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HYDROLOGY REPORT  
OUTER LOOP HIGHWAY  
ARIZONA CANAL TO  
THE SALT RIVER

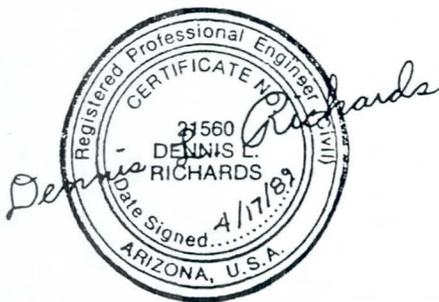
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FINAL  
HYDROLOGY REPORT  
OUTER LOOP HIGHWAY  
ARIZONA CANAL TO  
THE SALT RIVER

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April 1989



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PAZ-DC-02.3/PH/T3R01



## I. INTRODUCTION

The purpose of this report is to document the assumptions and methodologies used to develop the offsite hydrology for the Concept Drainage Plan for an approximate 6.3 mile reach of the Outer Loop Highway located between the Arizona Canal and the Salt River.

The following sections of this report present a discussion of drainage area characteristics, meteorological conditions, and the results of the hydrologic modeling process that was undertaken to develop the rainfall/runoff data required for the design of a cross-drainage system for this reach of the highway.

This report supersedes a "Preliminary Hydrology Report" published in December 1986 for this reach of the Outer Loop. The 1986 study was based on a highway alignment along Pima Road. This revised 1989 study is based on a new highway alignment that is located approximately one-quarter mile east of Pima Road.

## II. DRAINAGE BASIN CHARACTERISTICS

### 2.1 Drainage Area

Figure 2.1 presents a vicinity map showing the preliminary highway alignment and the perimeter of the drainage area that contributes runoff to this segment of the highway.

The contributing watershed is located entirely on the Salt River Indian Reservation. The watershed is bounded on the north by the Arizona Canal and on the south by the Salt River. The proposed Outer Loop Highway alignment forms the western boundary. The eastern watershed boundary is a function of land contours which determine grade breaks that direct flowage patterns towards the west or south. The total drainage area contributing runoff to the freeway is approximately 7.6 square miles.

The following subsections discuss specific characteristics of the watershed.

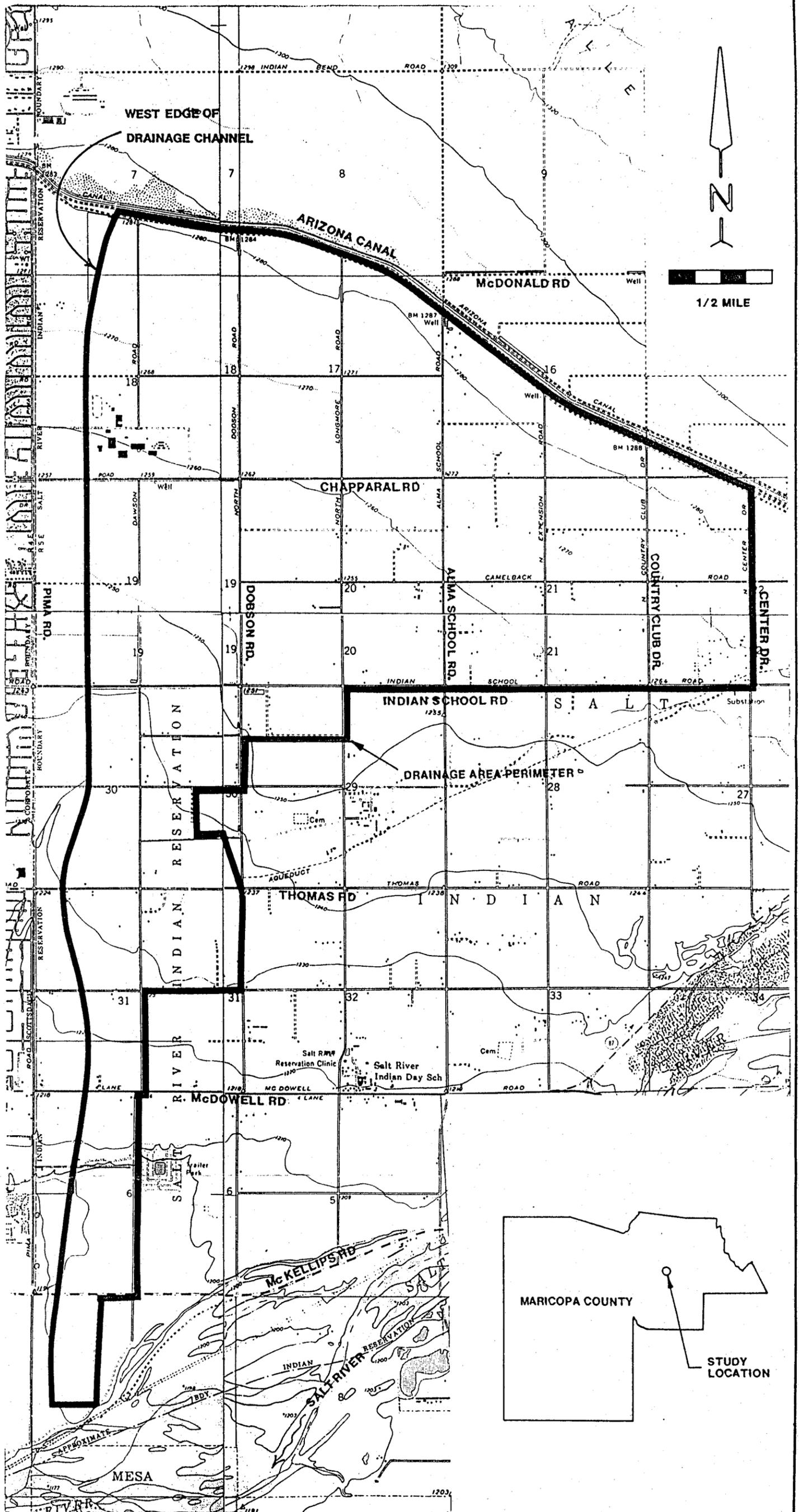
### 2.2 Land Use

Plate 1 is an aerial photograph of the project area showing the type of land use existing as of October 1986. As can be seen from this photograph, the primary land use on this portion of the Reservation is agriculture. Aside from a few scattered residential dwellings, the only other prominent land use is Scottsdale Community College which is located in the northwest quadrant of the watershed.

### 2.3 Soil Type and Vegetation

Soils information is needed in order to model the infiltration characteristics of the watershed. Such information is generally available from Soil Survey Reports published by the Soil Conservation Service (SCS). The watershed for this project was included in the Soil Survey of Aquila-Carefree Area, Parts of Maricopa and Pinal Counties, Arizona, U.S. Department of Agriculture, April 1986.

Using the standard SCS hydrologic soil group classification system, an estimate can be made of the runoff potential of the soils within any given sub-basin of the project watershed. The SCS system is based on four hydrologic soil groups, A through D. Soils in group A have very low runoff potential (i.e., high



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VICINITY MAP & DRAINAGE AREA

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FIGURE 2.1

infiltration rate), those in group B have moderately low runoff potential, those in group C have moderately high runoff potential, and those in group D have high runoff potential (i.e., very slow infiltration rate).

Plate 2 illustrates the composition of the project watershed in terms of hydrologic soil groups. The information in this figure is based on the previously referenced soil survey. As can be seen from Plate 2, the watershed is composed of three hydrologic soil groups (B, C, and D).

From the SCS soil survey, the following descriptions are provided for some of the major soil classifications comprising the drainage area.

1. Soil Group B - Mohall Clay Loam

This deep and well-drained soil is on fan terraces. It formed in alluvium derived dominantly from acid and basic igneous rock. Slope is 0 to 3 percent.

Typically, the surface layer is light brown clay loam about 2 inches thick. The upper 19 inches of the subsoil is light brown clay loam, and the lower 21 inches is light reddish brown and light brown, calcareous clay loam. The substratum to a depth of 60 inches or more is light reddish brown, calcareous extremely cobbly loamy sand.

Permeability of this Mohall soil is moderately slow. Available water capacity is high. Effective rooting depth is 60 inches or more. Runoff is slow, and the hazard to water erosion is slight.

2. Soil Group B - Rillito Loam

This deep and well-drained soil is on fan terraces. It formed in alluvium derived dominantly from acid and basic igneous rock. Most of the areas used as cropland have slopes of less than 1 percent.

Typically, the upper layer is light brown, calcareous loam about 11 inches thick. The next 13 inches is light brown, calcareous loam. The next 15 inches is light brown, calcareous, weakly lime- and silica-cemented gravelly loam. Below this to a depth of 60 inches

or more is light brown, calcareous, weakly cemented extremely gravelly loam.

Permeability of this Rillito soil is moderate. Available water capacity is moderate. Effective rooting depth is 60 inches or more. Runoff is slow, and the hazard of water erosion is slight.

3. Soil Group C - Contine Clay Loam

This deep and well-drained soil is on fan terraces. It formed in alluvium derived dominantly from acid and basic igneous rock. Slope is 0 to 3 percent.

Typically, the surface layer is brown, calcareous clay loam about 2 inches thick. The subsoil is reddish brown, calcareous clay loam and clay 28 inches thick. The substratum to a depth of 60 inches or more is light reddish brown, calcareous sandy loam.

Permeability of this Contine soil is slow. Available water capacity is high. Effective rooting depth is 60 inches or more. Runoff is slow, and the hazard of water erosion is slight.

4. Soil Group C - Contine Clay

This deep and well-drained soil is on fan terraces. It formed in alluvium derived dominantly from acid and basic igneous rock. Slope is 0 to 3 percent.

Typically, the surface layer is reddish brown, calcareous clay 12 inches thick. The subsoil is light reddish brown, calcareous clay 14 inches thick. The substratum to a depth of 60 inches or more is light reddish brown, calcareous gravelly clay.

Permeability of this Contine soil is slow. Available water capacity is high. Effective rooting depth is 60 inches or more. Runoff is slow, and the hazard to water erosion is slight.

Due to the agricultural nature of this watershed, vegetation density is a function of the type of crops being grown and the maturity of the crops. This parameter is accounted for in the "hydrologic condition" assigned to the land in SCS curve number charts. For this watershed, an average condition between "good" and "poor" was chosen.

The soils and vegetation data discussed in this section were used to select Soil Conservation Service (SCS) curve numbers. These curve numbers, which model the hydrologic abstractions and infiltration characteristics of the watershed, are discussed in more detail in Section 4.2 of this report.

#### 2.4 Existing Drainage Facilities

Due to the extensive agricultural land use in the watershed, the contributing drainage area contains a dense network of irrigation/drainage channels. Although this system is primarily used for moving irrigation water through the area, it also serves as a drainage system for storm runoff. Most of the channels in this system are relatively small (2- to 4-feet deep and 2- to 4-feet wide), and are unable to transport large rates of runoff that might occur during a major storm.

### III. RAINFALL CHARACTERISTICS

The hydrologic response of a watershed is dependent upon rainfall characteristics such as depth, duration, and the spatial and temporal distribution of the rainfall event. The rainfall depth is a function of the probability of occurrence and the duration of the event. This probability is expressed as a recurrence interval (50-year, 100-year, etc.), which is defined as the average interval of time within which the magnitude of an event will be equaled or exceeded once. Mathematically, recurrence interval is defined as the reciprocal of the probability of occurrence.

Evaluating storms with different recurrence intervals is required when considering the risk and economic factors associated with the design of a drainage system for a specific meteorological event. In order to incorporate a risk analysis into the freeway design process, ADOT has requested that both the 50- and 100-year storms be evaluated as part of the hydrologic analysis for the Outer Loop.

Rainfall depths for the project drainage area were developed using isopluvial maps and regression equations presented in the Precipitation-Frequency Atlas of the Western United States, Volume VIII - Arizona. Table 3.1 summarizes point precipitation values for the 50- and 100-year storms for durations of 5 minutes, 15 minutes and 1-, 2-, 3-, 6-, 12-, and 24-hours.

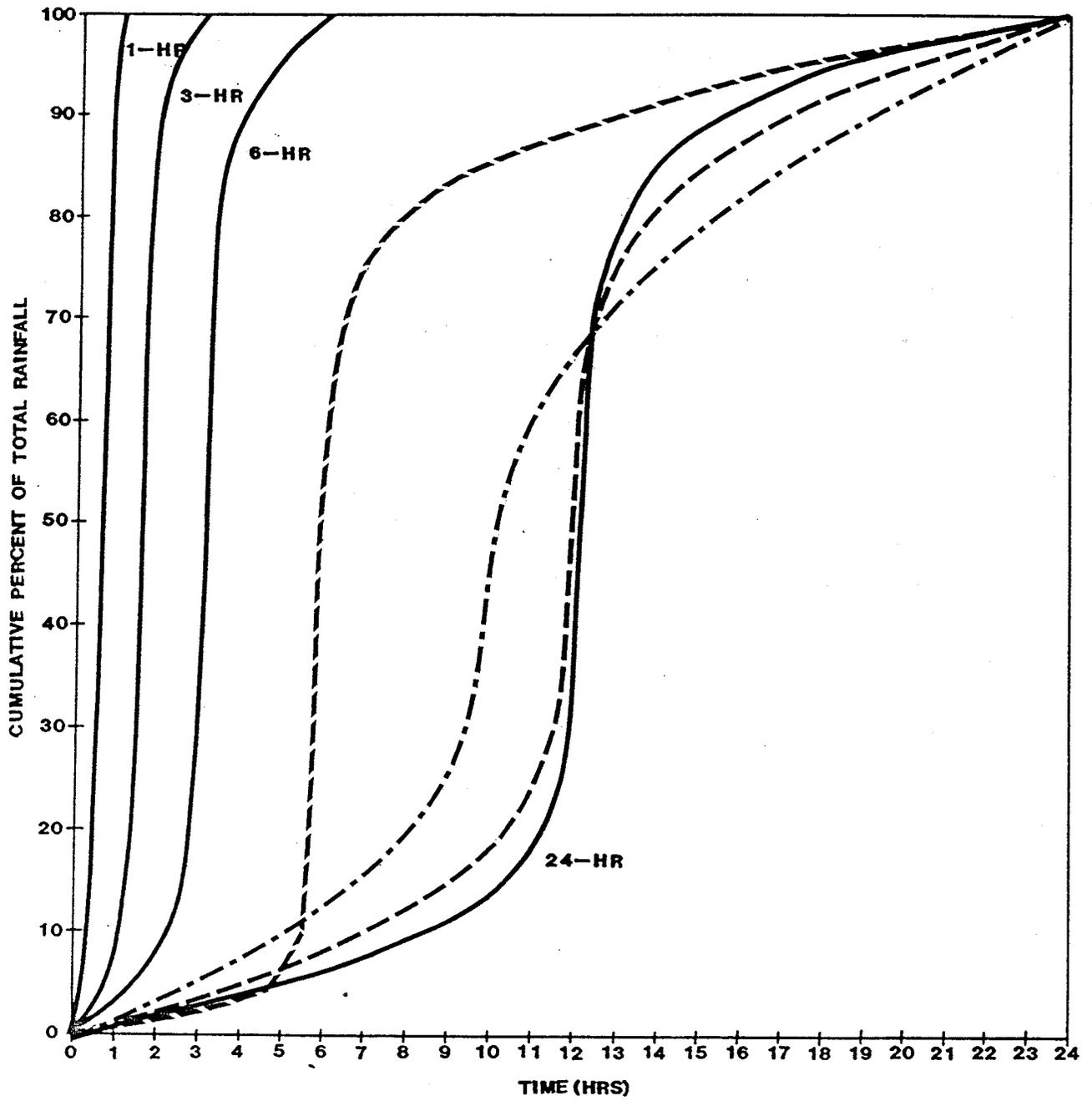
When using the hypothetical storm distribution in HEC-1, point rainfall values are automatically reduced (by the computer program) in accordance with procedures outlined in Weather Bureau Technical Paper No. 40, May 1961, to account for areal reduction of rainfall depths over large drainage areas. Due to the small size of the watershed under investigation, reduction in point precipitation values was used.

inserting a drainage area size of 0.1 square mile on the

Since De Leuw, Cather & Company and ADOT have previously used the HEC-1 hypothetical storm distribution for use in drainage improvements for the reach of the Outer Loop from the Salt River Indian Reservation, no attempt was made during this study to evaluate different storm distributions, i.e., the analysis was confined to the 100- and 50-year, 24-hour events. Figure 3.1 presents a graphical illustration of the selected rainfall distribution. The reader is referred to the Hydrology

*Incomplete  
references*

*no reference  
section*



LEGEND

- HEC-1 HYPOTHETICAL
- - - SCS 24-HR TYPE I
- - - SCS 24-HR TYPE II
- - - SCS 24-HR TYPE IIIA

**sla** SIMONS, Li & ASSOCIATES, INC.

**RAINFALL DISTRIBUTIONS**

FIGURE 3.1

Report, Outer Loop Freeway, CAP Aqueduct to Salt River Indian Reservation, August 19, 1986 by Simons, Li & Associates, Inc. for an overview of HEC-1 model sensitivity to several different rainfall distributions.

