.28	1.80	2.14	2.68	3.03	3.37	1.52	2.16	2.61	3.28	3.74	4.17	
E W PART OF HARQUAHALA VALLEY ABOVE LEUCE LOCATION	1.28	1.74	2.12	2.62	2.95	3.32	1.47	2.12	2.54	3.18	3.64	4.12	
	1.27	1.71	2.12	2.58	2.95	3.33	1.43	2.04	2.54	3.22	3.63	4.15	
	1.23	1.72	2.1	2.64	2.91	3.3	1.48	2.09	2.52	3.1	3.62	4.18	
	1.25	1.75	2.09	2.57	2.93	3.31	1.48	2.13	2.53	3.1	3.68	4.12	
	1.25	1.76	2.12	2.6	2.92	3.31	1.52	2.12	2.53	3.15	3.61	4.12	
Σ	6.23	8.68	10.55	13.01	14.66	16.57	7.38	10.50	12.66	15.25	18.18	20.64	
M	1.25	1.74	2.11	2.60	2.93	3.31	1.48	2.10	2.53	3.15	3.64	4.13	
ENTIRE AREA	Σ	66.04	90.79	107.98	132.98	150.59	167.96	78.53	109.24	130.91	163.84	185.36	207.55
	M	1.32	1.82	2.16	2.66	3.01	3.36	1.57	2.19	2.62	3.28	3.71	4.15

24 HOUR DURATION



ARIZONA
 R.A. Arthur 8/29/82
 HARQUAHUA VALLEY WATERSHED
 DERIVATION OF DRAINAGE AREAS TO USE FOR
 POINT-TO-AREAL ADJUSTMENTS