TABLE 3.1								
Rainfall Depths as a Function of Recurrence Interval and Duration								
Recurrence Interval (Years)	Point Precipitation Values (inches)							
	Duration							
	5-min	15-min	1-hr	2-hr	3-hr	6-hr	12-hr	24-hr
50	0.64	1.26	2.22	2.43	2.56	2.83	3.12	3.40
100	0.74	1.45	2.54	2.78	2.93	3.23	3.54	3.85

*These values are about  
0 to 0.3" higher than given  
in SLA report for Sect. 11*

#### IV. HYDROLOGIC MODEL (HEC-1)

A computerized rainfall/runoff model was developed for the watershed using the U.S. Army Corps of Engineers Flood Hydrograph Package (HEC-1). HEC-1 uses numerical parameters to describe the amount and temporal distribution of rainfall, the runoff characteristics of the watershed, and the hydraulic properties of channels that collect and convey the direct runoff to concentration points. The computer output provides a runoff hydrograph at user selected locations. These hydrographs can be used to design drainage channels, detention/retention basins, or to evaluate the capacity of existing drainage facilities.

This section of the report presents a detailed discussion of specific components of the computer model that were created to simulate the rainfall/runoff response of the watershed. The results of the modeling process are also presented (Section 4.4) along with a brief discussion of model verification (Section 4.5).

##### 4.1 Delineation of Drainage Sub-Basins

As discussed previously, there are several manmade features which effectively control the delineation of drainage area boundaries for this watershed. The proposed alignment of the Outer Loop sets the western boundary of the watershed while the Arizona Canal and the Salt River establish north and south boundaries, respectively. The eastern boundary of the watershed is a function of the land slope direction.

The drainage area has undergone extensive land leveling during its conversion from a natural desert environment to a network of irrigated agricultural fields. Consequently, the U.S.G.S. quadrangle maps were of no value in establishing drainage patterns or in delineating drainage sub-basins. In an effort to locate current topographic maps of the area, contact was established with the Bureau of Indian Affairs and the Salt River-Pima Maricopa Community Tribal Office. Although there were no topographic maps or engineering plans available for the agricultural fields and irrigation system, representatives from the Indian Community were most helpful in identifying the drainage patterns for the watershed. Using current (June 1986) aerial photographs (1" = 400'), Reservation personnel identified the drainage pattern for every field and irrigation lateral within the watershed boundaries. On the basis of this

information, the drainage area was divided into 91 sub-basins, which were considered to have homogeneous hydrologic properties. This large number of sub-basins provides a very high resolution for the HEC-1 input parameters that are used to model the hydrologic response of the watershed.

Due to the absence of current topographic maps, SLA conducted field surveys of several parcels in order to determine land slopes that are required for input into the HEC-1 model. This survey data also served as confirmation of the drainage pattern information provided by Reservation personnel.

#### 4.2 Interception/Infiltration

Precipitation losses due to interception and infiltration were modeled using the SCS curve number option in HEC-1. Selection of curve numbers was based on information gathered relative to type of soil cover, vegetation density, land use, and soil moisture conditions. An Antecedent Moisture Condition 2 (AMC 2) was assumed for all curve number selections. Based on this information, average curve numbers were developed for each of the 91 sub-basins to account for the combined effect of these drainage basin characteristics.

Numerous field inspections of the watershed revealed that the agricultural fields were graded to a configuration that would promote retention of water, i.e., there were often small berms constructed around the perimeter of the fields. Excess irrigation water that would pond along the downstream boundary of each field was drained through small culverts that connect to irrigation/drainage channels along the perimeter of adjacent fields. During major storms, these fields will probably function as very effective detention/retention basins with outflows being regulated, to some extent, by the culvert size draining each field and the extent of berming that surrounds each field. Such a condition could result in very minimal runoff relative to the design of a drainage interceptor channel for the proposed Outer Loop Highway.

Given the lack of current topographic data for this area, as well as the large number of sub-basins involved, there is no practical way that the detention/retention capacity of the fields could be modeled. Another variable that cannot be accounted for is the possibility of a future land use change that may or may not provide any onsite retention. Accordingly, as a conservative assumption for developing hydrology for the concept drainage design for the Outer

Loop, no onsite detention was considered in selecting curve numbers. Instead, curve numbers were based solely on existing land use, which is primarily agriculture. Assuming "straight row crops", with an average hydrologic condition between "good" and "poor", Table 2-2a. through c., Urban Hydrology For Small Watersheds, Technical Release No. 55, USDA, June 1986, was used as the basis for curve number selection. This same publication was also used to select curve numbers for the other land uses (desert shrub, Scottsdale Community College).

The curve numbers in TR-55 were developed from watersheds exposed to storms of 24-hour duration or less. Generally, these curve numbers are considered to be more applicable to long duration storms on the order of 24-hours. Research by Woodward (Runoff Curve Numbers for Semiarid Range and Forest Conditions, 1973) indicates the need to revise curve numbers upwards as storm duration decreases. However, since this hydrology study is based on a 24-hour event, the curve numbers from TR-55 require no adjustment for storm duration.

#### 4.3 Configuration of the Hydrologic Model

The HEC-1 model developed by the Corps of Engineers Hydrologic Engineering Center, was used to simulate runoff conditions in the study area. The kinematic wave option was used to determine the hydrologic response of the sub-basin areas and for routing the resulting hydrographs through the tributary channels of the basin. This option was selected because runoff processes can be simulated using measurable geographic features such as overland flow elements and the shape, boundary roughness, length, and slope of channel elements. Unlike unit hydrograph techniques, the kinematic wave approach also provides for a non-linear response of runoff characteristics, i.e., peak discharge does not necessarily increase linearly with direct runoff when using the kinematic wave methodology.

A network of sub-basins and connecting channels was configured that simulates the natural drainage pattern in the basin. Plate 3 presents an illustration of the drainage patterns, sub-basin boundaries, and concentration points used in the model.

The following subsections describe the parameters that were used to model the physical characteristics of the watershed.

#### 4.3.1 Kinematic Wave Parameters

The kinematic wave calculations used in HEC-1 are based on the hydraulic properties of simple geometric elements such as overland flow planes and prismatic channel cross-sections. These elements are selected and dimensioned so as to simulate the physical drainage characteristics of a specific watershed sub-basin.

Sub-basins can be described in terms of one or two planes discharging to a collector channel. For this project, overland flow planes were used to model runoff from the flat, agricultural fields. These planes are described in terms of an average overland flow length, slope, and roughness coefficient. The HEC-1 input data in APPENDIX A lists overland flow, kinematic wave parameters used for all the sub-basins in the study area.

The symmetrical shape and uniform slopes of the agricultural fields are ideally suited to overland flow plane modeling techniques. Lengths were measured from aerial photographs and average field slopes were based on survey levels completed by SLA. An average overland flow roughness value of 0.15 was used for all agricultural fields in the watershed. This value was based on an assumption of "row crops" and was thought to be a conservative average of roughness that might exist for a mixture of different crop conditions (grass and pasture, clover, small grain, row crops). This value (0.15) was taken from Table 4, Roughness Coefficients for Routing Surface Runoff, Engman, Journal of Irrigation and Drainage Engineering, February 1986.

Although runoff from the agricultural fields was easily simulated with overland flow planes, the selection of collector channel geometry to route the runoff from the fields was much more difficult. As discussed previously, irrigation water or rainfall runoff tends to pond at the downstream ends of the fields before draining into small ditches. As more and more water accumulates at the end of a field, some water may overtop the earth berms and either spill into earth drainage ditches along the edge of the roads, or begin flowing over the road or down the edge of a field as sheetflow. There is no simple solution to accurately modeling the complex combination of flow conditions that might occur at the end of each field. However, from field inspections, it was concluded that such flow would most likely be of the wide, shallow, sheet-flow variety. Accordingly, conveyance calculations were conducted for a channel

represented by the cross-section at the downstream end of a typical field. A prismatic cross-section was then found which exhibited similar conveyance characteristics. As a result, a channel with a bottom-width of 10-feet, side-slopes of 100:1, and a Mannings "n" = 0.045 was used to collect the overland flow from each field and route the flow towards the proposed Outer Loop drainage channel.

In order to simulate flow through the interceptor channel that would be constructed along the east side of the Outer Loop, a concrete-lined trapezoidal channel was modeled along the east side of the proposed highway. This interceptor channel used 1:1 side-slopes and a bottom width that varied from 10 feet to 35 feet. An "n" value of 0.012 was used to simulate the concrete lining. Based on elevation data along Pima Road, the channel bed-slope was held constant at 0.0032 ft/ft. APPENDIX A presents a complete listing of all collector channel geometry used in the HEC-1 model.

#### 4.3.2 Discussion of Key Modeling Assumptions

Although some features of this watershed are easily modeled with the options available in HEC-1, others present difficulties that must be resolved through the use of sound engineering judgement. Key modeling issues that fall within this latter category include: 1) detention/retention capacity of agricultural fields; 2) collector channel geometry used in routing operations; 3) routing through culverts that connect adjacent fields; and, 4) diversion of flow at road intersections. A brief discussion is presented on each of these issues to clarify the logic employed by SLA in developing the HEC-1 model.

1. Onsite detention/retention - This issue was previously discussed in Section 4.2. Although the agricultural fields in the watershed will undoubtedly retain some water due to the fact that they are constructed to allow maximum infiltration and containment of irrigation flows, there is no simple way to measure the amount of retention that might occur in each field. Modeling of the retention capability through reservoir routing would be extremely complex and time consuming for such a large number of sub-basins. Detailed topography of each field would also be required to develop stage-

discharge curves for a reservoir routing operation.

In consideration of these unknowns, a conservative approach (from a highway drainage system design standpoint) would be to ignore onsite retention and design the highway system on the assumption that there would be no reduction in flow due to onsite retention. This approach could also have some practical merit in that one might assume (as a worst-case condition) that the majority of the fields were being irrigated at the time the storm occurred. Under such conditions, the fields would obviously have their retention capacity reduced. Using this logic, SLA opted to ignore any onsite retention for use in the HEC-1 model.

2. Collector channel geometry - As noted previously, the routing of water through the drainage area is difficult to simulate because of the absence of any large, well-defined channels. The small irrigation/drainage channels along the edge of the fields will, for the most part, be capable of only carrying small flows on the order of 10 to 50 cfs. Any runoff in excess of this amount will either flow through the fields or down and across the roads. Since the majority of excess runoff from any fields would most likely flow in a wide shallow configuration (most probably confined to the downstream edge of each field), a wide, shallow trapezoidal cross-section was used to route water from field to field. Again, as a conservative assumption, no attempt was made to quantify hydrograph attenuation as a function of channel storage effects.
3. Routing through culverts - Present versions of HEC-1 have no capability to model pressure flow through closed conduits such as culverts. When flow being routed through a pipe exceeds approximately 90 percent of the pipe capacity, the program assumes that the capacity increases as required, with no upper limit to the size of pipe that would be required to pass the computed flow.

Most of the fields in the watershed are linked together by small culverts passing under the roads that separate the fields. These culverts join the small irrigation/drainage ditches that are located along the boundaries of each field. These culverts are normally located at the low point in one corner of the fields. In most instances, this is at a road intersection which is elevated 2- to 5-feet above the invert of the culvert. Under inlet control conditions, flow through the culvert will be a function of the headwater depth and culvert inlet geometry. This type of control will undoubtedly produce less culvert flow than would result using the HEC-1 procedure of increasing pipe size to pass all incoming flows. Accordingly, the methodology used by HEC-1 will not reflect the hydrograph attenuation that will most probably occur at each culvert/road intersection. As a result, pipes were not used for channel routing. Again, from a freeway drainage design standpoint, failure to acknowledge this probable ponding and detention storage will produce a conservative design, since larger discharges than will actually occur will probably be routed to the highway.

4. Diversion of flow - Each of the road intersections within the watershed creates a potential for a north/south or east/west diversion of flow at the intersection. Due to the symmetrical layout of the agricultural fields in the watershed, all drainage patterns are essentially north/south or east/west. Accordingly, as water begins to overtop an intersection, there is a potential for the flow to move either south or west. In most instances, the flow patterns provided by the Indian Reservation personnel were adhered to when routing flow from an intersection (these patterns only indicated the predominant flow direction of the irrigation laterals/drainage ditches, not the potential for a diversion). SLA field inspections of the area revealed six locations where there was some potential for a flow split. The splits at these locations were made on the basis of field inspections, engineering judgement, and a conservative approach that dictated taking the majority of the water westerly

towards the freeway.

#### 4.4 Hydrologic Modeling Results

Table 4.1 presents the results of the HEC-1 modeling that was accomplished to predict peak discharge values for the design of an interceptor channel along the proposed Outer Loop alignment. This table lists all the concentration points along the highway where the flow is incremented.

Concentration Point (north to south)	<sup>1/</sup> Q100 (cfs)	<sup>2/</sup> Q50 (cfs)
110	90	67
142	185	139
202	283	212
352	489	369
442	522	393
562	555	418
672	586	441
782	1674	1191
873	1748	1247
892	1751	1250
1022	1794	1281
1060	1795	1282
1082	1807	1291
1085	1810	1294
1112	1821	1302
1171	1818	1299
1205	2013	1442
1222	2044	1465
1240	2051	1470
1252	2064	1479
1253	2063	1479

Note: All discharge values are based on 24-hour precipitation applied to a HEC-1 hypothetical rainfall distribution of 24-hour duration.

<sup>1/</sup> Model No. 3FD1.24I

<sup>2/</sup> Model No. 3FD5.24I

#### 4.5 Verification of Model Results

In order to establish confidence in the results of the HEC-1 model, it is important to utilize an independent procedure to calculate peak discharge values that can be compared to the computerized modeling results. Due to the agricultural composition of this watershed, regional regression equations for specific areas of Arizona were considered inappropriate for this purpose. A preferred alternative procedure was found in the January 1975 edition of Technical Release 55 (TR55), Urban Hydrology for Small Watersheds. Chapter 4 and Appendices D and E of this publication contain procedures, tables, and figures for developing peak discharge estimates from agricultural watersheds ranging from 1 to 2000 acres in size. This procedure also contains an optional adjustment factor to simulate ponding that might occur in the agricultural fields under study for this reach of the Outer Loop. Calculations were performed both with and without this ponding adjustment and were based on a 100-year, 24-hour storm (Type II rainfall distribution) with 24-hour curve numbers.

Sub-Basins 270 and 730 were used for the verification analysis. Both of these sub-basins are approximately 80 acres in size and are homogeneous agricultural fields. Sub-Basin 270 is in Hydrologic Soil Group C, while Sub-Basin 730 is Hydrologic Soil Group B. The TR55 "with ponding" approach assumed two acres of each of these sub-basins would pond water.

The Cook Method (Chow, 1964, Chapter 21) was also used as a second method to verify results for the 50-year event. This empirical method, which is a much simpler procedure than TR55, computes peak discharge as a function of a "W" factor which provides a numerical rating for watershed: 1) relief; 2) infiltration; 3) vegetal cover; and, 4) surface storage.

The results of the verification calculations are summarized in Table 4.2. The TR55 procedure, with no ponding, produces excellent agreement with the HEC-1 results. As expected, the introduction of a ponding factor produced lower results than HEC-1. The Cook Method produced substantially higher peaks than either HEC-1 or TR55.

The data base used to develop the Cook Method is unknown. For the agricultural basin under investigation, the infiltration and surface storage components of the "W" factor are possible sources of error for this specific site. Since the TR55 procedure is a much more detailed and site-specific

procedure, it is given more weight than the Cook Method in the model verification analysis.

As a matter of technical interest, it should be noted that the June 1986 edition of TR55 no longer includes the peak discharge procedure used in the preceding model verification analysis. SCS personnel (Gary Conaway) in Portland, Oregon, were contacted in an effort to determine why the procedure was excluded from the 1986 edition of TR55. Mr. Conaway indicated it was eliminated because of weaknesses in application to urbanized environments. However, since urbanization is not assumed to occur on the Reservation agricultural fields, the 1975 TR55 procedure should still provide a valid estimate of peak discharge. Mr. Conaway concurred in the validity of this conclusion. Accordingly, the 1975 TR55 procedure was adopted for use in this study. Based on this analysis, it appears that the model is providing a reasonable prediction of runoff from the agricultural fields that comprise the vast majority of the watershed.

TABLE 4.2 Comparison of Peak Discharge Calculations for Verification of HEC-1 Model									
Sub-Basin	Area (ac)	CN	Q100 (cfs)			Q50 (cfs)			Cook Method
			HEC-1	TR55 (no ponding)	TR55 (2.5% ponding)	HEC-1	TR55 (no ponding)	TR55 (2.5% ponding)	
270	78.8	87	67	66	44	50	55	35	72
730	80.0	80	43	43	29	30	35	23	60

## V. CONCLUSIONS AND RECOMMENDATIONS

This report presents a technical overview of the engineering parameters that were used to create a computerized rainfall/runoff model of the offsite drainage intercepted by that reach of the Outer Loop Highway extending from the Arizona Canal to the Salt River. The model simulates the runoff response that would be associated with both the 100- and 50-year, 24-hour precipitation applied to the 24-hour hypothetical rainfall distribution generated by HEC-1.

When compared to peak discharge estimates generated from independent calculation methods, the model results were judged to be realistic, although probably somewhat conservative. This conservatism is believed to result from the following factors:

1. No onsite detention has been assumed when modeling the agricultural fields, even though such detention undoubtedly exists. As a result, predicted HEC-1 peak discharges are probably high.
2. Any constriction of flow (and the resultant ponding of water at elevated road intersections), due to the small pipes that link each field, has been ignored. This allows water to flow freely from field to field without any hydrograph attenuation due to reservoir storage effects.

A very basic and key assumption used in this analysis is the depiction of flowage patterns through the drainage area. SLA was not provided with new topographic maps of the watershed. Since the U.S.G.S. quadrangle maps do not reflect the land-leveling and irrigation system construction that has occurred in the watershed, complete reliance was placed on representatives of the Salt River Indian Reservation to identify the drainage patterns for the agricultural fields. Should future development occur in this watershed that would alter this existing drainage pattern, the inflow points to the proposed highway interceptor channel could be significantly altered. Depending on how such alterations might occur, the proposed channel capacity might be subjected to either an under- or over-design.

It is also important to emphasize that any future land-use changes that might alter this watershed towards a more urbanized condition will undoubtedly generate a potential for increased runoff, as the farmland is covered with more impervious surfaces such as asphalt streets, parking lots, rooftops, etc. If such changes are ever allowed to occur, it is important that effective drainage ordinances be enforced to insure that peak discharges are not increased along the Outer Loop Highway alignment.

Although not specifically intended for such a purpose, the possible conservatism that might exist in the peak discharge values from the HEC-1 model may help to offset any minor inadvertent increases in runoff due future land-use changes.

In summary, SLA recommends that the HEC-1 model presented in this report be adopted for use in the concept design of offsite drainage structures for this reach of the Outer Loop Highway. Both 100- and 50-year storm frequencies should be considered for engineering and economic comparisons.

APPENDIX A

\*\*\*\*

FLOOD HYDROGRAPH PACKAGE HEC-1 (IBM XT 512K VERSION) -FEB 1,1985  
U.S. ARMY CORPS OF ENGINEERS, THE HYDROLOGIC ENGINEERING CENTER, 609 SECOND STREET, DAVIS, CA. 95616  
\*\*\*\*

THIS HEC-1 VERSION CONTAINS ALL OPTIONS EXCEPT ECONOMICS, AND THE NUMBER OF PLANS ARE REDUCED TO 3

LINE	ID	1	2	3	4	5	6	7	8	9	10
1	ID	SIMONS, LI AND ASSOCIATES									
2	ID	OUTER LOOP FREEWAY, PHASE 3 (FROM THE ARIZONA CANAL TO THE SALT RIVER)									
3	ID	FINAL HYDROLOGY ANALYSIS FOR OFFSITE DRAINAGE, MARCH 26, 1989									
4	ID	MODEL 3FD1.24I - MARCH 1989 HIGHWAY ALIGNMENT									
5	ID	100 YEAR EVENT, 24 HOUR HYPOTHETICAL STORM DISTRIBUTION									
6	ID	24 HOUR CN VALUES									
7	ID	WITH LINED CHANNEL ALONG OUTER LOOP ALIGNMENT (n=.012)									
8	IT	5	26MAR89	0	289						
9	*DIAGRAM										
9	IO	5									
10	KK	550	SUB								
11	KM	RUNOFF FROM SUB 550									
12	BA	.1250									
13	PH		0.1	0.74	1.45	2.54	2.78	2.93	3.23	3.54	3.85
14	LS		83								
15	UK	1320	.0029	.15	100						
16	RK	2640	.0028	.045		TRAP	10	100			
17	KK	651	CP								
18	KM	ROUTE SUB 550 TO CP 651									
19	RK	1320	.0029	.045		TRAP	10	100			
20	KK	650	SUB								
21	KM	RUNOFF FROM SUB 650									
22	BA	.1250									
23	LS		87								
24	UK	1320	.0029	.15	100						
25	RK	2640	.0028	.045		TRAP	10	100			
26	KK	652	CP								
27	KM	COMBINE SUB 650 WITH CP 651									
28	HC	2									
29	KK	640	SUB								
30	KM	RUNOFF FROM SUB 640 AND ROUTE CP 652									
31	BA	.1250									
32	LS		80								
33	UK	1320	.0029	.15	100						
34	RK	2640	.0028	.045		TRAP	10	100	YES		
35	KK	430	SUB								
36	KM	RUNOFF FROM SUB 430									
37	BA	0.0409									
38	LS		80								
39	UK	400	.0029	.15	100						
40	RK	1650	.0028	.045		TRAP	10	100			
41	KK	420	SUB								
42	KM	RUNOFF FROM SUB 420 AND ROUTE SUB 430									
43	BA	0.1510									
44	LS		80								
45	UK	1500	.0029	.15	100						
46	RK	2600	.0028	.045		TRAP	10	100	YES		

LINE	ID	1	2	3	4	5	6	7	8	9	10
47	KK	501	CP								
48	KM	ROUTE SUB 420 TO CP 501									
49	RK	1320	.0029	.045		TRAP	10	100			
50	KK	500	SUB								
51	KM	RUNOFF FROM SUB 500									
52	BA	0.1250									
53	LS	80									
54	UK	1320	.0029	.15	100						
55	RK	2640	.0028	.045		TRAP	10	100			
56	KK	502	CP								
57	KM	COMBINE CP 501 WITH SUB 500									
58	HC	2									
59	KK	641	CP								
60	KM	ROUTE CP 502 TO CP 641									
61	RK	1320	.0029	.045		TRAP	10	100			
62	KK	642	CP								
63	KM	COMBINE SUB 640 WITH CP 641									
64	HC	2									
65	KK	630	SUB								
66	KM	RUNOFF FROM SUB 630 AND ROUTE CP 642									
67	BA	0.0597									
68	LS	80									
69	UK	1280	.0029	.15	100						
70	RK	1300	.0028	.045		TRAP	10	100	YES		
71	KK	490	SUB								
72	KM	RUNOFF FROM SUB 490									
73	BA	0.0625									
74	LS	80									
75	UK	1320	.0029	.15	100						
76	RK	1320	.0028	.045		TRAP	10	100			
77	KK	631	CP								
78	KM	ROUTE SUB 490 TO CP 631									
79	RK	1280	.0029	.045		TRAP	10	100			
80	KK	632	CP								
81	KM	COMBINE SUB 630 WITH CP 631									
82	HC	2									
83	KK	621	CP								
84	KM	ROUTE CP 632 TO CP 621									
85	RK	1320	.0028	.045		TRAP	10	100			

LINE	ID	1	2	3	4	5	6	7	8	9	10
86	KK	230		SUB							
87	KM			RUNOFF FROM SUB 230							
88	BA	0.0447									
89	LS			87							
90	UK	800	.0029	.015	100						
91	RK	1400	.0028	.045			TRAP	10	100		
92	KK	231		SUB							
93	KM			RUNOFF FROM SUB 231 AND ROUTE SUB 230							
94	BA	0.0105									
95	LS			87							
96	UK	680	.0028	.15	100						
97	RK	430	.0029	.045			TRAP	10	100	YES	
98	KK	281		CP							
99	KM			ROUTE SUB 231 TO CP 281							
100	RK	1320	.0029	.045			TRAP	10	100		
101	KK	290		SUB							
102	KM			RUNOFF FROM SUB 290							
103	BA	0.1131									
104	LS			83							
105	UK	1700	.0028	.15	100						
106	RK	1720	.0029	.045			TRAP	10	100		
107	KK	280		SUB							
108	KM			RUNOFF FROM SUB 280 AND ROUTE SUB 290							
109	BA	0.0317									
110	LS			87							
111	UK	1320	.0029	.15	100						
112	RK	670	.0028	.045			TRAP	10	100	YES	
113	KK	282		CP							
114	KM			COMBINE SUB 280 WITH CP 281							
115	HC	2									
116	KK	401		CP							
117	KM			ROUTE CP 282 TO CP 401							
118	RK	1300	.0029	.045			TRAP	10	100		
119	KK	410		SUB							
120	KM			RUNOFF FROM SUB 410							
121	BA	0.0625									
122	LS			80							
123	UK	1320	.0029	.15	100						
124	RK	1320	.0028	.045			TRAP	10	100		
125	KK	400		SUB							
126	KM			RUNOFF FROM SUB 400 AND ROUTE SUB 410							
127	BA	0.0606									
128	LS			83							
129	UK	1300	.0029	.15	100						
130	RK	1300	.0028	.045			TRAP	10	100	YES	

LINE	ID	1	2	3	4	5	6	7	8	9	10
131	KK	402	CP								
132	KM	COMBINE SUB 400 WITH CP 401									
133	HC	2									
134	KK	620	SUB								
135	KM	RUNOFF FROM SUB 620 AND ROUTE CP 402									
136	BA	0.1250									
137	LS		80								
138	UK	1320	.0028	.15	100						
139	RK	2640	.0029	.045		TRAP	10	100	YES		
140	KK	622	CP								
141	KM	COMBINE SUB 620 WITH CP 621									
142	HC	2									
143	KK	600	SUB								
144	KM	RUNOFF FROM SUB 600 AND ROUTE CP 622									
145	BA	0.1250									
146	LS		86								
147	UK	1320	.0029	.15	100						
148	RK	2640	.0028	.045		TRAP	10	100	YES		
149	KK	170	SUB								
150	KM	RUNOFF FROM SUB 170									
151	BA	0.1297									
152	LS		87								
153	UK	1200	.0029	.15	100						
154	RK	2650	.0028	.045		TRAP	10	100			
155	KK	221	CP								
156	KM	ROUTE SUB 170 TO CP 221									
157	RK	1320	.0029	.045		TRAP	10	100			
158	KK	220	SUB								
159	KM	RUNOFF FROM SUB 220									
160	BA	0.1250									
161	LS		87								
162	UK	1320	.0029	.15	100						
163	RK	2640	.0028	.045		TRAP	10	100			
164	KK	222	CP								
165	KM	COMBINE SUB 220 WITH CP 221									
166	HC	2									
167	KK	271	CP								
168	KM	ROUTE CP 222 TO CP 271									
169	RK	1300	.0029	.045		TRAP	10	100			
170	KK	270	SUB								
171	KM	RUNOFF FROM SUB 270									
172	BA	0.1231									
173	LS		87								
174	UK	1300	.0029	.15	100						
175	RK	2640	.0028	.045		TRAP	10	100			

LINE	ID	1	2	3	4	5	6	7	8	9	10
176	KK	272									
177	KM	COMBINE CP 271 WITH SUB 270									
178	HC	2									
179	KK	391									
180	KM	ROUTE CP 272 TO CP 391									
181	RK	1320	.0029	.045		TRAP	10	100			
182	KK	390									
183	KM	RUNOFF FROM SUB 390									
184	BA	0.1250									
185	LS		87								
186	UK	1320	.0029	.15	100						
187	RK	2640	.0028	.045		TRAP	10	100			
188	KK	392									
189	KM	COMBINE CP 391 WITH SUB 390									
190	HC	2									
191	KK	394									
192	KM	DIVERT FLOW FROM CP 392 TO SUB 380									
193	DT	393									
194	DI	0	12	26	50	2000					
195	DO	0	6	13	25	1975					
196	KK	471									
197	KM	ROUTE NON-DIVERTED FLOW FROM CP 392 (DIV 394) TO CP 471									
198	RK	1320	.0029	.045		TRAP	10	100			
199	KK	470									
200	KM	RUNOFF FROM SUB 470									
201	BA	0.1250									
202	LS		87								
203	UK	1320	.0029	.15	100						
204	RK	2640	.0028	.045		TRAP	10	100			
205	KK	472									
206	KM	COMBINE CP 471 WITH SUB 470									
207	HC	2									
208	KK	601									
209	KM	ROUTE CP 472 TO CP 601									
210	RK	1320	.0029	.045		TRAP	10	100			
211	KK	602									
212	KM	COMBINE SUB 600 WITH CP 601									
213	HC	2									
214	KK	604									
215	KM	DIVERT FLOW FROM CP 602 TO SUB 590									
216	DT	603									
217	DI	0	10	100	2000						
218	DO	0	5	50	1000						

LINE	ID	1	2	3	4	5	6	7	8	9	10
219	KK	721		CP							
220	KM			ROUTE NON-DIVERTED FLOW FROM CP 602 (DIV 604) TO CP 721							
221	RK	1320	.0029	.045		TRAP	10		100		
222	KK	720		SUB							
223	KM			RUNOFF FROM SUB 720							
224	BA	0.1250									
225	LS			84							
226	UK	1320	.0029	.15		100					
227	RK	2640	.0028	.045		TRAP	10		100		
228	KK	722		CP							
229	KM			COMBINE CP 721 WITH SUB 720							
230	HC	2									
231	KK	811		CP							
232	KM			ROUTE CP 722 TO CP 811							
233	RK	1320	.0029	.045		TRAP	10		100		
234	KK	760		SUB							
235	KM			RUNOFF FROM SUB 760							
236	BA	0.0625									
237	LS			87							
238	UK	660	.0029	.15		100					
239	RK	2640	.0028	.045		TRAP	10		100		
240	KK	751		CP							
241	KM			ROUTE SUB 760 TO CP 751							
242	RK	660	.0029	.045		TRAP	10		100		
243	KK	750		SUB							
244	KM			RUNOFF FROM SUB 750							
245	BA	0.0625									
246	LS			87							
247	UK	660	.0029	.15		100					
248	RK	2640	.0028	.045		TRAP	10		100		
249	KK	752		CP							
250	KM			COMBINE SUB 750 WITH CP 751							
251	HC	2									
252	KK	841		CP							
253	KM			ROUTE CP 752 TO CP 841							
254	RK	1320	.0029	.045		TRAP	10		100		
255	KK	850		SUB							
256	KM			RUNOFF FROM SUB 850							
257	BA	0.0701									
258	LS			81							
259	UK	1320	.0029	.10		100					
260	RK	1480	.0028	.045		TRAP	10		100		

LINE	ID	1	2	3	4	5	6	7	8	9	10
261	KK	840		SUB							
262	KM			RUNOFF FROM SUB 840 AND ROUTE SUB 850							
263	BA	0.0549									
264	LS		87								
265	UK	1320	.0029	.15	100						
266	RK	1160	.0028	.045		TRAP	10	100	YES		
267	KK	842		CP							
268	KM			COMBINE SUB 840 WITH CP 841							
269	HC	2									
270	KK	830		SUB							
271	KM			RUNOFF FROM SUB 830 AND ROUTE CP 842							
272	BA	0.1250									
273	LS		81								
274	UK	1320	.0029	.15	100						
275	RK	2640	.0028	.045		TRAP	10	100	YES		
276	KK	740		SUB							
277	KM			RUNOFF FROM SUB 740							
278	BA	0.1250									
279	LS		81								
280	UK	1320	.0029	.15	100						
281	RK	2640	.0028	.045		TRAP	10	100			
282	KK	831		CP							
283	KM			ROUTE SUB 740 TO CP 831							
284	RK	1320	.0029	.045		TRAP	10	100			
285	KK	832		CP							
286	KM			COMBINE CP 831 WITH SUB 830							
287	HC	2									
288	KK	820		SUB							
289	KM			RUNOFF FROM SUB 820 AND ROUTE CP 832							
290	BA	0.1250									
291	LS		80								
292	UK	1320	.0029	.15	100						
293	RK	2640	.0028	.045		TRAP	10	100	YES		
294	KK	730		SUB							
295	KM			RUNOFF FROM SUB 730							
296	BA	0.1250									
297	LS		80								
298	UK	1320	.0029	.15	100						
299	RK	2640	.0028	.045		TRAP	10	100			
300	KK	821		CP							
301	KM			ROUTE SUB 730 TO CP 821							
302	RK	1320	.0029	.045		TRAP	10	100			

LINE	ID	1	2	3	4	5	6	7	8	9	10
303	KK	822	CP								
304	KM	COMBINE SUB 820 WITH CP 821									
305	HC	2									
306	KK	824	DIV								
307	KM	DIVERT FLOW FROM CP 822 TO SUB 971									
308	KM	THIS DIVERT LEAVES THE SYSTEM & WILL NOT BE RETRIEVED. SUB 971 HAS ALSO									
309	KM	BEEN EXCLUDED FROM THE SYSTEM									
310	DT	823									
311	DI	0	10	26	50	100	1000				
312	DQ	0	5	13	20	40	400				
313	KK	810	SUB								
314	KM	RUNOFF FROM SUB 810 AND ROUTE NON-DIVERTED FLOW FROM CP 822 (DIV 824)									
315	BA	0.1250									
316	LS	80									
317	UK	1320	.0029	.15	100						
318	RK	2640	.0028	.045		TRAP	10	100	YES		
319	KK	812	CP								
320	KM	COMBINE CP 811 WITH SUB 810									
321	HC	2									
322	KK	800	SUB								
323	KM	RUNOFF FROM SUB 800 AND ROUTE CP 812									
324	BA	0.0928									
325	LS	80									
326	UK	1320	.0029	.15	100						
327	RK	1960	.0028	.045		TRAP	10	100	YES		
328	KK	804	DIV								
329	KM	DIVERT SUB 800 TO IRRIGATION POND (SUB 945)									
330	DT	805									
331	DI	0	10	26	50	100	2000				
332	DQ	0	5	13	25	50	50				
333	KK	801	SUB								
334	KM	RUNOFF FROM SUB 801 AND ROUTE NON-DIVERTED FLOW FROM SUB 800 (DIV 804)									
335	BA	0.0331									
336	LS	80									
337	UK	1320	.0029	.15	100						
338	RK	700	.0028	.045		TRAP	10	100	YES		
339	KK	130	SUB								
340	KM	RUNOFF FROM SUB 130									
341	BA	0.1042									
342	LS	87									
343	UK	1000	.0029	.15	100						
344	RK	2620	.0028	.045		TRAP	10	100			

LINE	ID	1	2	3	4	5	6	7	8	9	10
345	KK	161		CP							
346	KM			ROUTE SUB 130 TO CP 161							
347	RK	1260	.0029	.045		TRAP	10		100		
348	KK	160		SUB							
349	KM			RUNOFF FROM SUB 160							
350	BA	0.1193									
351	LS			87							
352	UK	1260	.0029	.15	100						
353	RK	2640	.0028	.045		TRAP	10		100		
354	KK	162		CP							
355	KM			COMBINE CP 161 WITH SUB 160							
356	HC	2									
357	KK	211		CP							
358	KM			ROUTE CP 162 TO CP 211							
359	RK	1320	.0029	.045		TRAP	10		100		
360	KK	210		SUB							
361	KM			RUNOFF FROM SUB 210							
362	BA	0.1217									
363	LS			87							
364	UK	1320	.0029	.15	100						
365	RK	2570	.0028	.045		TRAP	10		100		
366	KK	212		CP							
367	KM			COMBINE CP 211 WITH SUB 210							
368	HC	2									
369	KK	261		CP							
370	KM			ROUTE CP 212 TO CP 261							
371	RK	1300	.0029	.045		TRAP	10		100		
372	KK	260		SUB							
373	KM			RUNOFF FROM SUB 260							
374	BA	0.1203									
375	LS			87							
376	UK	1300	.0029	.15	100						
377	RK	2580	.0028	.045		TRAP	10		100		
378	KK	262		CP							
379	KM			COMBINE CP 261 WITH SUB 260							
380	HC	2									
381	KK	381		CP							
382	KM			ROUTE CP 262 TO CP 381							
383	RK	1320	.0029	.045		TRAP	10		100		

SCHEMATIC DIAGRAM OF STREAM NETWORK

INPUT LINE	(V) ROUTING	(---) DIVERSION OR PUMP FLOW
NO.	(.) CONNECTOR	(<---) RETURN OF DIVERTED OR PUMPED FLOW
10	550	
	V	
	V	
17	651	
	.	
20	.	650
	.	.
26	652.....	
	V	
	V	
29	640 ***	
	.	
35	.	430
	.	V
	.	V
41	.	420 ***
	.	V
	.	V
47	.	501
	.	.
50	.	500
	.	.
56	.	502.....
	.	V
	.	V
59	.	641
	.	.
62	642.....	
	V	
	V	
65	630 ***	
	.	
71	.	490
	.	V
	.	V
77	.	631
	.	.
80	632.....	
	V	
	V	
83	621	
	.	
86	.	230
	.	V
	.	V
92	.	231 ***
	.	V

101	.	.	290
	.	.	V
	.	.	V
107	.	.	280 ***
	.	.	.
113	.	282.....	.
	.	V	.
	.	V	.
116	.	401	.
	.	.	.
119	.	.	410
	.	.	V
	.	.	V
125	.	.	400 ***
	.	.	.
131	.	402.....	.
	.	V	.
	.	V	.
134	.	620 ***	.
	.	.	.
140	622.....	.	.
	V	.	.
	V	.	.
143	600 ***	.	.
	.	.	.
149	.	170	.
	.	V	.
	.	V	.
155	.	221	.
	.	.	.
158	.	.	220
	.	.	.
164	.	222.....	.
	.	V	.
	.	V	.
167	.	271	.
	.	.	.
170	.	.	270
	.	.	.
176	.	272.....	.
	.	V	.
	.	V	.
179	.	391	.
	.	.	.
182	.	.	390
	.	.	.
188	.	392.....	.
	.	.	.
193	.	-----)	393
191	.	394	.

199	.	.	470
	.	.	.
205	.	472.....	.
	.	V	.
	.	V	.
208	.	601	.
	.	.	.
211	.	602.....	.
	.	.	.
216	.	-----)	603
214	.	604	.
	.	V	.
	.	V	.
219	.	721	.
	.	.	.
222	.	720	.
	.	.	.
228	.	722.....	.
	.	V	.
	.	V	.
231	.	811	.
	.	.	.
234	.	760	.
	.	V	.
	.	V	.
240	.	751	.
	.	.	.
243	.	.	750
	.	.	.
249	.	752.....	.
	.	V	.
	.	V	.
252	.	841	.
	.	.	.
255	.	.	850
	.	.	V
	.	.	V
261	.	.	840 ***
	.	.	.
267	.	842.....	.
	.	V	.
	.	V	.
270	.	830 ***	.
	.	.	.
276	.	.	740
	.	.	V
	.	.	V
282	.	.	831
	.	.	.
	.	.	.

288	.	820 ***	
	.	.	
294	.		730
	.		V
	.		V
300	.		821
	.		.
	.		.
303	.	822.....	
	.	.	
	.	.	
310	.	----->	823
306	.	824	
	.	V	
	.	V	
313	.	810 ***	
	.	.	
	.	.	
319	.	812.....	
	.	V	
	.	V	
322	.	800 ***	
	.	.	
	.	.	
330	.	----->	805
328	.	804	
	.	V	
	.	V	
333	.	801 ***	
	.	.	
	.	.	
339	.	130	
	.	V	
	.	V	
345	.	161	
	.	.	
	.	.	
348	.		160
	.	.	.
	.	.	.
354	.	162.....	
	.	V	
	.	V	
357	.	211	
	.	.	
	.	.	
360	.		210
	.	.	.
	.	.	.
366	.	212.....	
	.	V	
	.	V	
369	.	261	
	.	.	
	.	.	
372	.		260
	.	.	.
	.	.	.
378	.	262.....	
	.	V	
	.	V	
381	.	381	

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384

393

V

V

387

380 \*\*\*

393

382.....

398

-----) 383

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384

V

V

401

461

404

460

410

462.....

V

V

413

591

418

-----) 603

416

603

V

V

419

590 \*\*\*

425

592.....

V

V

428

701

431

710

V

V

437

700 \*\*\*

443

702.....

V

V

446

802

449

950

457

-----) 805

455

805

458

946.....

V

V

461

945 \*\*\*

LINE	ID	1	2	3	4	5	6	7	8	9	10
384	KK	393	RET								
385	KM	RETRIEVE DIVERT 393 FROM SUB 390 (CP 392)									
386	DR	393									
387	KK	380	SUB								
388	KM	RUNOFF FROM SUB 380 AND ROUTE RETRIEVAL 393									
389	BA	0.1250									
390	LS		87								
391	UK	1320	.0029	.15	100						
392	RK	2640	.0028	.045		TRAP	10	100	YES		
393	KK	382	CP								
394	KM	COMBINE CP 381 WITH SUB 380									
395	HC	2									
396	KK	384	DIV								
397	KM	DIVERT FLOW FROM CP 382 TO SUB 450									
398	DT	383									
399	DI	0	10	100	2000						
400	DQ	0	5	50	1000						
401	KK	461	CP								
402	KM	ROUTE NON-DIVERTED FLOW FROM CP 382 (DIV 384) TO CP 461									
403	RK	1320	.0029	.045		TRAP	10	100			
404	KK	460	SUB								
405	KM	RUNOFF FROM SUB 460									
406	BA	0.1250									
407	LS		87								
408	UK	1320	.0029	.15	100						
409	RK	2640	.0028	.045		TRAP	10	100			
410	KK	462	CP								
411	KM	COMBINE CP 461 WITH SUB 460									
412	HC	2									
413	KK	591	CP								
414	KM	ROUTE CP 462 TO CP 591									
415	RK	1320	.0029	.045		TRAP	10	100			
416	KK	603	RET								
417	KM	RETRIEVE DIVERT 603 FROM SUB 600 (CP 602)									
418	DR	603									
419	KK	590	SUB								
420	KM	RUNOFF FROM SUB 590 AND ROUTE RET 603									
421	BA	0.1250									
422	LS		87								
423	UK	1320	.0029	.15	100						
424	RK	2640	.0028	.045		TRAP	10	100	YES		

LINE	ID	1	2	3	4	5	6	7	8	9	10
425	KK	592	CP								
426	KM	COMBINE CP 591 WITH SUB 590									
427	HC	2									
428	KK	701	CP								
429	KM	ROUTE CP 592 TO CP 701									
430	RK	1320	.0029	.045			TRAP	10	100		
431	KK	710	SUB								
432	KM	RUNOFF FROM SUB 710									
433	BA	0.0644									
434	LS		83								
435	UK	1360	.0028	.15	100						
436	RK	1320	.0029	.045			TRAP	10	100		
437	KK	700	SUB								
438	KM	RUNOFF FROM SUB 700 AND ROUTE SUB 710									
439	BA	0.0601									
440	LS		85								
441	UK	1320	.0029	.15	100						
442	RK	1270	.0028	.045			TRAP	10	100	YES	
443	KK	702	CP								
444	KM	COMBINE CP 701 WITH SUB 700									
445	HC	2									
446	KK	802	CP								
447	KM	ROUTE CP 702 TO CP 802									
448	RK	1320	.0029	.045			TRAP	10	100		
449	KK	950	SUB								
450	KM	RUNOFF FROM SUB 950									
451	BA	0.0928									
452	LS		80								
453	UK	1960	.0028	.15	100						
454	RK	1300	.0015	.045			TRAP	10	100		
455	KK	805	RET								
456	KM	RETRIEVE DIVERT 805 FROM SUB 800									
457	DR	805									
458	KK	946	CP								
459	KM	COMBINE SUB 950 WITH RET 805									
460	HC	2									
461	KK	945	SUB								
462	KM	RUNOFF FROM SUB 945 AND ROUTE CP 946									
463	KM	THIS SUB-BASIN IS PRIMARILY AN IRRIGATION POND									
464	BA	0.0030									
465	LS		98								
466	UK	75	10.0	0.01	100						
467	RK	600	.0005	0.01			TRAP	150	1	YES	

LINE	ID	1	2	3	4	5	6	7	8	9	10
468	KK	940									
			SUB								
469	KM										
			RUNOFF FROM SUB 940								
470	BA	0.0294									
471	LS		80								
472	UK	1170	.0020	.15	100						
473	RK	700	.0020	.045			TRAP	10	100		
474	KK	803									
			CP								
475	KM										
			COMBINE CP 802, SUB 945, SUB 940, AND SUB 801								
476	HC		4								
477	KK	806									
			DIV								
478	KM										
			DIVERT FLOW FROM CP 803 TO SUB 960								
479	KM										
			THIS DIVERT LEAVES THE SYSTEM AND WILL NOT BE RETRIEVED. SUB 960 HAS								
480	KM										
			ALSO BEEN EXCLUDED FROM THE SYSTEM.								
481	DT	807									
482	DI	0	80	100	280	300	1000				
483	DQ	0	0	20	200	200	200				
484	KK	790									
			SUB								
485	KM										
			RUNOFF FROM SUB 790 AND ROUTE DIV 806								
486	BA	0.1250									
487	LS		82								
488	UK	1320	.0029	.15	100						
489	RK	2640	.0028	.045			TRAP	10	100	YES	
490	KK	383									
			RET								
491	KM										
			RETRIEVE DIVERT 383 FROM SUB 380 (CP 382)								
492	DR	383									
493	KK	451									
			CP								
494	KM										
			ROUTE RET 383 TO CP 451								
495	RK	1320	.0029	.045			TRAP	10	100		
496	KK	450									
			SUB								
497	KM										
			RUNOFF FROM SUB 450 AND ROUTE CP 451								
498	BA	0.1250									
499	LS		87								
500	UK	1320	.0029	.15	100						
501	RK	2640	.0028	.045			TRAP	10	100	YES	
502	KK	581									
			CP								
503	KM										
			ROUTE SUB 450 TO CP 581								
504	RK	1320	.0029	.045			TRAP	10	100		
505	KK	580									
			SUB								
506	KM										
			RUNOFF FROM SUB 580								
507	BA	0.1250									
508	LS		86								
509	UK	1320	.0029	.15	100						
510	RK	2640	.0028	.045			TRAP	10	100		

LINE	ID	1	2	3	4	5	6	7	8	9	10
511	KK	582		CP							
512	KM			COMBINE CP 581 WITH SUB 580							
513	HC	2									
514	KK	681		CP							
515	KM			ROUTE CP 582 TO CP 681							
516	RK	1300	.0029	.045			TRAP	10	100		
517	KK	680		SUB							
518	KM			RUNOFF FROM SUB 680							
519	BA	0.1231									
520	LS			85							
521	UK	1300	.0029	.15	100						
522	RK	2640	.0028	.045			TRAP	10	100		
523	KK	682		CP							
524	KM			COMBINE CP 681 WITH SUB 680							
525	HC	2									
526	KK	791		CP							
527	KM			ROUTE CP 682 TO CP 791							
528	RK	1320	.0029	.045			TRAP	10	100		
529	KK	792		CP							
530	KM			COMBINE CP 791 WITH SUB 790							
531	HC	2									
532	KK	780		SUB							
533	KM			RUNOFF FROM SUB 780 AND ROUTE CP 792							
534	BA	0.0625									
535	LS			87							
536	UK	1320	.0029	.15	100						
537	RK	1320	.0028	.045			TRAP	10	100	YES	
538	KK	110		SUB							
539	KM			RUNOFF FROM SUB 110							
540	BA	.1895									
541	LS			87							
542	UK	1500	.0029	.15	100						
543	RK	3950	.0028	.045			TRAP	10	100		
544	KK	141		CP							
545	KM			ROUTE SUB 110 TO CP 141							
546	RK	1365	.0032	.012			TRAP	10	1		
547	KK	140		SUB							
548	KM			RUNOFF FROM SUB 140							
549	BA	.1900									
550	LS			87							
551	UK	1365	.0029	.15	100						
552	RK	3950	.0028	.045			TRAP	10	100		

LINE	ID	1	2	3	4	5	6	7	8	9	10
553	KK	142	CP								
554	KM	COMBINE SUB 140 WITH CP 141									
555	HC	2									
556	KK	201	CP								
557	KM	ROUTE CP 142 TO CP 201									
558	RK	1320	.0032	.012			TRAP	10		1	
559	KK	200	SUB								
560	KM	RUNOFF FROM SUB 200									
561	BA	0.1889									
562	LS		87								
563	UK	1320	.0029	.15	100						
564	RK	3990	.0028	.045			TRAP	10		100	
565	KK	202	CP								
566	KM	COMBINE SUB 200 WITH CP 201									
567	HC	2									
568	KK	351	CP								
569	KM	ROUTE CP 202 TO CP 351									
570	RK	2640	.0032	.012			TRAP	10		1	
571	KK	250	SUB								
572	KM	RUNOFF FROM SUB 250									
573	BA	0.0616									
574	LS		87								
575	UK	1320	.0029	.15	100						
576	RK	1300	.0028	.045			TRAP	10		100	
577	KK	251	SUB								
578	KM	RUNOFF FROM SUB 251									
579	BA	0.0621									
580	LS		87								
581	UK	1310	.0029	.15	100						
582	RK	1320	.0028	.045			TRAP	10		100	
583	KK	252	CP								
584	KM	COMBINE SUB 250 WITH SUB 251									
585	HC	2									
586	KK	371	CP								
587	KM	ROUTE CP 252 TO CP 371									
588	RK	1320	.0029	.045			TRAP	10		100	
589	KK	370	SUB								
590	KM	RUNOFF FROM SUB 370									
591	BA	0.0625									
592	LS		87								
593	UK	1320	.0029	.15	100						
594	RK	1320	.0028	.045			TRAP	10		100	

LINE	ID	1	2	3	4	5	6	7	8	9	10
595	KK	372		CP							
596	KM			COMBINE CP 371 WITH SUB 370							
597	HC	2									
598	KK	350		SUB							
599	KM			RUNOFF FROM SUB 350 & ROUTE CP 372							
600	BA	.1920									
601	LS		91								
602	UK	1980	.0029	.08	100						
603	RK	2690	.0028	.045		TRAP	10	100	YES		
604	KK	352		CP							
605	KM			COMBINE CP 351 & SUB 350							
606	HC	2									
607	KK	441		CP							
608	KM			ROUTE CP 352 TO CP 441							
609	RK	1320	.0032	.012		TRAP	15	1			
610	KK	440		SUB							
611	KM			RUNOFF FROM SUB 440							
612	BA	.0644									
613	LS		87								
614	UK	1320	.0029	.15	100						
615	RK	1350	.0028	.045		TRAP	10	100			
616	KK	442		CP							
617	KM			COMBINE SUB 440 & CP 441							
618	HC	2									
619	KK	561		CP							
620	KM			ROUTE CP 442 TO CP 561							
621	RK	1360	.0032	.012		TRAP	15	1			
622	KK	560		SUB							
623	KM			RUNOFF FROM SUB 560							
624	BA	.0659									
625	LS		87								
626	UK	1330	.0029	.15	100						
627	RK	1340	.0028	.045		TRAP	10	100			
628	KK	562		CP							
629	KM			COMBINE SUB 560 & CP 561							
630	HC	2									
631	KK	671		CP							
632	KM			ROUTE CP 562 TO CP 671							
633	RK	1280	.0032	.012		TRAP	15	1			



LINE	ID	1	2	3	4	5	6	7	8	9	10
676	KK	900		SUB							
677	KM			RUNOFF FROM SUB 900 AND ROUTE CP 922							
678	BA	0.0373									
679	LS		80								
680	UK	1300	.0029	.15	100						
681	RK	800	.0028	.045		TRAP	10	100	YES		
682	KK	910		SUB							
683	KM			RUNOFF FROM SUB 910							
684	BA	0.0649									
685	LS		80								
686	UK	1360	.0028	.15	100						
687	RK	1330	.0029	.045		TRAP	10	100			
688	KK	901		CP							
689	KM			COMBINE SUB 910 WITH SUB 900							
690	HC	2									
691	KK	880		SUB							
692	KM			RUNOFF FROM SUB 880 AND ROUTE CP 901							
693	BA	0.0266									
694	LS		80								
695	UK	920	.0029	.15	100						
696	RK	1100	.0028	.045		TRAP	10	100	YES		
697	KK	872		CP							
698	KM			ROUTE SUB 880 TO CP 872							
699	RK	670	.0028	.045		TRAP	10	100			
700	KK	873		CP							
701	KM			COMBINE CP 872, CP 871, AND SUB 870							
702	HC	3									
703	KK	891		CP							
704	KM			ROUTE CP 873 TO CP 891							
705	RK	700	.0032	.012		TRAP	30	1			
706	KK	890		SUB							
707	KM			RUNOFF FROM SUB 890							
708	BA	.0406									
709	LS		82								
710	UK	800	.0029	.10	100						
711	RK	1390	.0028	.045		TRAP	10	100			
712	KK	892		CP							
713	KM			COMBINE SUB 890 WITH CP 891							
714	HC	2									
715	KK	1020		SUB							
716	KM			RUNOFF FROM SUB 1020 & ROUTE CP 892							
717	BA	.0182									
718	LS		79								
719	UK	600	.0028	.10	100						
720	RK	1370	.0032	.012		TRAP	30	1	YES		

LINE	ID	1	2	3	4	5	6	7	8	9	10
721	KK	1040	SUB								
722	KM		RUNOFF FROM SUB 1040								
723	BA	0.0625									
724	LS		80								
725	UK	1300	.0028	.15	100						
726	RK	1340	.0029	.045		TRAP	10	100			
727	KK	1031	CP								
728	KM		ROUTE SUB 1040 TO CP 1031								
729	RK	800	.0028	.045		TRAP	10	100			
730	KK	1030	SUB								
731	KM		RUNOFF FROM SUB 1030 AND ROUTE CP 1031								
732	BA	0.0544									
733	LS		80								
734	UK	1100	.0028	.15	100						
735	RK	2000	.0029	.045		TRAP	10	100	YES		
736	KK	1070	SUB								
737	KM		RUNOFF FROM SUB 1070								
738	BA	0.0699									
739	LS		80								
740	UK	1500	.0029	.15	100						
741	RK	1300	.0028	.045		TRAP	10	100			
742	KK	1032	CP								
743	KM		COMBINE SUB 1070 WITH SUB 1030								
744	HC		2								
745	KK	1021	CP								
746	KM		ROUTE CP 1032 TO CP 1021								
747	RK	240	.0028	.045		TRAP	10	100			
748	KK	1022	CP								
749	KM		COMBINE SUB 1020 & CP 1021								
750	HC		2								
751	KK	1060	SUB								
752	KM		RUNOFF FROM SUB 1060 & ROUTE CP 1022								
753	BA	.0161									
754	LS		80								
755	UK	1270	.0028	.15	100						
756	RK	1370	.0032	.012		TRAP	30	1	YES		
757	KK	1083	CP								
758	KM		ROUTE SUB 1060 TO CP 1083								
759	RK	1260	.0032	.012		TRAP	30	1			
760	KK	1090	SUB								
761	KM		RUNOFF FROM SUB 1090								
762	BA	0.0413									
763	LS		80								
764	UK	800	.0029	.15	100						
765	RK	1740	.0028	.045		TRAP	10	100			

LINE	ID	1	2	3	4	5	6	7	8	9	10
766	KK	1080									
			SUB								
767	KM										
			RUNOFF FROM SUB 1080 AND ROUTE SUB 1090								
768	BA	.0294									
769	LS		77								
770	UK	1300	.0028	.10	100						
771	RK	1500	.0029	.045			TRAP	10	100	YES	
772	KK	1082									
			CP								
773	KM										
			COMBINE SUB 1080 WITH CP 1083								
774	HC	2									
775	KK	1084									
			CP								
776	KM										
			ROUTE CP 1082 TO CP 1084								
777	RK	705	.0032	.012			TRAP	30	1		
778	KK	1081									
			SUB								
779	KM										
			RUNOFF FROM SUB 1081								
780	BA	.0330									
781	LS		79								
782	UK	700	.0029	.10	100						
783	RK	1900	.0028	.045			TRAP	10	100		
784	KK	1085									
			CP								
785	KM										
			COMBINE CP 1084 & SUB 1081								
786	HC	2									
787	KK	1111									
			CP								
788	KM										
			ROUTE CP 1085 TO CP 1111								
789	RK	760	.0032	.012			TRAP	30	1		
790	KK	1110									
			SUB								
791	KM										
			RUNOFF FROM SUB 1110								
792	BA	.0709									
793	LS		80								
794	UK	1000	.0029	.15	100						
795	RK	1560	.0028	.045			TRAP	10	100		
796	KK	1112									
			CP								
797	KM										
			COMBINE CP 1111 & SUB 1110								
798	HC	2									
799	KK	1171									
			CP								
800	KM										
			ROUTE CP 1112 TO CP 1171								
801	RK	2800	.0032	.012			TRAP	30	1		
802	KK	1203									
			CP								
803	KM										
			ROUTE CP 1171 TO CP 1203								
804	RK	2010	.0032	.012			TRAP	30	1		

LINE	ID	1	2	3	4	5	6	7	8	9	10
805	KK	1071									
			SUB								
806	KM										
			RUNOFF FROM SUB 1071								
807	BA	0.0226									
808	LS		80								
809	UK	900	.0029	.15	100						
810	RK	1350	.0028	.045			TRAP	10	100		
811	KK	1072									
			SUB								
812	KM										
			RUNOFF FROM SUB 1072								
813	BA	0.0440									
814	LS		77								
815	UK	700	.0029	.10	100						
816	RK	1700	.0028	.045			TRAP	10	100		
817	KK	1074									
			CP								
818	KM										
			COMBINE SUB 1072 WITH SUB 1071								
819	HC	2									
820	KK	1073									
			SUB								
821	KM										
			RUNOFF FROM SUB 1073 AND ROUTE CP 1074								
822	BA	0.0430									
823	LS		80								
824	UK	1290	.0029	.15	100						
825	RK	930	.0028	.045			TRAP	10	100	YES	
826	KK	1092									
			CP								
827	KM										
			ROUTE SUB 1073 TO CP 1092								
828	RK	1320	.0029	.045			TRAP	10	100		
829	KK	1091									
			SUB								
830	KM										
			RUNOFF FROM SUB 1091								
831	BA	0.0353									
832	LS		82								
833	UK	600	.0029	.10	100						
834	RK	2000	.0028	.045			TRAP	10	100		
835	KK	1093									
			CP								
836	KM										
			COMBINE CP 1092 WITH SUB 1091								
837	HC	2									
838	KK	1122									
			CP								
839	KM										
			ROUTE CP 1093 TO CP 1122								
840	RK	1300	.0029	.045			TRAP	10	100		
841	KK	1121									
			SUB								
842	KM										
			RUNOFF FROM SUB 1121								
843	BA	0.0515									
844	LS		80								
845	UK	800	.0029	.15	100						
846	RK	1830	.0028	.045			TRAP	10	100		

LINE	ID	1	2	3	4	5	6	7	8	9	10
847	KK	1131	SUB								
848	KM		RUNOFF FROM SUB 1131								
849	BA	0.0379									
850	LS		80								
851	UK	1320	.0029	.15	100						
852	RK	800	.0028	.045		TRAP	10	100			
853	KK	1132	CP								
854	KM		COMBINE SUB 1131 WITH SUB 1121								
855	HC	2									
856	KK	1133	CP								
857	KM		ROUTE CP 1132 TO CP 1133								
858	RK	1320	.0029	.045		TRAP	10	100			
859	KK	1130	SUB								
860	KM		RUNOFF FROM SUB 1130								
861	BA	0.0379									
862	LS		80								
863	UK	1320	.0029	.15	100						
864	RK	800	.0028	.045		TRAP	10	100			
865	KK	1134	CP								
866	KM		COMBINE CP 1133 WITH SUB 1130								
867	HC	2									
868	KK	1120	SUB								
869	KM		RUNOFF FROM SUB 1120 AND ROUTE CP 1134								
870	BA	0.0853									
871	LS		80								
872	UK	1300	.0029	.15	100						
873	RK	1830	.0028	.045		TRAP	10	100	YES		
874	KK	1123	CP								
875	KM		COMBINE CP 1122 WITH SUB 1120								
876	HC	2									
877	KK	1181	CP								
878	KM		ROUTE CP 1123 TO CP 1181								
879	RK	2750	.0029	.045		TRAP	10	100			
880	KK	1150	SUB								
881	KM		RUNOFF FROM SUB 1150								
882	BA	.0670									
883	LS		80								
884	UK	1400	.0029	.15	100						
885	RK	2260	.0028	.045		TRAP	10	100			
886	KK	1172	CP								
887	KM		ROUTE SUB 1150 TO CP 1172								
888	RK	1320	.0029	.045		TRAP	10	100			

LINE	ID	1	2	3	4	5	6	7	8	9	10
889	KK	1170									
			SUB								
890	KM										
			RUNOFF FROM SUB 1170								
891	BA	.0671									
892	LS		80								
893	UK	1320	.0029	.15	100						
894	RK	2820	.0028	.045			TRAP	10	100		
895	KK	1173									
			CP								
896	KM										
			COMBINE CP 1181, CP 1172, AND SUB 1170								
897	HC	3									
898	KK	1201									
			CP								
899	KM										
			ROUTE CP 1173 TO CP 1201								
900	RK	1650	.0029	.045			TRAP	10	100		
901	KK	1200									
			SUB								
902	KM										
			RUNOFF FROM SUB 1200								
903	BA	.1033									
904	LS		80								
905	UK	1650	.0029	.15	100						
906	RK	2120	.0028	.045			TRAP	10	100		
907	KK	1202									
			CP								
908	KM										
			COMBINE CP 1201 WITH SUB 1200								
909	HC	2									
910	KK	1204									
			CP								
911	KM										
			ROUTE CP 1202 TO CP 1204								
912	RK	1600	.0010	.055			TRAP	50	100		
913	KK	1205									
			CP								
914	KM										
			COMBINE CP 1204 & CP 1203								
915	HC	2									
916	KK	1220									
			SUB								
917	KM										
			RUNOFF FROM SUB 1220 AND ROUTE CP 1205								
918	BA	.0710									
919	LS		80								
920	UK	1300	.0028	.15	100						
921	RK	3330	.0032	.012			TRAP	35	1	YES	
922	KK	1230									
			SUB								
923	KM										
			RUNOFF FROM SUB 1230								
924	BA	0.1399									
925	LS		80								
926	UK	1100	.0028	.15	100						
927	RK	1170	.0029	.045			TRAP	10	100		
928	KK	1221									
			CP								
929	KM										
			ROUTE SUB 1230 TO CP 1221								
930	RK	600	.0028	.045			TRAP	10	100		



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598	.	350 ***	
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628	.	562.....	
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643	.	781	
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646	782.....		
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649	871		

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952 1253

(\*\*\*) RUNOFF ALSO COMPUTED AT THIS LOCATION

RUNOFF SUMMARY  
 FLOW IN CUBIC FEET PER SECOND  
 TIME IN HOURS, AREA IN SQUARE MILES

OPERATION	STATION	PEAK FLOW	TIME OF PEAK	AVERAGE FLOW FOR MAXIMUM PERIOD			BASIN AREA	MAXIMUM STAGE	TIME OF MAX STAGE
				6-HOUR	24-HOUR	72-HOUR			
HYDROGRAPH AT	550	52.	13.42	23.	7.	7.	.13		
ROUTED TO	651	52.	13.67	23.	6.	6.	.13		
HYDROGRAPH AT	650	68.	13.33	27.	8.	8.	.13		
2 COMBINED AT	652	116.	13.50	49.	14.	14.	.25		
HYDROGRAPH AT	640	148.	13.83	68.	20.	20.	.38		
HYDROGRAPH AT	430	33.	12.75	7.	2.	2.	.04		
HYDROGRAPH AT	420	65.	13.50	30.	9.	9.	.19		
ROUTED TO	501	65.	13.67	30.	9.	9.	.19		
HYDROGRAPH AT	500	43.	13.58	20.	6.	6.	.13		
2 COMBINED AT	502	107.	13.67	50.	15.	15.	.32		
ROUTED TO	641	107.	13.83	50.	14.	14.	.32		
2 COMBINED AT	642	255.	13.83	117.	34.	34.	.69		
HYDROGRAPH AT	630	271.	13.92	125.	37.	37.	.75		
HYDROGRAPH AT	490	22.	13.42	10.	3.	3.	.06		
ROUTED TO	631	22.	13.75	10.	3.	3.	.06		
2 COMBINED AT	632	293.	13.92	135.	40.	40.	.81		
ROUTED TO	621	292.	14.00	135.	39.	39.	.81		
HYDROGRAPH AT	230	89.	12.33	10.	3.	3.	.04		
HYDROGRAPH AT	231	94.	12.42	13.	4.	4.	.06		
ROUTED TO	281	87.	12.75	14.	4.	4.	.06		
HYDROGRAPH AT	290	38.	13.67	20.	6.	6.	.11		
HYDROGRAPH AT	280	53.	13.33	27.	8.	8.	.14		
2 COMBINED AT	282	129.	12.75	40.	11.	11.	.20		
ROUTED TO	401	129.	12.92	41.	12.	12.	.20		
HYDROGRAPH AT	410	22.	13.42	10.	3.	3.	.06		
HYDROGRAPH AT	400	47.	13.42	21.	6.	6.	.12		
2 COMBINED AT	402	169.	13.00	62.	18.	18.	.32		
HYDROGRAPH AT	620	183.	13.50	80.	23.	23.	.45		

HYDROGRAPH AT	600	479.	14.17	233.	69.	69.	1.39
HYDROGRAPH AT	170	75.	13.25	28.	8.	8.	.13
ROUTED TO	221	75.	13.42	28.	8.	8.	.13
HYDROGRAPH AT	220	68.	13.33	27.	8.	8.	.13
2 COMBINED AT	222	143.	13.33	55.	16.	16.	.25
ROUTED TO	271	141.	13.50	54.	15.	15.	.25
HYDROGRAPH AT	270	67.	13.33	27.	8.	8.	.12
2 COMBINED AT	272	207.	13.50	81.	23.	23.	.38
ROUTED TO	391	206.	13.58	80.	23.	23.	.38
HYDROGRAPH AT	390	68.	13.33	27.	8.	8.	.13
2 COMBINED AT	392	269.	13.58	106.	30.	30.	.50
DIVERSION TO	393	244.	12.67	84.	23.	23.	.50
HYDROGRAPH AT	394	25.	12.67	23.	8.	8.	.50
ROUTED TO	471	25.	13.83	23.	7.	7.	.50
HYDROGRAPH AT	470	68.	13.33	27.	8.	8.	.13
2 COMBINED AT	472	93.	13.33	49.	15.	15.	.63
ROUTED TO	601	92.	13.50	48.	15.	15.	.63
2 COMBINED AT	602	554.	14.08	281.	84.	84.	2.01
DIVERSION TO	603	277.	14.08	141.	42.	42.	2.01
HYDROGRAPH AT	604	277.	14.08	141.	42.	42.	2.01
ROUTED TO	721	277.	14.25	140.	42.	42.	2.01
HYDROGRAPH AT	720	56.	13.42	24.	7.	7.	.13
2 COMBINED AT	722	316.	14.17	161.	48.	48.	2.14
ROUTED TO	811	316.	14.25	161.	48.	48.	2.14
HYDROGRAPH AT	760	52.	12.92	14.	4.	4.	.06
ROUTED TO	751	51.	13.08	14.	4.	4.	.06
HYDROGRAPH AT	750	52.	12.92	14.	4.	4.	.06
2 COMBINED AT	752	101.	13.08	28.	8.	8.	.13
ROUTED TO	841	101.	13.25	28.	8.	8.	.13
HYDROGRAPH AT	850	36.	13.00	12.	3.	3.	.07
HYDROGRAPH AT	840	67.	13.17	24.	7.	7.	.13
2 COMBINED AT	842	167.	13.17	52.	15.	15.	.25

HYDROGRAPH AT	740	46.	13.50	21.	6.	6.	.13
ROUTED TO	831	46.	13.75	21.	6.	6.	.13
2 COMBINED AT	832	237.	13.67	92.	26.	26.	.50
HYDROGRAPH AT	820	259.	14.00	110.	32.	32.	.63
HYDROGRAPH AT	730	43.	13.58	20.	6.	6.	.13
ROUTED TO	821	43.	13.83	20.	6.	6.	.13
2 COMBINED AT	822	300.	14.00	129.	38.	38.	.75
DIVERSION TO	823	120.	14.00	52.	16.	16.	.75
HYDROGRAPH AT	824	180.	14.00	77.	22.	22.	.75
HYDROGRAPH AT	810	200.	14.33	94.	28.	28.	.88
2 COMBINED AT	812	516.	14.25	255.	76.	76.	3.01
HYDROGRAPH AT	800	530.	14.42	266.	79.	79.	3.11
DIVERSION TO	805	50.	14.42	50.	19.	19.	3.11
HYDROGRAPH AT	804	480.	14.42	216.	60.	60.	3.11
HYDROGRAPH AT	801	485.	14.58	220.	62.	62.	3.14
HYDROGRAPH AT	130	69.	13.08	23.	6.	6.	.10
ROUTED TO	161	68.	13.33	23.	6.	6.	.10
HYDROGRAPH AT	160	66.	13.25	26.	7.	7.	.12
2 COMBINED AT	162	135.	13.25	48.	14.	14.	.22
ROUTED TO	211	135.	13.50	49.	14.	14.	.22
HYDROGRAPH AT	210	66.	13.33	26.	7.	7.	.12
2 COMBINED AT	212	199.	13.50	74.	21.	21.	.35
ROUTED TO	261	198.	13.58	74.	21.	21.	.35
HYDROGRAPH AT	260	66.	13.25	26.	7.	7.	.12
2 COMBINED AT	262	259.	13.58	99.	28.	28.	.47
ROUTED TO	381	258.	13.67	99.	28.	28.	.47
HYDROGRAPH AT	393	244.	13.58	84.	23.	23.	.00
HYDROGRAPH AT	380	278.	13.83	109.	30.	30.	.13
2 COMBINED AT	382	531.	13.75	208.	58.	58.	.59
DIVERSION TO	383	266.	13.75	104.	29.	29.	.59
HYDROGRAPH AT	384	266.	13.75	104.	29.	29.	.59
ROUTED TO	461	265.	13.92	103.	29.	29.	.59

2 COMBINED AT	462	319.	13.83	129.	37.	37.	.72
ROUTED TO	591	318.	13.92	129.	36.	36.	.72
HYDROGRAPH AT	603	277.	14.08	141.	42.	42.	.00
HYDROGRAPH AT	590	300.	14.42	161.	49.	49.	.13
2 COMBINED AT	592	594.	14.17	290.	85.	85.	.84
ROUTED TO	701	593.	14.33	290.	85.	85.	.84
HYDROGRAPH AT	710	26.	13.33	12.	3.	3.	.06
HYDROGRAPH AT	700	55.	13.33	24.	7.	7.	.12
2 COMBINED AT	702	629.	14.25	311.	92.	92.	.97
ROUTED TO	802	629.	14.42	312.	92.	92.	.97
HYDROGRAPH AT	950	23.	14.08	14.	4.	4.	.09
HYDROGRAPH AT	805	50.	13.25	50.	19.	19.	.00
2 COMBINED AT	946	73.	14.08	63.	23.	23.	.09
HYDROGRAPH AT	945	73.	14.17	63.	23.	23.	.10
HYDROGRAPH AT	940	10.	13.50	5.	1.	1.	.03
4 COMBINED AT	803	1191.	14.50	598.	178.	178.	4.23
DIVERSION TO	807	200.	14.50	193.	58.	58.	4.23
HYDROGRAPH AT	806	991.	14.50	406.	120.	120.	4.23
HYDROGRAPH AT	790	998.	14.67	423.	124.	124.	4.36
HYDROGRAPH AT	383	266.	13.75	104.	29.	29.	.00
ROUTED TO	451	265.	13.92	103.	29.	29.	.00
HYDROGRAPH AT	450	287.	14.17	126.	36.	36.	.13
ROUTED TO	581	287.	14.33	126.	36.	36.	.13
HYDROGRAPH AT	580	63.	13.33	26.	7.	7.	.13
2 COMBINED AT	582	326.	14.25	150.	43.	43.	.25
ROUTED TO	681	325.	14.42	149.	43.	43.	.25
HYDROGRAPH AT	680	59.	13.33	24.	7.	7.	.12
2 COMBINED AT	682	361.	14.33	173.	50.	50.	.37
ROUTED TO	791	360.	14.42	172.	50.	50.	.37
2 COMBINED AT	792	1345.	14.67	595.	174.	174.	4.73
HYDROGRAPH AT	780	1353.	14.75	606.	177.	177.	4.79
HYDROGRAPH AT	110	90.	13.58	40.	11.	11.	.19

HYDROGRAPH AT	140	96.	13.50	40.	12.	12.	.19
2 COMBINED AT	142	185.	13.58	80.	23.	23.	.38
ROUTED TO	201	185.	13.58	80.	23.	23.	.38
HYDROGRAPH AT	200	99.	13.42	40.	12.	12.	.19
2 COMBINED AT	202	283.	13.50	121.	35.	35.	.57
ROUTED TO	351	282.	13.58	121.	35.	35.	.57
HYDROGRAPH AT	250	34.	13.17	13.	4.	4.	.06
HYDROGRAPH AT	251	35.	13.17	14.	4.	4.	.06
2 COMBINED AT	252	69.	13.17	27.	8.	8.	.12
ROUTED TO	371	68.	13.33	27.	8.	8.	.12
HYDROGRAPH AT	370	35.	13.17	14.	4.	4.	.06
2 COMBINED AT	372	103.	13.25	40.	11.	11.	.19
HYDROGRAPH AT	350	218.	13.25	89.	25.	25.	.38
2 COMBINED AT	352	489.	13.50	209.	60.	60.	.95
ROUTED TO	441	489.	13.50	209.	60.	60.	.95
HYDROGRAPH AT	440	36.	13.17	14.	4.	4.	.06
2 COMBINED AT	442	522.	13.50	222.	63.	63.	1.01
ROUTED TO	561	522.	13.50	222.	63.	63.	1.01
HYDROGRAPH AT	560	36.	13.17	14.	4.	4.	.07
2 COMBINED AT	562	555.	13.50	236.	67.	67.	1.08
ROUTED TO	671	555.	13.50	236.	67.	67.	1.08
HYDROGRAPH AT	670	35.	13.17	13.	4.	4.	.06
2 COMBINED AT	672	586.	13.50	250.	71.	71.	1.14
ROUTED TO	781	586.	13.50	250.	71.	71.	1.14
2 COMBINED AT	782	1674.	14.67	848.	248.	248.	5.93
ROUTED TO	871	1671.	14.67	848.	248.	248.	5.93
HYDROGRAPH AT	870	28.	13.50	12.	4.	4.	.07
HYDROGRAPH AT	930	21.	13.42	10.	3.	3.	.06
ROUTED TO	921	21.	13.58	10.	3.	3.	.06
HYDROGRAPH AT	920	18.	13.92	10.	3.	3.	.07
2 COMBINED AT	922	39.	13.67	20.	6.	6.	.13
HYDROGRAPH AT	900	51.	13.58	26.	8.	8.	.17

2 COMBINED AT	901	73.	13.50	36.	11.	11.	.23
HYDROGRAPH AT	880	81.	13.50	40.	12.	12.	.26
ROUTED TO	872	81.	13.67	41.	12.	12.	.26
3 COMBINED AT	873	1748.	14.67	901.	263.	263.	6.26
ROUTED TO	891	1747.	14.67	901.	263.	263.	6.26
HYDROGRAPH AT	890	32.	12.75	8.	2.	2.	.04
2 COMBINED AT	892	1751.	14.67	907.	265.	265.	6.30
HYDROGRAPH AT	1020	1751.	14.75	909.	266.	266.	6.31
HYDROGRAPH AT	1040	22.	13.42	10.	3.	3.	.06
ROUTED TO	1031	22.	13.67	10.	3.	3.	.06
HYDROGRAPH AT	1030	38.	13.75	18.	5.	5.	.12
HYDROGRAPH AT	1070	22.	13.58	11.	3.	3.	.07
2 COMBINED AT	1032	60.	13.67	29.	9.	9.	.19
ROUTED TO	1021	60.	13.75	29.	9.	9.	.19
2 COMBINED AT	1022	1794.	14.67	938.	274.	274.	6.50
HYDROGRAPH AT	1060	1795.	14.75	940.	275.	275.	6.52
ROUTED TO	1083	1795.	14.75	940.	274.	274.	6.52
HYDROGRAPH AT	1090	21.	13.08	7.	2.	2.	.04
HYDROGRAPH AT	1080	30.	13.42	11.	3.	3.	.07
2 COMBINED AT	1082	1807.	14.75	951.	277.	277.	6.59
ROUTED TO	1084	1807.	14.75	951.	277.	277.	6.59
HYDROGRAPH AT	1081	21.	12.92	5.	2.	2.	.03
2 COMBINED AT	1085	1810.	14.75	956.	279.	279.	6.62
ROUTED TO	1111	1810.	14.75	955.	279.	279.	6.62
HYDROGRAPH AT	1110	31.	13.17	12.	3.	3.	.07
2 COMBINED AT	1112	1821.	14.75	966.	282.	282.	6.69
ROUTED TO	1171	1818.	14.75	966.	281.	281.	6.69
ROUTED TO	1203	1816.	14.83	966.	281.	281.	6.69
HYDROGRAPH AT	1071	11.	13.08	4.	1.	1.	.02
HYDROGRAPH AT	1072	26.	12.83	7.	2.	2.	.04
2 COMBINED AT	1074	36.	12.92	10.	3.	3.	.07
HYDROGRAPH AT	1073	50.	13.08	17.	5.	5.	.11

HYDROGRAPH AT	1091	29.	12.83	7.	2.	2.	.04
2 COMBINED AT	1093	67.	13.17	24.	7.	7.	.14
ROUTED TO	1122	67.	13.42	23.	7.	7.	.14
HYDROGRAPH AT	1121	26.	13.08	9.	2.	2.	.05
HYDROGRAPH AT	1131	13.	13.42	6.	2.	2.	.04
2 COMBINED AT	1132	39.	13.08	15.	4.	4.	.09
ROUTED TO	1133	39.	13.33	15.	4.	4.	.09
HYDROGRAPH AT	1130	13.	13.42	6.	2.	2.	.04
2 COMBINED AT	1134	52.	13.33	20.	6.	6.	.13
HYDROGRAPH AT	1120	78.	13.67	34.	10.	10.	.21
2 COMBINED AT	1123	142.	13.50	57.	16.	16.	.36
ROUTED TO	1181	138.	14.00	56.	16.	16.	.36
HYDROGRAPH AT	1150	22.	13.67	11.	3.	3.	.07
ROUTED TO	1172	22.	13.92	10.	3.	3.	.07
HYDROGRAPH AT	1170	23.	13.67	11.	3.	3.	.07
3 COMBINED AT	1173	181.	13.92	76.	22.	22.	.49
ROUTED TO	1201	181.	14.17	76.	22.	22.	.49
HYDROGRAPH AT	1200	30.	13.83	16.	5.	5.	.10
2 COMBINED AT	1202	209.	14.17	90.	27.	27.	.59
ROUTED TO	1204	208.	14.50	90.	26.	26.	.59
2 COMBINED AT	1205	2013.	14.75	1052.	307.	307.	7.29
HYDROGRAPH AT	1220	2021.	14.83	1061.	310.	310.	7.36
HYDROGRAPH AT	1230	56.	13.25	23.	7.	7.	.14
ROUTED TO	1221	56.	13.33	23.	7.	7.	.14
2 COMBINED AT	1222	2044.	14.83	1082.	316.	316.	7.50
HYDROGRAPH AT	1240	2051.	14.83	1087.	318.	318.	7.54
ROUTED TO	1251	2050.	14.83	1087.	317.	317.	7.54
HYDROGRAPH AT	1250	20.	13.83	11.	3.	3.	.07
2 COMBINED AT	1252	2064.	14.83	1097.	321.	321.	7.62
ROUTED TO	1253	2063.	14.83	1097.	321.	321.	7.62

APPENDIX B

LINE	ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10
1	ID SIMONS, LI AND ASSOCIATES
2	ID OUTER LOOP FREEWAY, PHASE 3 (FROM THE ARIZONA CANAL TO THE SALT RIVER)
3	ID FINAL HYDROLOGY ANALYSIS FOR OFFSITE DRAINAGE, MARCH 26, 1989
4	ID MODEL 3FD5.24I - MARCH 1989 HIGHWAY ALIGNMENT
5	ID 50 YEAR EVENT, 24 HOUR HYPOTHETICAL STORM DISTRIBUTION
6	ID 24 HOUR CN VALUES
7	ID WITH LINED CHANNEL ALONG OUTER LOOP ALIGNMENT (n=.012)
8	IT 5 26MAR89 0 289
9	IO 5

\*\*\* NOLIST \*\*\*

RUNOFF SUMMARY  
 FLOW IN CUBIC FEET PER SECOND  
 TIME IN HOURS, AREA IN SQUARE MILES

OPERATION	STATION	PEAK FLOW	TIME OF PEAK	AVERAGE FLOW FOR MAXIMUM PERIOD			BASIN AREA	MAXIMUM STAGE	TIME OF MAX STAGE
				6-HOUR	24-HOUR	72-HOUR			
HYDROGRAPH AT	550	38.	13.58	18.	5.	5.	.13		
ROUTED TO	651	38.	13.83	18.	5.	5.	.13		
HYDROGRAPH AT	650	50.	13.42	22.	6.	6.	.13		
2 COMBINED AT	652	85.	13.58	40.	12.	12.	.25		
HYDROGRAPH AT	640	108.	14.08	54.	16.	16.	.38		
HYDROGRAPH AT	430	23.	12.83	6.	2.	2.	.04		
HYDROGRAPH AT	420	46.	13.67	24.	7.	7.	.19		
ROUTED TO	501	46.	13.92	24.	7.	7.	.19		
HYDROGRAPH AT	500	30.	13.83	16.	5.	5.	.13		
2 COMBINED AT	502	76.	13.83	39.	12.	12.	.32		
ROUTED TO	641	76.	14.08	39.	12.	12.	.32		
2 COMBINED AT	642	184.	14.08	93.	28.	28.	.69		
HYDROGRAPH AT	630	196.	14.17	99.	30.	30.	.75		
HYDROGRAPH AT	490	15.	13.67	8.	2.	2.	.06		
ROUTED TO	631	15.	14.00	8.	2.	2.	.06		
2 COMBINED AT	632	211.	14.17	107.	32.	32.	.81		
ROUTED TO	621	210.	14.25	107.	32.	32.	.81		
HYDROGRAPH AT	230	69.	12.33	9.	2.	2.	.04		
HYDROGRAPH AT	231	72.	12.42	11.	3.	3.	.06		
ROUTED TO	281	73.	12.67	11.	3.	3.	.06		
HYDROGRAPH AT	290	28.	13.92	16.	5.	5.	.11		
HYDROGRAPH AT	280	38.	13.50	21.	6.	6.	.14		
2 COMBINED AT	282	98.	12.75	33.	9.	9.	.20		
ROUTED TO	401	98.	12.92	33.	9.	9.	.20		
HYDROGRAPH AT	410	15.	13.67	8.	2.	2.	.06		
HYDROGRAPH AT	400	33.	13.58	17.	5.	5.	.12		
2 COMBINED AT	402	124.	13.00	49.	14.	14.	.32		
HYDROGRAPH AT	620	129.	13.67	63.	19.	19.	.45		

HYDROGRAPH AT	600	344.	14.42	184.	56.	56.	1.39
HYDROGRAPH AT	170	56.	13.33	23.	7.	7.	.13
ROUTED TO	221	56.	13.58	23.	7.	7.	.13
HYDROGRAPH AT	220	50.	13.42	22.	6.	6.	.13
2 COMBINED AT	222	106.	13.50	45.	13.	13.	.25
ROUTED TO	271	106.	13.75	45.	13.	13.	.25
HYDROGRAPH AT	270	50.	13.42	22.	6.	6.	.12
2 COMBINED AT	272	153.	13.67	66.	19.	19.	.38
ROUTED TO	391	152.	13.83	66.	19.	19.	.38
HYDROGRAPH AT	390	50.	13.42	22.	6.	6.	.13
2 COMBINED AT	392	199.	13.75	87.	25.	25.	.50
DIVERSION TO	393	174.	12.83	65.	18.	18.	.50
HYDROGRAPH AT	394	25.	12.83	22.	7.	7.	.50
ROUTED TO	471	25.	13.83	22.	7.	7.	.50
HYDROGRAPH AT	470	50.	13.42	22.	6.	6.	.13
2 COMBINED AT	472	75.	13.42	43.	13.	13.	.63
ROUTED TO	601	75.	13.67	43.	13.	13.	.63
2 COMBINED AT	602	406.	14.42	227.	69.	69.	2.01
DIVERSION TO	603	203.	14.42	113.	34.	34.	2.01
HYDROGRAPH AT	604	203.	14.42	113.	34.	34.	2.01
ROUTED TO	721	203.	14.50	113.	34.	34.	2.01
HYDROGRAPH AT	720	41.	13.58	19.	6.	6.	.13
2 COMBINED AT	722	232.	14.42	130.	40.	40.	2.14
ROUTED TO	811	231.	14.58	130.	39.	39.	2.14
HYDROGRAPH AT	760	39.	13.00	12.	3.	3.	.06
ROUTED TO	751	39.	13.17	12.	3.	3.	.06
HYDROGRAPH AT	750	39.	13.00	12.	3.	3.	.06
2 COMBINED AT	752	77.	13.17	23.	7.	7.	.13
ROUTED TO	841	77.	13.33	23.	6.	6.	.13
HYDROGRAPH AT	850	26.	13.17	10.	3.	3.	.07
HYDROGRAPH AT	840	48.	13.33	19.	6.	6.	.13
2 COMBINED AT	842	125.	13.33	42.	12.	12.	.25

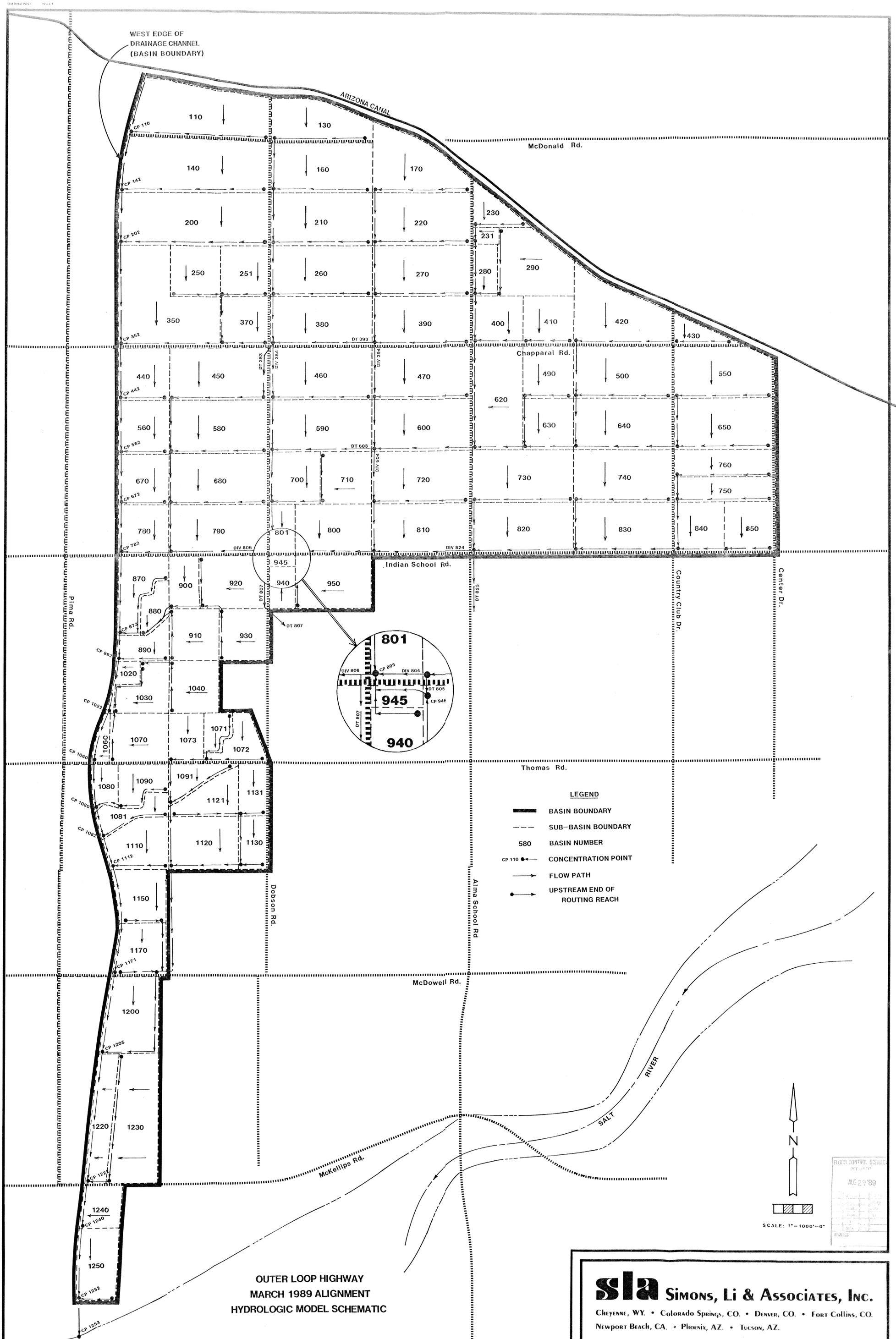
HYDROGRAPH AT	740	33.	13.75	16.	5.	5.	.13
ROUTED TO	831	32.	14.00	16.	5.	5.	.13
2 COMBINED AT	832	174.	13.83	74.	22.	22.	.50
HYDROGRAPH AT	820	189.	14.17	88.	26.	26.	.63
HYDROGRAPH AT	730	30.	13.83	16.	5.	5.	.13
ROUTED TO	821	30.	14.08	16.	5.	5.	.13
2 COMBINED AT	822	219.	14.17	103.	30.	30.	.75
DIVERSION TO	823	88.	14.17	41.	13.	13.	.75
HYDROGRAPH AT	824	132.	14.17	62.	18.	18.	.75
HYDROGRAPH AT	810	147.	14.58	75.	22.	22.	.88
2 COMBINED AT	812	378.	14.58	204.	61.	61.	3.01
HYDROGRAPH AT	800	389.	14.75	213.	64.	64.	3.11
DIVERSION TO	805	50.	14.75	49.	18.	18.	3.11
HYDROGRAPH AT	804	339.	14.75	163.	46.	46.	3.11
HYDROGRAPH AT	801	343.	14.83	166.	47.	47.	3.14
HYDROGRAPH AT	130	52.	13.25	19.	5.	5.	.10
ROUTED TO	161	52.	13.42	19.	5.	5.	.10
HYDROGRAPH AT	160	50.	13.42	21.	6.	6.	.12
2 COMBINED AT	162	102.	13.42	40.	11.	11.	.22
ROUTED TO	211	102.	13.58	40.	11.	11.	.22
HYDROGRAPH AT	210	49.	13.42	21.	6.	6.	.12
2 COMBINED AT	212	150.	13.58	61.	17.	17.	.35
ROUTED TO	261	149.	13.67	60.	17.	17.	.35
HYDROGRAPH AT	260	49.	13.42	21.	6.	6.	.12
2 COMBINED AT	262	196.	13.67	81.	23.	23.	.47
ROUTED TO	381	194.	13.83	81.	23.	23.	.47
HYDROGRAPH AT	393	174.	13.75	65.	18.	18.	.00
HYDROGRAPH AT	380	201.	14.08	85.	24.	24.	.13
2 COMBINED AT	382	389.	13.92	166.	47.	47.	.59
DIVERSION TO	383	195.	13.92	83.	24.	24.	.59
HYDROGRAPH AT	384	195.	13.92	83.	24.	24.	.59
ROUTED TO	461	194.	14.08	83.	24.	24.	.59

2 COMBINED AT	462	235.	14.00	103.	30.	30.	.72
ROUTED TO	591	234.	14.17	103.	30.	30.	.72
HYDROGRAPH AT	603	203.	14.42	113.	34.	34.	.00
HYDROGRAPH AT	590	222.	14.67	130.	40.	40.	.13
2 COMBINED AT	592	437.	14.42	233.	70.	70.	.84
ROUTED TO	701	437.	14.50	232.	69.	69.	.84
HYDROGRAPH AT	710	19.	13.58	9.	3.	3.	.06
HYDROGRAPH AT	700	40.	13.50	19.	6.	6.	.12
2 COMBINED AT	702	465.	14.42	249.	75.	75.	.97
ROUTED TO	802	465.	14.50	249.	74.	74.	.97
HYDROGRAPH AT	950	16.	14.42	11.	3.	3.	.09
HYDROGRAPH AT	805	50.	13.58	49.	18.	18.	.00
2 COMBINED AT	946	66.	14.42	59.	21.	21.	.09
HYDROGRAPH AT	945	66.	14.50	59.	21.	21.	.10
HYDROGRAPH AT	940	7.	13.67	4.	1.	1.	.03
4 COMBINED AT	803	869.	14.67	477.	144.	144.	4.23
DIVERSION TO	807	200.	14.67	186.	54.	54.	4.23
HYDROGRAPH AT	806	669.	14.67	291.	91.	91.	4.23
HYDROGRAPH AT	790	674.	14.92	304.	94.	94.	4.36
HYDROGRAPH AT	383	195.	13.92	83.	24.	24.	.00
ROUTED TO	451	194.	14.08	83.	24.	24.	.00
HYDROGRAPH AT	450	212.	14.42	100.	30.	30.	.13
ROUTED TO	581	211.	14.58	100.	29.	29.	.13
HYDROGRAPH AT	580	47.	13.50	21.	6.	6.	.13
2 COMBINED AT	582	241.	14.50	120.	35.	35.	.25
ROUTED TO	681	240.	14.67	119.	35.	35.	.25
HYDROGRAPH AT	680	44.	13.50	20.	6.	6.	.12
2 COMBINED AT	682	267.	14.58	138.	41.	41.	.37
ROUTED TO	791	266.	14.67	138.	41.	41.	.37
2 COMBINED AT	792	934.	14.92	442.	134.	134.	4.73
HYDROGRAPH AT	780	941.	15.00	451.	137.	137.	4.79
HYDROGRAPH AT	110	67.	13.75	33.	10.	10.	.19

HYDROGRAPH AT	140	72.	13.67	33.	10.	10.	.19
2 COMBINED AT	142	139.	13.67	66.	19.	19.	.38
ROUTED TO	201	139.	13.75	66.	19.	19.	.38
HYDROGRAPH AT	200	73.	13.58	33.	10.	10.	.19
2 COMBINED AT	202	212.	13.67	99.	29.	29.	.57
ROUTED TO	351	211.	13.75	99.	29.	29.	.57
HYDROGRAPH AT	250	25.	13.25	11.	3.	3.	.06
HYDROGRAPH AT	251	26.	13.25	11.	3.	3.	.06
2 COMBINED AT	252	51.	13.25	22.	6.	6.	.12
ROUTED TO	371	51.	13.50	22.	6.	6.	.12
HYDROGRAPH AT	370	26.	13.25	11.	3.	3.	.06
2 COMBINED AT	372	76.	13.42	33.	9.	9.	.19
HYDROGRAPH AT	350	165.	13.42	73.	21.	21.	.38
2 COMBINED AT	352	369.	13.58	171.	50.	50.	.95
ROUTED TO	441	368.	13.58	171.	50.	50.	.95
HYDROGRAPH AT	440	26.	13.33	11.	3.	3.	.06
2 COMBINED AT	442	393.	13.58	183.	53.	53.	1.01
ROUTED TO	561	392.	13.67	182.	53.	53.	1.01
HYDROGRAPH AT	560	27.	13.33	12.	3.	3.	.07
2 COMBINED AT	562	418.	13.58	194.	56.	56.	1.08
ROUTED TO	671	417.	13.67	194.	56.	56.	1.08
HYDROGRAPH AT	670	25.	13.25	11.	3.	3.	.06
2 COMBINED AT	672	441.	13.58	205.	59.	59.	1.14
ROUTED TO	781	441.	13.67	205.	59.	59.	1.14
2 COMBINED AT	782	1191.	14.92	647.	196.	196.	5.93
ROUTED TO	871	1189.	14.92	647.	196.	196.	5.93
HYDROGRAPH AT	870	21.	13.67	10.	3.	3.	.07
HYDROGRAPH AT	930	15.	13.58	8.	2.	2.	.06
ROUTED TO	921	15.	13.75	8.	2.	2.	.06
HYDROGRAPH AT	920	13.	14.25	8.	2.	2.	.07
2 COMBINED AT	922	27.	13.92	16.	5.	5.	.13
HYDROGRAPH AT	900	36.	13.83	20.	6.	6.	.17

2 COMBINED AT	901	51.	13.75	28.	8.	8.	.23
HYDROGRAPH AT	880	57.	13.75	32.	9.	9.	.26
ROUTED TO	872	57.	13.92	32.	9.	9.	.26
3 COMBINED AT	873	1247.	14.92	688.	208.	208.	6.26
ROUTED TO	891	1246.	14.92	688.	208.	208.	6.26
HYDROGRAPH AT	890	24.	12.83	6.	2.	2.	.04
2 COMBINED AT	892	1250.	14.92	693.	209.	209.	6.30
HYDROGRAPH AT	1020	1249.	15.00	695.	210.	210.	6.31
HYDROGRAPH AT	1040	15.	13.67	8.	2.	2.	.06
ROUTED TO	1031	15.	13.92	8.	2.	2.	.06
HYDROGRAPH AT	1030	27.	13.92	14.	4.	4.	.12
HYDROGRAPH AT	1070	15.	13.83	9.	3.	3.	.07
2 COMBINED AT	1032	43.	13.92	23.	7.	7.	.19
ROUTED TO	1021	43.	14.00	23.	7.	7.	.19
2 COMBINED AT	1022	1281.	14.92	717.	217.	217.	6.50
HYDROGRAPH AT	1060	1282.	15.00	719.	217.	217.	6.52
ROUTED TO	1083	1282.	15.00	719.	217.	217.	6.52
HYDROGRAPH AT	1090	15.	13.17	5.	2.	2.	.04
HYDROGRAPH AT	1080	21.	13.58	9.	3.	3.	.07
2 COMBINED AT	1082	1291.	15.00	728.	219.	219.	6.59
ROUTED TO	1084	1291.	15.00	727.	219.	219.	6.59
HYDROGRAPH AT	1081	15.	13.00	4.	1.	1.	.03
2 COMBINED AT	1085	1294.	15.00	731.	220.	220.	6.62
ROUTED TO	1111	1293.	15.00	731.	220.	220.	6.62
HYDROGRAPH AT	1110	22.	13.33	9.	3.	3.	.07
2 COMBINED AT	1112	1302.	15.00	740.	223.	223.	6.69
ROUTED TO	1171	1299.	15.08	739.	222.	222.	6.69
ROUTED TO	1203	1299.	15.08	739.	222.	222.	6.69
HYDROGRAPH AT	1071	7.	13.25	3.	1.	1.	.02
HYDROGRAPH AT	1072	18.	13.00	5.	1.	1.	.04
2 COMBINED AT	1074	25.	13.00	8.	2.	2.	.07
HYDROGRAPH AT	1073	35.	13.17	14.	4.	4.	.11

HYDROGRAPH AT	1091	21.	12.92	5.	2.	2.	.04
2 COMBINED AT	1093	48.	13.33	19.	5.	5.	.14
ROUTED TO	1122	48.	13.58	19.	5.	5.	.14
HYDROGRAPH AT	1121	19.	13.17	7.	2.	2.	.05
HYDROGRAPH AT	1131	9.	13.58	5.	1.	1.	.04
2 COMBINED AT	1132	27.	13.25	12.	3.	3.	.09
ROUTED TO	1133	27.	13.50	12.	3.	3.	.09
HYDROGRAPH AT	1130	9.	13.58	5.	1.	1.	.04
2 COMBINED AT	1134	37.	13.50	16.	5.	5.	.13
HYDROGRAPH AT	1120	55.	13.83	26.	8.	8.	.21
2 COMBINED AT	1123	101.	13.75	45.	13.	13.	.36
ROUTED TO	1181	100.	14.17	44.	13.	13.	.36
HYDROGRAPH AT	1150	15.	13.83	8.	2.	2.	.07
ROUTED TO	1172	15.	14.17	8.	2.	2.	.07
HYDROGRAPH AT	1170	16.	13.92	8.	2.	2.	.07
3 COMBINED AT	1173	130.	14.17	60.	18.	18.	.49
ROUTED TO	1201	130.	14.42	60.	18.	18.	.49
HYDROGRAPH AT	1200	21.	14.08	12.	4.	4.	.10
2 COMBINED AT	1202	150.	14.42	71.	21.	21.	.59
ROUTED TO	1204	149.	14.83	71.	21.	21.	.59
2 COMBINED AT	1205	1442.	15.08	806.	242.	242.	7.29
HYDROGRAPH AT	1220	1449.	15.08	814.	244.	244.	7.36
HYDROGRAPH AT	1230	39.	13.33	18.	5.	5.	.14
ROUTED TO	1221	39.	13.50	18.	5.	5.	.14
2 COMBINED AT	1222	1465.	15.08	829.	250.	250.	7.50
HYDROGRAPH AT	1240	1470.	15.08	834.	251.	251.	7.54
ROUTED TO	1251	1468.	15.17	834.	250.	250.	7.54
HYDROGRAPH AT	1250	14.	14.17	9.	3.	3.	.07
2 COMBINED AT	1252	1479.	15.17	842.	253.	253.	7.62
ROUTED TO	1253	1479.	15.17	841.	253.	253.	7.62



WEST EDGE OF DRAINAGE CHANNEL (BASIN BOUNDARY)

ARIZONA CANAL

McDonald Rd.

Chapparral Rd.

Indian School Rd.

Thomas Rd.

McDowell Rd.

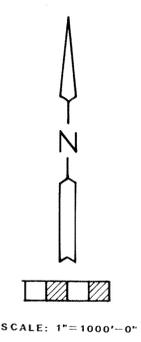
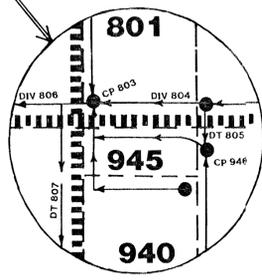
McKellips Rd.

SALT RIVER

**sla** SIMONS, LI & ASSOCIATES, INC.  
 CHEYENNE, WY. • COLORADO SPRINGS, CO. • DENVER, CO. • FORT COLLINS, CO.  
 NEWPORT BEACH, CA. • PHOENIX, AZ. • TUCSON, AZ.

OUTER LOOP HIGHWAY  
 MARCH 1989 ALIGNMENT  
 HYDROLOGIC MODEL SCHEMATIC

- LEGEND**
- BASIN BOUNDARY
  - SUB-BASIN BOUNDARY
  - 580 BASIN NUMBER
  - CP 110 ● CONCENTRATION POINT
  - FLOW PATH
  - UPSTREAM END OF ROUTING REACH



FLORIDA CONTROL DISTRICT  
 PETS 1000  
 AUG 29 '89


BLANKS

WEST EDGE OF DRAINAGE CHANNEL (BASIN BOUNDARY)

ARIZONA CANAL

McDonald Rd.

Chapparral Rd.

Indian School Rd.

Thomas Rd.

McDowell Rd.

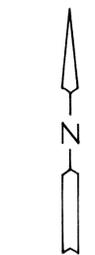
McKellips Rd.

RIVER

SALT

LEGEND

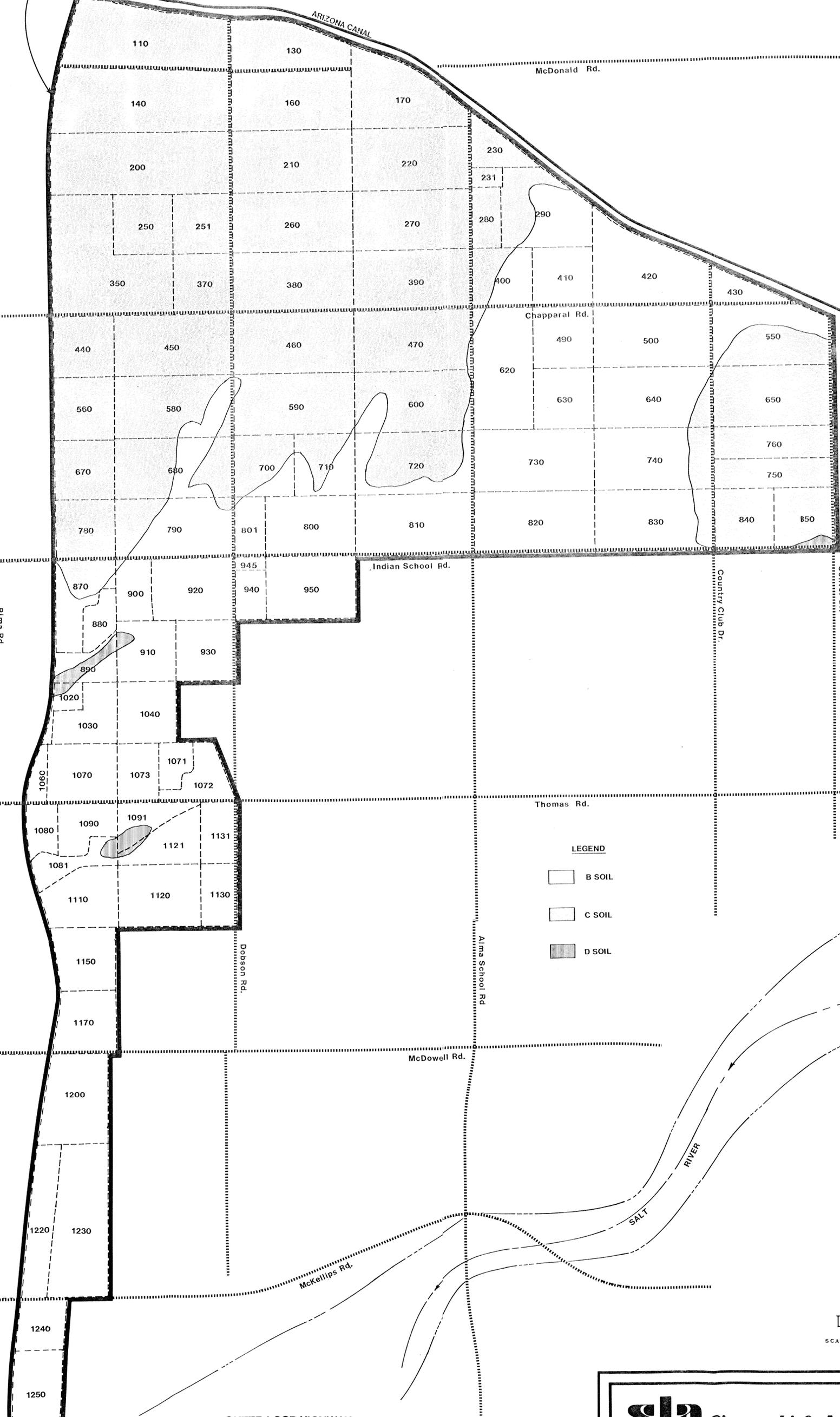
-  B SOIL
-  C SOIL
-  D SOIL



SCALE: 1"=1000'-0"

AUG 29 '89

DATE	29	89
BY	SLA	
CHECKED		
DESIGNED		
ENGINEER		



OUTER LOOP HIGHWAY  
MARCH 1989 ALIGNMENT  
HYDROLOGIC SOIL GROUP MAP

**sla** SIMONS, LI & ASSOCIATES, INC.  
 CHEYENNE, WY. • COLORADO SPRINGS, CO. • DENVER, CO. • FORT COLLINS, CO.  
 NEWPORT BEACH, CA. • PHOENIX, AZ. • TUCSON, AZ.

